COLLISION DAMAGE REPAIR - INT. BT. NO.4

-Repair concrete and apply wrap

Repair concrete and apply wrap. (See Special Provisions)

(See Special Provisions)

-Exist. Confinement Reinf. 3′-0″ر (Exist Col.) Substructure repair (Unformed)

SECTION A-A

* Fiber wrap shall be overlapped per the manufacturer's recommendations.

As a minimum, fiber wrap shall provide for strength equivalent to confinement provided by existing steel.

height picture Cracks in concrete appear to be approximately 6'above ground line. -Concrete damaged and spalled on both impact and reverse side of column. Substructure Repair (Unformed) 10'-0" damage Existing Ground Line ELEVATION North Column

Concrete spalled @ top of column —

2,

Estimated Qu	uantities	
I tem		Total
Substructure Repair (Unformed)	Sq. Ft.	12
Fiber Wrap	Sq. Ft.	115
Epoxy Pressure Injecting	Lin. Ft.	15

Notes:

The cost of all labor, equipment and material to apply the bonding agent and place repair concrete shall be considered covered by the contract unit price for Substructure Repair (Unformed).

The cost of all labor, equipment and material to apply the fiber wrap as shown shall be considered completely covered by the contract unit price for fiber wrap.

Cost of any required excavation for bridge repairs will be considered completely covered by the contract unit price for other items.

REPAIR DETAILS OF INTERMEDIATE BENT NO.4

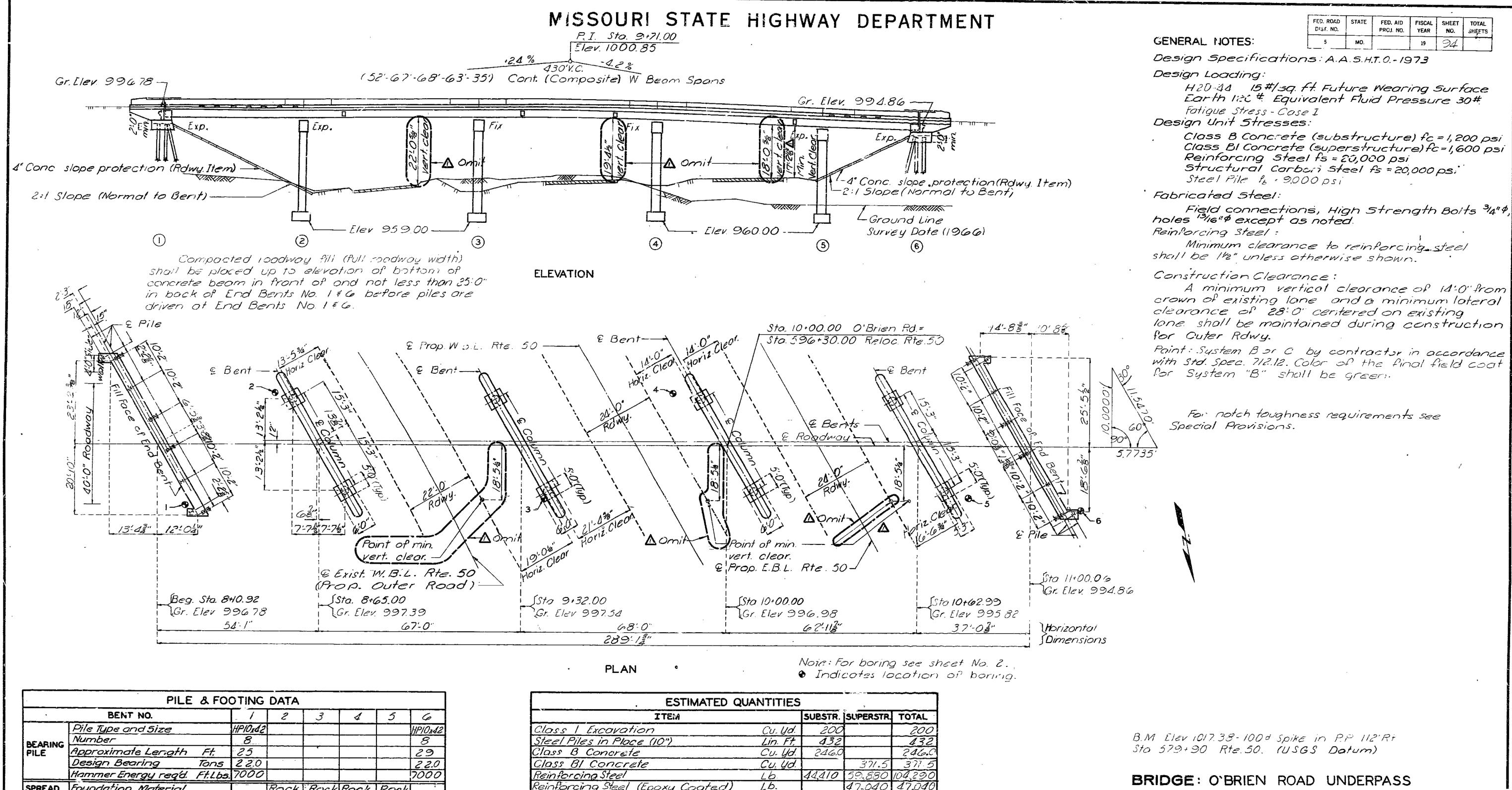
Outline of old work is indicated by light dashed lines. Heavy lines indicate new work.

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 1 of 1

5/13/2016 ΜO 50 SHEET NO JACKSON JOB NO. K16C9370 CONTRACT ID. PROJECT NO. BRIDGE NO. A24462

"THIS MEDIA SHOULD NOT BE CONSIDERED A CERTIFIED DOCUMENT."



Rock Rock Rock Rock 9.7 9.7 9.7 10.5 SPREAD Foundation Material FUOTINGS Design Bearing Tons / Sq. Ft.

Minimum energy requirement of hammer based on plan length and design bearing value of piles. All pile shall be driven to practical refusal.

Steel Piles in Place (10") Class B Concrete Cu. yd. 246.0 Class BI Concrete Cu. yd. 371.5 Reinforcing Steel Reinforcing Steel (Epoxy Coated) Eabricated Structural Carbon Steel Painting Tans Lin. Ft. 432 432 246.0 246.0 246.0 246.0 246.0 246.0 246.0 246.0 246.0 246.0 246.0 246.0 246.0 246.0 247.0 371.5 371.5 47.040 47.040 47.040 47.040 700 70													
ITEM		SUBSTR.	SUPERSTR.	TOTAL									
Class Excavation	Cu. Yd.	200		200									
Steel Piles in Ploce (10")	Lin. Ft.	432		432									
Class B Concrete	Cu. yd.	246.0		246.0									
Class BI Concrete	Cu. yd.		371.5	371.5									
Reinforcing Steel	Lb.	11,110	59,880	104,290									
Reinforcing Steel (Epoxy Coated)	Lb.		47,040	47,040									
Fabricated Structural Carbon Steel	Lb.		198,100	198,100									
Painting	Tons		98.7										
Bridge Rail (One Tube)	Lin. Ft.		<i>583</i>	583									
Pedestrian Ferrice (72")	Lin Ft.	<u>.</u>	309	309									
Steel Reinf. Elastomeric Exp. Jt. Seal (2.	Vin Ft		24	94									

Note: All concrete and reinforcement in end posis, parapets. and curbs is included with superstructure quantities. See Special Provisions for Epoxy cooted reinforcing STATE ROAD : RELOCATED ROUTE 50

ABOUT: NEAR UNITY VILLAGE

PROJECT NO. U-50-1 (9)

STA. 596+30.00

JOB NO. 4-U-50-27A

RTE. 50

JACKSON

COUNTY

STD. 611.60 STD. 706.30 A A2446

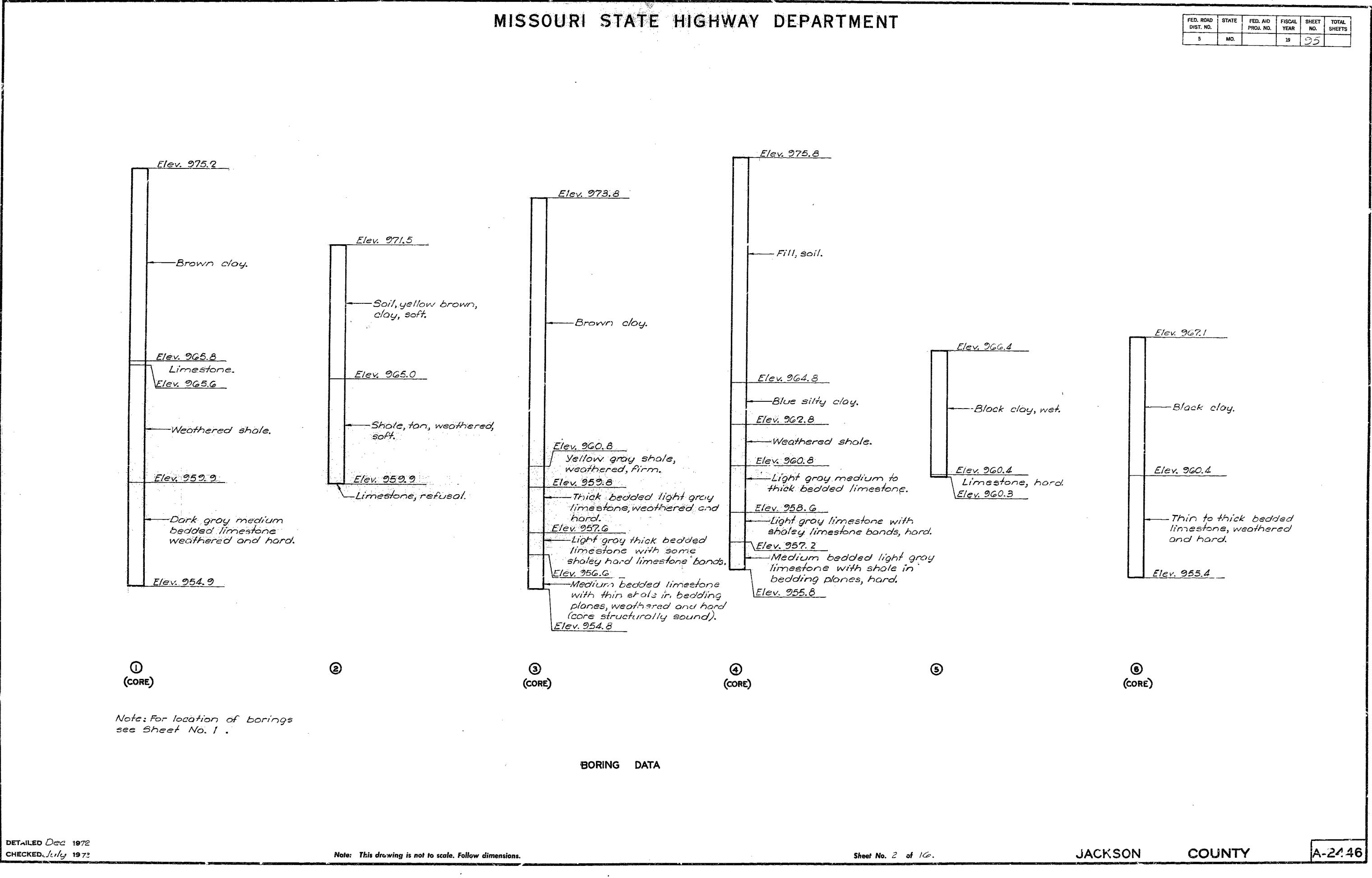
DESIGNED May 1972 DETAILED FEB 1973 CHECKED July 1973

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Note: This drawing is not to scale. Follow dimensions.

Sheet No. / of 16. A Revised Nov 13,1975

DATE 9/10/75



07V

FOR INFORMATION ONLY A24462, Sht. 4 MISSOURI STATE HIGHWAY DEPARTMENT FED. '(OAD STATE FED. AID FISCAL SHEET TOTAL DIST. NO. PROJ. NO. YEAR NO. SHEETS 11'-0" 51 #5 U4 Spaced some as V-Bars 24-#5-VI@12"ds. (Each Face) 22-#5-V1@12" ds. (Each Face) 12"5-#5-V2@12" cts. rElev. 996.87 Syrum obt & Bent except as shown Gr. Elev. 796.78 ot & Readway-Elex 996.780 Fill Foce r-2-#4-H5 Key 2" x 4" Pile Cut-off Elev. 990.8% Elok 989.36 Note: Top of backwall and expansion device for 16" 7-4016 at 12'ds 12 3-408 16" 10-44-02@ abt. 12" ds. 16" 44"

eabt 12'ds. 14.44-02 Spaced as Shown End Bents No. 186 to conform to crown of 1 4=5-15012ds 12 7-#4-V7@12'cts 11 2" 38-#4-UI Spaced of Shown SECTION AT & (Ec. Face) Backwall above upper construction ELEVATION joints shall not be poured until the 6-95" superstructure slob has been poured 231-18" in the adjacent span. ELEVATION B-3 -Summ libt. & Bent except as shown -Elev. 997.26 at Top of Curb 23-18" Elev. 996.87 of Top of Wing 10'-118 - E Roadway 2010" 24 72" -Elev. 99742 of Top of Curb FILL Face of End Bent Broth 191x 3/Typ, & Bearing 10'-118" 10-118 - Elev. 997.08 10'-118" 10-118" 46-#5-VI \$ 12" cts. (Ea. Face) 5.45-12012ts at Top of Wing Const. It. SECTION THRU WING SECTION THRU WING Key 2'x4" C Shippe (Nominal) Note: Field bending shall be required of PART SECTION C-C PART SECTION J-J wings for #6! Dars in backwalls with Expansion Note: All reinforcing bors in tops of Device and for F bars when necessary substructure beams or caps shall to conform to slope of wing. See Sheet No. 12 & 13 for reinforcement 111-0" be spaced to clear anchor, bolts rElev 997.42 Elev. 997.26for bearings by at least 2". PLAN of end posts, parapets and curbs. Const. Joint-12-102" 26-42" 58 30° /- E Roadway SECTION D-D -£ Pile at Bott. of Beam Fill Face of End Bent-Key 21 x 4' Butt splice (if required) (Nominal) Top of lower section to be cut square -Elev. 989.36 581 16 10-4-U1 @obt. 12"cts 16"10-4-U1@obt. 12"cis. 16"10-4-U1 @obt. 12"cts. 16"74-U1@obt. 12'3-4U216"10-4-U2@obt. 12"cts. 16" 44 2" 76-#4-V8@12"cts. 12"5-#5-V6@12"cts. 3" 3: 88" E Pile 3: 6-58" (Ea. Face) (Ea. Face) PLAN OF BEAM STEEL PILE SPLICE CBELOW LOWER CONSTRUCTION JOINT) ELEVATION A-A DETAILS OF END BENT NO. I DETAILED FEB. 1973 A-2446 CHECKED July 1973 JACKSON COUNTY Note: This drawing is not to scale. Follow dimensions. Sheet No. 3 of 16.

Note: This drawing is not to scale. Follow dimensions.

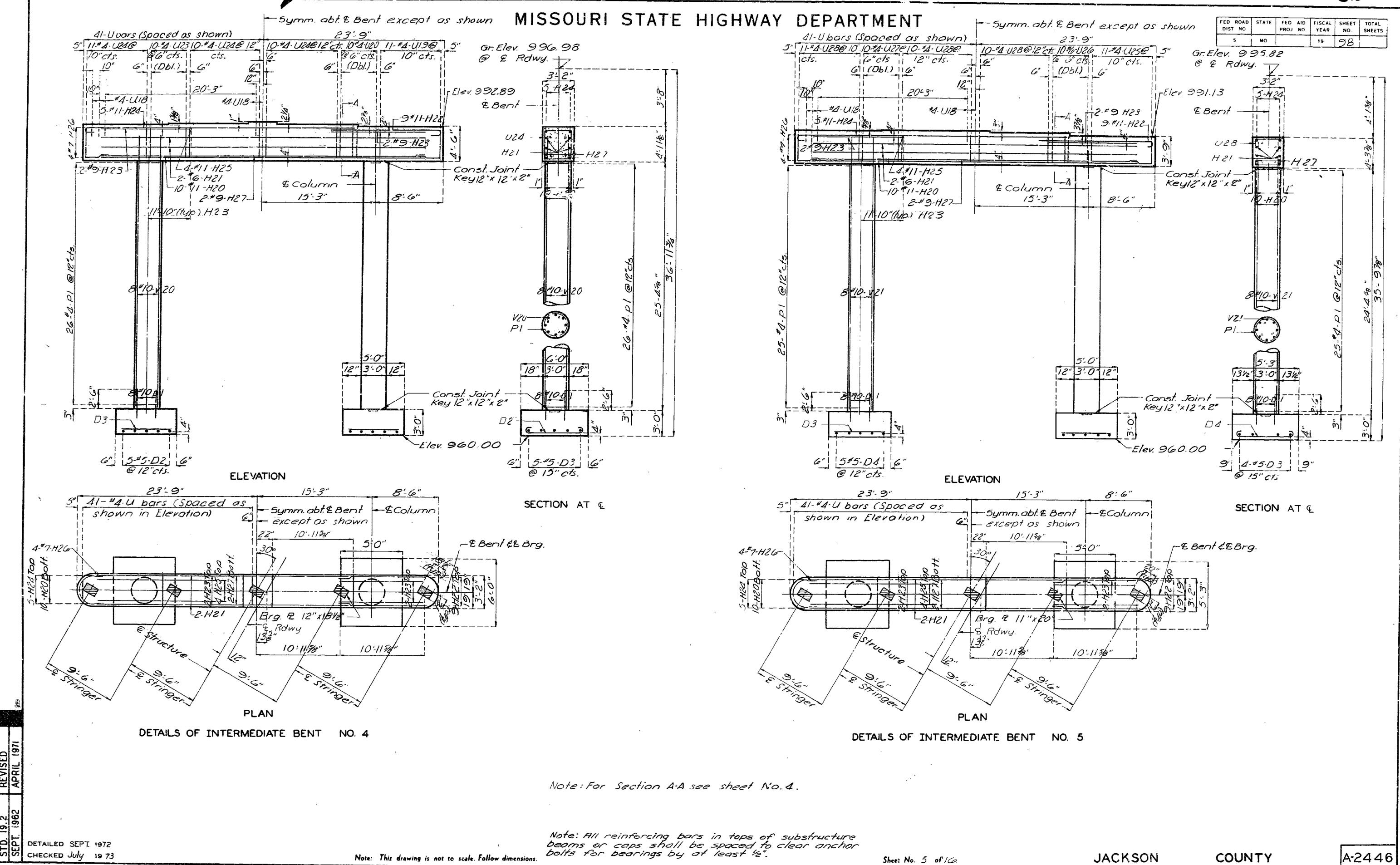
A-2446

JACKSON

Sheet No. 1 of 16.

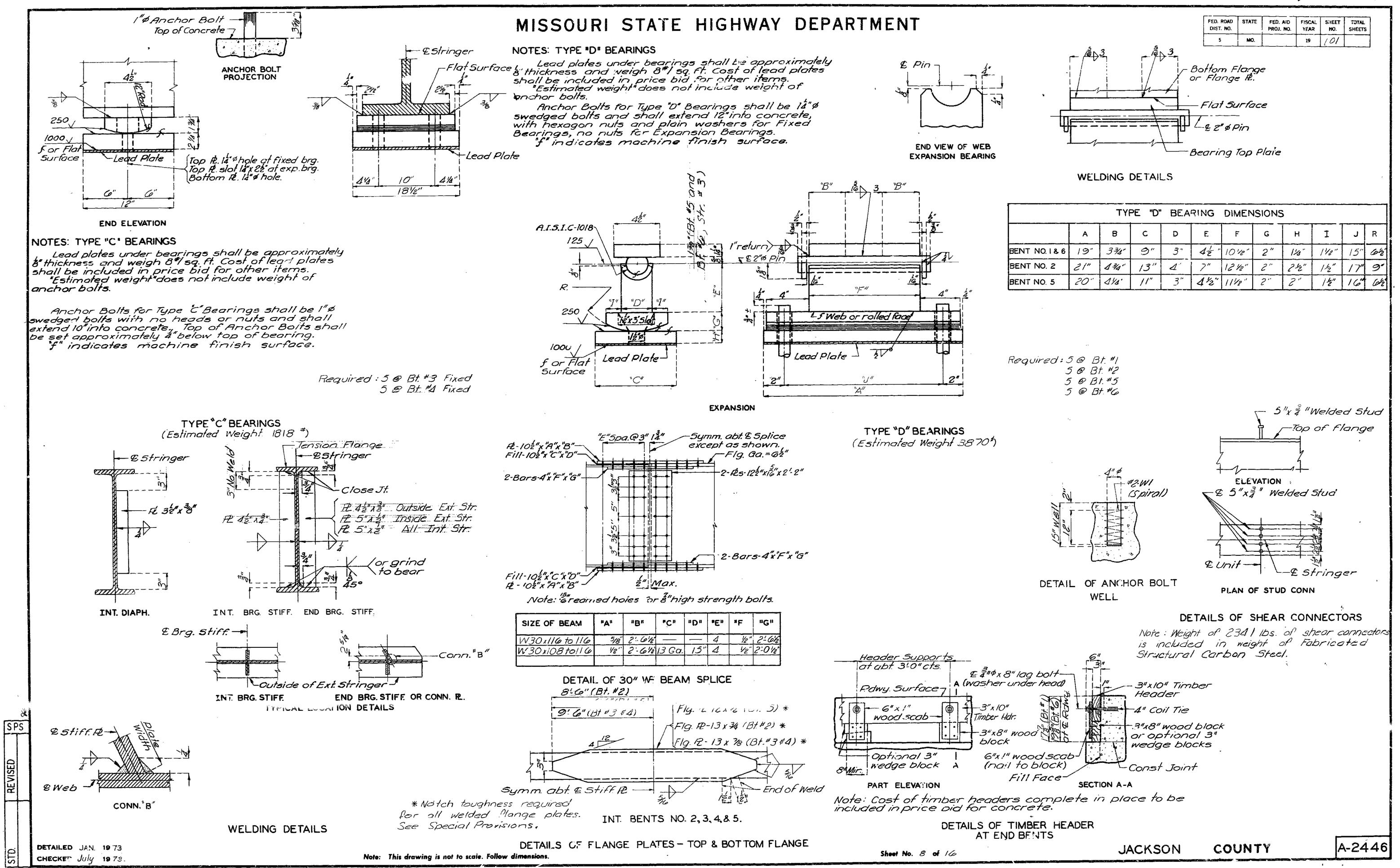
COUNTY

CHECKED July 19 73



FOR INFORMATION ONLY A24462, Sht. 7 MISSOURI STATE HIGHWAY DEPARTMENT 51-#5-U/3 Spaced same as V-Bars 11'-0" 12 22 5 VI @ 12 ds (Each Face) V2@12"cts 12" 24- #5-VI @ 12" cts (Each Face) Symm obt & Bent except as shown--Elev. 994.66 Gr. Elev. 994.86 @ & Roadway-Se SELLEN 9474 86 @FIII Face -2-#G-HG Pile Cut-off Elev. 988.89 Elev. 987.39-Note: Top of backwall and expansion device for End Bents No. 146 to conform to crown of @abt. 12"cts. @abt. 12" ats roadway slab. 3" 4 #5 VB0/26 12 7-# 4-VI40 12" cts. 30-#4-U12 Spaced as Shown 13-402 Spaced as Shown Spaced as Shown
ELEVATION Backwall above upper construction joints shall not be poured until the SECTION AT & superstructure slob has been poured in the adjacent span. ELEVATION F-F -Elev. 995.50 I Elev. 994.66 at Top of Wing at Top of Curb - Symm obt. E. Bent except as shown 23'-18" 4-35" 24-75" Elev. 995.01 at Top of Curb -Elev 99580 at Top of Wing FBrg R. 19" . 9" (Typ.) FILL Face of End Bent-& Bearing 10-113" 10'-118" 10:118" | 2:28 8-94" 46-#5-NI @ 12" cts. (Ea Face) 5-16 12012"cts -Const. It.-SECTION THRU WING SECTION THRU WING E St. Ber E SHIBOL PART SECTION H-H E Skillinger FART SECTION G-G Note: Field bending shall be required of Note: All reinforcing bors in tops of wings for #64 bors in bockwalls with substructure beams or caps shall Expansion Device and for F bars when be spaced to clear onchor bolts 1/-0" necessary to conform to slope of wing. for bearings by at least \$". See Sheet No. 12 \$13 for reinforcement Elev 995.50-PLAN of end posts, parapets and curbs. #6091cts 25-24" SECTION D-D Fill Face of -E Pile at Bott, of Beam End Bent --Const. Joint Butt splice (if required) Key 2" x 4" Top of lower section to be out squarey -Elev 987.39 52" 16" 10-"4-U12@abt 12" cts. 16" 10-"4-U12@abt 12" cts. 16" 9#4-U12@abt 12"cts. 12" 16" 8-#4-111@abt 12"cts. 12" 16" 10-#4-U2@abt 12" cts. 16" 44" 2 76-#4-V16012"cts 185-#5-V15012"cts 3" (Ea. Face) (Eo. Foce) -£ Pile-10'-2" 10'-2" 6'-55" PLAN OF BEAM
(BELOW LOWER CONSTRUCTION JOINT) STEEL PILE SPLICE ELEVATION E-E DETAILED FEB. 1973 DETAILS OF END BENT NO. 6 A-2446 JACKSON COUNTY CHECKED July 19 73 Sheet No. G of 16. Note: This drawing is not to scale. Follow dimensions.

253



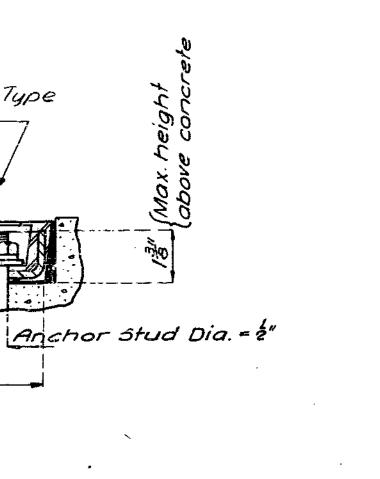
& Stud Bolt and Wing Type

Threaded Insert -

Anti-Skid Tread Design

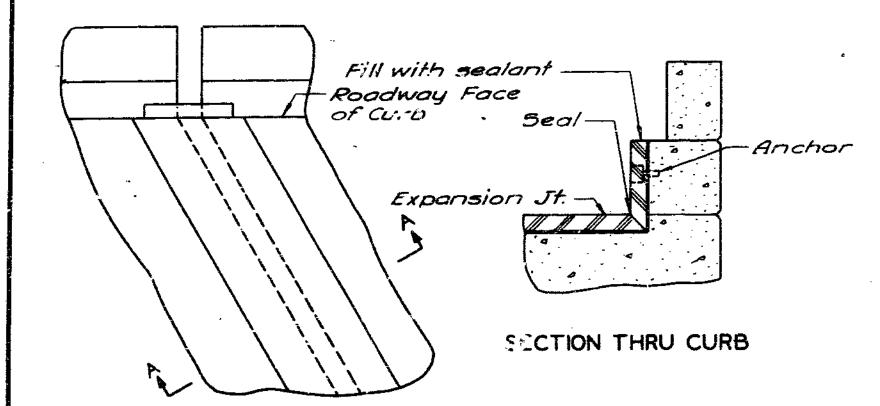
SECTION A - A

MISSOURI STATE HIGHWAY DEPARTMENT



P.I. Sta. 9+71.00 Elev. 1000.85 - Sto 7+70,00 Sto.11.40.00 -Fill Face Bt. 1 & Bt. #2 (£ Bt. #3 2 Bt. #4 Sta. 9132.00 Sto. 10-62.99 Sto. 10.00.00 Elev. 99739 Elev. 996.78 Elev. 937,54

PROFILE GRADE ELEVATIONS



Location	″A″	"B"	"C"	<i>"D"</i>
Berit No.1 to Splice (SI)	238	2	15/8	24
Splice (SI) to Splice (SE)	varies	varies	varies	varies
Splice (S2) to Splice (S3)	24	134	13/8	21/4
Splice(S3)to Splice(S4)	vories	varies	varies	raries
Splice(SA)to Bent No.6	218	13/1	1/2	21/4

74-Spa. @ 5:0 1.

SECTION THRU STRINGERS (Normal to & Roadway)

TABLE (F VARIA	BLE DIME	NSIONS
TEMP. °F	DIM. "A"	DIM. "B"	DIM. "C"
110	78"	98"	1"
90	8"	94"	18"
70	84"	10"	18"
60	82"	104"	18"
50	88"	108"	2"
40	84"	102"	28"
30	9"	103	28."
10	94"	11"	28"
-10	98"	118"	3"

Joint Seal for 2" movement

Note: Plan dimensions are based on installation at 60°F. Expansion joint width shall be adjusted during installation for compliance with the above table. See Special Provisions

Stringer*/	Ċ	1	offer.	100	Ó	1	100	130	اخ	K.	101/2	120	ان	<u>-7</u> 4		oj.	5	0	Ö	6	خا
Stringer"?	ő	igo	Ļ	E.	Ö	179	•	4		. 	Jo.		 		 			1/2	Ö	Ö	Ö
Stringer *3	0	12	êķ.	e'i	Ö	6/2	in	13	Ö	14	ok ²	7.4	5	"A	in	-	Ó	116	ō	Č	0
Stringer *4	ď	1/0	6,	13.	Ö	1.5	10	2/6	Ö	1/2	igo.	elle	Ō	1/4	1/2	de	Ö	110	0	0	Ó
Stringer "5	0	16,1	"B	ija Ija	Ö	15/6	3/10	0.0	Ó	10	19/10	eg.	Ö	14"	750	40	0	16,	0	Ö	0
	•																				_
E Brg. Stift		5/1	5/0	S/1	ي تو	S/1	S/4	34	چ چ	5/1	s _a	يري	<i>§</i> 4	S/A	S/A	5/4	3/1	مري	3/1	\$4 1	54
	İ	.S=						1:0				-				-		-	35		
		Sp	on	(1-,	2)	Sn	ar	(2.	3)	31	2 <i>0:</i>	263	? - 1	151	2 <i>01</i>	26	1-5).51	2 <i>0</i> 2	2/5	-6

| Spon(1-2)| Spon(2-3)| Spon(3-4)| Spon(4-3/Spon(3-6)

Spon(1-2) Spon(2-3) Spon(3-4) Spon(4-5) Spon(5-6)

DEAD LOAD DEFLECTION

Note: 10 % of dead load deflection due to weight of Structural steel

Stringer *1	140	24%	33.	100 mgs	34"	3%	3%	300	Sep.	To	S S	296	2%	3/4"	3/2	242"	12	23%	636	Rici	أة
Stringer *2	14,	2 mg	275	2/4.	21/6	29.	S	3/2	S. S. S.	13	29%	Str.	21.	215.	يان	200	4.0	113.	up.	1.11	4
Stringer #3	.je	21.	"#Z	ONO U	1,60	1.35	28.	246	77	13.45	23	/4" "	24"	2/3.	22	245	13.	1116	160	138	","
Stringer *4	12	23.	376	290	Sin	135	يار س	345	100	248"	250	140	25.	κ.	34"	28.	2to	115	13:	113	
Stringer*5	O.A.	35.	32,	3,4%	40	2	3,0	3%	10	28%	234	120	250	35."	34.	3/4	2/2	25.	The.	1/20	1,41
			. 1					7	B	9 t	<i>†.</i> 9	P	ک	10	5						
t of Top Flange — & Bry. Stiff.—			_	, ,		·						<u> </u>							<u></u>		,
t bry, stiff.		5/4	5/4	5/4	5/4	Sa	£/4	5/8	3/4	¼	5/1	Sá	S/A	S/4	5/4	5/4	SA	1/4	54	5/4	ş
	[.	5=	52	40'		5=	6	2'-0	~	S	68	3:0) [*]	5=0	23	C	"	5 =3	5	0"	

JACKSON COUNTY

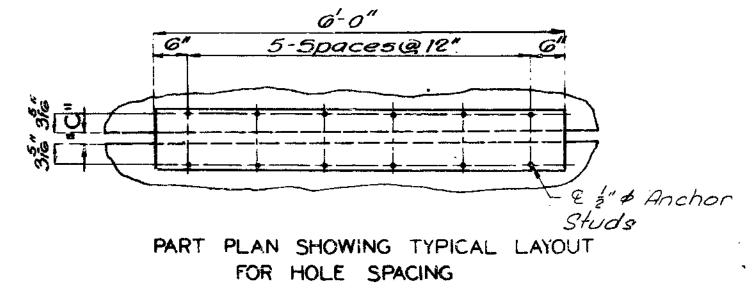
THEORETICAL SLAB HAUNCHING DIAGRAM

Note: The expansion joint shall be set, anchored, bonded and sealed as recommended by the manufacturer and as set forth in the Special Provisions. Archors shall be cast in place. Reinforcing steel shall be shifted as required to clear ancher studs.

Accurately locate the hole spacing for g"studs on both sides of the expansion void at a distance of 3% From the edge of the concrete and snap a chalk line on both sides of the expansion void. Layout transverse hale spacing along the chalk line in accordance with the shop drawings and the Typical Layout as shown on this sheet. Insure that the holes are directly apposite each other (square).

First section of expansion joint shall be installed starting at & of roadway. Tighten all nuts to 40 foot pounds. Retighten to 40 foot pounds 30 minutes after initial tightening. Wire brush bolt cavity and coat with sealant. Fill cavity with sealant to a depth of 2" and push plug down to snap lock. Scrape off all excess sealant.

Payment for furnishing and installing the expansion joint, including anchior bolt assembly, shall be made under unit price bid per lineal foot of joint.



DETAILS OF STEEL REINFORCED ELASTOMERIC EXPANSION JOINT SEAL AT BENTS NO. I \$ 6.

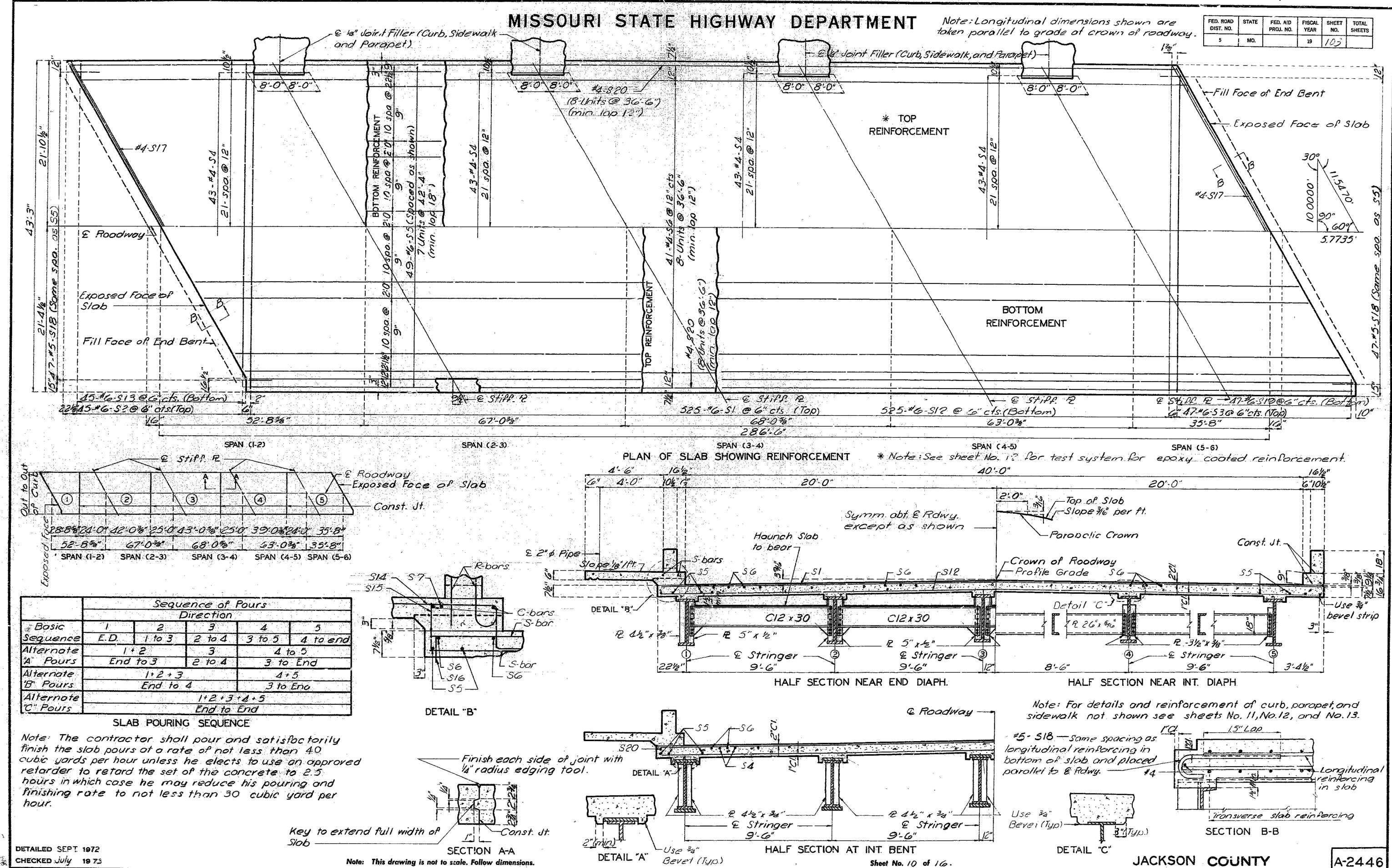
DETAILED JAN 1973 CHECKED July 19 72

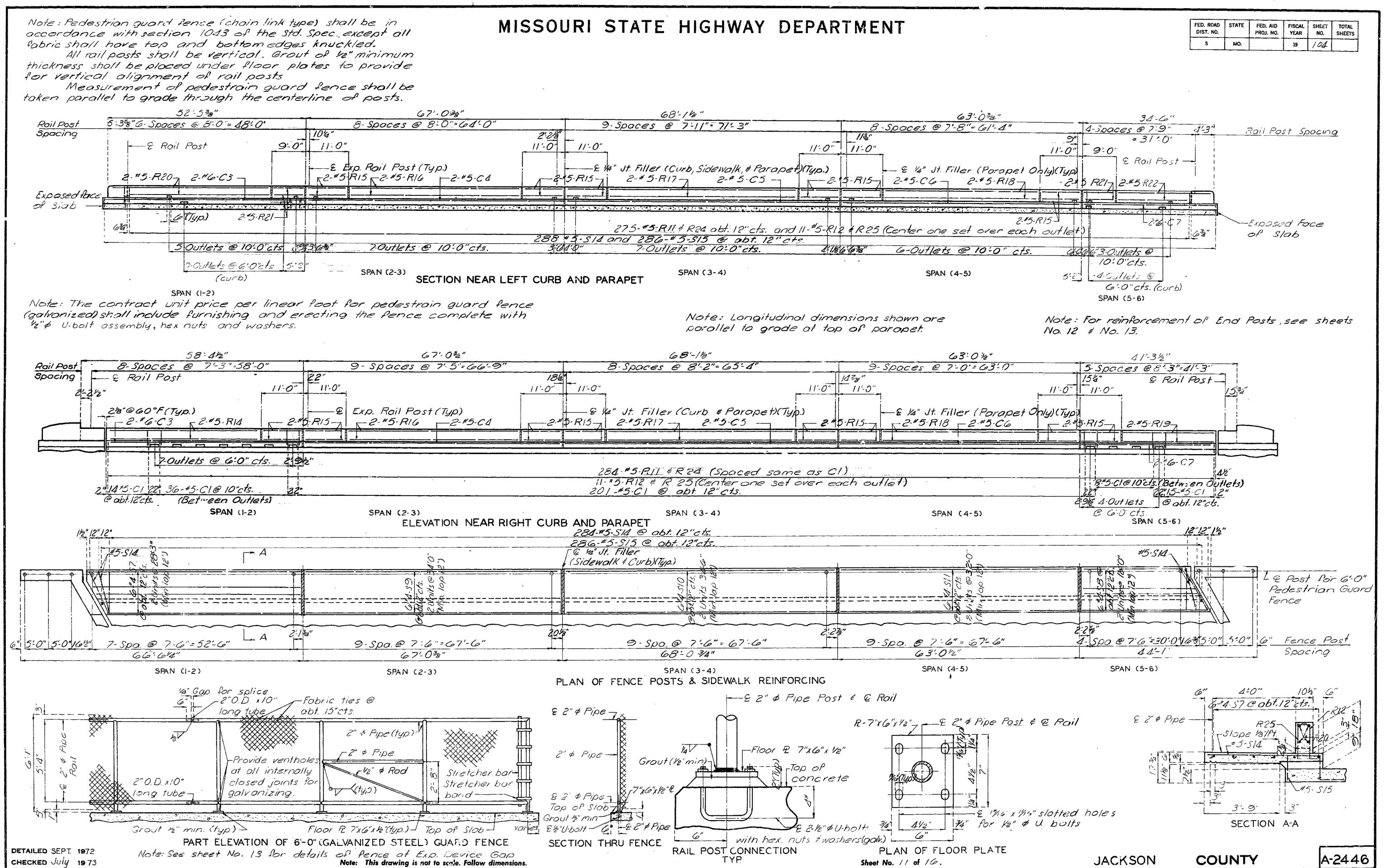
te" Sealant

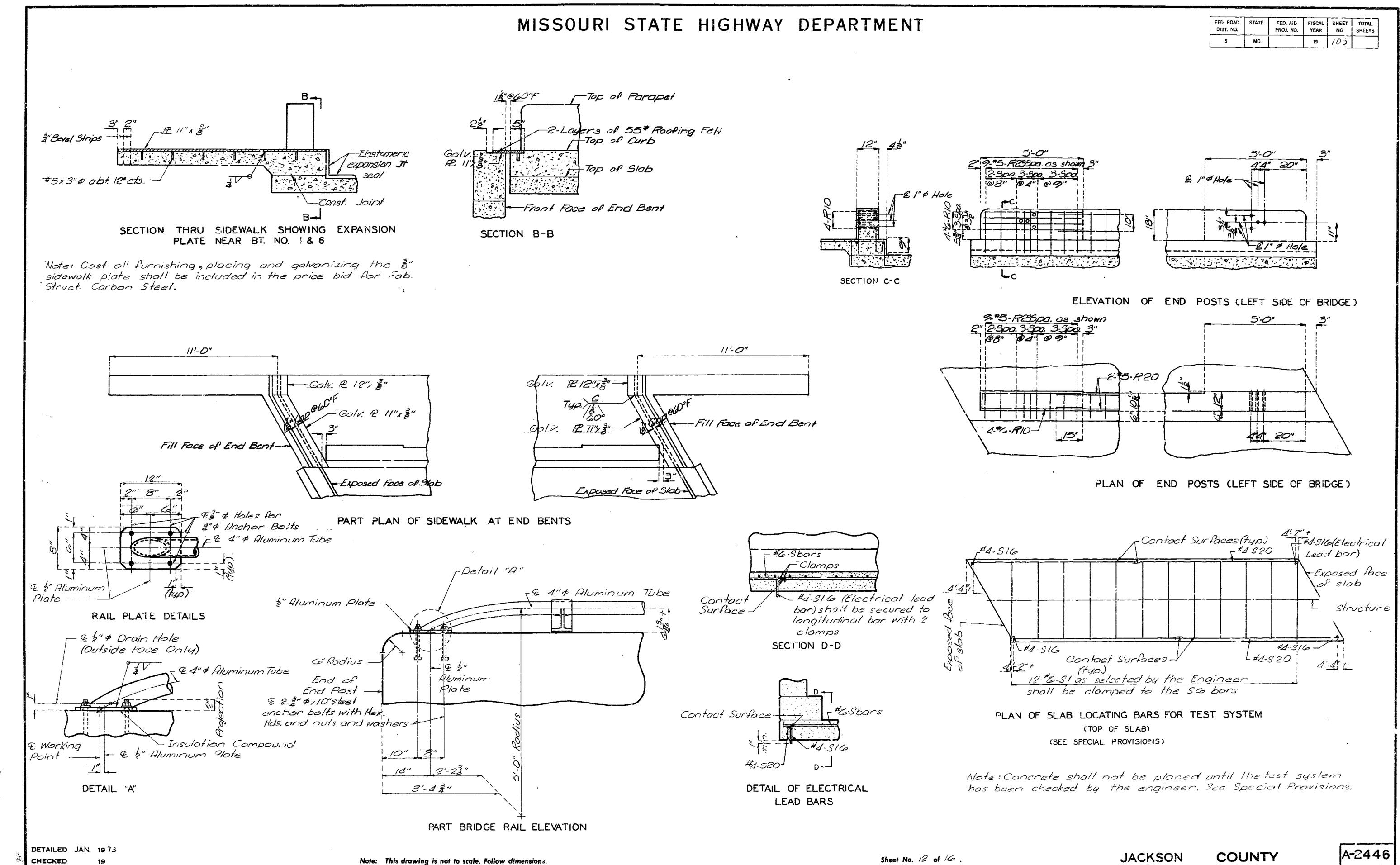
Note: This drawing is not to scale. Follow dimensions.

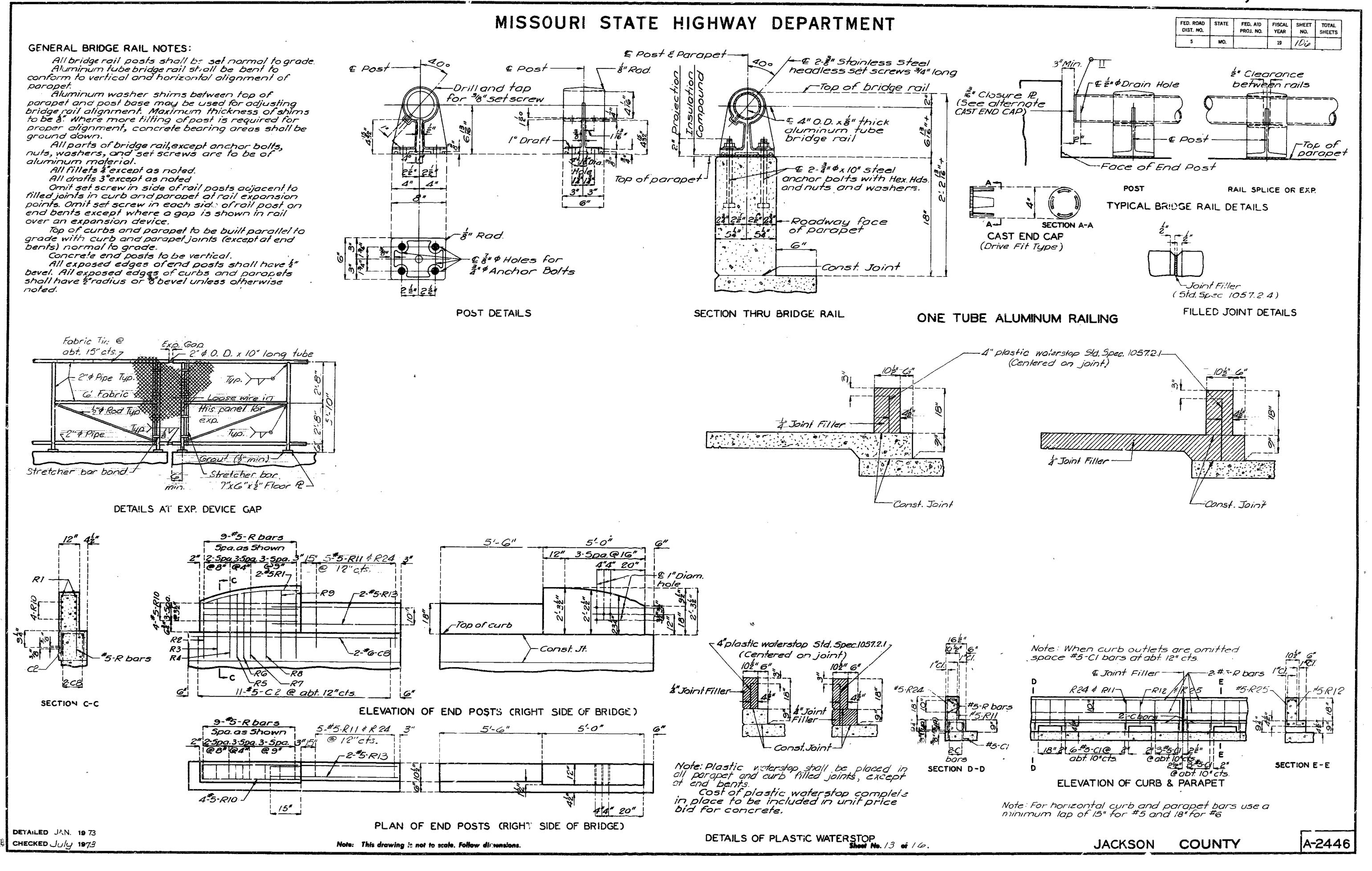
Sheet No. 9 of 16.

N





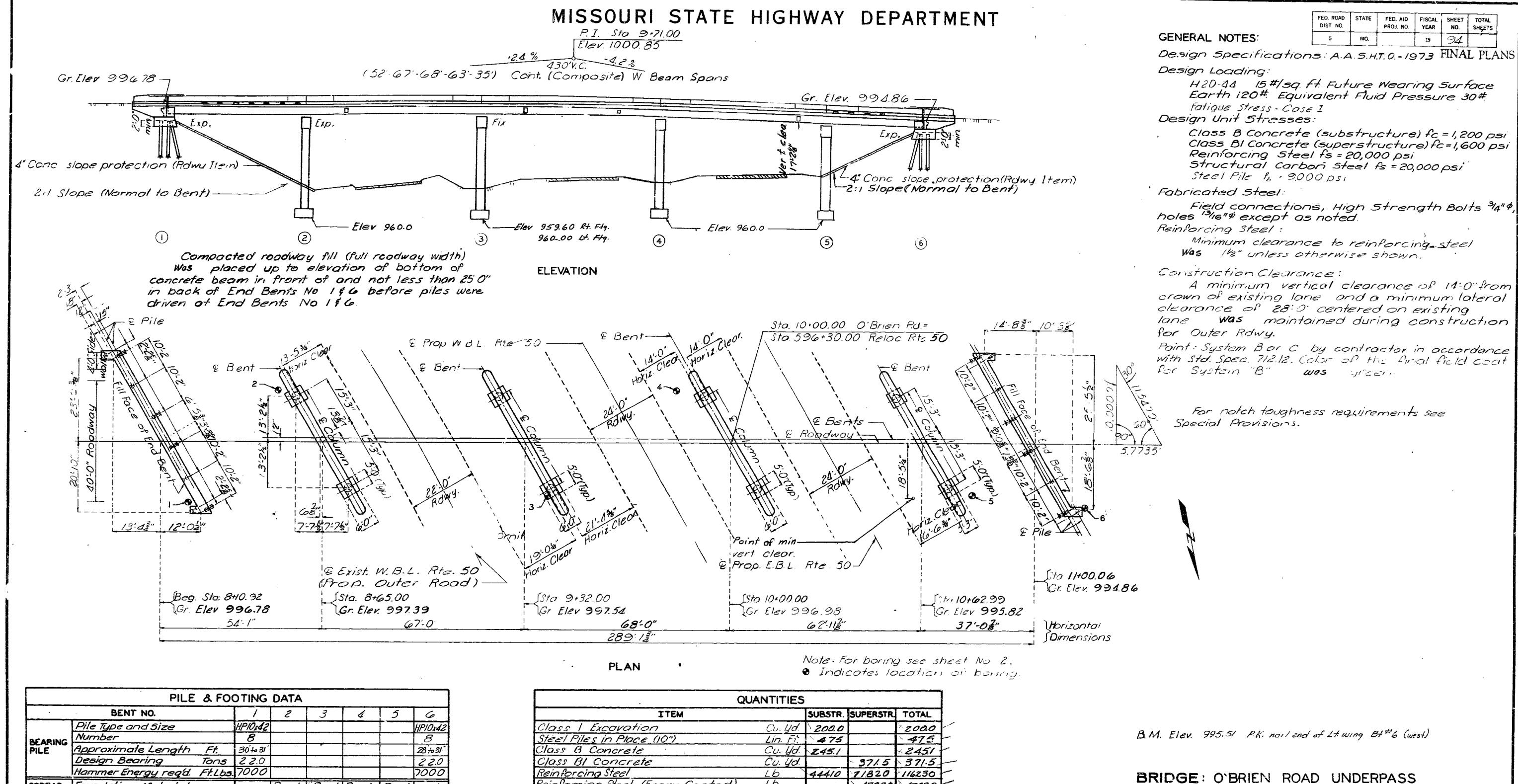




				М	ISSOURI STATE HIG	HWAY DEPARTMENT		FED ROAD STATE FED. AID FISCAL SHEET TOTAL
					REINFORCING STEEL			DIST NO PROJ NO YEAR NO SHEETS
" MA		3800	DIMENSIONS	NAL STH TH	MARK NO.	DIMENSIONS	A T T T T	STANDARD HOOKS STIRPUP HOOKS
REOD	LOCATION	PE N STR	B C D E F	WEIG WEIG	TOCATION NOITA OF EACH	B C D E F	NOMI LENG	Detailing Hook Detailing @dor 25 Min. 10 10 10 10 10 10 10 10 10 10 10 10 10
N SIZE	A A	STE	FT. IN. FT. IN. FT. IN. FT. I	N. FT. IN. FT. IN. FT. IN. FT. IN. LBS.	S S S S S S S S S S S S S S S S S S S	IN. FT. IN. FT. IN. FT. IN. FT. IN. FT.	IN. FT. IN. FT.IN. FT.IN. LBS.	25"Min - 180° 90° 0' 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1
	END BENT NO 1				10 503 FOOTING _ 20 X 4	6.000	4 6 4 5 47	5ize of 90 DEG Hooks 90 90 90 135° 5ize of 180 DEG Hooks All Grades 90 90 90 135° Grade 40 Ksi D=6d for #3 thru#8 ZBeam ZBeam D=5d for #3 thru#11 D=8d for #9 #0 d #11
3 6FI	ı	15 X	12.125 6 0.000 12.125 6.000 10.5 12.125 3 7.000 12.125 10.500 4.0			4.000	44 4 44 4 2355	END HOOK DIMENSIONS STIRRUP HOOK DIMENSIONS
3 612	2 WING	19 X	12.125 3 7.000 12.125 10.500 %.0	500 10.500 8.000 3 7 3 6 25	┃	7. 000	14 9 14 9 705	BAR GRADE 40 ALL GRADES 90° HOOK 135° HOOK
	1 BEAM 2 BEAM	18 X	53 2.000 ° 53 2.000 °	53 2 55 2 750 53 2 53 2 160	<u> </u>	3.000	18 10 18 10 256 37 5 37 5 994	#3 5" 23" 6" SIZE ("" A OF G A OF G H
4 9H3	B EAM	17 X	19 4.500	20 5 20 5 278	4 11H25 BEAH 17 X 13	7.000	14 9 14 9 313	#4 6" $3\frac{1}{2}$ " 8" #3 $1\frac{1}{2}$ " 4 " 4 " $2\frac{1}{2}$ " #5 7" $4\frac{1}{2}$ " 10" #4 2" $4\frac{1}{2}$ " $4\frac{1}{2}$ " 3 " #6 8" $5\frac{1}{4}$ " 12" #5 $2\frac{1}{2}$ " 6" $5\frac{1}{2}$ " $3\frac{3}{2}$ "
	BEAM BACKWALL	17 X 20 X	37 3.875 27 4.000	27 4 27 4 146	 	4.000 2 11.000	9 3 8 3 135 44 4 44 4 30 <i>1</i>	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$
L	BACKWALL	20 X	51 1.000 10 9.000	51 1 51 1 153 10 9 10 9 129	54 401 COLUMN 14 W 3	0.000		#9 12" 8" 19" Note: Uniess otherwe noted
	s WING	20 X V 2	 _ _ _	4 10 4 10	54 4P1 COLUMN 16 X 2	9.000	9 6 9 6 343	#10 13" 9" 22" diameter D is the same for all tends and hooks #11 14° 10" 2'0" on a var #14 2'2" 202" 2'-1" #18 2'-11" 2'-3" 3'-5"
ļ	INCR. = 32.000	20 X	10 1.500 7 2.500	10 2 10 2 68 7 3 7 3 22	┃ ╌╶ ╸ ┩┈┈┈┩┈┈┈	1.000 4 6.000 2 11.000 4 6.000 1.000 4 3.000 2 11.000 4 3.000	· · · · · · · · · · · · · · · · · · ·	
2 6H1	O MING	20 X	9 10.000	9 10 9 10 30	20 4U16 BEAM 13 S X 2	3.333 4 6.000 23.333 4 6.000	13 8 13 5 179	Note: All Standard Hooks and Bends other than 180 DEG. to be bent with some procedure as for 90 DEG. Standard Hooks.
2 6Hi	1 SIDEWALK	20 X	4 10.000	4 10 4 10 15	20 4U17 BEAM 13 S X 2 9 4U18 BEAM 10 X	6.000 2 11.000	12 10 12 7 168 3 11 3 9 23	Note: Hooks and hends shall be in accordance with the procedures as shown on this sheet.
	. WING	25 X	21.000 6 9.250 23.000 2 0.500 6 4.125 2 0.000	2 8.000 6 2.750 10 5 10 4 51		7.600		Nominal Lengths are based on out to out dimensions shown in bending diagrams and
	WING	19 X	5 8.000 2 11.000	2 6.500 5 9.750 10 5 10 4 31 8 7 8 5 25	16 10V18 COLUMN 20 X 28	7.500	28 8 28 8 1974	Payweights are based on Actual Lengihs.
2 614	VING	19 X	4 10.000 3 11.000	8 9 8 7 26	10 2WI BEAM 22 X 1 INTERMEDIATE BENT	2.000 9.125	19 9 19 9 33.	5~ stirrup. X~ bar is included in substructure quantities. Length ~ Total lengths are measured along
38 4U1	BEAM	13 S X	2 9.000 2 11.500 3 3.000 2 11.000	12 8 12 4 313	NO 3			centerline bar to the nearest inch. V~ bar dimensions vary in equal increments
<u> </u>	BEAM	13 S X	2 9.000 2 8.500 3 3.000 2 8.000 6.000 2 9.000	12 2 11 10 111 3 9 3 7 36		0.000	5 0 5 0 344 6 2 6 2 64	between dimensions shown on this line and the following line. No. Ea. ~ Number of bors of each length.
<u> </u>	BACKWALL BEAM	10 X	2 5.000 9.00Q 20.000 2 9.000	5 7 5 4 284 6 1 5 9 78	10 503 FOOTING 20 X 4 4	6.000	4 6 6 6 47	* All hooks and bends for shape No. 12 (only) are based on D=5d.
	BEAM	10 X	20.000 3 0.000	6 4 6 0 72	10 11H2O BEAM 20 X 44	4.000	44 4 44 4 2355	Gre based on 15 30.
	BEAM BEAM	11 X	5 7.000 2 9.000 4 0.00H 6 0.000 2 9.000 4 4.000	12 4 12 0 123 13 1 12 9 130		╼╼╼┈╬╼╼┈┈┈┈┼	44 4 44 4 266 14 9 14 9 705	BENDING DIAGRAMS
3 4 U9	KING	10 X	2 11.000 6.000	6 4 6 2 12	4 9H23 BEAM 20 X 18), 000	18 10 18 10 256	
2 4010	O WING	10 X	3 11.000 6.000	8 4 8 2 11	5 11H24 BEAM 17 X 36 1 4 11H25 BEAM 17 X 13	7.000	37 5 37 5 994 14 9 14 9 313	C C C E
	BACKWALL BACKWALL	20 X	5 7.000 6 1.000	5 7 5 7 536		4.000 2 11.000	8 3 8 3 135	SHAPE 6 SHAPE 7 K E D D D D D D D D D D D D D D D D D D
i i	WING	20 X	5 0.000	5 0 5 0 7	2 9H27 BEAM 20 X 444 2	4.000	244 4 444 - 301	
I	WING WING	20 X	5 10.000	4 2 4 2 6 5 10 5 10 49	54 4P1 COLUMN 16 X 2	9.000	9 6 9 6 343	B D H H K C F SHAPE IS B
5v6				5 5 5 5 50		6.000 2 11.000	3 11 3 9 23	* SHAPE 12 SHAPE 13 SHAPE 14 K SHAPE 16
14 447	, WING INCR. = 4.000 I	20 X V 2	4 0.600	4 0 4 0 28		1.000 4 3.000 2 11.000 4 3.000 3.333 4 3.000	15 1 14 10 109 13 2 12 11 173	A - SHAPE 17 B Leg H AB C F
12 448	WING INCR. = 5.375 I	20 X V 2	2 0.000	2 0 2 0	 	1.000 4 5.500 2 11.000 4 5.500	15 6 15 3 316	SHAPE 20 SHAPE 19 SHAPE 21 SHAPE 23
	BEAM	20 X	2 9.000	2 9 2 9 8	20 4U22 BEAN 13 S X 23	3.333 4 5.500 23.333 4 5.500	13 7 13 + 178	B Vertical B D H B
2 671	O BEAM	20 X	3 0.000	3 0 3 0 9	16 10V19 COLUMN 20 X 29 6	6.000	29 6 29 6 2031	HE C CHIHL K D C K C K
≥ 10 2W1	A B WELLS	22 X	12.000 9.125	19 9 19 9 33	10 2W1 bEAM 22 X 12	2.000 9.125	19 9 19 9 33	SHAPE 24 SHAPE 25 SHAPE 26 SHAPE 27
3	NO. 2	41			INTERMEDIATE BENT NO. 4			E HI B K D K F
20	FOOTING FOOTING	20 X		5 0 5 0 344	16 1051 FOOTING 20 X 5 0	0.000	5 0 5 0 344	B C D HE C E
	D AUG 1973	18 X	5 0.000	6 2 6 2 64	10 502 FOOTING 18 X 5 C	0.000	6 2 6 2 64	SHAPE 28 SHAPE 29 SHAPE 30
CHECKED				Note: This drawing is not to scale. Follow dimensi	ons.	Sheet No. 14 of	16.	JACKSON COUNTY A-2446

		- Na			COMPLETE BU		SSOURI STAT	E HIGH	WAY	DEPAR	RTMENT				FE∵ R C ST ⊢ - 5	NO PPOJ NO YEAR NO SHEETS MO 19 / 08
	(6) (S) (S)		DOSCALCIONO		COMPLETE BIL	L OF	REINFORCING STEEL						[<u> </u>	STANDARD HOOKS	STIRRUP HOOKS
MARK NO.	PE NO.		DIMENSIONS		MINA NGT!	EIGH	MARK NO. LOCATION	0 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8		DIME	NSIONS	,,	NOMINAL LENGTH ACTUAL LENGTH	IGHT	Detailing Hook Detailing Dimension AorG Dimension	6d or 22°Min.
ARK TOCATION	<u> </u>	U U	E	n	K RO REP	3	ARK ARK	HAPE N CBSTR CBSTR O EA.		U		"			Ador 180° 90° 21	Signal H
2 0	ο ο ο ο ο ο ο ο ο ο ο ο ο ο ο ο ο ο ο	N. FT. IN. FT. IN	FT. IN. FT. IN	N. FT. IN.	FT. IN. FT. IN. FT. IN.	LBS.	Z 0 ∑ 4 9H1 BEAM	の いい ラZ FT I 16 X 53 2.0		FT. IN. FT	IN. FT. IN	FT. IN.	FT. IN. FT. IN. FT. IN.	LBS.	22"Min. 180 90 F	135°
101 503 FOULTING		,,,,			7 0 7 0		2 6H2 BEAM	20 X 53 2.0	00				53 2 53 2	160	Size of 90 DEG Hooks 5ize of 180 GEG Hooks All Grades Grade 40 KSI D=6d for #3 thru D=5d for #3 thru #11 D=8d for #9, 10 4 #1 D=10d for #14 & #18 D=10d for #14 & #1 END HOOK DIMENSIONS	#8 LeBeam LeBeam STIRRUPS
10 11H20 BEAM	20 X 44 4.0				44 4 44 4	2355 266	8 4H5 BACKY LL 2 6H6 BACKWALL	20 X 27 4.0 20 X 51 1.0					27 4 27 4 51 1 51 1	146 153		STIRRUP HOOK DIMENSIONS
4 6H21 BEAM 9 11H22 BEAM	20 X 44 4,0				14 9 14 9	705	e 6H7 WING	20 X 10 9.0					10 9 10 9		BAR GRADE 40 ALL GRADES	GRADES 40-50-60 Ksi 90° HOCK 135° HOOK
4 9H23 BEAM	20 X 18 10.5				19 10 13 10	256	2 6H11 SIDEWALK 4 9H12 BEAM	20 X 4 10.0 17 X 16 4.0					4 10 4 10	!5	#3 5" 23" 6"	BAR D HOOK HOCK APPROX.
5 11H24 BEAM 4 11H25 BEAM	17 X 36 3.0			+	14 9 14 9	994 313	4 9H13 BEAM	20 X 13 1.0					17 4 17 4			#3 $1\frac{1}{2}$ " 4 " 4 " $2\frac{1}{2}$ " #4 2 " $4\frac{1}{2}$ " $4\frac{1}{2}$ " 3 "
8 7H26 BEAM	7 X 3 4.0	 			8 3 8 3	135	4 OHLA SEAM	17 X 29 11.0					30 11 30 11	420	#6 8" 5" 12" #7 9" 6a" 14"	#5 $2\frac{1}{2}$ " 6 " $5\frac{7}{2}$ " $3\frac{3}{4}$ " #6 3 " $6\frac{7}{2}$ " $6\frac{7}{2}$ " $4\frac{7}{2}$ "
2 9H27 BEAM	20 X 44 4.0	000			44 4 44 4	301	2 6H15 WING 2 6H16 WING	20 X 6 1.0					9 3 9 3	18 28	#8 10" 7" 16"	Note: Uniess otherwise noted
52 4P1 COLUMN	16 X 2 9.0	000			9696	3,30	2 0H17 WAG	20 X 7 9.0	00				7 9 7 9	23		diameter D is the same for all bends and hooks
							2 6H18 WING 2 6H19 WING	20 X 10 2.0 20 X 5 3.0	_ 				10 10 2 5 3 5 3	31	#10 13" 9" 22" #11 14" 10" 2'0" #14 2'2" 202" 2'7" #18 2'1!' 2'3" 3'-5"	on a bar.
9 4U18 BEAM	10 X 2 11.0	6.000 2 11.000 60 4 3.000 2 11.000	 		3 11 3 9 15 1 14 10		2 0127 1110	20 1					7 3 7 3		1	s and Bends other than
20 4U20 BEAM	13 S X 23.3	33 4 3.000 23.333	4 3.000		13 2 12 11	173	2 675 WING		00 6 4.750	 			5 11.000 10 2 10 1	30	180 DEG. to be beni with	h same procedure as
20 4U23 BEAM 31 4U24 BEAM		33 4 5.000 23.333 00 4 5.000 2 11.000	· 		13 6 13 3	177 314			00 23 2.875	-		6 1.500	22 5.000 27 3 27 2 7 7 7 5	82	Note: Hooks and hends	s shall be in accordance is shown on this sheet.
JI 4024 DEAR	13 S X 2 11.0	3,000 2,11000	75000				2 6T8 WING	19 X 5 8.0	3 11.000				9 7 9 5	28	Nominal Lenaths of	are based on out to out
16 10V20 COLUMN	20 X 27 9.0	00			27 9 27 9	1911	13 4UZ BEAM	13 5 /1 2 9.0	00 2 8-500	3 3.000 2	8-000	<u> </u>	12 2 11 10	103	i Pouveinte ora	bending diagrams and ators use. Dased on Actual Lengths.
10 2W1 BEAM	22 X 12.0	00 9.125			19 9 19 9	33		1G X		2 9.000	-		3 9 3 7		5~ stirrup.	in substructure quantities
INTERMEDIATE]							10 X		2 9.000			6 1 5 9	78	Length ~ Total leng centerline bar to the r	in substructure quantities. gths are measured along nearest inch.
NO. 5	20 X 5 0.0	00			5 0 5 0	344		10 X 11 X		3 0.000 4 2 9.000 4	0.000		12 4 12 0	123	lbetween dimensions st	vary in equal increments nown on this line and
8 503 FOOTING	[00			4 6 4 6.	38	5 7U8 BEAM	11 X	6 0.000	2 9.000 4	4.000		13 1 12 9	130	the following line. No. Ea. ~ Number o	of bors of each length.
10 504 FOOTING	18 X 3 9.0	00			4 11 4 11.	51	2 4U9 WING :	10 X	2 11.000 3 11.000	╃────			8 4 8 2	8	* All hooks and ben are based on D = Ed.	nds for shupe No.12(only)
10 11H2O BEAM	20 X 44 4.0	4. ,			44 4 44 4	2355		13 S X 2 9.0		3 3.000 3	0.875		-2 11 12 8	76		
4 6H21 BEAM	20 X 44 4.0	00			44 4 44 4	266		13 S X 2 9.0		3 3.000 3	4.000		13 6 15 2	264	BENDING	DIAGRAMS
9 11H22 BEAM 4 9H23 BEAM	20 X 13 7.0			-	14 9 14 9 18 10 18 10		51 5U13 BACKWALL	10 X	2 7.000	9.000			5 11 5 8	301		B E B B
5 11H24 BEAM	17 X 36 3.0	00			37 5 37 5	994	92 5V1 BACKWALL	20 X 5 7.0	00				5 7 5 7	536	B	HCCCEE
4 11H25 BEAM 8 7H26 BEAM	17 × 13 7.0				14 9 14 9 B 3 8 3	313	10 5V2 BACKWALL 2 6V9 BEAM	20 X 6 1.0 20 X 2 9.0					2 9 2 9	63 8	SHAPE 6 SHAPE 7	D E D
2 SH27 BEAM	20 X 44 4.0	00 2 11.000			44 4 44 4	30 <i>i</i>		20 X 4 0.0					4 0 4 0	5	B - B -	8 SHAPE 9 SHAPE 10 SHAPE 11
								20 X 5 2.0					5 2 5 2	7	B D C E	THE DATE
50 4P1 COLUMN	16 X 2 9.0	00			9696	317	8 5V13 WING 2	20 X V 2 2 2.0					2 2 2 2			SHAPE 15
9 4U18 BEAM	10 x	6.000 2 11.000			3 11 3 9	23	INCR. = 3.875 IN.	4 1.0	00				4 1 4 1	29	* SHAPE 12 SHAPE 13 " SHAPE 1	
11 4U25 BEAM 20 4U26 BEAM		00 3 6.000 2 11.000			13 7 13 4	98 153	10 5V15 WING 2	20 X V 2 2 1.0					2 1 2 1	60	A + SHA: E 17 B Leg	H B Spot Weld #2 bors D/E H AB
20 4U27 BEAM			3 6.000 3 9.875		12 3 12 0	160	INCR. = 5.250 IN.	4 3.0					4 3 4 3	25	1	SHAPE 21 SHAPE 23
31 4:J28 BEAM	13 S X 2 11.0	00 3 9.875 2 11.006	3 9.875		14 3 14 0	290	2 6V17 BEAM 2	20 X 3 3.0	10				3 3 3 3	10	B Vertical	
16 10V21 COLUMN	20 X 26 8.0	06			26 8 25 8	1836	10 2W1 A B WELLS 2	22 X 12.0	0 9.125				19 9 19 9	33	BH C CHH LEG	B D H B D H
							SUPERSRUCTURE								K D K K D SHAPE 24 SHAPE 25	SHAPE 26 SHAPE 27
10 2W1 BEAM END BENT NO 6	12.0	00 9.125			19 9 19 9	33	272 5C1 CURE 1 22 5C2 CURB 1	10	12.750	 	6.000		3 9 3 5	969 71	ĘĘ, K	
3 6F1 WING	15 X 12.1	25 6 0.000 12.125	6.000 10.500	0 6.000	10.500 8 0 8 0	36	4 6C3 CURB 2	20 53 10.0					53 10 53 10	323	D B E H	B K D K F
2 6F2 WING	15 X 12.1	25 3 7.000 12.125	10.500 6.000	0 10.500	6.000 5 7 5 6	17	4 5C4 CURB 2	20 66 9.0					66 9 66 9	278	▋	н с Е
			<u> </u>				4 5C5 CURB	20 67 11.0	0				67 11 67 11	285	SHAPE 28 SHARE 29	SHAPE 30
DETAILED AUG 1973 CHECKED Aug 1973					ring is not to scale. Follow						Sheet No. /			•	JACKSON CO	OUNTY A-2446

		MISSOURI STATE HIGHWAY DEPA	DTMENT	FED ROAD STATE FED AID FISCAL SHEET TOTAL
		COMPLETE BILL OF REINFORCING STEEL		DIST NO PROJ. NO YEAR NO SHEETS
MARK	DIMENSIONS	AT AT TO MARK O (S) (S) DI	MENSIONS 볼드 로드 노	STANDARD HOOKS STIRRUP HOOKS
MARK ONO. LOCATIO		MEG P C C D B C C C C C C C C C C C C C C C C	NOMIN LENGT LENGT	Detailing Hook Detailing 6d or 000 10 10 100 100 100 100 100 100 100
AARK	N SUBSTANCE D E F H	ET TO ET IN LEG C Z & TEMPERO		4 d or 180° 90° 01 81 8 Aoro 8 22 Ming
Ζ ω ≥	0 0 0 0 2 1 0 2 0 0 0 0 0 0 0 0 0 0 0 0	569 5 R24 PARAPET 19 2 0.375 7.000	2 7 2 6 1480	
4 5C6 CURB	20 62 9.000	62 9 62 9 262 22 15 R25 PARAPET 19 19.000 6.000	2 1 2 0 46	5ize of 900EG Hooks 0 90° 00° 135° 5ize of 1800EG Hooks All Grades Grade 40 Ksi 0=6d for #3 thru#8 2 & Beam 0=5d for #3 thru#11 0=8d for #9, 10 \$ #11 0=10d for #14 \$ #18 0=10d for #14 \$ #18 STIRRUPS
4 6C7 CURB 4 6C8 CURB	20 36 10.000	36 10 36 10 221 (6 4 5 20 SLAB 20 36 6,000 10 9 10 9 64	36 6 36 6 39	END HOOK DIMENSIONS STIRRUP HOOK DIMENSIONS
				BAR GRADE 40 ALL GRADES BAR D 90° HOOK 135° HOOK
4 5R1 END POST 2 5R2 END POST	20 4 9.000 2 8.125 9.000	6 1 5 10 12		#3 5" 22" 6" SIZE (17.) HOOK APPROX.
2 5R3 END POST	10 2 10.500 0.900	5 10 5 7 12		$*5$ 7" $4\frac{1}{2}$ " 10" $*4$ 2" $4\frac{1}{2}$ " $4\frac{1}{2}$ " 3 "
2 5R4 END POST	10 3 1.000 9.000	6 11 6 8 14 7 1 6 10 14		$\begin{array}{c ccccccccccccccccccccccccccccccccccc$
2 5R5 END POST 2 5R6 END POST	10 3 2.000 9.000 10 3 2.750 9.000	7 3 7 0 15		#8 10" 7" 16" Note: Unless otherwise noted
2 5R7 END POST	10 3 3.375 9.000	7 4 7 1 15		for all bends and hooks
2 5RB END POST	10 3 4.250 9.000 10 3 4.625 9.000	7 6 7 3 15 7 6 7 3 30		#14 2 [!] -2" 20½" 2 [!] -7" #18 2 [!] -11" 2 [!] -3" 3 [!] -5"
16 SR10 END POST	10 4 9.000 7.750	10 2 9 11 166		Note: All Standard Hooks and Bends other than
509 SRIL PARAPET	12 2 0.375 7.000	2 7 2 4 54		180 DEG. to be bent with same procedure os for 90 DEG. Standard hooks.
22 SR12 PARAPET 8 SR13 PARAPET	20 6 9.000 19.000 6.000	6 9 6 9 56		Note: Hooks and hends shall be in accordance with the procedures as shown on this sheet.
4 5R14 PARAPET	20 41 7.000	41 7 41 7 174		Nominal Lengths are based on out to out dimensions shown in bending diagrams and are listed for fabricators use.
56 5R15 PARAPET 8 5R16 PARAPET	20 10 9.000	10 9 10 9 628		Payweights are based on Actual Lengths,
8 5R17 PARAPET	20 45 11.000	45_11_45_11383_		5~ stirrup. X~ bar is included in substructure quantities.
8 5R18 PARAPET 4 5R19 PARAPET	20 40 9.000 20 2 <i>d</i> 3.000 -	24 3 24 3 101		X~ bar is included in substructure quantities, Length ~ Total lengths are measured along centerline bar to the nearest inch.
4 5R20 PARAPET	20 39 8.000	39 8 39 8 166		V-bar dimensions vary in equal increments between dimensions shown on this line and the following line.
8 5R21 PARAPET	20 6 9.000	8 9 8 9 73		No. Ea Number of bars of each length.
4 5R22 PARAPET 18 5R23 PARAPET	20 21 9.000 12 2 0.375 10.000	21 9 21 9 91 5 6 5 4 100		* All hooks and bends for shape No.12(only) are based on D=5d.
* 527. 6S1 SLAB	20 43 0.000	43 0 43 0 34037		
* 45 652 SLAB	75 IN. 40 11.000	2 10 2 10 40 11 1478 40 11 1478 40 12 40 11 1478 40 11 1478		BENDING DIAGRAMS
* 47, 653 SLAB	20 V / 2 5.000	2 5 2 5		
INCR. = 10.		42 3 4 2 3 1576		H B C C C E
* 172 454 SLAB	20 16 0.000	16 0 16 0 1838 42 4 47 4 9700		SHAPE 6 SHAPE 7
*330 456 SLAB	20 36 6.000	36 6 36 6 8046		SHAPE 8 SHAPE 9 SHAPE 10 SHAPE 11
12 457 SIDEWALK 12 658 SIDEWALK	20 28 3.000 20 18 0.000	28 3 28 3 226 18 0 18 0 144		B A D C E B D E
				C D C K SHAPE IS SHAPE IS
12 459 SIDEWALK	20 34 6.000	34 0 34 0 273		* SHAPE 12 SHAPE 13 SHAPE 14 K Shape 16
12 4511 SIDEWALK	20 32 9.000	34 6 34 6 277 E E E E E E E E E E E E E E E E E E		A - SHAPE 17 B Vertical H B 2 B H AB C F
525 6512 SLAB	20 43 0.000	43 0 43 0 33908		SHAPE 20 SHAPE 19 SHAPE 21 SHAPE 23
45 6513 SLAB	20 V 1 2 10.000 40 11.000	2 10 2 10 40 11 40 11 1478		B Vertical B D H
788 5514 SIDEWALK	23 5 8.000 14.000 15.000 15.000 0.010 4 8.000	0.010 8 1 7 10 2353		C C H H C C K C K
0 286 5515 SIDEWALK 4 4516 SLAB	20 15.000 8.500 6.000	15 15 373		SHAPE 24 SHAPE 25 SHAPE 26 SHAPE 27
2 4S17 SLAB	20 46 9.250	46 9 46 9 62		- K
94 5518 SLAB	17 22.000	2 5 2 5 237		D B K D K F
47 651948 1NCR. = 10.3	20 V 1 2 5.000 5 IN. 42 3.000	2 5 2 5 42 3 42 3 157 U		BE C HET C E H
2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	* Bors to be coated with B	<u></u>	<u> </u>	
CHECKED AUG 1973	Note: This dray	vinctuded for festing purposes. ving is not to scale. Follow dimensions.	Sheet No. 16 of 16.	JACKSON COUNTY A-2446



	PILE & FUL	DIING	DAIA				1
•	BENT NO.	/	2	3	4	5	6
	Pile Type and Size	HP10142					HPIOx42
BEARING	Number	8					8
PILE	Approximate Length Ft.	30 to 31'					28 +031
	Design Bearing Tons	22.0					22.0
	Hammer Energy read Ft.Lbs.	7000					7000
SPREAD	Foundation Material		ROCK	Rock	Rock	Rock	,
FOOTINGS	Design Bearing Tons / Sq. Ft.		9.7	9.7	9.7	10.5	

Minimum energy requirement of hammer based on plan length and design bearing value of piles.
All pile were driven to Absolute refusal.

, WO/		,		
ITEM		SUBSTR.	SUPERSTR.	TOTAL
Class Excavation	Cu. Yd.	200.0		× 200.0
Steel Piles in Place (10")	Lin. Fr.	475		475
Class B Concrete	Cu. Yd.	Z45.1		× 245.1
Class BI Concrete	Cu. yd.		371.5	371.5
Reinforcing Steel	16	44410	7/820	· 116230
Reinforcing Steel (Epoxy Coated)	Lb.		47220	47220
Fabricated Structural Carbon Steel	Lb.		2/9070	219010
Painting	Tons		108.4	108.4
Bridge Rail (One Tube)	Lin. Ft.		583	583
Pedestrian Fence (72")	Lin Ft.		309	309
Steel Reinf. Elastomeric Exp. Jt. Seal(2'	1) Lin Ft.		94	94
rst Holes (Contingent Item)	Lin Ft.	\ 3Z		32
Handling Reinforcing Steel (Contingent Item) F.A.		723.71	723.71
Vote: All concrete and reinford	cement	in er	od posts	POFO

and curbs was included with superstructure quantities.

See Special Provisions for Epoxy cooted reinforcing

STATE ROAD: RELOCATED ROUTE 50

ABOUT: NEAR UNITY VILLAGE

PROJECT NO. U-50-1 (9)

STA. 596+30.00

JOB NO. 4-U-50-27A

RTE. 50

JACKSON

COUNTY

STD. 611.60 STD. 706.30

DESIGNED May 1972 DETAILED FEB 1973

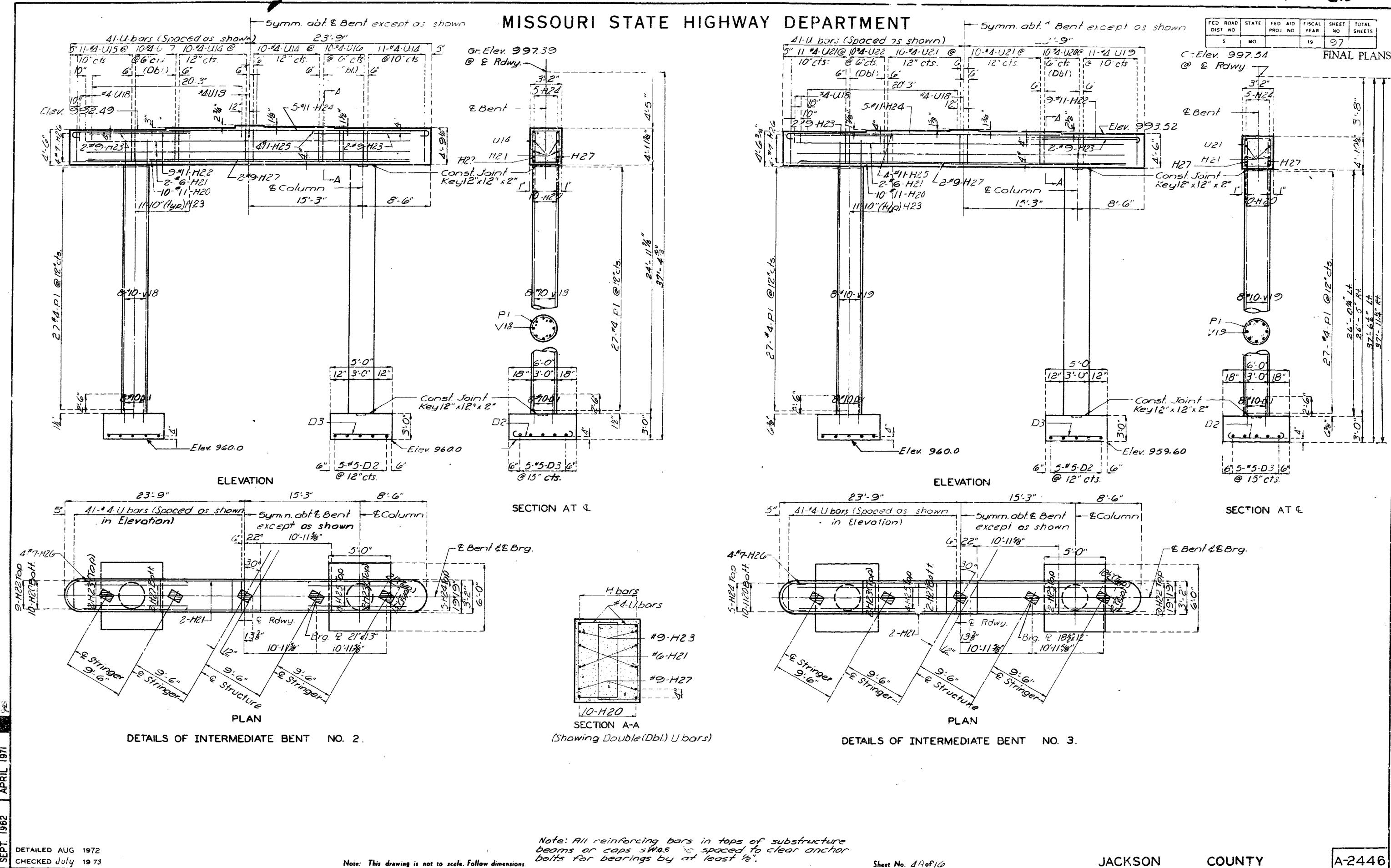
CHECKED July 1973

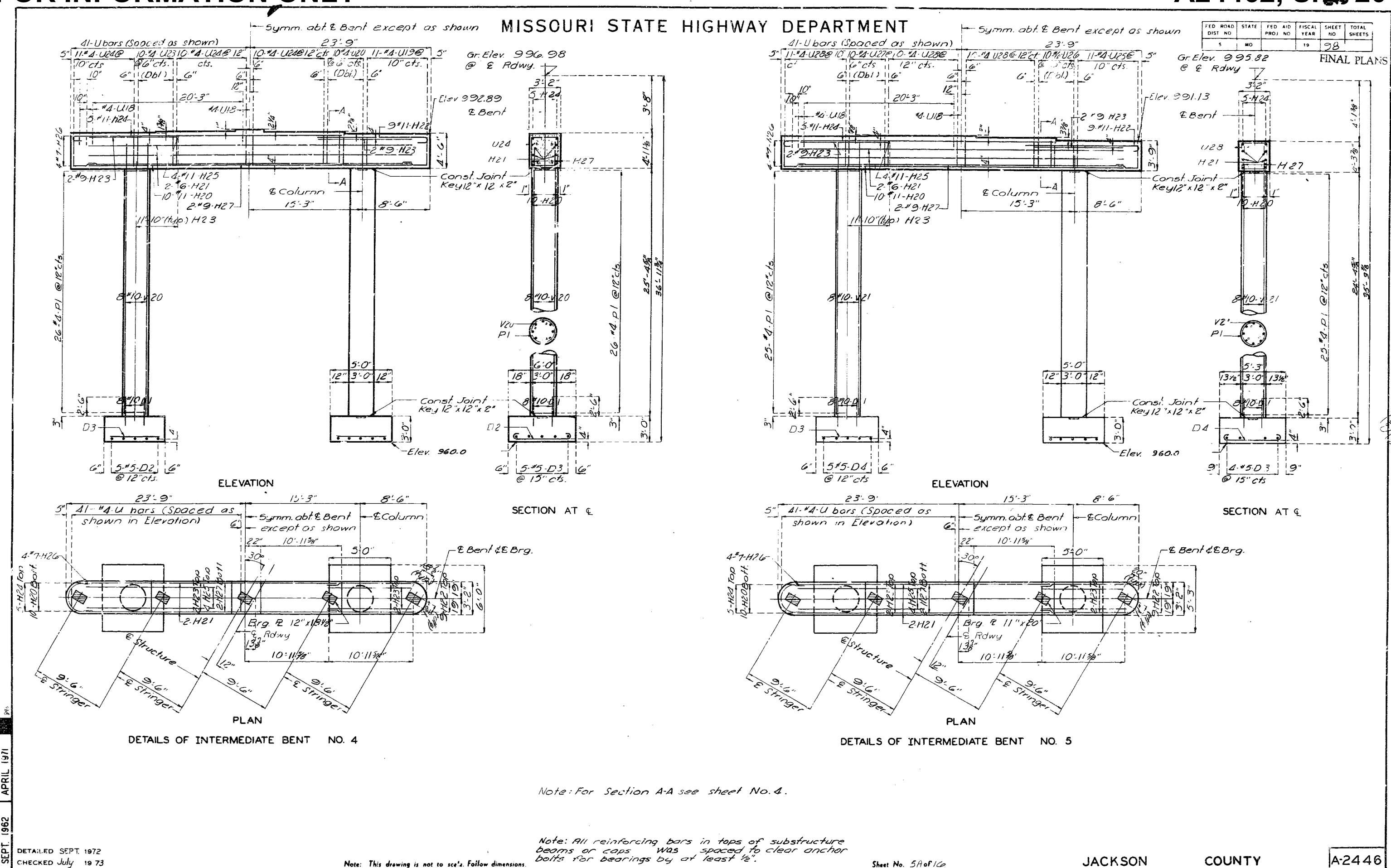
Note: This drawing is not to scale. Follow dimensions

Sheet No. 1A of 16. A Revised Nov. 13,1975

DATE 9/10/75

A-2446





A24462 Sht 21

DEAN DAVID

THIS SHEET HAS BEEN SIGNED, SEALED AND DATED ELECTRONICALLY.

DATE PREPARED

1/16/2014

JACKSON JOB NO.

J4P2191

CONTRACT ID.

PROJECT NO.

BRIDGE NO.

A24461

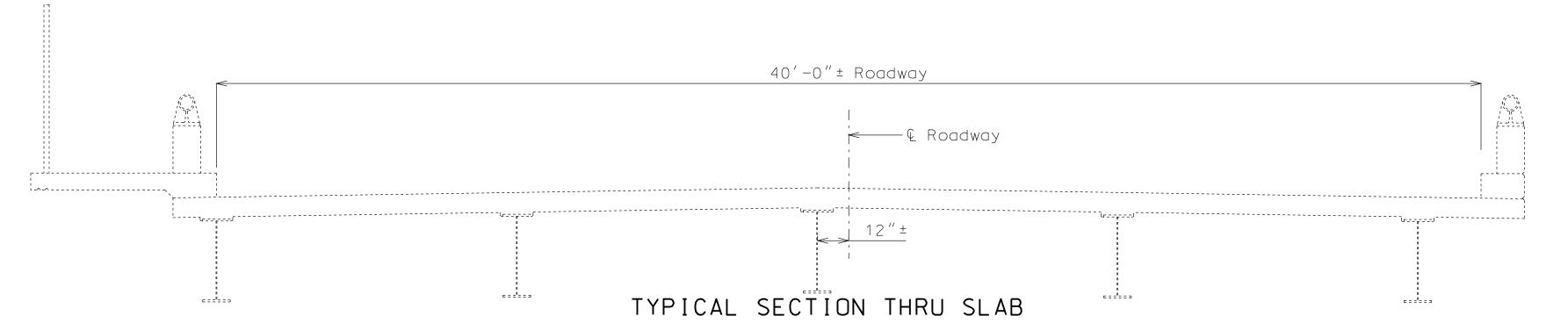
PE-ZU.

50

RGE 32W

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

U.I.P. AND REHABILITATE EXISTING (52'-67'-68'-63'-35') CONTINUOUS COMPOSITE WIDE FLANGE SPANS (SKEW 30° R.A.)



GENERAL NOTES:

Design Specifications:

2002 - AASHTO LFD (17th Edition) Standard Specifications Bridge Deck Rating = 6

Design Loading:

H20-44 (1973 & New Construction)

Design Unit Stresses:

Class B-1 Concrete (Slab, Curb & Parapet) f'c = 4,000 psiReinforcing Steel (Grade 60) fy = 60,000 psi

Reinforcing Steel:

Minimum clearance to reinforcing steel shall be $1\frac{1}{2}''$, unless otherwise shown.

Expansion Joints:

The concrete for slab, curb and parapet replacement shall be Class B-1.

Payment for backwall and slab concrete, complete-in-place, for expansion joint replacement will be considered completely covered by the contract unit price for Class B-1 concrete per cu. yard.

Payment for furnishing and installing slab reinforcing steel and backwall reinforcing steel, complete-in-place, for expansion joint replacement will be considered completely covered by the contract unit price for Reinforcing Steel (Bridges).

Concrete Protective Coatings:

Protective coating for concrete bents and piers (Urethane) shall be applied as shown on plans and in accordance with Sec 711.

Exsiting Bearings Protective Coatings:

Protective Coating: Existing bearings at End Bent No. 1 & 6 with System G in accordance with Sec 1081.

The cost of cleaning and coating of existing bearings shall be considered completely covered by the contract unit price for Cleaning and Coating Existing Bearing per each.

Miscellaneous:

Bars bonded in old concrete not removed shall be cleanly stripped and embedded into new concrete where possible. If length is available, old bars shall extend into new concrete at least 40 diameters for smooth bars and 30 diameters for deformed bars, unless otherwise noted.

Outline of old work is indicated by light dashed lines. Heavy lines indicate new work.

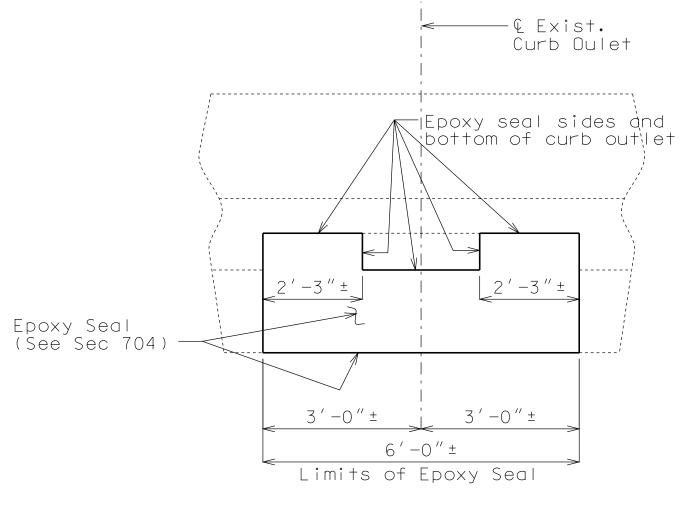
Contractor shall verify all dimensions in field before ordering new material.

The area exposed by the removal of concrete and not covered with new concrete shall be coated with an approved qualified special mortar in accordance with Sec 704.

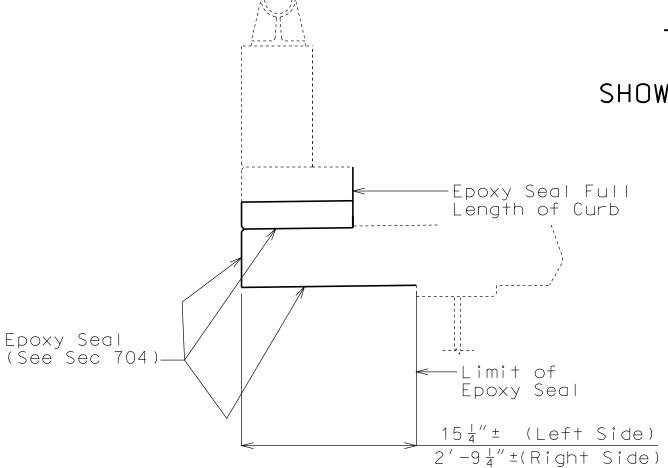
The existing skid plates on the sidewalk shall be reinstalled with $\frac{3}{8}$ "Ø diameter counter sunk cone expansion anchors and they shall have a minimum ultimate pullout strength of 3900 lbs. in concrete with f'c= 4,000 psi.

Traffic Handling:

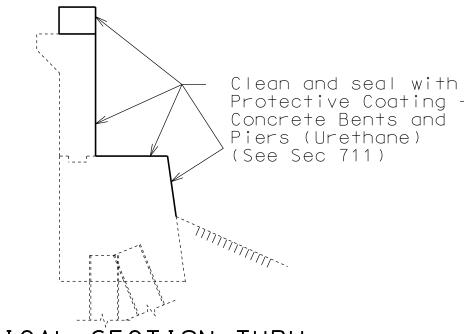
Bridge A24461 will be closed to traffic during construction. See Roadway Plans for Traffic Control.



TYPICAL ELEVATION OF EXISTING CURB OUTLET SHOWING LIMITS OF EPOXY SEAL



TYPICAL SECTION
THRU CURB OUTLET SHOWING
LIMITS OF EPOXY SEAL

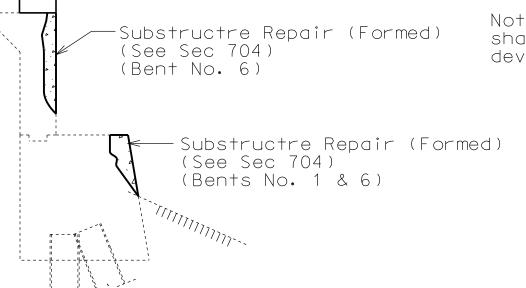


TWP 47N

SEC/SUR 1

TYPICAL SECTION THRU
END BENTS NO. 1 & 6 SHOWING
PROTECTIVE COATING

Note: Cleaning and coating of existing bearings shall be done prior to installation of expansion device and new concrete.



TYPICAL SECTION THRU
END BENTS NO. 1 & 6
SHOWING SUBSTRUCTURE REPAIR

Estimated Quantities		
I tem		Total
Removal of Existing Expansion Joints & Adjacent Concrete	linear foot	92
Remove and Replace Curb & Parapet	linear foot	18
Class B-1 Concrete	cu, yard	11.2
Substructure Repair (Formed)	sq. foot	30
Clean and Epoxy Seal	sq. foot	452
Reinforcing Steel (Bridges)	pound	940
Protective Coating - Concrete Bents & Piers (Urethane)	lump sum	1
Cleaning and Coating Existing Bearings	each	10
Strip Seal Expansion Joint System	linear foot	92

REPAIRS TO BRIDGE: O'BRIEN RD. OVER RTE. 50

STATE ROAD FROM RTE. 7 TO RTE. 291

ABOUT 0.5 MILE SOUTHEAST OF RTE. 291

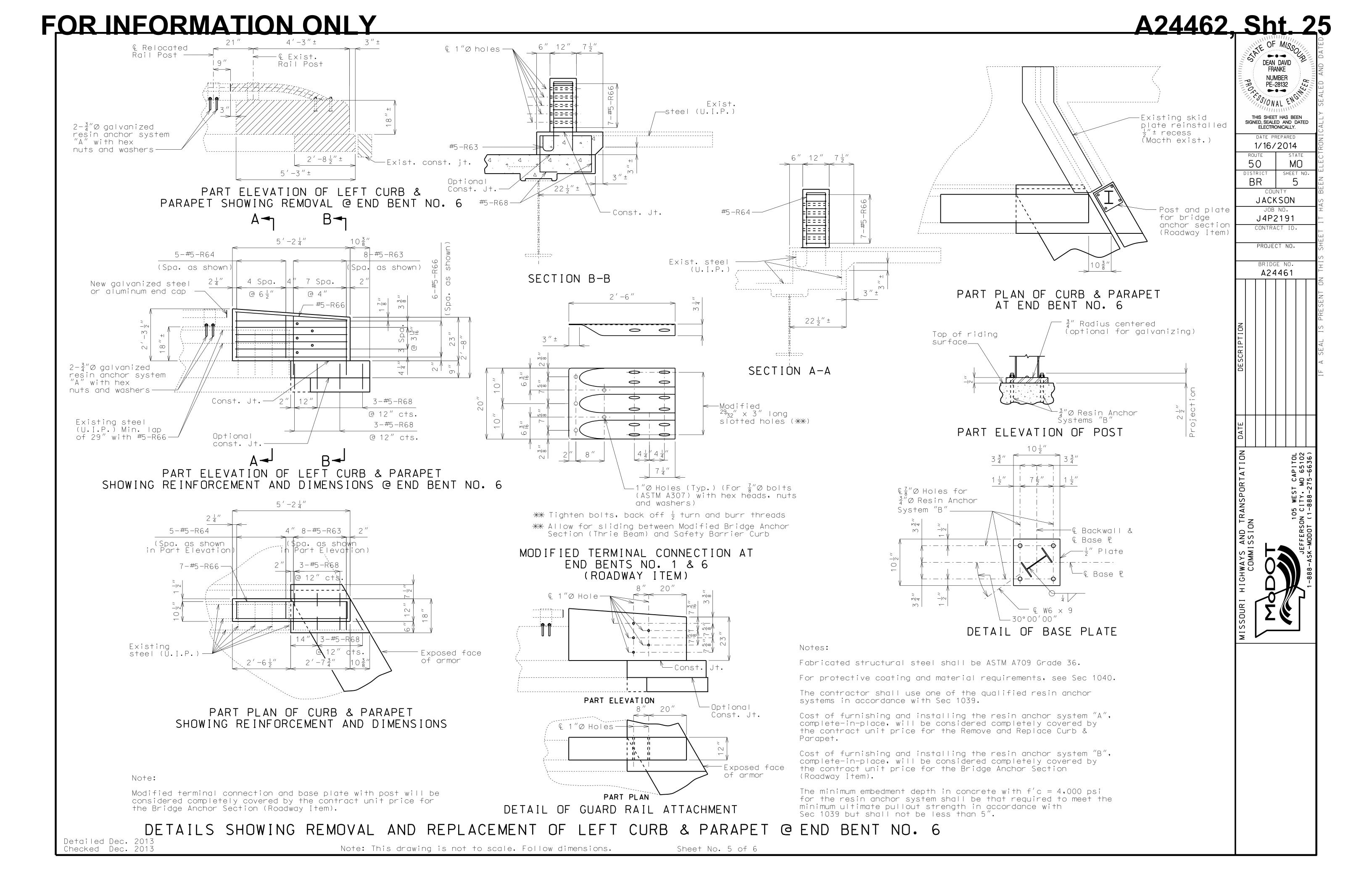
STA. 596+30.00±(MATCH EXISTING)

STD. 606.22 STD. 706.35

Detailed Nov. 2013 Checked Dec. 2013 FEBIRES INECORPHA: To CON CONSIDER Apr. 2009 A24462 Sht 22 OF MISSO Steel armor GENERAL NOTES: $-3/4'' \varnothing \times 8''$ Welded Expansion joint system shall be fabricated -Concrete Removal Line $2\frac{1}{2}$ Gap at 60°F DEAN DAVID 1/2"Ø Machine bolt at abt. 18" cts. Extrusion shear connector in one section, except for stage -Detail "A" alternating at Franke Use two hex nuts to set gap before construction and when the length is over 50 (Normal) NUMIL PE-28132 concrete placement. Gap may be set cts. feet. A complete joint penetration groove 5/16" Plate and 5/16" angle (See Detail "B") 2" Gap at 60°F anytime up to but not exceeding 2 welded splice shall be required. Welds hours before concrete placement. shall be ground flush to provide a smooth Const. Jt. Cut machine bolt flush with steel surface. The expansion joint system shall be Exist. Long. armor after concrete on each side fabricated and installed to the crown and Reinforcement (U.I.P.) Exposed has taken initial set.— 9/16"Ø Holes for grade of the roadway. THIS SHEET HAS BEEN SIGNED, SEALED AND DATED ELECTRONICALLY. face Bottom of steel 1/2"Ø machine bolts The strip seal gland shall be installed in armor DATE PREPARED joints in one continuous piece without field sidewalk DETAIL OF JOINT ARMOR 1/16/2014 splices. Factory splicing will be permitted #6-S1 (Typ.) for joints in excess of 53 feet. $\frac{1}{4}$ " (Typ.) 50 MO Structural steel for the expansion joint -Exist. Transverse. system shall be ASTM A709 Grade 36 except Reinforcement (U.I.P.) the steel armor may be ASTM A709 Grade 50W. Anchors for the expansion joint system Const. Jt. Key + —Haunch slab to bear shall be in accordance with Sec 1037. Strip JACKSON ½'; Gap' seal expansion joint system shall be in JOB NO. - \bigcirc 1/2"Ø Machine bolt and nut accordance with Sec 717. å† 60° F Tack weld--1/2"Ø Machine bolt J4P2191 -Optional with 9/16"Ø hole (use exist, holes) @ abt. 18" cts. Fill Face Const. Jt. CONTRACT ID. in the top flange Structural steel for the expansion joint Cut machine bolt of End Bent ---system shall be coated with a minimum of two _(Remove bolt after concrete has Note: Strip seal gland not shown for clarity. flush with steel armor :set.) coats of inorganic zinc primer (5 mils PROJECT NO. Rail post not shown for clarity. after concrete on minimum) or galvanized in accordance with each side has taken Exist. Vertical ASTM A123. Anchors need not be protected PART ELEVATION OF LEFT CURB & PARAPET initial set. Reinforcement BRIDGE NO. from overspray. (U.I.P.) A24461 DETAIL "A" Plan dimensions are based on installation at 60°F. The expansion gap and other dimensions 1 g" ± (Match exist.) shall be increased or decreased $\frac{1}{8}$ " for SECTION A-A each 10° fall or rise in temperature at Note: Strip seal gland not shown for clarity. Outside edge installation. of sidewalk _ $2\frac{1}{2}$ " Gap at 60°F New longitudinal reinforcing steel shall be placed and existing longitudinal reinforcing └─Single layer gland, multiple-layer steel shall be cut/bent so that ends shall glands not allowed not be more than ±1" from vertical leg of the -Existing skid plate steel armor at the expansion joint system. Strip seal gland size = 3" reinstalled ½" ± recess (Match exist.) Concrete shall be forced under and around DETAIL OF GLAND steel armor and anchors. Proper Const. Jt. consolidation of the concrete shall be — Const. Jt. achieved by localized internal vibration. Reinstalled existing skid plate -Optional Const. Jt. 🛶 For details of post and plate for bridge anchor section, see Sheet No. 4. Provide ½" recess Cost of fabricating and reinstalling existing to allow -Optional skid plates with $\frac{3}{8}$ cone expansion anchors existing skid Const. will be considered incidental to other plate movement Joint pay items. at sidewalk — —New parapet Extend strip *f*,*i*, - - *f* - - - - - - - - - -For rail post details, seal gland 3" past edge see Sheet No. 4.of slab (Typ.) -5/16" Plate and Note: Strip seal gland not shown for clarity. 9/16" Ø Holes at abt. -Exist. Galvanized 5/16" angle cts. (For 1/2' Rail post not shown for clarity. $12'' \times \frac{3}{8}''$ (U. I.P.) PART ELEVATION OF RIGHT machine bolts) € 1/2"Ø Machine bolts 3/4" Ø x 8" Long Welded Shear. and nut with CURB & PARAPET Connector Studs (Spaced 9/16"Ø holes -Exist. Galvanized Gutter Line (use exist. holes) in top flange alternately at abt. 9" cts.) $11'' \times \frac{3}{8}''$ (Remove bolt after $-29/16" \times 1"$ Slotted holes PART SECTION C-C YS MM I (U.I.P.) for 1/2"Ø machine bolts concrete has set.) 1/2"Ø Machine bolt at about 18" cts. __5/16" Plate Cut machine bolt flush with steel armor ½" Ø Countersunk after concrete on each side has taken £ 9/16"_x 1" Slotted holes Socket Head Cap initial set. (Typ.) for 1/2"Ø machine bolts — Cut line-Screws and Anchors — Angle 3 1/2 x 3 1/2 @ abt. 12" cts. x 5/16 x 6" long and 5/16" plate Angle 3 1/2 x 3 1/2 (Match Exist.) \times $^{5}/16 \times 6"$ long $_{-}$ (See Detail "B") PLAN OF LEFT SKID PLATE — @ Exist. Girder SHOWING DIMENSIONS Const. Jt. DETAIL "B" @ END BENT NO. 6 End of strip — Q Exist. seal gland fence post Fill Face Optional 1 11"± of End Bent-Const. 15 " -Exist. Galvanized Joint — $2\frac{1}{2}'' \pm$ Plate $12'' \times \frac{3}{8}''$ 3/4"Ø x 8" Welded (Match Exist.) shear connector studs (U.I.P.) alternately spaced at Countersunk 2<u>1</u>" Gap at 60° F about 9" cts. (Typ.) 20°00′00″ -Exist. Galvanized Socket Head Cap Screws 2-Layers of 30# | Roofing Felt and Anchors , (U.I.P.) 9/16" Ø Holes at abt. 18" cts. (For 1/2" Ø (Match Exist.) between plate machine bolts) and recess _ ¦" Recess with ½"± Recess with 3/4" Ø x 8" Long Welded Shear reinstalled reinstalled existing Connector Studs (Spaced existing " skid plate alternately at abt. 9" cts.) ≨″Skid Йlate PART PLAN Working point— PART SECTION B-B PLAN OF LEFT SKID PLATE SHOWING DIMENSIONS PART SECTION DETAILS OF STRIP SEAL AT END BENTS NO. 1 & 6 @ END BENT NO. 1 OF SIDEWALK (End Bent No. 1 shown, End Bent No. 6 similar) SHOWING RAISED PLATE Detailed Dec. 2013 Checked Dec. 2013 Note: This drawing is not to scale. Follow dimensions. Sheet No. 2 of 6

FOR INFORMATION ONLY A24462 Sht 23 Exist. rail and Limits of Curb & Parapet Existing steel (U.I.P.) rail post Existing DEAN DAVID FRANKE longitudinal reinforcement Removal & Replacement — Const. Jt. (U.I.P.) € Exist. 2″Ø 2 ½" Gap Rail Post (U.I.P.)— Exist. anchor bolts @ 60° F NUMBER PE-28132 PE-zo. (U.I.P. or replace) Const. Jt. (Optional) #5-R1--Match exist. Exist. Reinforcement curb dimensions (U.I.P.) (Typ.)-THIS SHEET HAS BEEN SIGNED, SEALED AND DATED ELECTRONICALLY. DATE PREPARED Note: $22\frac{1}{2}''$ ± -Const. Jt. Const. Jt. — 1/16/2014 For details of existing 50 skid plate, see Sheet No. 2. SECTION C-C Const. Jt. Note: Slab steel not shown for clarity. -Horizontal leg shall be bent (Optional) Const. Jt. — Existing skid plate in field to follow & joint (Optional) JACKSON reinstalled SECTION B-B *** JOB NO. *** *2-#5-R1 (Spa. equally) Existing skid plate J4P2191 reinstalled-CONTRACT ID. ** 3-#5-R1 (Spa. equally) PROJECT NO. *** 2-#5-R2 (Spa. with #5-R1 bars) **** 3-#5-R2 (Spa. with #5-R1 bars) BRIDGE NO. -Concrete A24461 removal PART ELEVATION RIGHT SIDE SHOWING CURB line & PARAPET REINFORCEMENT (Right side End Bent No. 1 shown, right side End Bent No. 6 similar) -Concrete removal 12"± 2'-6"± line #6-S2 Concrete Removal Line-(Top & Bottom) (Field Bend) #6-S1 (Top & Bo\(\psi\)+om) (Field Bend) PART SECTION THRU END BENT SHOWING REMOVAL (End bent No. 1 shown, End Bent No. 6 similar) € Structure — $2' - 3\frac{3}{4}'' \pm$ € Structure top of slab @ 60° F (Typ.) ----#5-S3 -Fill Face of Fill Face of End Bent No. 6 End Bent No. 1— └─Exist. transverse Siteel (Typ.) — Exist. longitudinal Steel (Typ.) Fill Face of End Bent ----> Exist. steel (U.I.P.) Exist. Steel (U.I.P.) 2 ½" Gap -2-#6-S1(Typ.)— @ 60° F $2' - 8 \frac{1}{8}'' \pm$ 2 <u>1</u> " Gap SECTION A-A @ 60° F PART PLAN OF SLAB @ $2' - 8 \frac{1}{8}'' \pm$ (End bent No. 1 shown, End Bent No. 6 similar) END BENT NO. 6 Note: Existing reinforcement (U.I.P.) not shown for clarity. PART PLAN OF SLAB @ END BENT NO. 1 Notes: Note: Existing reinforcement (U.I.P.) Remove existing stirrups within limits of curb & parapet removal. not shown for clarity. All exposed edges of new curb & parapet shall match existing curb & parapet. PART ELEVATION D-D Payment for curb removal and all new concrete and reinforcement for curb & parapet, complete-in-place, will be considered completely covered by the contract unit SHOWING REINFORCEMENT price for Remove and Replace Curb & Parapet per linear foot. Existing rail posts located within limits of curb and parapet removal shall be (End Bent No. 1 shown, End Bent No. 6 similar) reattached to the new curb. Contractor may clean and reuse existing anchor bolts as approved by the engineer or replace the anchor bolts in-kind. Cost of rail post reattachement will be considered completely covered by the contract unit price for Detailed Dec. 2013 Class B-1 Concrete. Checked Dec. 2013 Note: This drawing is not to scale. Follow dimensions. Sheet No. 3 of 6

FOR INFORMATION ONLY A24462 Sht 24 OF MISSO € Existing Rail Post— Note: DEAN DAVID FRANKE $7\frac{1}{2}$ " 12" 6" For details of existing $\frac{3}{8}$ " skid plate -& Relocated Rail Post $5' - 3\frac{5}{8}''$ at End Bent No. 1, see Sheet No. 2. NUMBER PE-28132 PE-zo. · (£ 1"Ø holes THIS SHEET HAS BEEN SIGNED, SEALED AND DATED ELECTRONICALLY. Exist. DATE PREPARED steel (U.I.P.)— Existing skid plate 1/16/2014 reinstalled $\frac{1}{2}$ "± recess ==== (Match exist.) — 50 - #5 -R3 Exist. Const. Jt.- $7\frac{1}{2}''$ 12" 6" Post and plate Const. Jt. for bridge JACKSON anchor section PART ELEVATION OF LEFT CURB JOB NO. & PARAPET SHOWING REMOVAL Fill Face-J4P2191 $22\frac{1}{2}'' \pm$ CONTRACT ID. Note: Existing rail post anchor bolts shall be cut off flush. Optional Const. Jt.-PART PLAN OF LEFT CURB & PARAPET AT END BENT NO. 1 PROJECT NO. BRIDGE NO. A24461 8-#5-R3 5-#5-R4 Exist. steel (U.I.P.) (Spa. as shown) (Spa. as shown) 3" Radius centered (optional for galvanizing) 7 Spa. 4 Spa. Top of riding SECTION B-B surface a 4" @ 7\3" New galvanized steel or aluminum end cap $22\frac{1}{2}'' \pm$ 0 0 "Ø Resin Anchor PART ELEVATION OF POST SECTION A-A - Modified 29,3" x 3" long Const. Jt.-Existing slotted holes (**) steel (Ŭ.I.P.) 3-#5-R8 Min. lap of 29" with #5-R6 or R7 @ 12" cts. 4-#5-R8 $\frac{7}{8}$ % Holes for Exist. steel ¾″Ø Resin Anchor @ 12" cts. $\cdot 1''\emptyset$ Holes (Typ.) (For $\frac{7}{8}''\emptyset$ bolts (U.I.P.) Systems (ASTM A307) with hex heads, nuts $2' - 6\frac{1}{2}''$ $2'-11\frac{1}{4}''$ and washers) Optional # Tighten bolts, back off $\frac{1}{2}$ turn and burr threads € Backwall & Const. Jt. ** Allow for sliding between Modified Bridge Anchor Section (Thrie Beam) and Safety Barrier Curb AND TRA MODIFIED TERMINAL CONNECTION AT END BENTS NO. 1 & 6 —€ Base P (ROADWAY ITEM) PART ELEVATION OF LEFT CURB & PARAPET SHOWING REINFORCEMENT AND DIMENSIONS -€ 1″Ø Hole $5' - 5\frac{3}{4}''$ 8-#5-R3 4" 5-#5-R4 \$þa∙ as shown| (Spa. as shown DETAIL OF BASE PLATE brt Elevatilor Part Elevation) Notes: 12" cts. Fabricated structural steel shall be ASTM A709 Grade 36. Const. Jt.— For protective coating and material requirements, see Sec 1040. Optional Const. Jt. — The contractor shall use one of the qualified resin anchor PART ELEVATION systems in accordance with Sec 1039. Cost of furnishing and installing the resin anchor system, complete-in-place, will be considered completely covered by -Existing steel - £ 1"Ø Holes (U.I.P.) the contract unit price for the Bridge Anchor Section Field Bend #5-R3-(Roadway Item). 7-#5-R6-The minimum embedment depth in concrete with f'c = 4,000 psi Exposed face for the resin anchor system shall be that required to meet the minimum ultimate pullout strength in accordance with of armor -Exposed face Sec 1039 but shall not be less than 5". PART PLAN Modified terminal connection and base plate with post will be PART PLAN OF CURB & PARAPET considered completely covered by the contract unit price for DETAIL OF GUARD RAIL ATTACHMENT the Bridge Anchor Section (Roadway Item). SHOWING REINFORCEMENT AND DIMENSIONS DETAILS SHOWING REMOVAL AND REPLACEMENT OF LEFT CURB & PARAPET @ END BENT NO. 1 Detailed Dec. 2013 Checked Dec. 2013 Note: This drawing is not to scale. Follow dimensions. Sheet No. 4 of 6



FOR NEGOROMATS FOR FONELY A24462 Sht 26 BILL OF REINFORCING STEEL BILL OF REINFORCING STEEL DEAN DAVID FRANKE NOMINAL LENGTH ACTUAL LENGTH MARK NO. LENGTH ACTUAL LENGTH MARK DIMENSIONS DIMENSIONS REO'D. REQ'D. NO. NUMBER SO/ONAL LOCATION SIZE SHAPE 6 SHAPE 8 SHAPE 7 9 N FT. IN. |FT. IN.|FT. IN.|FT. IN.|FT. IN.|FT. IN.|FT. IN.|FT.IN.|FT.IN.| THIS SHEET HAS BEEN SIGNED, SEALED AND DATED ELECTRONICALLY. DATE PREPARED 6 S1 442 1/16/2014 298 5 S3 2 3.00 SHAPE 9 SHAPE 10 SHAPE 11 50 MO DISTRICT :URB/PARAPE **JACKSON** URB/PARAPI JOB NO. URB/PARAPI J4P2191 CONTRACT ID. URB/PARAP SHAPE 13 INCREMENT 9.000 PROJECT NO. 0.625 IN CURB/PARAPE 23.000 9.000 BRIDGE NO. A24461 NCREMENT 2 0.000 9.000 0.500 INC URB/PARAPI 4 11.000 7.750 SHAPE 14 SHAPE 15 URB/PARAPI 12.50 5 R63 URB/PARAP NCREMENT 9.000 2 7.000 SHAPE 17 0.625 IN CURB/PARAPI 23.000 SHAPE 18 INCREMENT 9.000 2 0.000 0.500 INC SHAPE 20 SHAPE 19 URB/PARAPI SPOT WELD AASHTO M32 7.750 URB/PARAPI 12.500 SIZE W5 WIRE (TYP.) SHAPE 21 TOTALS 623 740 1363 SHAPE 22 SHAPE 23 Girder AND TI 623 740 1363 TOTAL S M I SHAPE 25 SHAPE 24 SHAPE 27 SHAPE 26 SHAPE 28 SHAPE 29 <u>► B</u> TWO ADDITIONAL #6-S1 ARE INCLUDED IN THE BAR BILL FOR TESTING. 6d FOR #4 AND #5, 12d FOR #6\ END HOOK DIMENSIONS STIRRUP HOOK DIMENSIONS DETAILING DIMENSION ALL GRADES GRADES 40 - 50 - 60 KSI 180° HOOKS 90° HOOKS ALL STANDARD HOOKS AND BENDS OTHER THAN 180 DEGREE ARE TO BE BENT WITH SAME SIZE (IN.) PROCEDURE AS FOR 90 DEGREE STANDARD HOOKS. HOOKS AND BENDS SHALL BE IN ACCORDANCE WITH THE PROCEDURES AS SHOWN ON THIS SHEET. 90° HOOK 135° HOOK AORG J AORG (IN.) HOOK HOOK APPROX. SIZE SHAPE 31 SHAPE 32 #3 2 1/4" 5" 3" = EPOXY COATED REINFORCEMENT. SHAPE 30 #4 3" 6" | 4" | 8" STIRRUP. 4 1/2" 4 1/2" 3" = BAR IS INCLUDED IN SUBSTRUCTURE QUANTITIES. #5 | 3 3/4" | 7" | 5" | 10" 2 1/2" | 6" | 5 1/2" | 3 3/4" = BAR DIMENSIONS VARY IN EQUAL INCREMENTS BETWEEN DIMENSIONS SHOWN ON THIS LINE HOOK -DETAILING DIMENSION #6 | 4 1/2" | 8" | 6" | 12" AND THE FOLLOWING LINE. 12" | 8" | 4 1/2" 4 1/2" NO. EA. = NUMBER OF BARS OF EACH LENGTH.
NOMINAL LENGTHS ARE BASED ON OUT TO OUT DIMENSIONS SHOWN IN BENDING DIAGRAMS AND
ARE LISTED FOR FABRICATORS USE. (NEAREST INCH) #7 | 5 1/4" | 10" | 7" | 14" | 14" | #8 6" 11" 8" 16" NOTE: UNLESS OTHERWISE NOTED DIAMETER #9 | 9 1/2" | 15" | 11 3/4" | 19" | ACTUAL LENGTHS ARE MEASURED ALONG CENTERLINE BAR TO THE NEAREST INCH. IS THE SAME FOR ALL BENDS AND HOOKS 135° STIRRUP PAYWEIGHTS ARE BASED ON ACTUAL LENGTHS. #10 | 10 3/4" | 17" | 13 1/4" | 22" FOUR ANGLE OR CHANNEL SPACERS ARE REQUIRED FOR EACH COLUMN SPIRAL. SPACERS ARE TO #11 | 12" | 19" | 14 3/4" | 2'-0" PLACED ON INSIDE OF SPIRALS, LENGTH AND WEIGHT OF COLUMN SPIRALS DO NOT INCLUDE SHAPE 36 4d OR 2 1/2" MIN. SHAPE 34 #14 | 18 1/4" | 2'-3" | 21 3/4" | 2'-7" SPLICES OR SPACERS. REINFORCING STEEL (GRADE 60) FY = 60,000 PSI. (SHAPE 35 SHALL BE A DEFORMED OR PLAIN SHAPE 33 SPIRAL BAR OR WIRE.) Detailed Dec. 2013 SHAPE 35 Checked Dec. 2013 Note: This drawing is not to scale. Follow dimensions. BENDING DIAGRAMS Sheet No. 6 of 6