

MODOT BRANSON TOWER

EXISTING TOWER

AS BUILT DRAWINGS, C. 1967

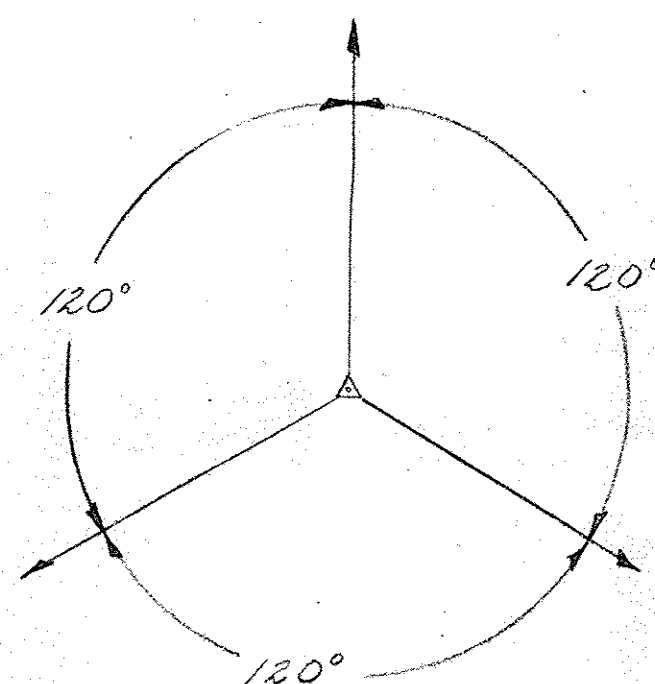
The following drawings are provided for information purposes only and to the best of our knowledge represent the existing radio tower structure, foundations and shelter. Exact locations of existing antennas, feed lines, lighting and other attachments have changed.

BILL OF MATERIAL				
ITEM	QUAN.	MARK NO.	DESCRIPTION	DWG. NO.

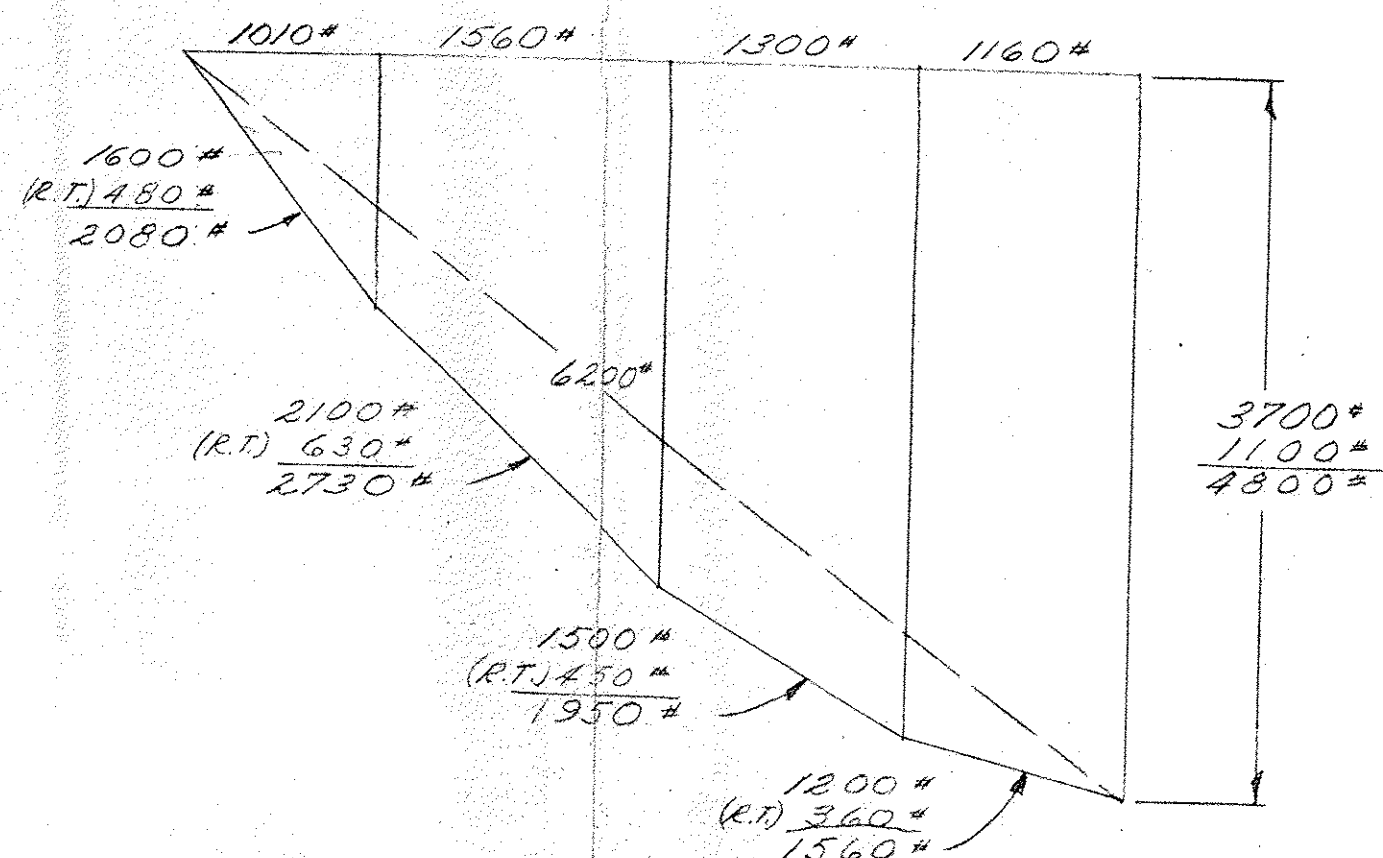
WIND LOADING
30 PSF w/1" R. ICE
6.1A

#45 TOWER = 19.6 #/FT.
LINES = 6.0 #/FT.
TOTAL = 26 #/FT

WIND ON GUYS = 2 #/FT.



PLAN VIEW
NO SCALE



GUY STRESS DIAGRAM
SCALE 1"=1,000'

BASE REACTION
 $3700 \times 2 = 7400 \#$
 $17 \times 150 = 2550 \#$ (TOWER WT.)
 $= 600 \#$ (GUYS w/ ICE) $\div 2$
 $10,550$

MAX. LEG STRESS @ LOWER GUY LEVEL
 $10,550 \div 3 = 3520 \#$ (DIRECT)
 $26 \times (35)^2 = 2590 \#$ (BENDING)
 $10 \times 866 \times 1.42 = 6,110$

MODEL 456 LEG CAPACITY = 6,480 #

NOTE: RESIDUAL TENSION = 30% OF LIVE LOAD.

ANTENNA = 120 #
TOWER & LINES = 520 #
GUYS = 370 #
TOTAL = 1010 #

ANTENNA = 210 #
TOWER & LINES = 1040 #
GUYS = 310 #
1560 #

TOWER & LINES = 1040 #
GUYS = 260 #
1300 #

TOWER & LINES = 910 #
GUYS = 250 #
1160 #

GUY TYPE	ULTIMATE STRENGTH	INITIAL TENSION
1/4" E.H.S.	6650 #	700 #
3/8" E.H.S.	11,200 #	1100 #

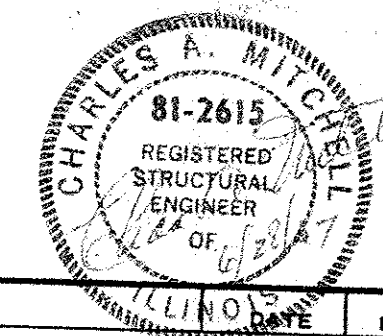
ADD 10% TO GUY LENGTHS SHOWN FOR FINISHED CUT LENGTHS.

ITEM	DWG. NO.
#456 TOWER SECTION	C-630645-R
BASE PIER DETAILS	A-670710 A-670711
ANCHOR DETAILS	C-620643-R

PAL NUTS TO BE SUPPLIED FOR ALL TOWER BOLTS.

CHAMFER EXPOSED EDGES OF ALL CONC.

SITES: RIDGELY
BRANSON
WARDELL



NO.	DESCRIPTION	DATE	BY
	REVISIONS		
ROHN			
TITLE STRESS & ERECTION DATA - 150' #456 FOR MOTOROLA FOR MO. STATE HWY.			
THIS DRAWING IS THE PROPERTY OF ROHN. IT IS NOT TO BE REPRODUCED, COPIED, OR TRACED IN WHOLE OR IN PART WITHOUT OUR WRITTEN CONSENT.		FILE NO. 789A	
SCALE NOTED	MATERIAL	FINISH	WT.
DWN BY R.M.	DATE 6-19-67	UNLESS OTHERWISE SPECIFIED DIMENSIONS ARE GIVEN IN INCHES	
CKD BY O.H.	DATE 6-20-67	TOLERANCES DEC. FRAC. ANGLES	DWG NO. C-670616
APP'D C.H.	DATE		RI

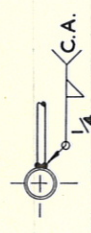
TOWER ELEV.
SCALE 1"=20'

R.M. 7-19-67
RI - Revised Base Pier Ref. Drawg.

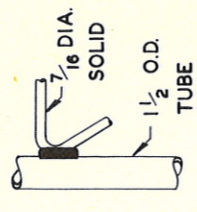
C-630655R

TOWER SPECIFICATIONS

DISTANCE BETWEEN SIDE RAILS (CENTER TO CENTER)	17"
OVERALL LENGTH OF SECTION	10'-5 1/4"
WEIGHT PER SECTION	95 LBS.
SIDE RAIL DIAMETER AND GAUGE	1 1/2" O.D. 1/8" TH.
CROSS SECTIONAL AREA—ONE LEG	.5203 SQ. IN.
GROSS ALLOWABLE VERTICAL LOAD ON THE BOTTOM TOWER SECTION	33,570 LBS.
MAXIMUM ALLOWABLE AXIAL COMPRESSION OF THE CROSS SECTION OF ONE SIDE RAIL	11,190 LBS.
MEASURED TENSILE STRENGTH OF ONE SIDE RAIL	37,400 LBS.
MEASURED TENSILE STRENGTH OF ONE BOLTED LEG JOINT	37,400 LBS.
MAXIMUM ALLOWABLE TENSION IN EACH BOLTED LEG JOINT	14,960 LBS.
SAFE MOMENT OF RESTRAINT	13,730 FT. LBS.
L—UNBRACED LENGTH OF SIDE RAIL (DISTANCE BETWEEN CROSSPIECES)	15 1/2"
R—RADIUS OF GYRATION OF SIDE RAIL	.4897"
L/R FOR MAIN LEG MEMBER	31.7
WIND LOAD PER LINEAL FOOT OF TOWER AT THE HORIZONTAL WIND PRESSURES (PER SQUARE FOOT OF FLAT SURFACE)	
LISTED BELOW:	30 LBS.
	40 LBS.
	50 LBS.

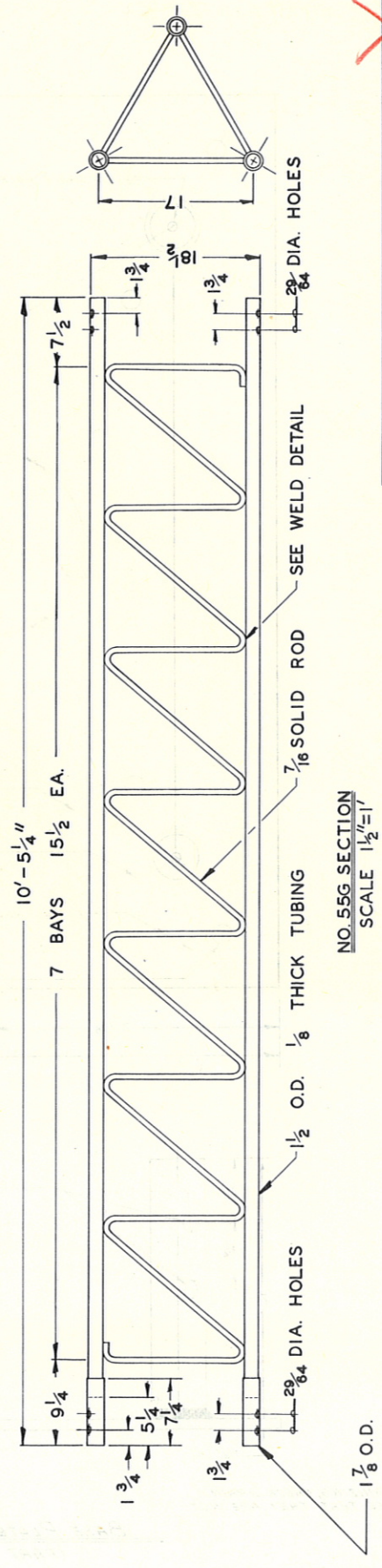


PLAN



ELEVATION

WELD DETAIL
SCALE 3/4"=1'



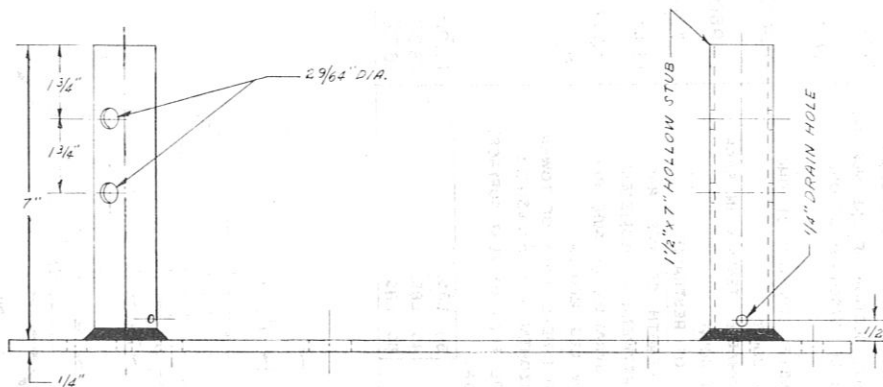
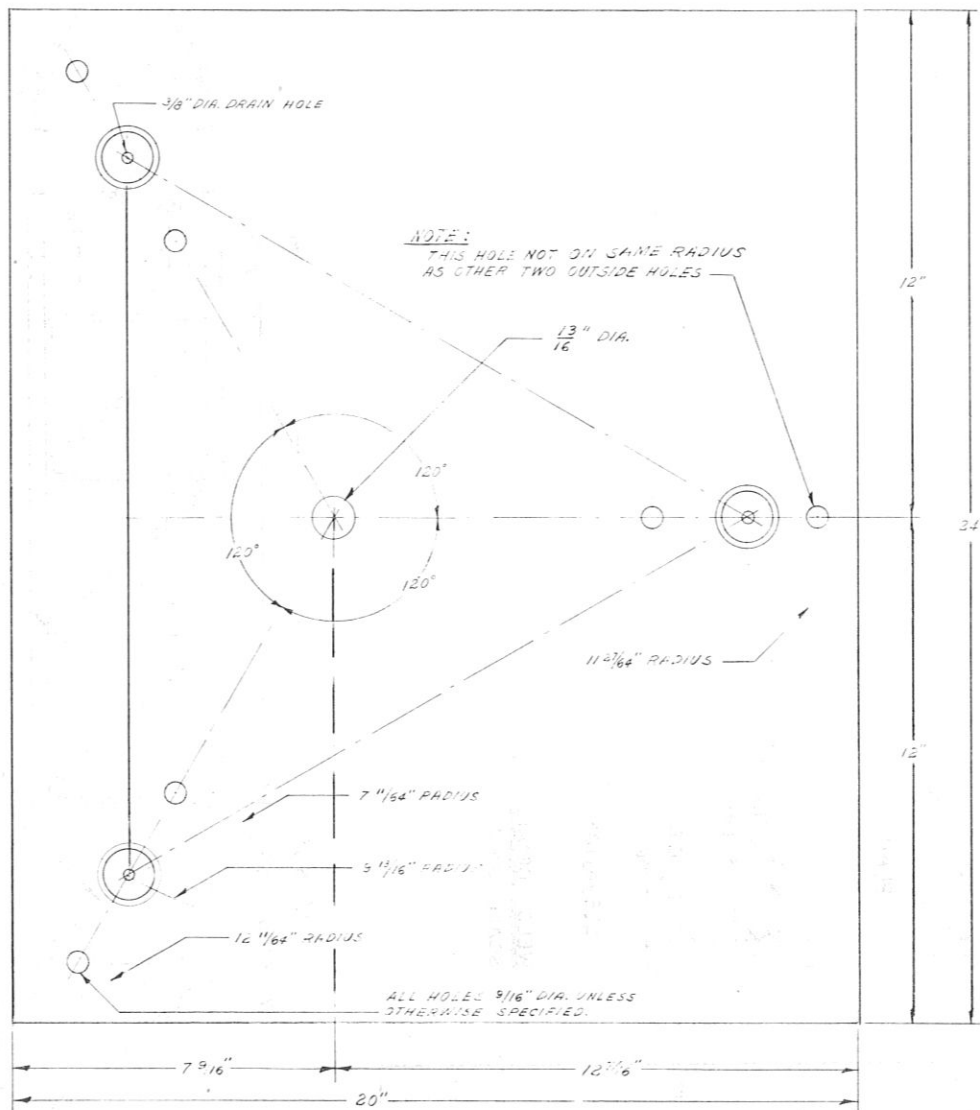
NO. 55G SECTION
SCALE 1 1/2"=1'

NOTE: ALL DIMENSIONS IN INCHES EXCEPT AS NOTED

DRAWN <i>R.M.</i>	CUSTOMER	TITLE
CHECKED <i>agq</i>		MODEL 55 TOWER SECTION
APPROVED <i>DWC</i>		DRAWING NO.
DATE 6-25-63	ROHN MANUFACTURING CO. PEORIA, ILLINOIS	C-630655-R
SCALE NOTED		

REVISED 9-24-64

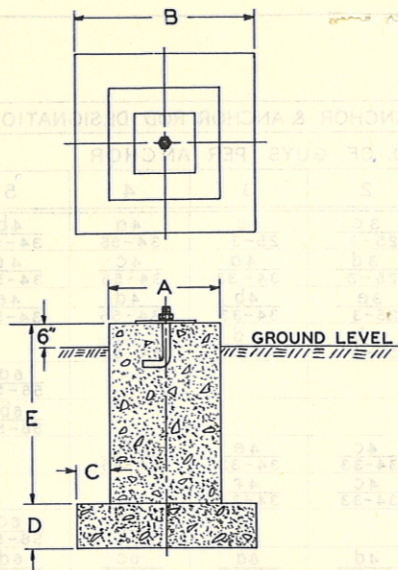
C-640655-R



NOTE: AFTER GALVANIZING, CHECK DRAIN HOLES TO SEE THAT THEY ARE NOT PLUGGED.

BASE PLATE FOR CONCRETE PIER
(PART NO. BPC-55)

DRAWN BY SUNDA	CUSTOMER ROHN MANUFACTURING CO. PEORIA, ILLINOIS	TITLE BASE PLATE FOR MODEL 55 TOWER
CHECKED M-A		DRAWING NO. C-640655-R
APPROVED JAH		
DATE 11-1-65		
SCALE		



CONCRETE BASE DETAIL

NOTE:
REINFORCING RODS ARE
RECOMMENDED TO BE USED
IN ALL CONCRETE BASES.
RODS SHOULD HAVE A
MINIMUM DIAMETER OF $\frac{1}{2}$ "
WITH MAXIMUM SPACING OF 12"

CONCRETE BASE SCHEDULE

CB NO.	DOWNWARD FORCE "F", LBS.	CONCRETE		DOWNWARD FORCE "F _T ", LBS.	DIMENSIONS				
		CU. YDS.	LBS.		A	B	C	D	E
1	14,040	.52	1,960	16,000	2'-0"	2'-0"	0"	0"	3'-6"
2	21,940	.81	3,060	25,000	2'-6"	2'-6"	0"	0"	3'-6"
3	32,310	.98	3,690	36,000	2'-6"	3'-0"	3"	6"	3'-6"
4	44,060	1.31	4,940	49,000	2'-6"	3'-6"	6"	8"	4'-4"
5	58,260	1.52	5,740	64,000	2'-6"	4'-0"	9"	1'-0"	4'-0"
6	74,180	1.81	6,820	81,000	2'-6"	4'-6"	1'-0"	1'-3"	3'-9"
7	89,640	2.74	10,360	100,000	3'-0"	5'-0"	1'-0"	1'-3"	4'-9"
8	108,980	3.18	12,020	121,000	3'-0"	5'-6"	1'-3"	1'-6"	4'-6"
9	130,130	3.67	13,870	144,000	3'-0"	6'-0"	1'-6"	1'-8"	4'-4"
10	149,980	5.03	19,020	169,000	3'-6"	6'-6"	1'-6"	1'-8"	5'-4"
11	172,610	6.19	23,390	196,000	4'-0"	7'-0"	1'-6"	1'-8"	5'-4"
12	198,050	7.13	26,950	225,000	4'-0"	7'-6"	1'-9"	2'-0"	5'-0"
13	223,500	8.60	32,500	256,000	4'-0"	8'-0"	2'-0"	2'-2"	5'-10"
14	250,520	10.18	38,480	289,000	4'-6"	8'-6"	2'-0"	2'-2"	5'-10"
15	278,990	11.91	45,010	324,000	5'-0"	9'-0"	2'-0"	2'-2"	5'-10"

ABOVE VALUES BASED ON SOIL BEARING PRESSURE OF 4000 POUNDS PER
SQUARE FOOT.

DRAWN L. HOFFMAN	CUSTOMER	TITLE
CHECKED		
APPROVED		
DATE JUN. 24, 1961	ROHN MFG. CO. PEORIA, ILLINOIS	DRAWING NO.
SCALE NONE		C-610621

GUY WIRE				CABLE CLAMPS		CONCRETE ANCHOR & ANCHOR ROD DESIGNATION					TURN-BUCKLE SIZE,* IN.	
SIZE AND TYPE	ULTIMATE STRENGTH, LBS.	INITIAL TENSION, LBS.	WEIGHT, LBS./FT.	SIZE IN.	NO. PER CONN.	NO. OF GUYS PER ANCHOR						
						1	2	3	4	5		
$\frac{3}{16}$ " H.S.-7	2,850	300	.073	$\frac{3}{16}$	3	$\frac{3a}{25-3}$	$\frac{3c}{25-3}$	$\frac{3e}{25-3}$	$\frac{4a}{34-55}$	$\frac{4b}{34-55}$	$\frac{3}{8}$	
$\frac{3}{16}$ " E.H.S.-7	3,990	400				$\frac{3b}{25-3}$	$\frac{3d}{25-3}$	$\frac{4a}{34-33}$	$\frac{4c}{34-55}$	$\frac{4d}{34-55}$		
$\frac{1}{4}$ " H.S.-7	4,750	500	$\frac{1}{4}$	$\frac{3c}{25-3}$		$\frac{3e}{25-3}$	$\frac{4b}{34-33}$	$\frac{4d}{34-55}$	$\frac{4e}{34-55}$	$\frac{1}{2}$		
$\frac{1}{4}$ " E.H.S.-7	6,650	700		$\frac{3d}{25-3}$		$\frac{4b}{34-33}$	$\frac{4d}{34-33}$	$\frac{4e}{34-55}$				
$\frac{5}{16}$ " H.S.-7	8,000	800	$\frac{5}{16}$	4						$\frac{6a}{56-55}$	$\frac{5}{8}$	
$\frac{5}{16}$ " E.H.S.-7	8,950	900								$\frac{6b}{56-55}$		
$\frac{3}{8}$ " H.S.-7	10,800	1,100	$\frac{3}{8}$			$\frac{3e}{25-3}$	$\frac{4c}{34-33}$	$\frac{4e}{34-33}$	$\frac{4f}{34-55}$		$\frac{1}{2}$	
$\frac{5}{16}$ " E.H.S.-7	11,200	1,200	$\frac{5}{16}$			$\frac{3e}{25-3}$	$\frac{4c}{34-33}$	$\frac{4f}{34-33}$				
$\frac{7}{16}$ " H.S.-7	14,500	1,500	$\frac{7}{16}$			$\frac{4a}{34-33}$	$\frac{4d}{34-33}$	$\frac{6a}{56-33}$	$\frac{6c}{56-55}$	$\frac{6d}{56-55}$	$\frac{5}{8}$	
$\frac{3}{8}$ " E.H.S.-7	15,400	1,600	$\frac{3}{8}$			$\frac{4b}{34-33}$	$\frac{4e}{34-33}$	$\frac{6a}{56-33}$	$\frac{6c}{56-55}$	$\frac{6d}{56-55}$		
$\frac{1}{2}$ " H.S.-7	18,800	1,900	$\frac{1}{2}$			5					$\frac{8a}{57-55}$	$\frac{3}{4}$
$\frac{7}{16}$ " E.H.S.-7	20,800	2,100	$\frac{7}{16}$				$\frac{6a}{56-33}$	$\frac{6b}{56-33}$	$\frac{6e}{56-33}$	$\frac{8a}{57-55}$	$\frac{8c}{57-55}$	
$\frac{9}{16}$ " H.S.-7	24,500	2,500	$\frac{9}{16}$		$\frac{6a}{56-33}$		$\frac{6d}{56-33}$	$\frac{8a}{57-33}$	$\frac{8c}{57-55}$	$\frac{8e}{57-55}$	$\frac{7}{8}$	
$\frac{1}{2}$ " E.H.S.-7	26,900	2,700	$\frac{1}{2}$		$\frac{8a}{57-33}$		$\frac{8a}{57-33}$	$\frac{8b}{57-33}$	$\frac{8d}{57-55}$			
$\frac{5}{8}$ " H.S.-7	29,600	3,000	$\frac{5}{8}$							$\frac{10a}{58-55}$	$\frac{1}{4}$	
$\frac{9}{16}$ " E.H.S.-7	35,000	3,500	$\frac{9}{16}$							$\frac{10a}{58-55}$		
$\frac{5}{8}$ " E.H.S.-7	42,400	4,300	$\frac{5}{8}$	5				$\frac{10a}{58-33}$	$\frac{10c}{58-55}$	$\frac{10e}{58-55}$	$\frac{1}{4}$	
$\frac{9}{16}$ " B.S.-19	38,000	3,800	$\frac{9}{16}$		$\frac{8a}{57-33}$		$\frac{8b}{57-33}$					1
$\frac{5}{8}$ " B.S.-19	48,000	4,800	$\frac{5}{8}$		$\frac{10a}{58-33}$		$\frac{10a}{58-33}$	$\frac{10a}{58-33}$	$\frac{10b}{58-55}$	$\frac{10d}{58-55}$	$\frac{1}{4}$	
$\frac{3}{4}$ " B.S.-19	68,000	6,800	$\frac{3}{4}$		$\frac{10a}{58-33}$		$\frac{10a}{58-33}$	$\frac{10e}{58-33}$	$\frac{12b}{59-55}$	$\frac{12d}{59-55}$		$\frac{1}{2}$
$\frac{7}{8}$ " B.S.-19	92,000	9,200	$\frac{7}{8}$		$\frac{10a}{58-33}$		$\frac{10c}{58-33}$	$\frac{12c}{59-33}$	$\frac{12c}{59-55}$		$\frac{3}{4}$	
1" B.S.-19	122,000	12,200	1	$\frac{10a}{58-33}$	$\frac{10e}{58-33}$		$\frac{12c}{59-33}$					

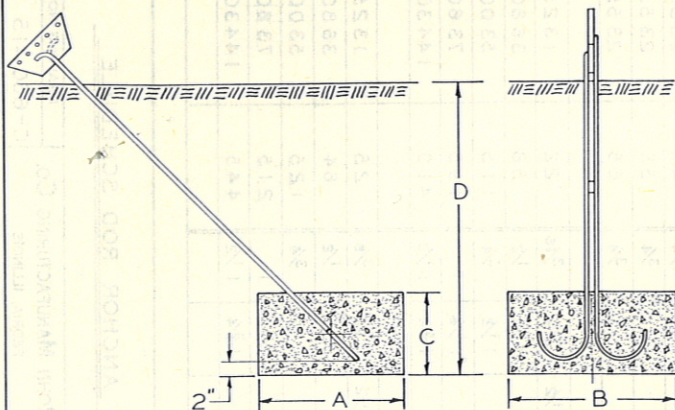
* ALL TURNBUCKLES ARE EYE-AND-JAW TURNBUCKLES

EXAMPLE FOR ABOVE:

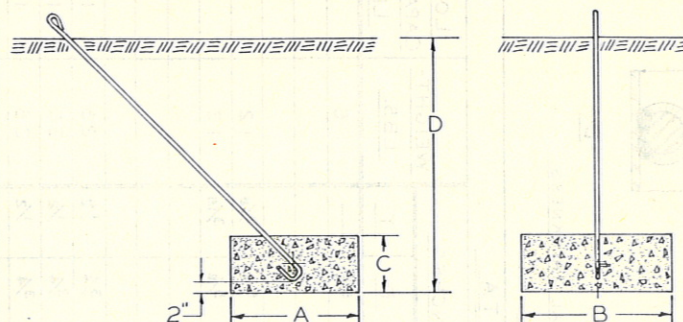
$\frac{6b}{56-55}$ — CONCRETE ANCHOR 6b (DWG.NO. C-620643-R)
 $\frac{56-55}{56-55}$ — ANCHOR ROD GAC 56-55 (DWG.NO. C-620642-R)

NOTE: THE ABOVE CHART IS TO SERVE AS A GUIDE ONLY. THE CONCRETE ANCHORS AND ANCHOR RODS LISTED ABOVE ARE BASED UPON MAXIMUM STRENGTHS OF GUY WIRES AND DO NOT NECESSARILY INDICATE THOSE WHICH WOULD BE RECOMMENDED AS A RESULT OF A TOWER DESIGN. THEREFORE, IN MANY CASES, THE RECOMMENDED ANCHORS AND RODS WILL BE SMALLER THAN THOSE GIVEN ABOVE.

DRAWN AED	ROHN MFG. CO. PEORIA, ILLINOIS	TITLE
CHECKED <i>ck</i>		GUY ACCESSORY SCHEDULE
APPROVED <i>RAH</i>		DRAWING NO.
DATE 8-4-64		C-640717
SCALE NONE		



ANCHOR DETAIL FOR GAC-34-57-58-59
(FOR GAC-56 SEE DWG. NO. C-620642-R)



ANCHOR DETAIL FOR GAC-25

NOTE: REINFORCING RODS ARE RECOMMENDED TO BE USED IN ALL ANCHORS. RODS SHOULD HAVE A MINIMUM DIAMETER OF 1/2" WITH A MAXIMUM SPACING OF 12".

CONCRETE ANCHOR DATA

DEPTH D, FT.	ROD NO.	BLOCK NO.	ANCHOR DIMENSIONS, FT.			WEIGHT LBS.	CONCRETE CU. YDS.	HOLDING POWER, LBS.
			A	B	C			
3	GAC-25	3a	1.5	1.5	1	310	.08	1,810
		3b	2	2	1	560	.15	2,640
		3c	2.5	2.5	1	870	.23	3,630
		3d	3	3	1	1,260	.33	4,800
		3e	3	4	1	1,680	.44	6,050
4	GAC-34	4a	3	3	1.5	1,890	.50	7,000
		4b	3	4	1.5	2,520	.67	8,500
		4c	3	5	1.5	3,150	.84	10,400
		4d	3	6	1.5	3,780	1.00	12,000
		4e	4	6	1.5	5,050	1.33	15,200
		4f	5	6	1.5	6,300	1.67	18,300
6	GAC-56	6a	3	4	1.5	2,520	.67	20,100
		6b	3	5	1.5	3,150	.84	23,200
		6c	3	6	1.5	3,780	1.00	26,300
		6d	4	6	1.5	5,050	1.33	31,700
		6e	5	6	1.5	6,300	1.67	36,600
8	GAC-57	8a	3	5	1.5	3,150	.84	44,300
		8b	3	6	1.5	3,780	1.00	49,400
		8c	4	6	1.5	5,050	1.33	57,000
		8d	5	6	1.5	6,300	1.67	64,500
		8e	6	6	1.5	7,550	2.00	72,100
10	GAC-58	10a	3	6	1.5	3,780	1.00	83,500
		10b	4	6	1.5	5,050	1.33	94,100
		10c	5	6	1.5	6,300	1.67	104,800
		10d	6	6	1.5	7,550	2.00	115,200
		10e	6	8	1.5	10,080	2.67	136,400
12	GAC-59	12a	4	6	1.5	5,050	1.33	144,800
		12b	5	6	1.5	6,300	1.67	158,700
		12c	6	6	1.5	7,550	2.00	172,500
		12d	6	8	1.5	10,080	2.67	200,500
		12e	8	8	1.5	13,440	3.56	233,200

DRAWN M. BUENDIA

CHECKED ck

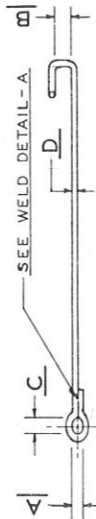
APPROVED PA

DATE 7-22-64

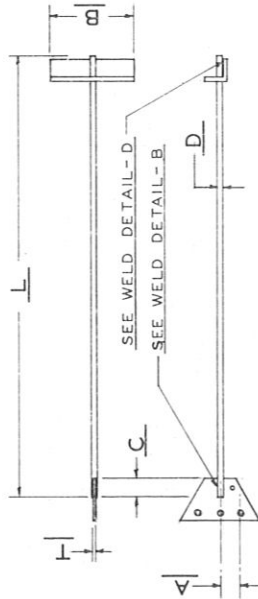
SCALE NONE

ROHN MFG. CO.
PEORIA, ILLINOIS

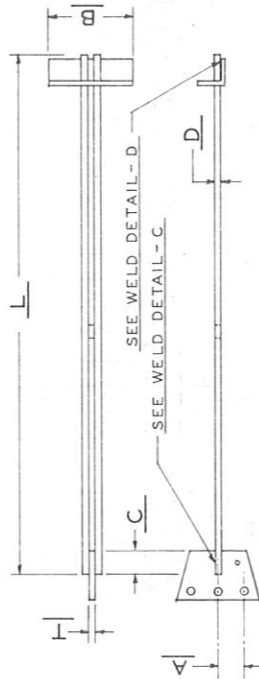
TITLE
DETAILS
OF
CONCRETE ANCHORS
DRAWING NO.
C-620643-R



TYPES- GAR-25 & GAC-25



TYPES- GAC- 75, 100, 34, 56 & 57



TYPES- GAC- 58 & 59

ANCHOR ROD DETAILS

NOTE: WELD SIZE TO BE ONE-HALF OF MATERIAL THICKNESS

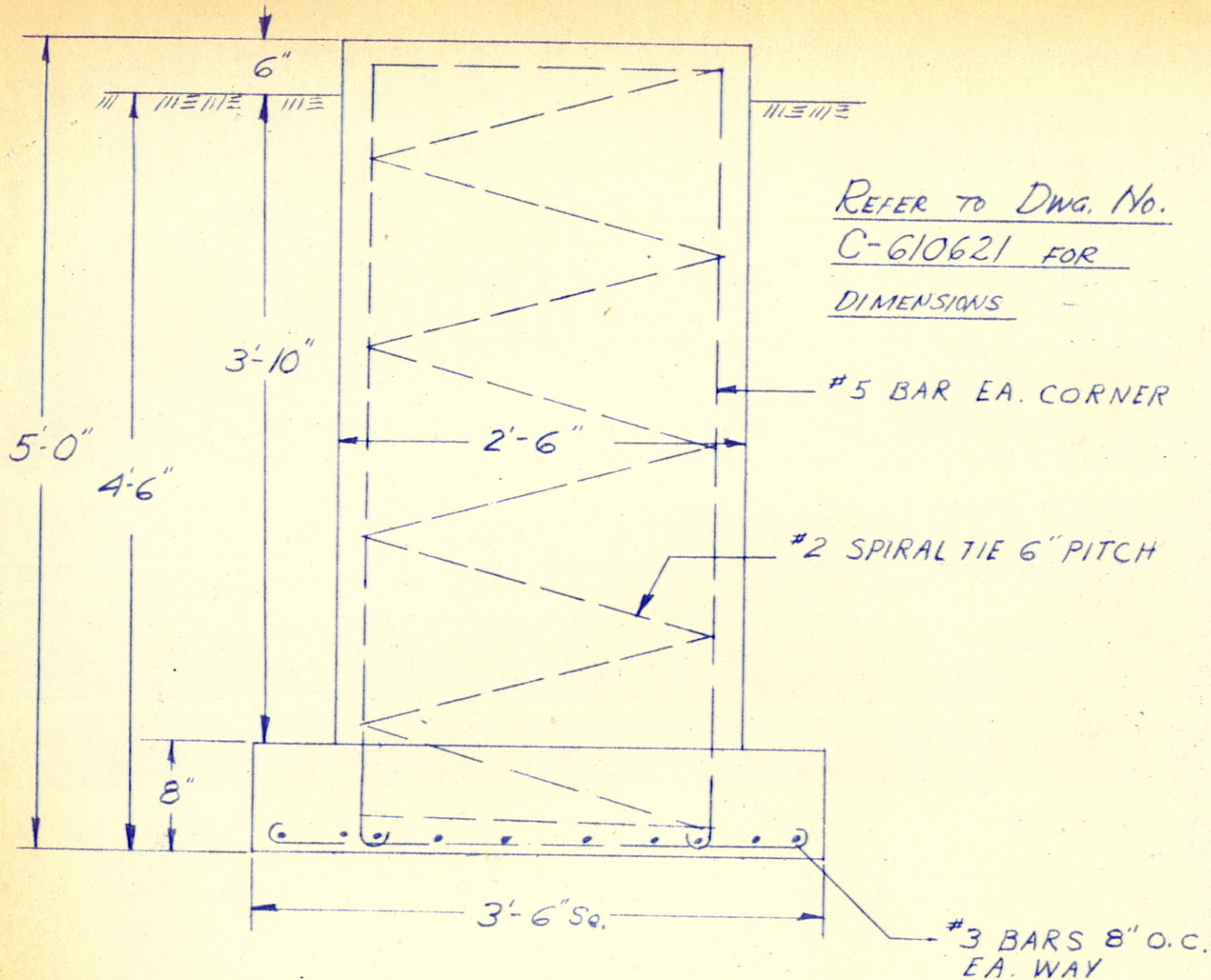
ANCHOR ROD DATA

PART NUMBER	EQUALIZER PLATE TYPE	DIMENSIONS IN INCHES						WEIGHT- LBS.	LOAD CAPACITY- LBS.
		L	A	B	C	D	T		
GAR-25	EYE	64	3/4	4	1 1/2	5/8	---	B	9200
GAC-25-3	EE	64	2	4	1 1/2	5/8	3/16	12	9200
GAC-25-5	EE	64	2	4	1 1/2	5/8	3/16	13	9200
GAC-75-1	EJ	120	1 1/16	12	3	3/4	1/2	25	13250
GAC-75-3	EJ	120	2 1/2	12	3	3/4	1/2	30	13250
GAC-75-5	EJ	120	2 1/2	12	3	3/4	1/2	35	13250
GAC-100-1	EJ	120	1 5/16	12	4	1	3/4	45	23550
GAC-100-3	EJ	120	3	12	4	1	3/4	55	23550
GAC-100-5	EJ	120	3	12	4	1	3/4	65	23550
GAC-34-33	EJ	84	2	12	2 1/2	3/4	3/16	22	13250
GAC-56-33	EJ	120	2 1/2	12	3	1 1/4	1/2	58	36800
GAC-57-33	EJ	168	3	12	4	1 1/2	3/4	115	53000
GAC-58-33	EJ	192	4	12	6	1 1/4	1	195	73600
GAC-59-33	EJ	240	4	12	6	1 3/4	1 1/2	410	144300
GAC-34-55	EJ	84	2	12	2 1/2	3/4	3/8	25	13250
GAC-56-55	EJ	120	2 1/2	12	3	1 1/4	1/2	64	36800
GAC-57-55	EJ	168	3	12	4	1 1/2	3/4	125	53000
GAC-58-55	EJ	192	4	12	6	1 1/4	1	215	73600
GAC-59-55	EJ	240	4	12	6	1 3/4	1 1/2	445	144300

- 1) SEE DRAWING NO. C-660416 FOR EQUALIZER PLATE DETAILS.
- 2) TYPE GAC-25 RODS ARE SUPPLIED WITH TYPE 2534-EE EQUALIZER PLATES.
- 3) PART NO. SUFFIXES- 1, 3, 5, 33 & 55 DENOTES- 1, 3 OR 5 HOLES IN PLATE.
- 4) TYPE-EE EQUALIZER PLATES ARE SUPPLIED IN PAIRS FOR EYE & EYE TURNBUCKLES.
- 5) TYPE-EJ EQUALIZER PLATES ARE USED WITH EYE & JAW TURNBUCKLES.
- 6) 7)

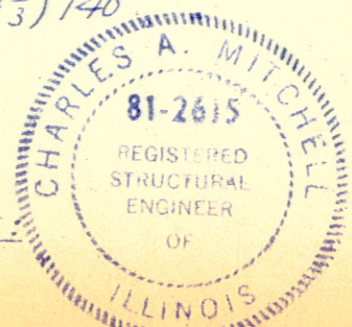
NO.	REVISION	BY	DATE	DRAWN	CHK	ANCHOR ROD SCHEDULE	
1	REDRAWN		7-25-66				
				CHECKED			
				APPROVED			
				DATE	7-25-66	DRAWING NO.	
				SCALE	NONE	C-660 415 A	
				ROHN MANUFACTURING Co. PEORIA, ILLINOIS			

CB-4 BASE PIER



$$\begin{aligned} \text{WT. OF PIER} &= (3\frac{1}{2} \times 3\frac{1}{2} \times \frac{8}{12} + 2\frac{1}{2} \times 2\frac{1}{2} \times 4\frac{1}{3}) 140 \\ &= \underline{4,940 \#} \end{aligned}$$

USING SOIL BEARING PRESSURE OF 3000 PSF:

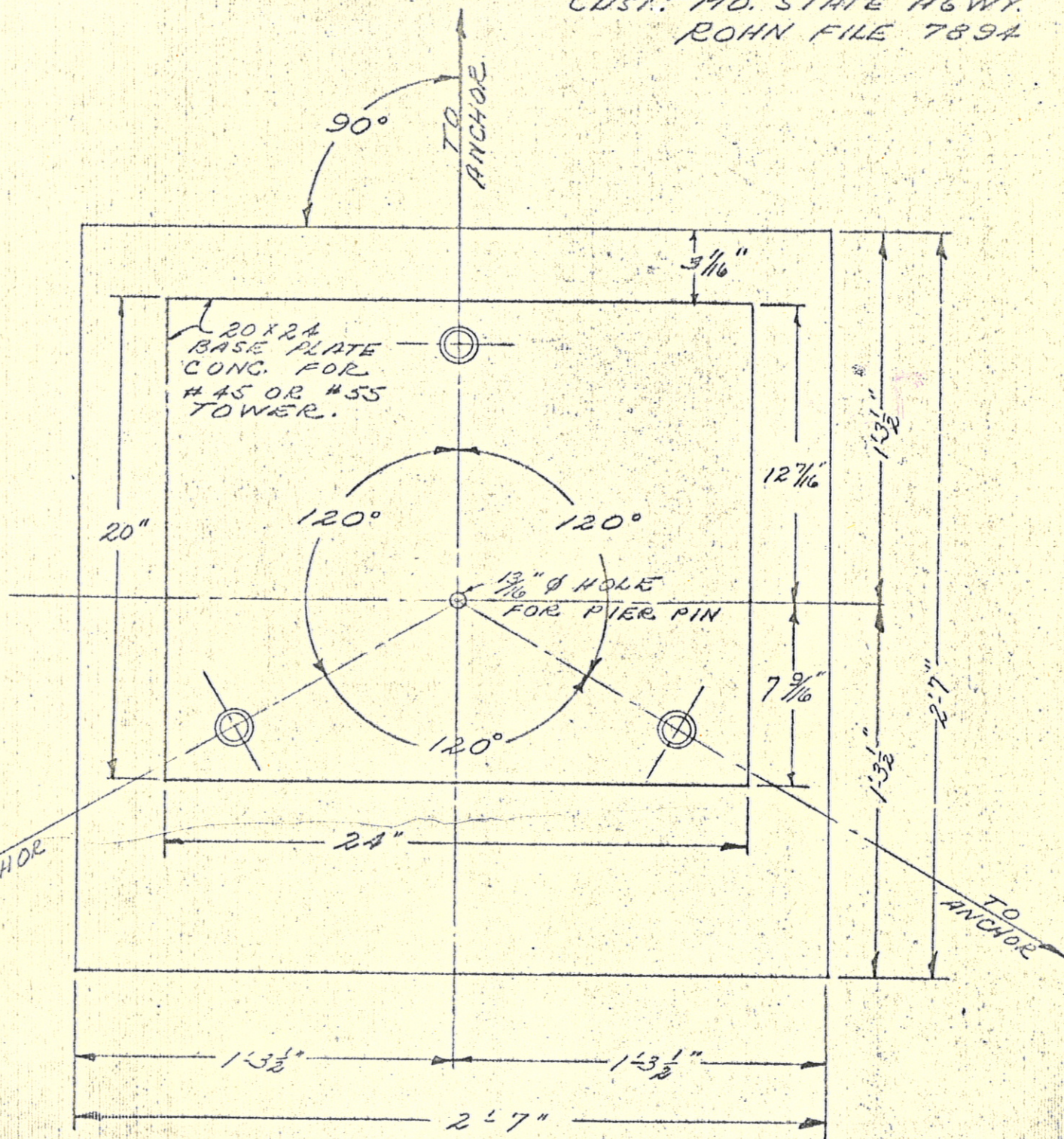


$$\text{ALLOWABLE BASE REACTION} = \text{TOTAL ALLOW. BEARING} - \text{WT.}$$

$$= 3\frac{1}{2} \times 3\frac{1}{2} \times 3000 - 4940$$

$$= 36,750 - 4,940 = \underline{\underline{31,810 \#}}$$

CUST: MO. STATE HGWY.
ROHN FILE 7894



NOTE: CHAMFER ALL EXPOSED EDGES OF CONC.

PLAN VIEW OF TOP
OF SPECIAL CB-4 BASE
SCALE 1" = 6"

SIZES: FLAT; RIDGELY; WARDELL; BRANSON.

own: ram 7-19-67

CHKO: WDV 7-19-67

APPROVED;

ROHN MFG CO.
DWG. A-670710

DWG. A-670710

NOTE: SEE DWG. A-670711
FOR ELEV. VIEW.