

(82.9') PRESTRESSED CONCRETE NU-GIRDER SPAN

Notes:

Roadway fill shall be completed to the final roadway section and up to the elevation of the bottom of the concrete beam within the limits of the structure and for not less than 25 feet in back of the fill face of the end bents before any piles are driven for any bents falling within the embankment section.

Hydrologic Data	
Drainage Area = 9.3 mi ²	
Design Flood Frequency = 50 years	
Design Flood Discharge = 1,060 cfs	
Design Flood (D.F.) Elevation = 303.0	
Base Flood (100-year)	
Base Flood Elevation = 303.5	
Base Flood Discharge = 1,210 cfs	
Estimated Backwater = 0.1 ft	
Average Velocity thru Opening = 2.5 ft/s	
Freeboard (50-year)	
Freeboard = 1.4 ft	
Roadway Overtopping	
Overtopping Flood Discharge = N/A	
Overtopping Flood Frequency = >500 years	
500 Year Flood Elevation = 304.8	

⊕ Indicates location of borings.

Notice and Disclaimer Regarding Boring Log Data

The locations of all subsurface borings for this structure are shown on the plan sheet(s) for this structure. The boring data for all locations indicated, as well as any other boring logs or other factual records of subsurface data and investigations performed by the department for the design of the project, are shown on Sheet(s) No. 23 & 24 and may be included in the Electronic Bridge Deliverables. They will also be available from the Project Contact upon written request. No greater significance or weight should be given to the boring data depicted on the plan sheets than is given to the subsurface data available from the district or elsewhere.

The Commission does not represent or warrant that any such boring data accurately depicts the conditions to be encountered in constructing this project. A contractor assumes all risks it may encounter in basing its bid prices, time or schedule of performance on the boring data depicted here or those available from the district, or on any other documentation not expressly warranted, which the contractor may obtain from the Commission.

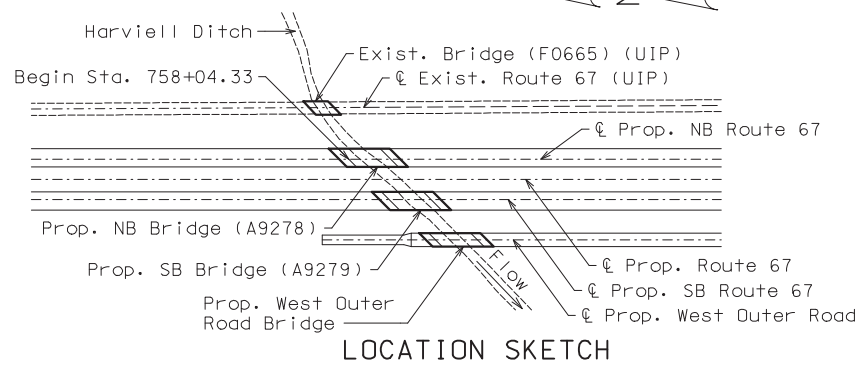
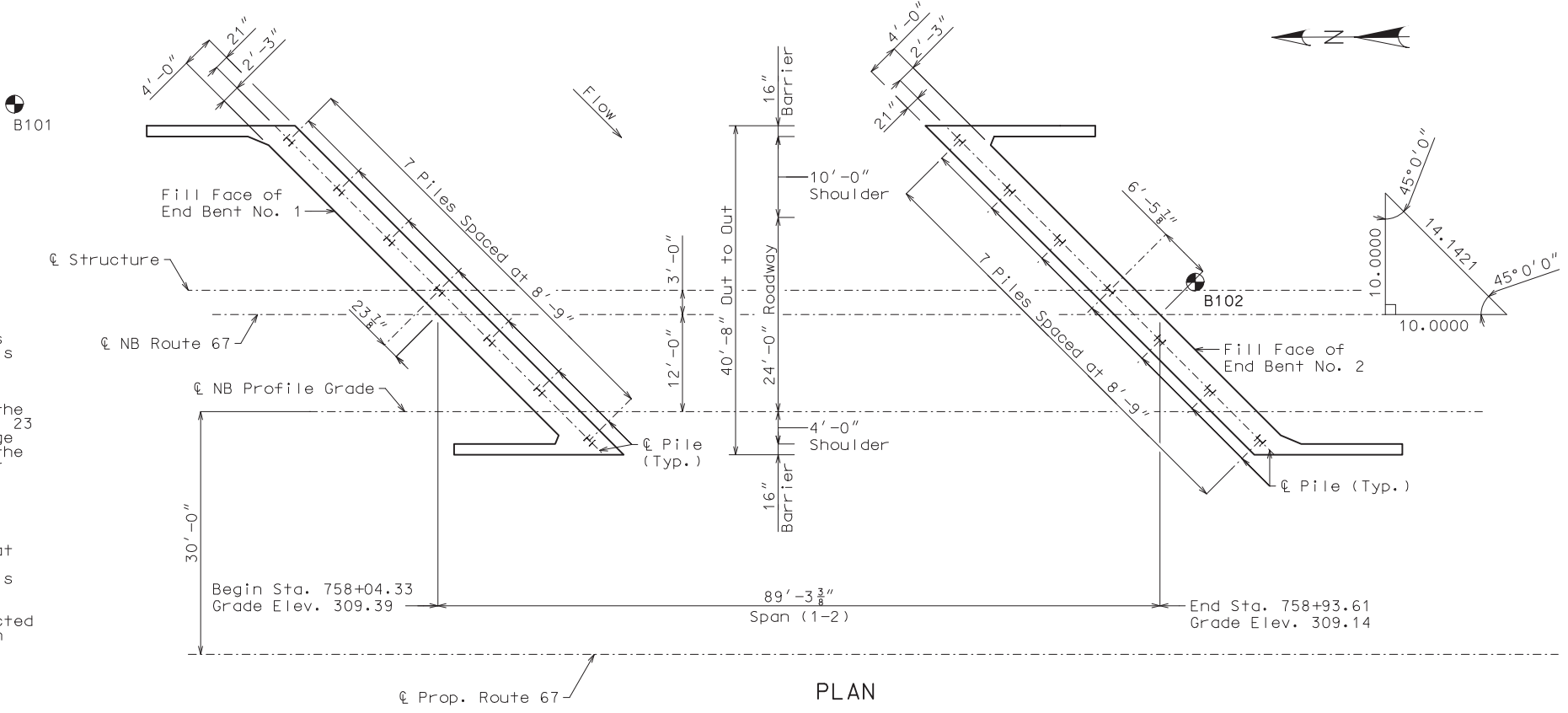
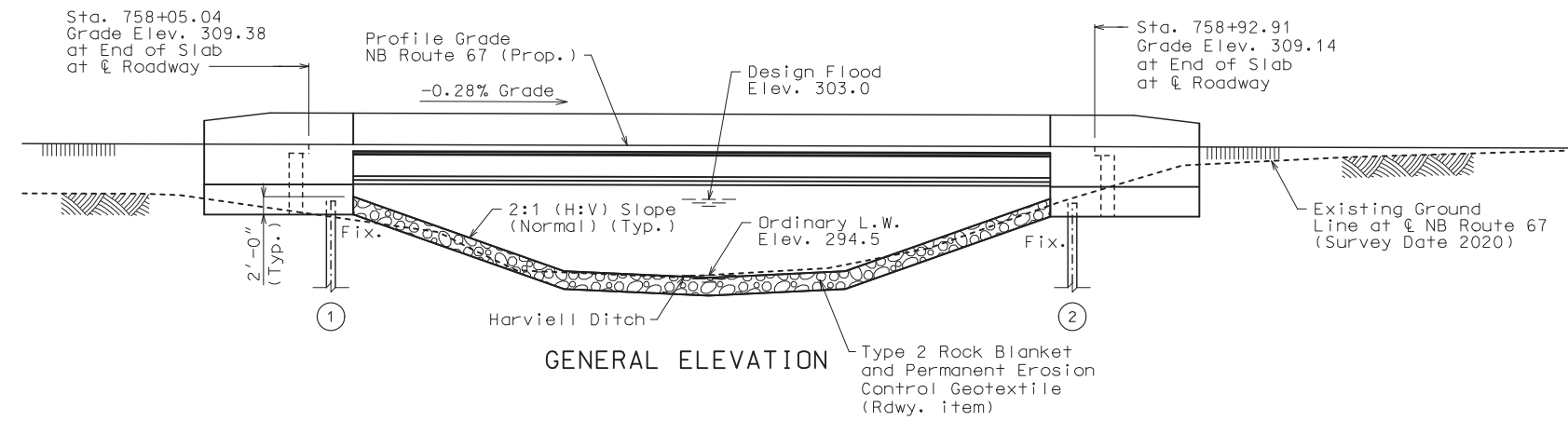
General Notes:

Longitudinal dimensions are measured horizontal.

For General Notes, Estimated Quantities and Foundation Data see Sheet No. 2.

B.M. #32
 N: 282085.74
 E: 814254.75
 Sta. 719+83.75 Exist. Rte 67
 Elev. 308.27
 Chisled "Q" on SW corner of west barrier curb on bridge F0665

Control Point #129
 N: 281264.97
 E: 814315.18
 Sta. 728+04.49 Exist. Rte 67
 Elev. 302.52
 5/8" rebar with MoDOT cap



BRIDGE: ROUTE 67 NB OVER HARVIELL DITCH

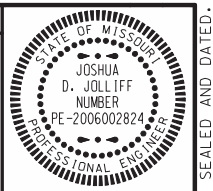
ROUTE 67 FROM ROUTE 158 TO ROUTE 142
 ABOUT 3.2 MILES NORTH OF ROUTE 142
 BEGINNING STATION 758+04.33 (℄ NB ROUTE 67)

GENERAL PLAN AND ELEVATION

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 1 of 24

Designed Sep. 2022
 Detailed Oct. 2022
 Checked Oct. 2022



THIS SHEET HAS BEEN SIGNED, SEALED AND DATED ELECTRONICALLY.

DATE PREPARED	3/3/2023
ROUTE	67
STATE	MO
DISTRICT	BR
SHEET NO.	1
COUNTY	BUTLER
JOB NO.	J9P3751
CONTRACT ID.	
PROJECT NO.	
BRIDGE NO.	A9278

DESCRIPTION	DATE

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

105 WEST CAPITOL
 JEFFERSON CITY, MO 65102
 1-888-ASK-MODOT (1-888-275-6636)

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General Notes:

Design Specifications:

2020 AASHTO LRFD Bridge Design Specifications (9th Ed.)
2011 AASHTO Guide Specifications for LRFD Seismic Bridge Design (2nd Ed.)
and 2014 Interim Revisions (Seismic)
Seismic Design Category = C
Design earthquake response spectral acceleration coefficient at
1.0 second period, $S_{D1} = 0.34$
Acceleration Coefficient (effective peak ground
acceleration coefficient), $A_s = 0.37$

Design Loading:

Vehicular = HL-93
Future Wearing Surface = 35 lb/sf
Earth = 120 lb/cf
Equivalent Fluid Pressure = 45 lb/cf (Min.)
Superstructure: Non-composite for dead load. Composite for live load.

Design Unit Stresses:

Class B Concrete (Substructure) $f'c = 3,000$ psi
Class B-2 Concrete (Superstructure, except Prestressed
Girders and Barrier) $f'c = 4,000$ psi
Class B-1 Concrete (Barrier) $f'c = 4,000$ psi
Reinforcing Steel (Grade 60) $fy = 60,000$ psi
Structural Steel HP Pile (ASTM A709 Grade 50S) $fy = 50,000$ psi

For precast prestressed panel stresses, see Sheet No. 12.

For prestressed girder stresses, see Sheets No. 10 & 11.

Neoprene Pads:

Neoprene bearing pads shall be 60 durometer and shall be in
accordance with Sec 716.

Joint Filler:

All joint filler shall be in accordance with Sec 1057 for preformed sponge
rubber expansion and partition joint filler, except as noted.

Reinforcing Steel:

Minimum clearance to reinforcing steel shall be 1 1/2", unless otherwise
shown.

Traffic Handling:

Structure to be closed during construction. Traffic to be maintained on
other routes during construction. See roadway plans for traffic control.

Miscellaneous:

MoDOT Construction personnel will indicate the type of joint filler option
used under the precast panels for this structure:

- Constant Joint Filler
- Variable Joint Filler

Estimated Quantities for Slab on Concrete NU-Girder

Item	Total
Class B-2 Concrete cu. yard	161
Reinforcing Steel (Epoxy Coated) pound	33,320

Notes:

The table of Estimated Quantities for Slab on Concrete
NU-Girder represents the quantities used by the State
in preparing the cost estimate for concrete slabs. The
area of the concrete slab will be measured to the
nearest square yard longitudinally from end of slab to
end of slab and transversely from out to out of bridge
slab (or with the horizontal dimensions as shown on
the plan of slab). Payment for prestressed panels,
conventional forms, all concrete and epoxy coated
reinforcing steel will be considered completely
covered by the contract unit price for the slab.
Variations may be encountered in the estimated
quantities but the variations cannot be used for an
adjustment in the contract unit price.

Method of forming the slab shall be as shown on the
plans and in accordance with Sec 703. All hardware for
forming the slab to be left in place as a permanent
part of the structure shall be coated in accordance
with ASTM A123 or ASTM B633 with a thickness class SC
4 and a finish type I, II or III.

The Estimated Quantities for Slab on Concrete
NU-Girder are based on skewed precast prestressed end
panels.

Class B-2 Concrete quantity is based on minimum top
flange thickness and minimum joint material thickness.

The prestressed panel quantities are not included in
the table of Estimated Quantities for Slab on Concrete
NU-Girder.

Estimated Quantities

Item	Substr.	Superstr.	Total
Class 1 Excavation cu. yard	75		75
Bridge Approach Slab (Major) sq. yard		171	171
Galvanized Structural Steel Piles (12 in.) linear foot	1106		1106
Pile Point Reinforcement each	14		14
Class B Concrete (Substructure) cu. yard	62.1		62.1
Type D Barrier linear foot		235	235
Slab on Concrete NU-Girder sq. yard		398	398
NU 43, Prestressed Concrete NU-Girder linear foot		335	335
Slab Drain each		14	14
Vertical Drain at End Bents each	2		2
Plain Neoprene Bearing Pad each		8	8

All concrete above the construction joint in the end bents is included in the
Estimated Quantities for Slab on Concrete NU-Girder.

All reinforcement in the end bents is included in the Estimated Quantities for Slab
on Concrete NU-Girder.

Cost of L4x4 ASTM A709 Grade 36 HP pile anchors and 3/4-inch diameter ASTM F3125
Grade A325 Type 1 bolts, complete in place, will be considered completely covered by
the contract unit price for Galvanized Structural Steel Piles (12 in.).

Foundation Data

Type	Design Data	Bent Number	
		1	2
Load Bearing Pile	Pile Type and Size	HP 12x53	HP 12x53
	Number	7	7
	Approximate Length Per Each	ft 79	79
	Pile Point Reinforcement	ea All	All
	Min. Galvanized Penetration (Elev.)	ft 281	281
	Pile Driving Verification Method	DF	DF
	Resistance Factor	0.4	0.4
Minimum Nominal Axial Compressive Resistance	kip	523	523

DF = FHWA-modified Gates Dynamic Pile Formula

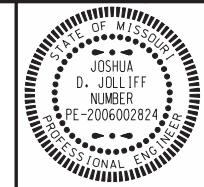
Load Bearing Pile:

Minimum Nominal Axial Compressive Resistance = $\frac{\text{Maximum Factored Loads}}{\text{Resistance Factor}}$

All piles shall be galvanized down to the minimum galvanized penetration
(elevation).

Pile point reinforcement need not be galvanized. Shop drawings will not
be required for pile point reinforcement.

The contractor shall make every effort to achieve the minimum galvanized
penetration (elevation) shown on the plans for all piles. Deviations in
penetration less than 5 feet of the minimum will be considered acceptable
provided the contractor makes the necessary corrections to ensure the
minimum penetration is achieved on subsequent piles.



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DATE PREPARED
3/3/2023

ROUTE STATE
67 MO

DISTRICT SHEET NO.
BR 2

COUNTY
BUTLER

JOB NO.
J9P3751

CONTRACT ID.

PROJECT NO.

BRIDGE NO.
A9278

DESCRIPTION

DATE

MISSOURI HIGHWAYS AND TRANSPORTATION
COMMISSION

105 WEST CAPITAL
JEFFERSON CITY, MO 65102
1-888-ASK-MODOT (1-888-278-6636)

MoDOT

COMMISSION

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ESTIMATED QUANTITIES AND GENERAL NOTES

Detailed Oct. 2022
Checked Oct. 2022

Note: This drawing is not to scale. Follow dimensions.

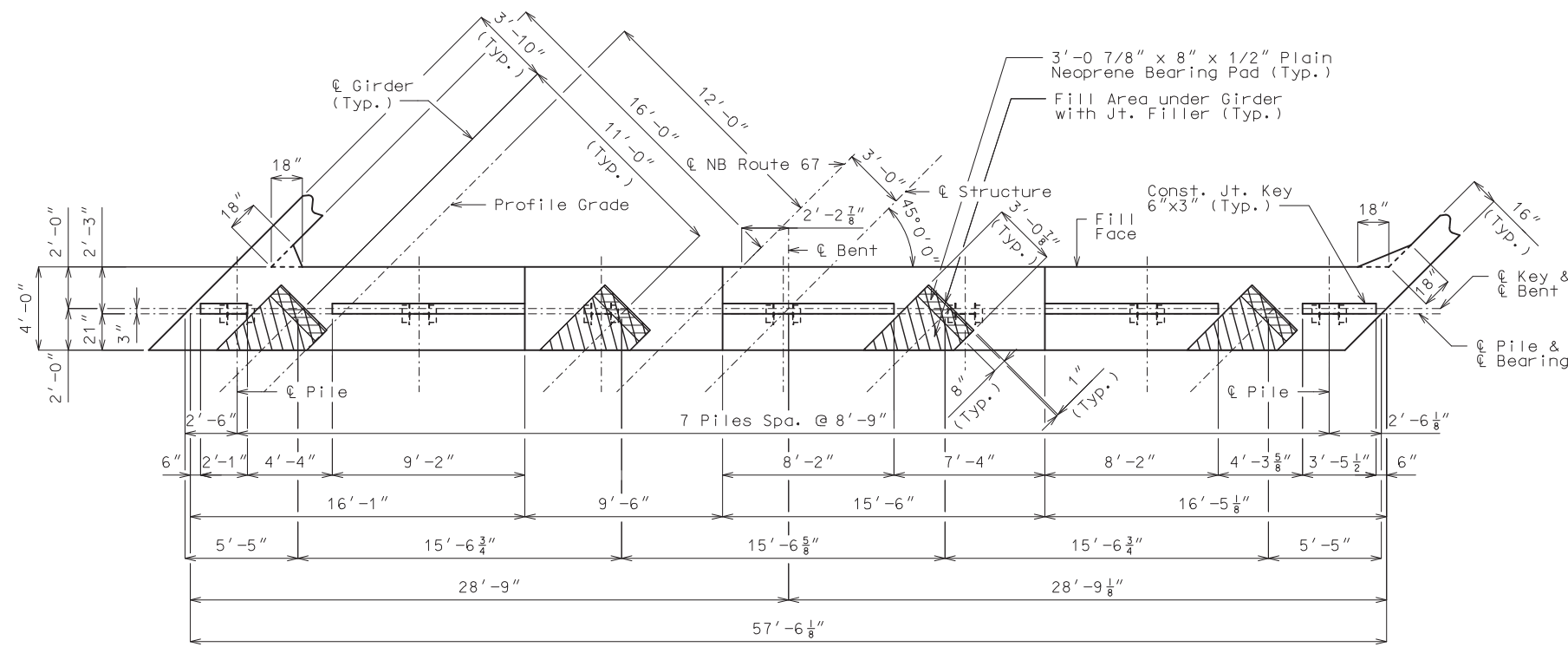
Sheet No. 2 of 24

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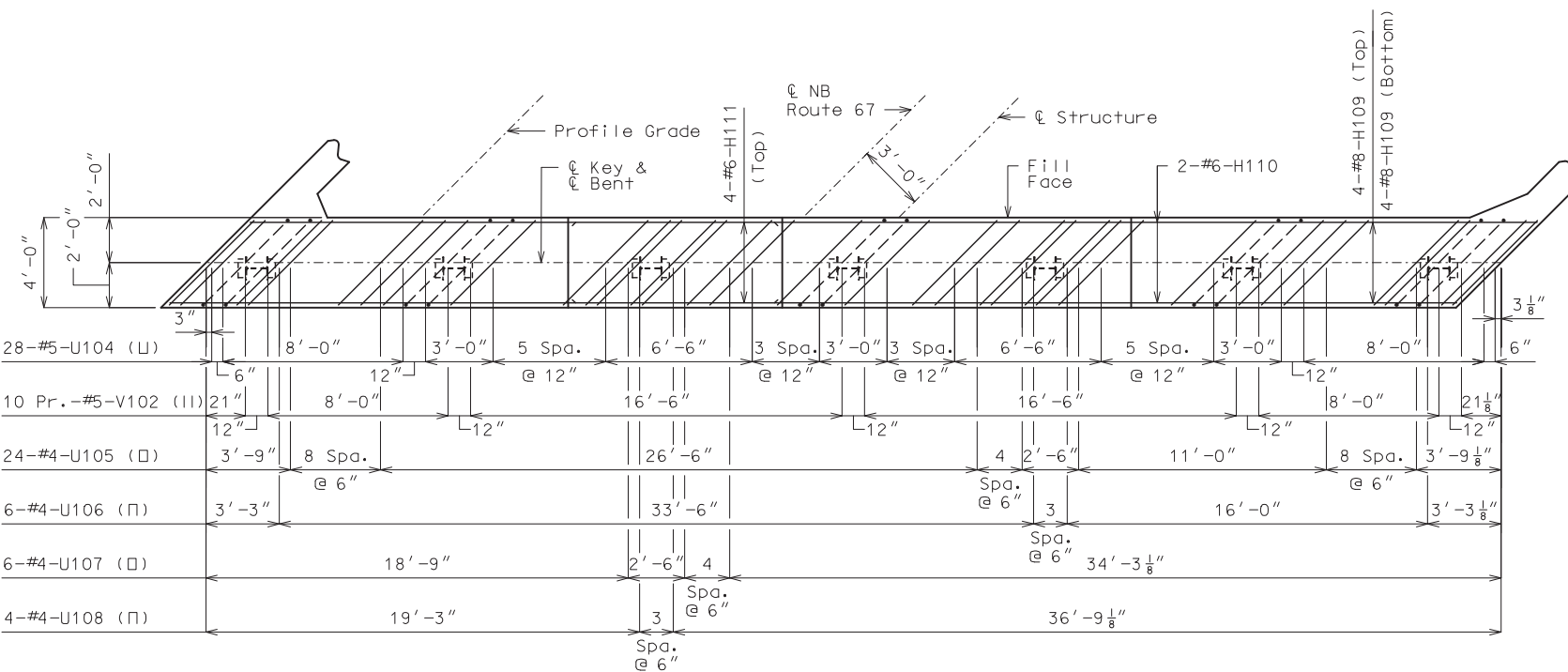
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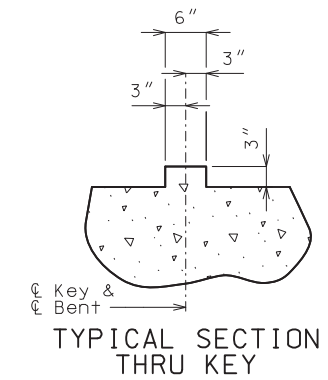


PLAN OF BEAM SHOWING DIMENSIONS

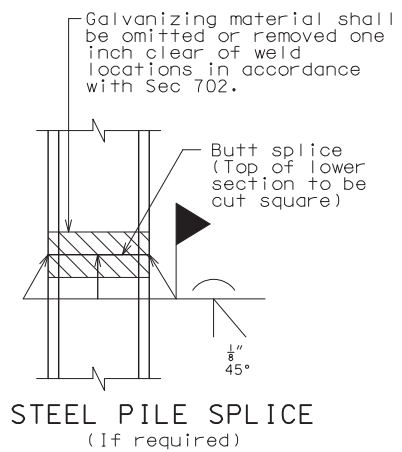


PLAN OF BEAM SHOWING REINFORCEMENT
(Keys not shown for clarity)

DETAILS OF END BENT NO. 1



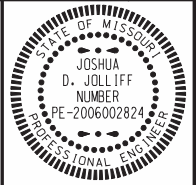
TYPICAL SECTION THRU KEY



STEEL PILE SPLICE
(If required)

General Notes:

- For details of End Bent No. 1 not shown, see Sheets No. 4 & 5.
- For details of Vertical Drain at End Bents, see Sheet No. 6.
- The U bars and pairs of V bars shall be placed parallel to centerline of roadway.
- Reinforcing steel shall be shifted to clear piles, U bars shall clear piles by at least 1 1/2".



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3/3/2023

ROUTE 67 STATE MO
DISTRICT BR SHEET NO. 3

COUNTY BUTLER
JOB NO. J9P3751
CONTRACT ID.

PROJECT NO.

BRIDGE NO. A9278

DESCRIPTION	DATE

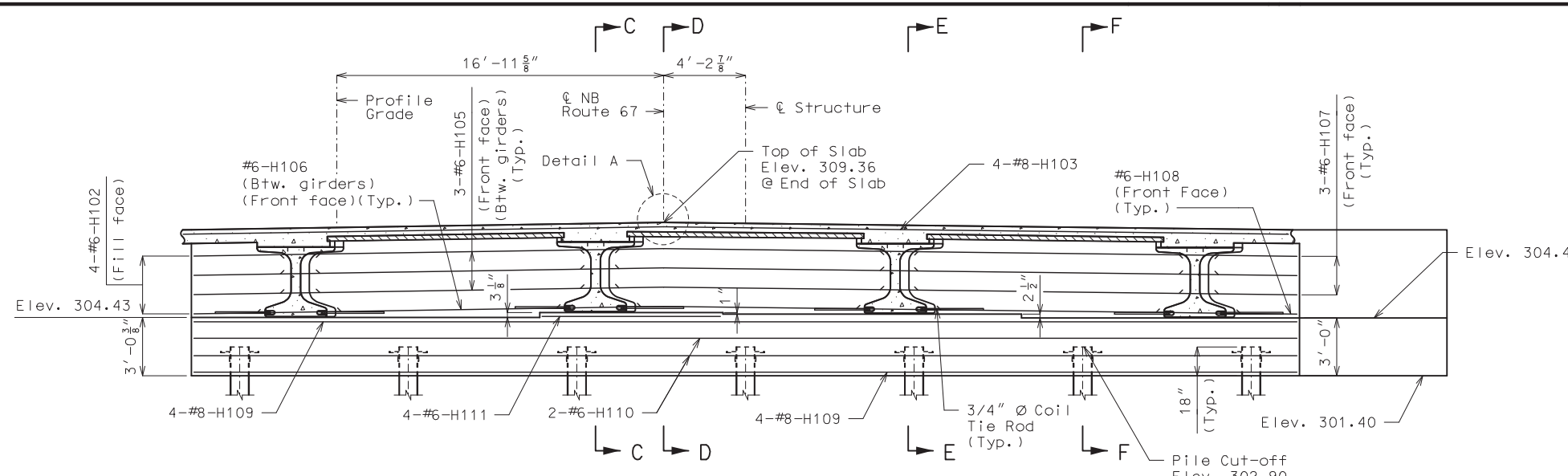
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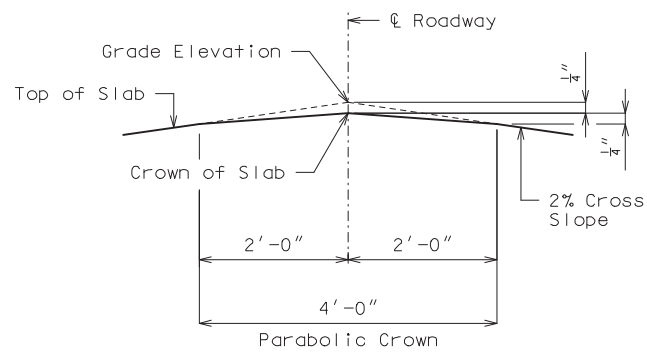
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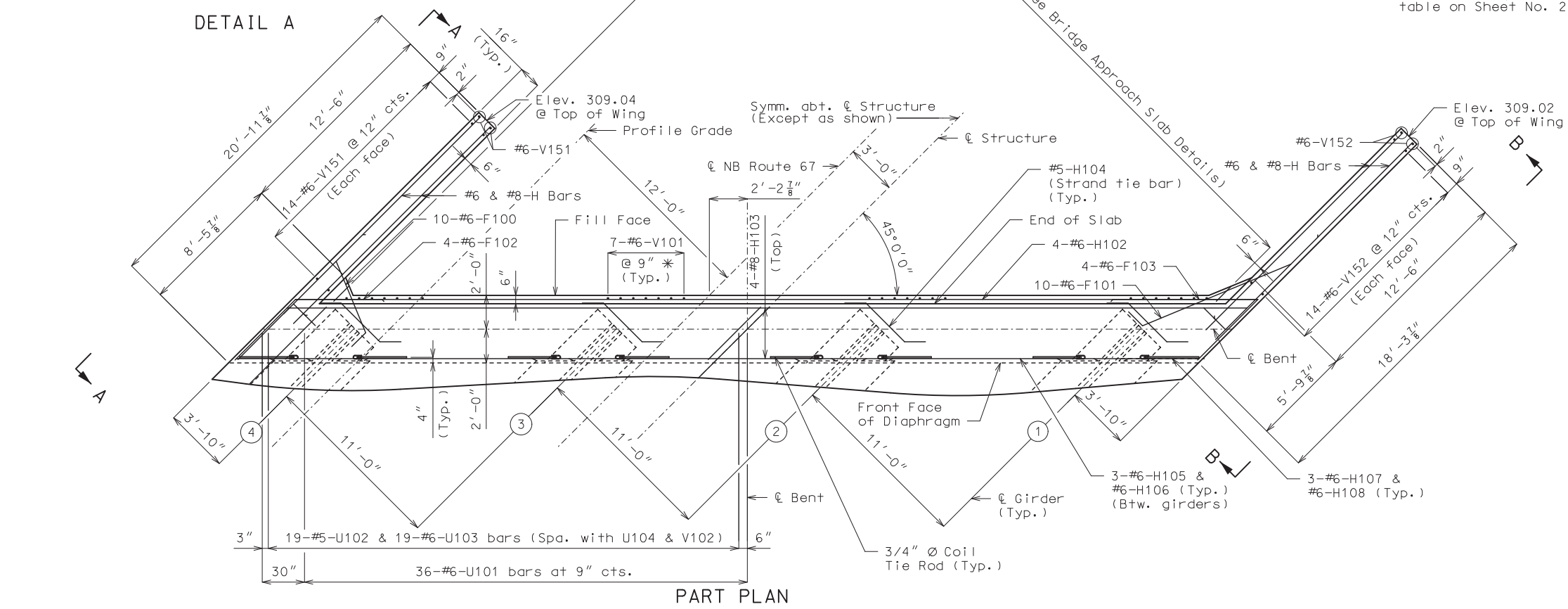
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SECTION NEAR END BENT
Keys not shown for clarity.



DETAIL A



PART PLAN

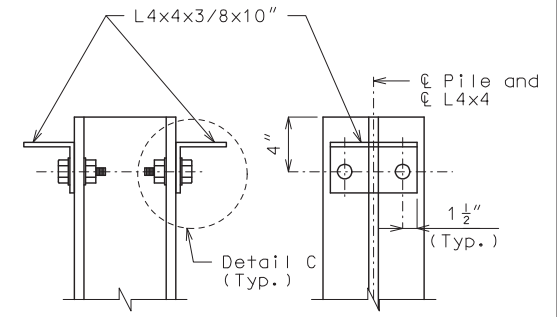
DETAILS OF END BENT NO. 1

General Notes:

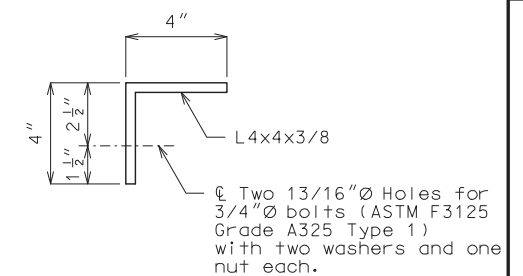
- For details of End Bent No. 1 not shown, see Sheets No. 3 & 5.
- For Sections C-C, D-D, E-E, & F-F, see Sheet No. 5.
- For Elevations A-A & B-B, see Sheet No. 5.
- All concrete in the end bent above top of beam and below top of slab shall be Class B-2.
- For location of Coil Tie Rods and #5-H104 (Strand Tie Bar), see Sheets No. 10 and 11.
- For details of Vertical Drain at End Bents, see Sheet No. 6.
- Reinforcing steel shall be shifted to clear piles, U bars shall clear piles by at least 1 1/2".
- The #6-F100 and #6-F101 bars shall be bent in the field to clear girders.
- The U bars shall be placed parallel to centerline of roadway.
- Strands at end of girders shall be field bent or, if necessary, cut in field to maintain 1 1/2-inch minimum clearance to fill face of end bent.
- For details and reinforcement of the Type D Barrier, see Sheet No. 18.
- For details of Bridge Approach Slab, see Sheet No. 19.

Item	Quantity
Class 1 Excavation	cu. yard 10
Galvanized Structural Steel Piles (12 in.)	linear foot 553
Pile Point Reinforcement	each 7
Class B Concrete (Substructure)	cu. yard 31.0

Note: These quantities are included in the Estimated Quantities table on Sheet No. 2.

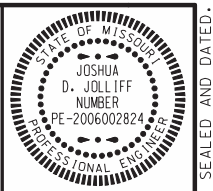


DETAILS OF HP PILE ANCHORS



DETAIL C

Angles shall be coated with a minimum of two coats of non-aluminum epoxy mastic primer to provide a dry film thickness of 4 mils minimum, 8 mils maximum, or galvanized in accordance with Sec 1081. Bolts, washers and nuts shall be galvanized in accordance with AASHTO M 232 (ASTM A153), Class C.



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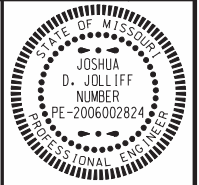
DATE PREPARED
3/3/2023
ROUTE 67 STATE MO
DISTRICT BR SHEET NO. 4
COUNTY BUTLER
JOB NO. J9P3751
CONTRACT ID.
PROJECT NO.
BRIDGE NO. A9278

DATE	DESCRIPTION

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION
105 WEST CAPITOL JEFFERSON CITY, MO 65102
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ROUTE 67 STATE MO

DISTRICT BR SHEET NO. 5

COUNTY BUTLER

JOB NO. J9P3751

CONTRACT ID.

PROJECT NO.

BRIDGE NO. A9278

DESCRIPTION

DATE

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105 WEST CAPITOL JEFFERSON CITY, MO 65102

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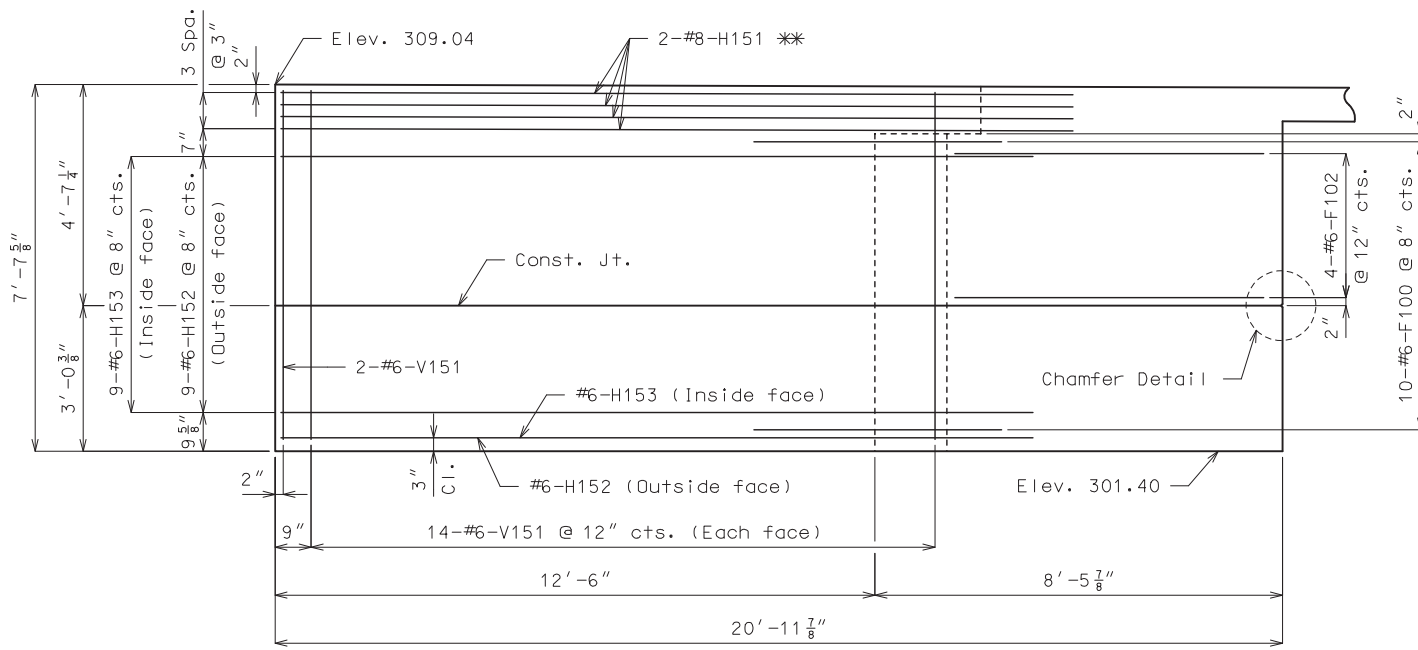
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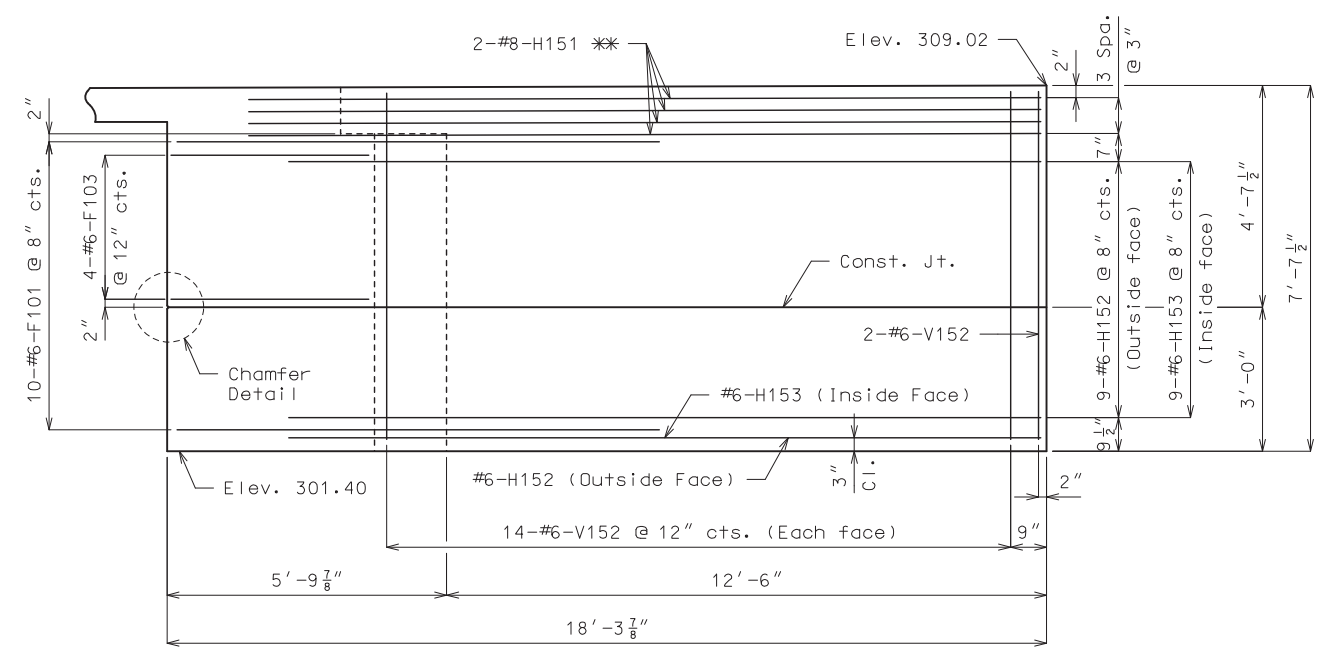
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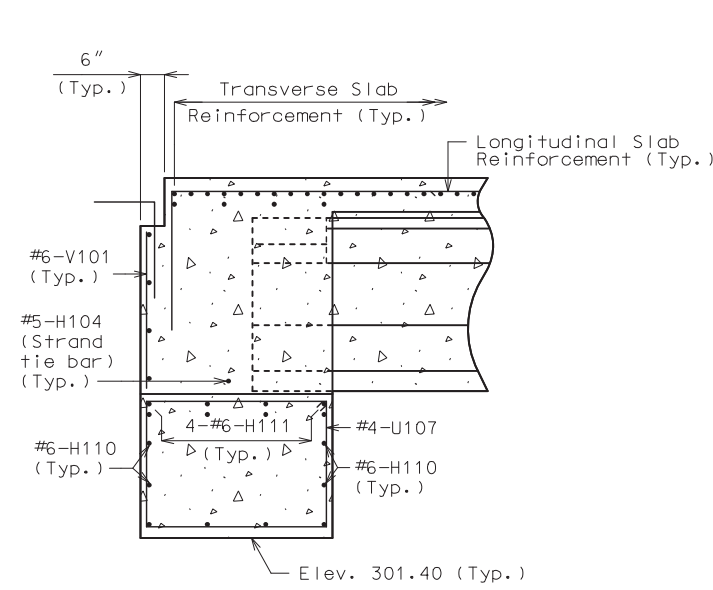
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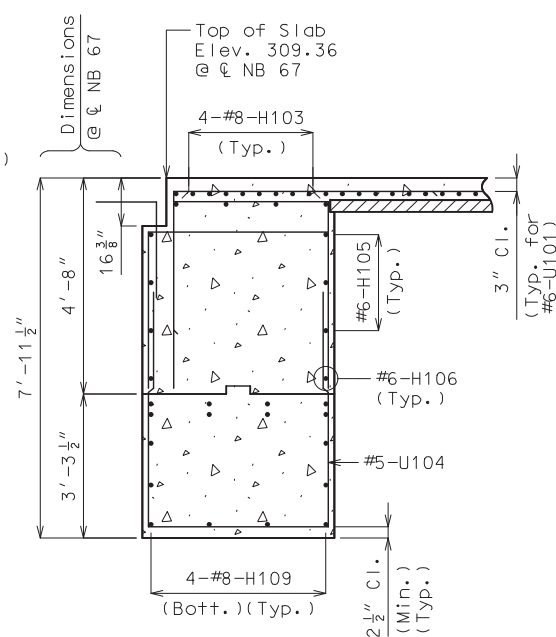
ELEVATION A-A



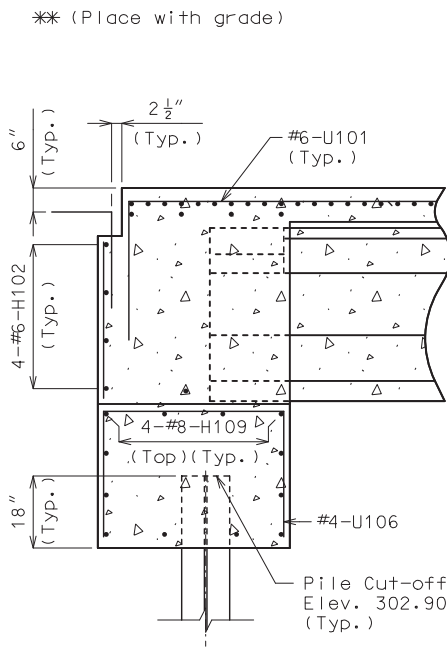
ELEVATION B-B



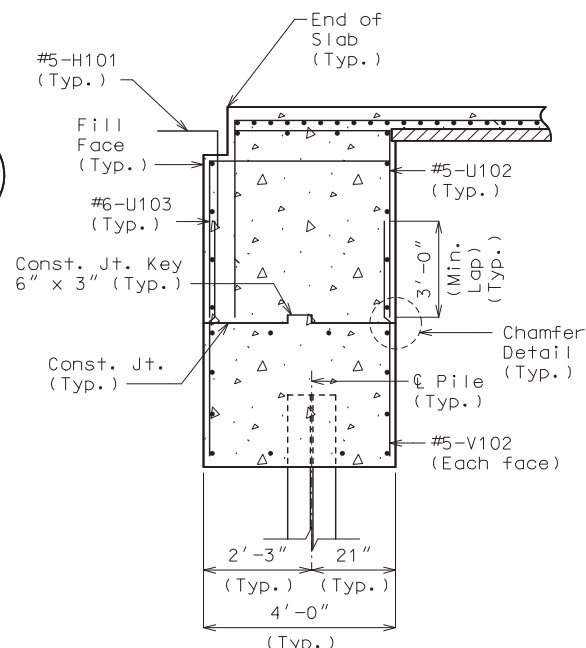
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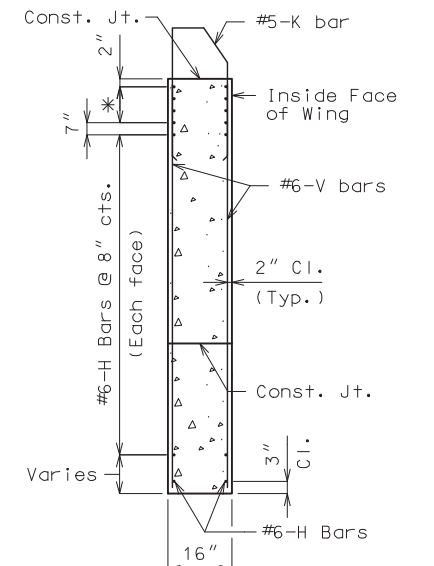
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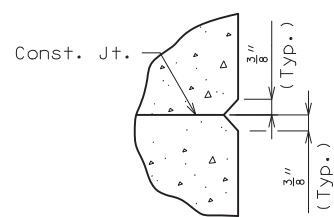
SECTION E-E



SECTION F-F



TYPICAL SECTION THRU WING



CHAMFER DETAIL

DETAILS OF END BENT NO. 1

General Notes:

For details of End Bent No. 1 not shown, see Sheets No. 3 & 4.

All concrete in the end bent above top of beam and below top of slab shall be Class B-2.

The #6-F100 and #6-F101 bars shall be bent in field to clear girders.

For details and reinforcement of the Type D Barrier, see Sheet No. 18.

For details of Vertical Drain at End Bents, see Sheet No. 6.

For location of #5-H104 (Strand tie bar), see Sheets No. 10 and 11.

For location of Sections C-C, D-D, E-E, & F-F, see Sheet No. 4.

For location of Elevations A-A & B-B, see Sheet No. 4.

For details of Bridge Approach Slab, see Sheet No. 19.

Detailed Oct. 2022 Checked Oct. 2022

Note: This drawing is not to scale. Follow dimensions.

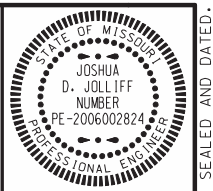
Sheet No. 5 of 24

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3/3/2023

ROUTE 67 STATE MO

DISTRICT BR SHEET NO. 6

COUNTY BUTLER

JOB NO. J9P3751

CONTRACT ID.

PROJECT NO.

BRIDGE NO. A9278

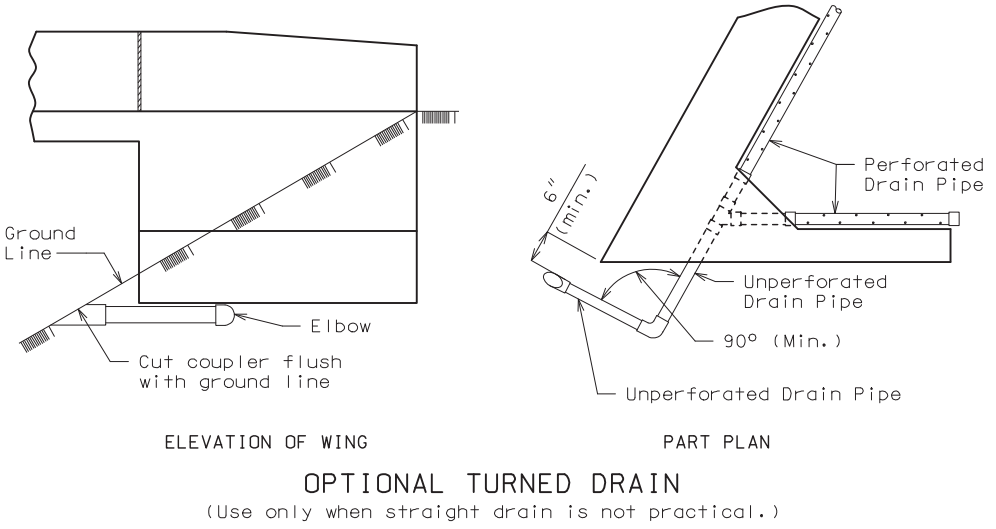
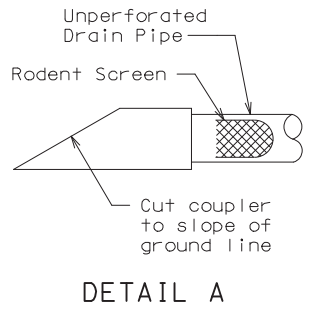
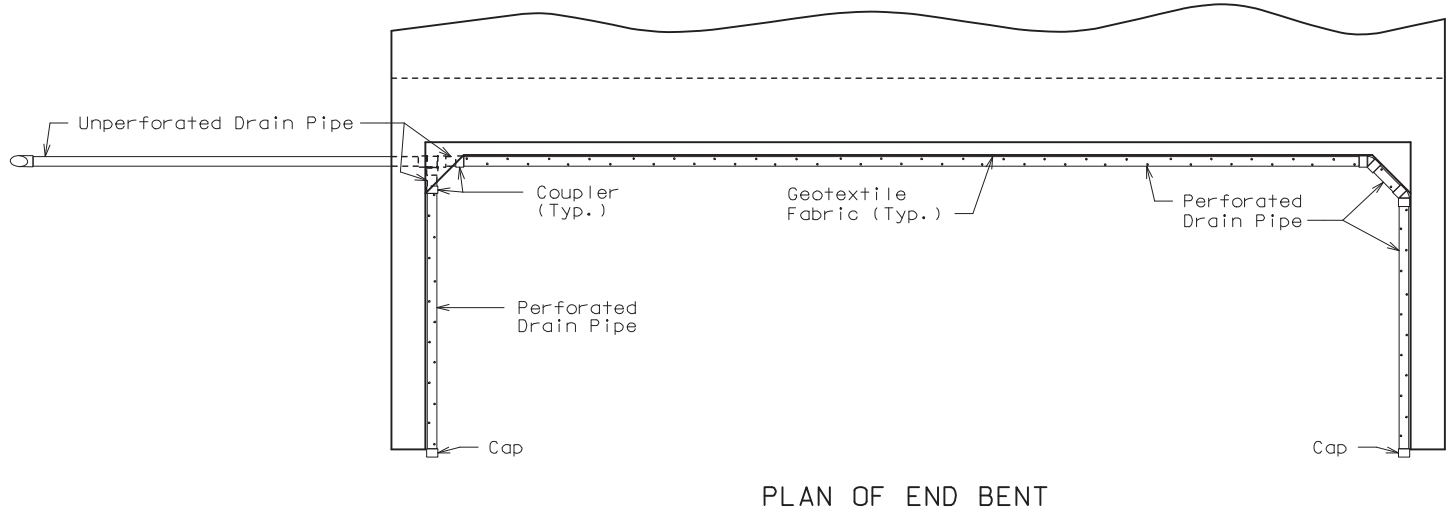
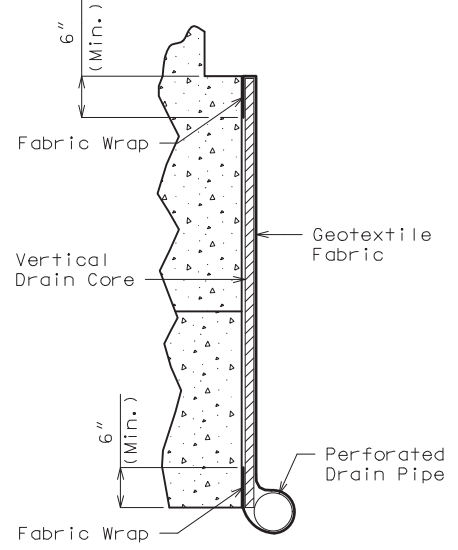
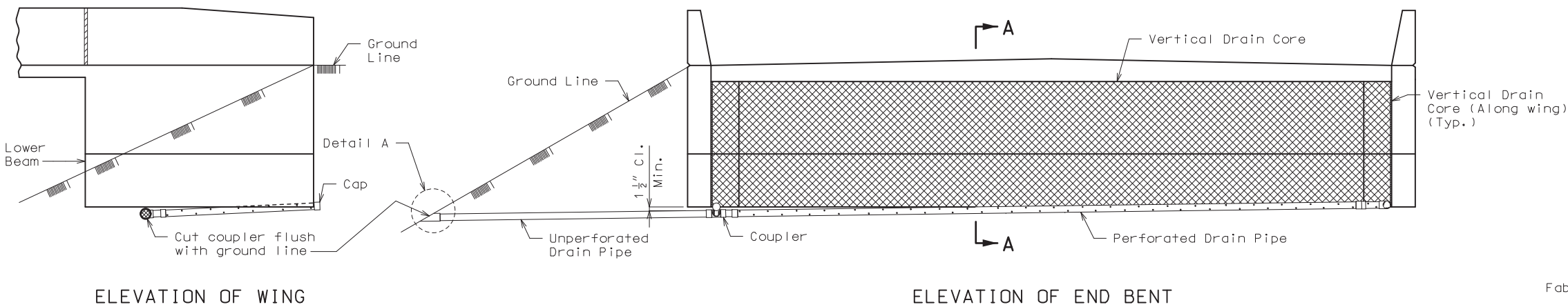
DATE	DESCRIPTION

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General Notes:

All drain pipe shall be sloped 1 to 2 percent.

Drain pipe may be either 6-inch diameter corrugated metallic-coated steel pipe underdrain, 4-inch diameter corrugated polyvinyl chloride (PVC) drain pipe, or 4-inch diameter corrugated polyethylene (PE) drain pipe.

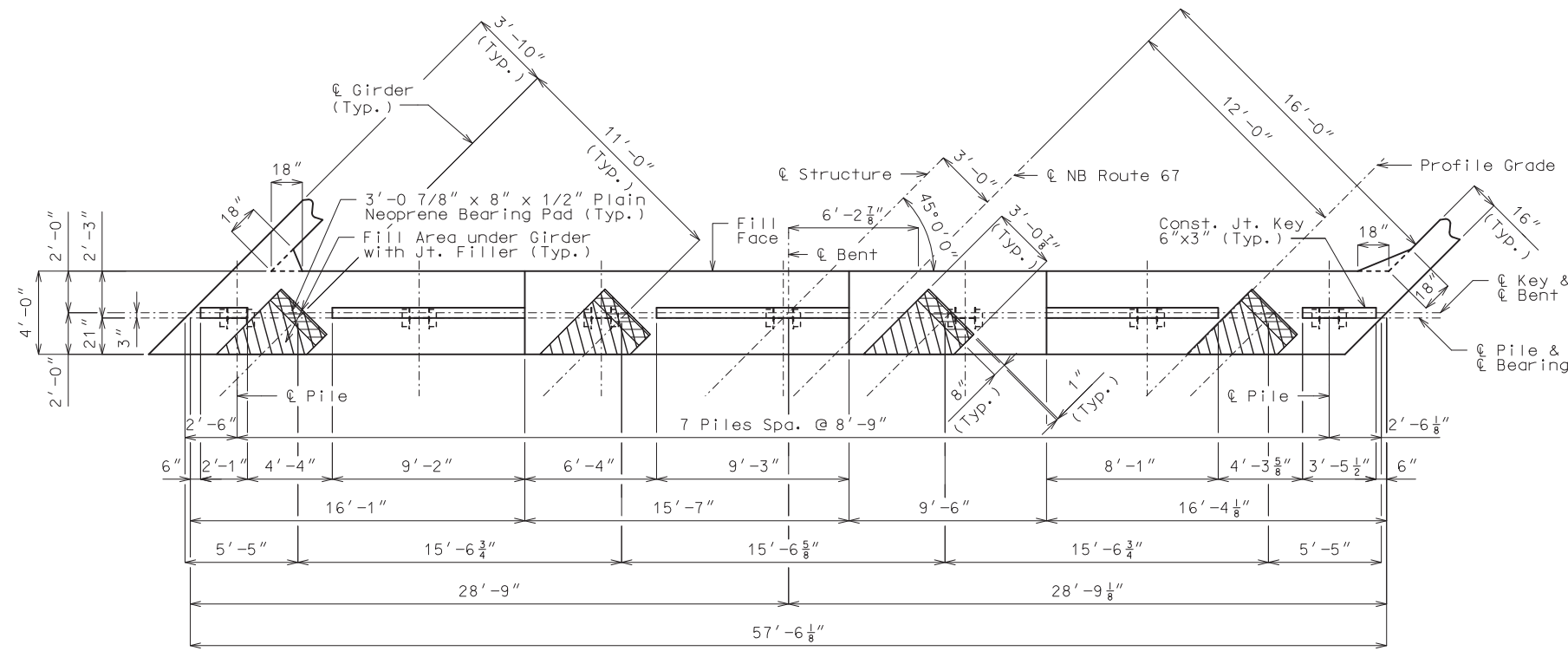
Drain pipe shall be placed at fill face of end bent and inside face of wings. The pipe shall slope to lowest grade of ground line, also missing the lower beam of end bent by a minimum of 1 1/2 inches.

Perforated pipe shall be placed at fill face side and inside face of wings at the bottom of end bent and plain pipe shall be used where the vertical drain ends to the exit at ground line.

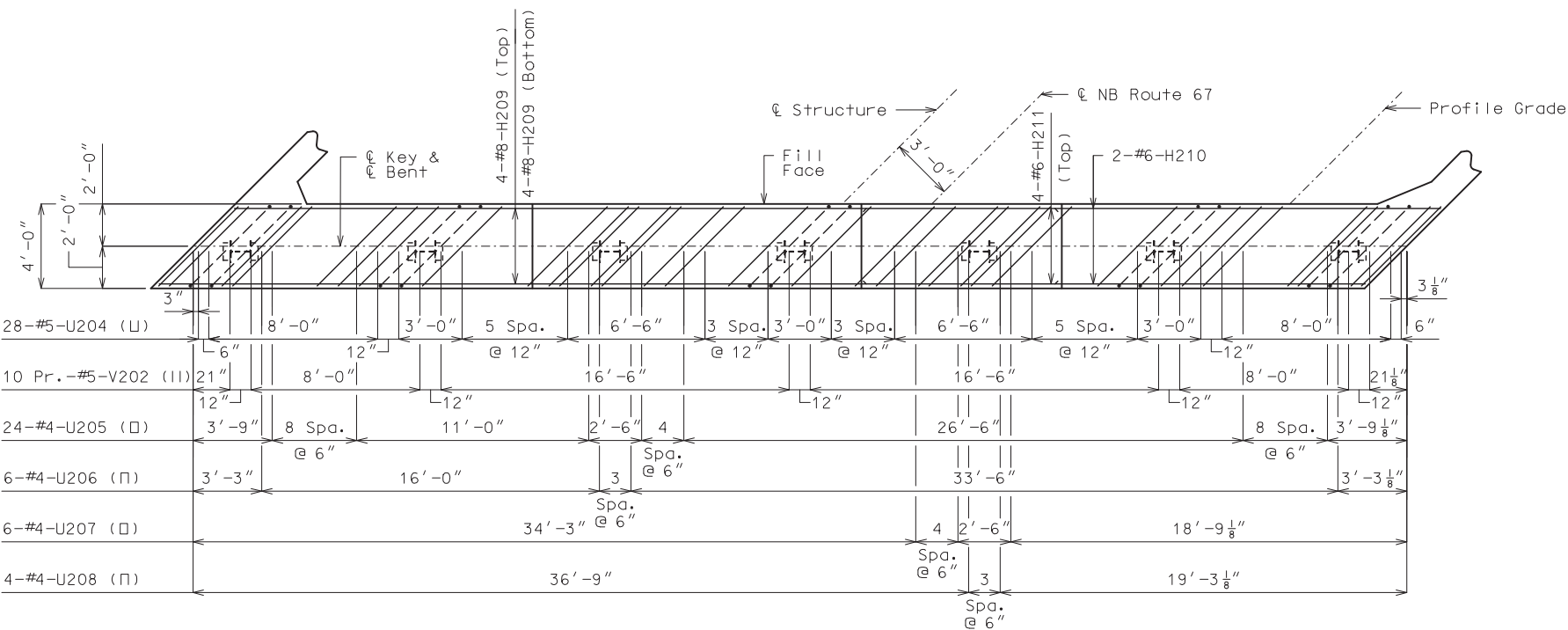
VERTICAL DRAIN AT END BENTS
(Squared end bent shown, skewed end bent similar)

Detailed Oct. 2022
Checked Oct. 2022

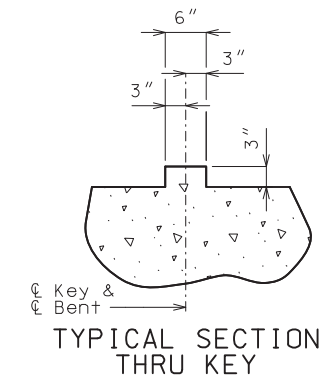
Note: This drawing is not to scale. Follow dimensions. Sheet No. 6 of 24



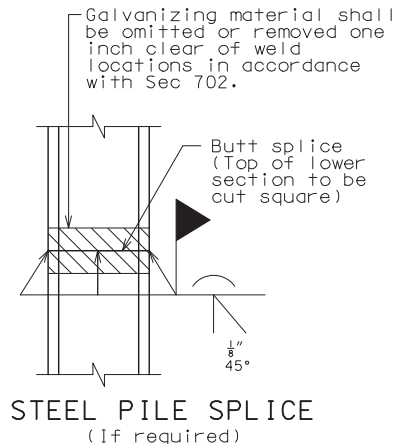
PLAN OF BEAM SHOWING DIMENSIONS



PLAN OF BEAM SHOWING REINFORCEMENT
(Keys not shown for clarity)



TYPICAL SECTION THRU KEY



STEEL PILE SPLICE
(If required)

General Notes:
 For details of End Bent No. 2 not shown, see Sheets No. 8 & 9.
 For details of Vertical Drain at End Bents, see Sheet No. 6.
 The U bars and pairs of V bars shall be placed parallel to centerline of roadway.
 Reinforcing steel shall be shifted to clear piles, U bars shall clear piles by at least 1 1/2".

DETAILS OF END BENT NO. 2

Detailed Oct. 2022
 Checked Oct. 2022

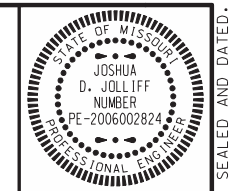
Note: This drawing is not to scale. Follow dimensions.

Sheet No. 7 of 24

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3/3/2023



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DATE PREPARED
 3/3/2023

ROUTE 67 STATE MO
 DISTRICT BR SHEET NO. 7

COUNTY BUTLER
 JOB NO. J9P3751
 CONTRACT ID.

PROJECT NO.

BRIDGE NO. A9278

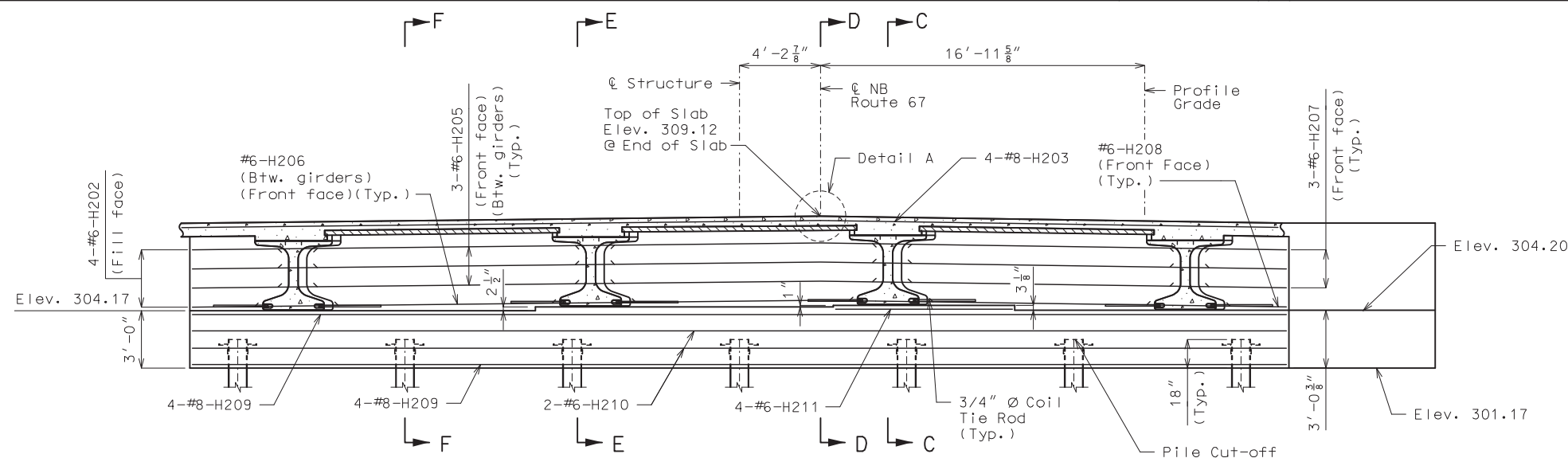
DATE	DESCRIPTION

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

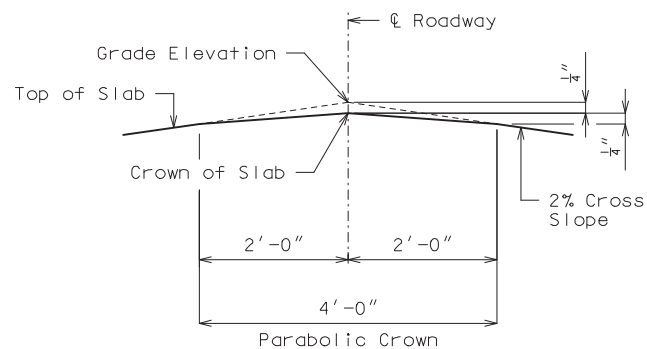
105 WEST CAPITOL
 JEFFERSON CITY, MO 65102
 1-888-ASK-MODOT (1-888-275-6636)

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 6851 WISCONSIN AVE, SUITE 300
 SPRINGFIELD, MO 65807 (417) 869-6009
 ENGINEERING CORPORATION - 000631

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SECTION NEAR END BENT
Keys not shown for clarity.



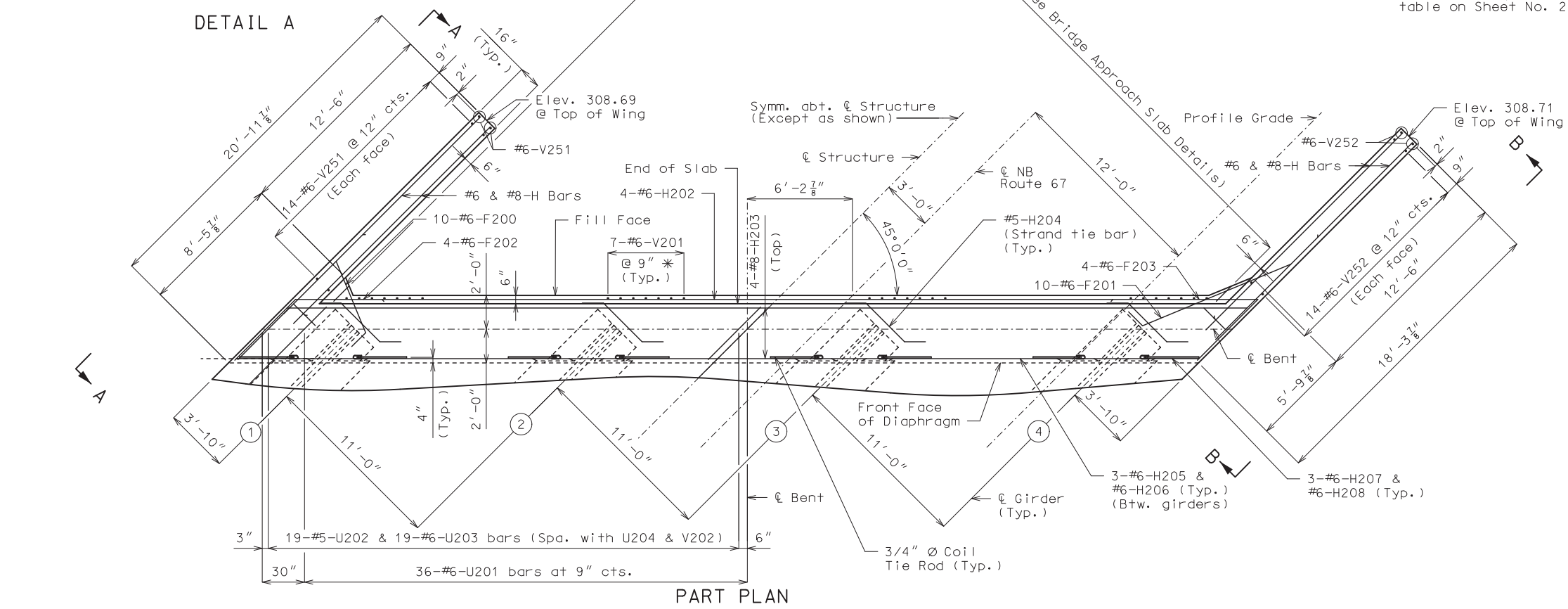
DETAIL A

General Notes:

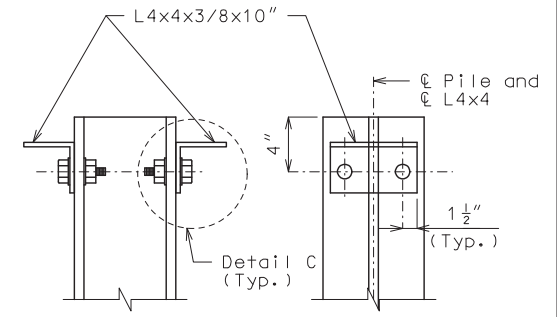
- For details of End Bent No. 2 not shown, see Sheets No. 7 & 9.
- For Sections C-C, D-D, E-E, & F-F, see Sheet No. 9.
- For Elevations A-A & B-B, see Sheet No. 9.
- All concrete in the end bent above top of beam and below top of slab shall be Class B-2.
- For location of Coil Tie Rods and #5-H204 (Strand Tie Bar), see Sheets No. 10 and 11.
- For details of Vertical Drain at End Bents, see Sheet No. 6.
- Reinforcing steel shall be shifted to clear piles, U bars shall clear piles by at least 1 1/2".
- The #6-F200 and #6-F201 bars shall be bent in the field to clear girders.
- The U bars shall be placed parallel to centerline of roadway.
- Strands at end of girders shall be field bent or, if necessary, cut in field to maintain 1 1/2-inch minimum clearance to fill face of end bent.
- For details and reinforcement of the Type D Barrier, see Sheet No. 18.
- For details of Bridge Approach Slab, see Sheet No. 19.

Item	Quantity
Class 1 Excavation	cu. yard 65
Galvanized Structural Steel Piles (12 in.)	linear foot 553
Pile Point Reinforcement	each 7
Class B Concrete (Substructure)	cu. yard 31.1

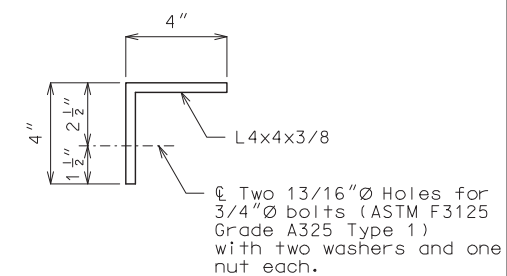
Note: These quantities are included in the Estimated Quantities table on Sheet No. 2.



DETAILS OF END BENT NO. 2



DETAILS OF HP PILE ANCHORS



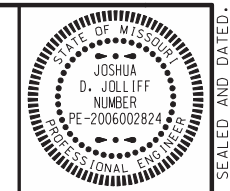
DETAIL C

Angles shall be coated with a minimum of two coats of non-aluminum epoxy mastic primer to provide a dry film thickness of 4 mils minimum, 8 mils maximum, or galvanized in accordance with Sec 1081. Bolts, washers and nuts shall be galvanized in accordance with AASHTO M 232 (ASTM A153), Class C.

Detailed Oct. 2022
Checked Oct. 2022

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 8 of 24



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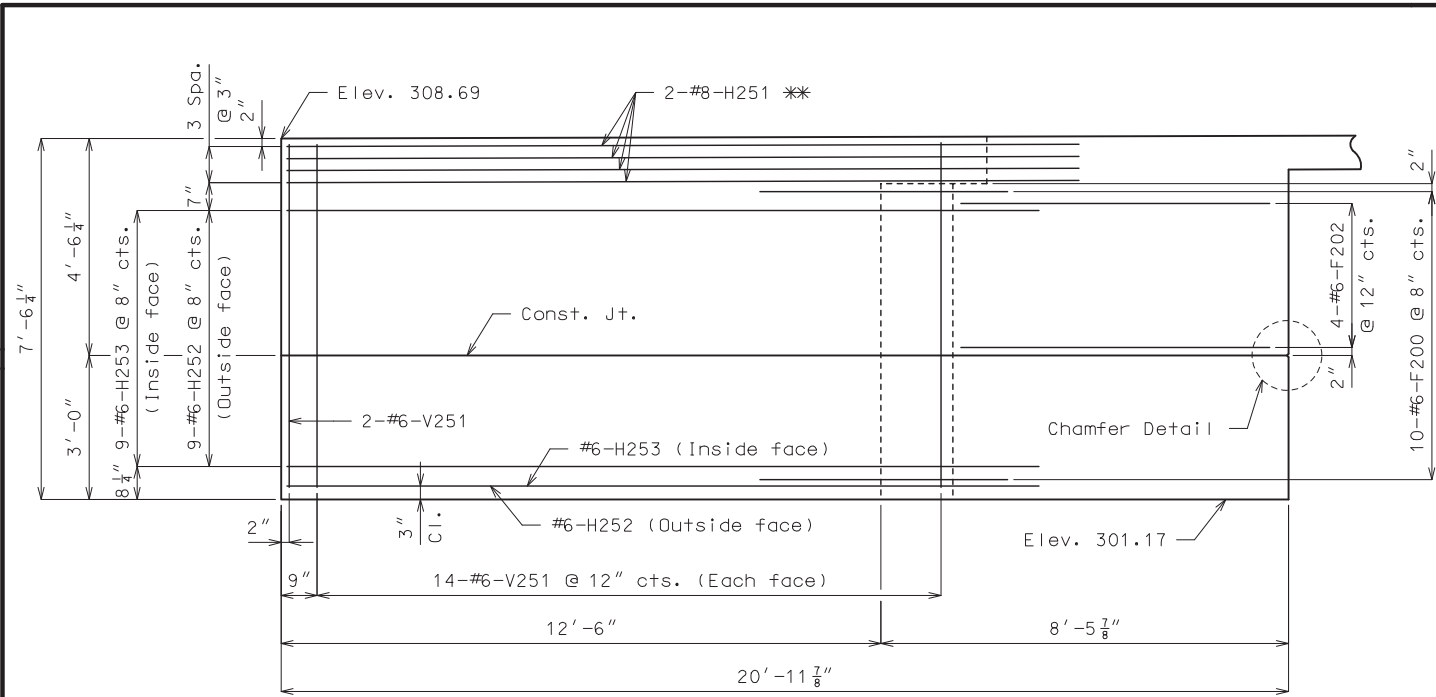
DATE PREPARED	3/3/2023
ROUTE	67
DISTRICT	BR
COUNTY	BUTLER
JOB NO.	J9P3751
CONTRACT ID.	
BRIDGE NO.	A9278

DESCRIPTION	DATE

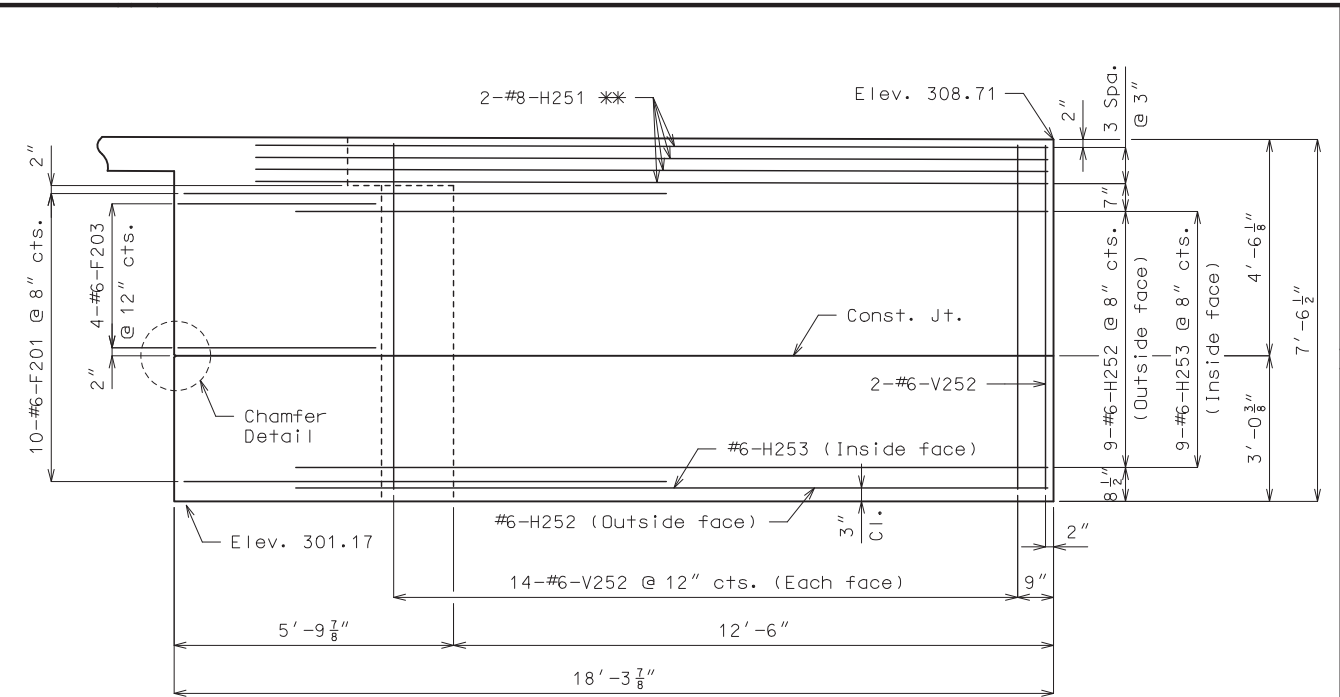
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 105 WEST CAPITOL JEFFERSON CITY, MO 65102
 1-888-ASK-MODOT (1-888-275-6636)

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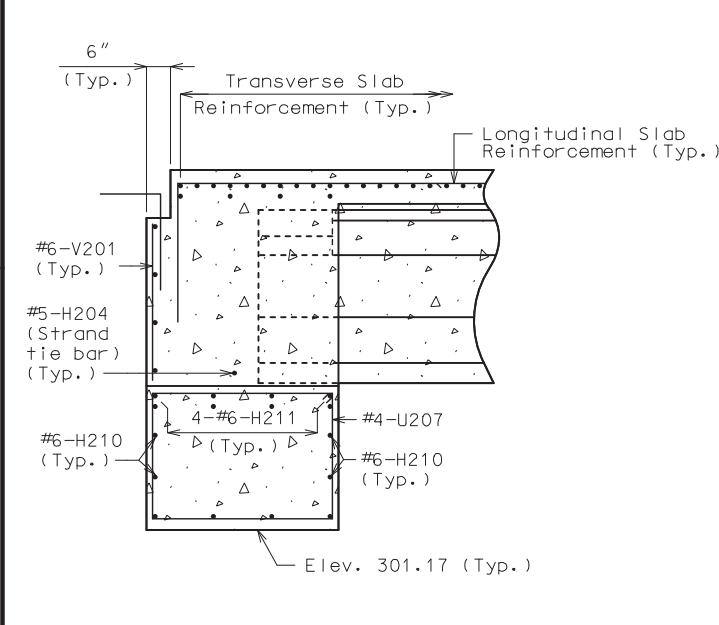
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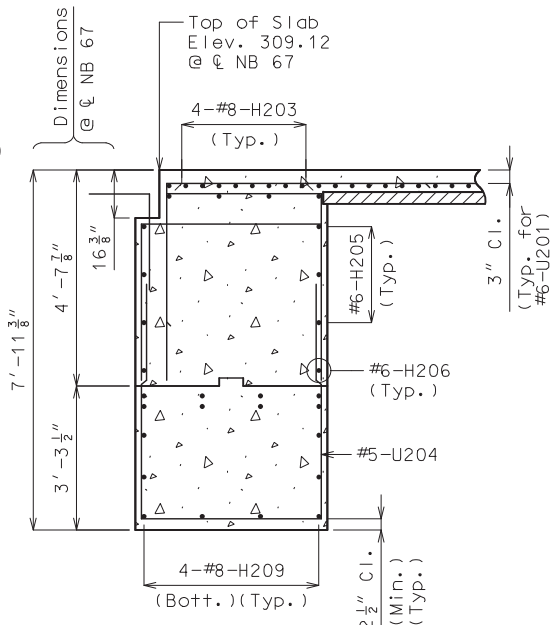
ELEVATION A-A



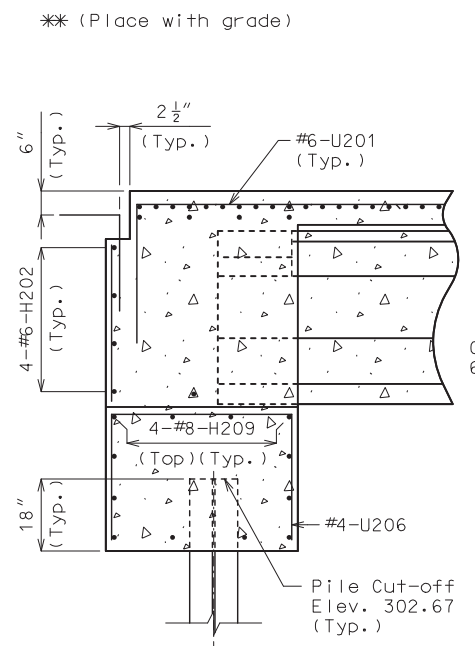
ELEVATION B-B



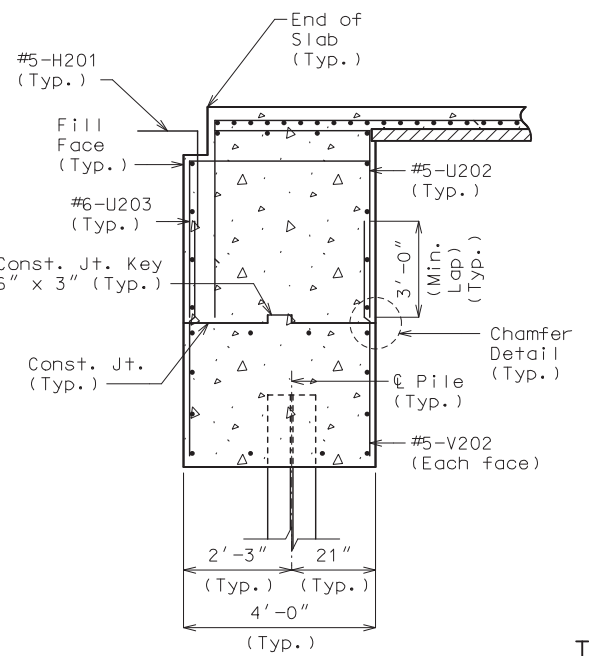
SECTION C-C



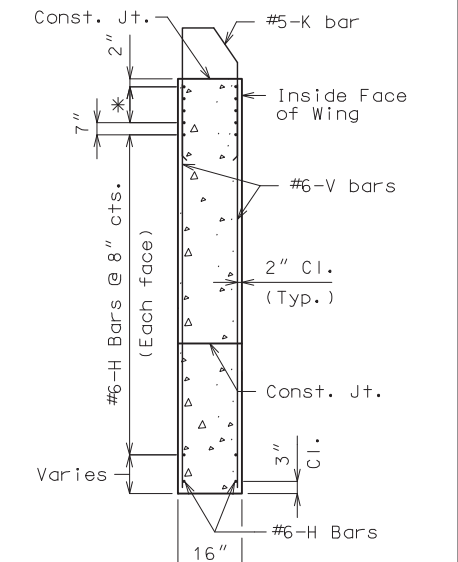
SECTION D-D



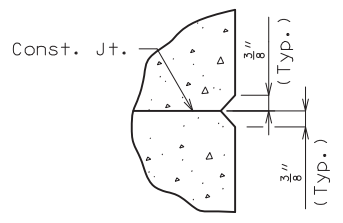
SECTION E-E



SECTION F-F



TYPICAL SECTION THRU WING



CHAMFER DETAIL

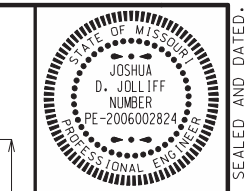
DETAILS OF END BENT NO. 2

Detailed Oct. 2022
Checked Oct. 2022

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 9 of 24

pw:\cmtengr-pw.bentley.com\cmt-projects\Documents\Projects\MoDOT\20040908\Draw\Structures\Sheets\Harviell Ditch\A9278\B-A9278-009-J9P3751_End_Bent_2_Details.dgn



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DATE PREPARED: 3/3/2023
ROUTE: 67 STATE: MO
DISTRICT: BR SHEET NO.: 9

COUNTY: BUTLER
JOB NO.: J9P3751
CONTRACT ID.

PROJECT NO.
BRIDGE NO.: A9278

DESCRIPTION	DATE

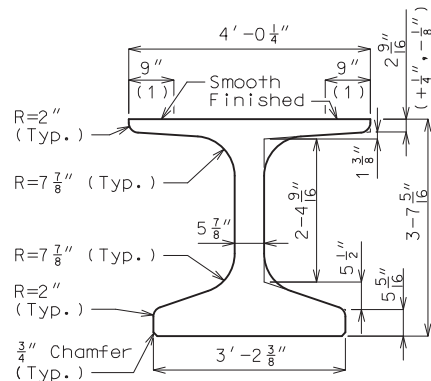
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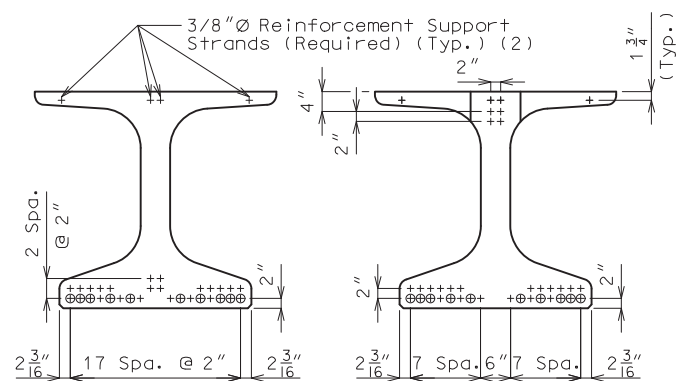
IF A SEAL IS PRESENT ON THIS SHEET IT HAS BEEN ELECTRONICALLY SEALED AND DATED. REV.

(1) Fabricator shall apply a bond breaker to this region excluding where joint filler will be applied.

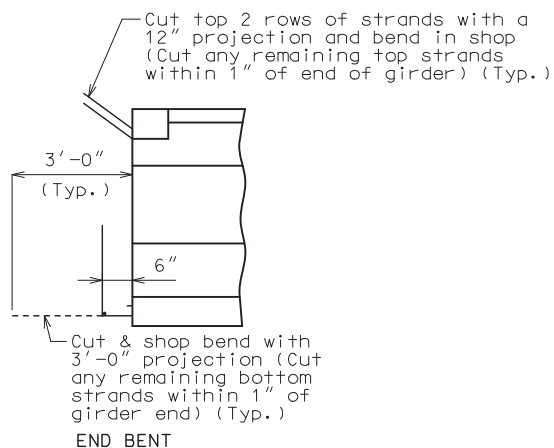
(2) Outer strands tensioned to 2.02 kips/strand and inner strands to 8 kips/strand. Placed symmetrical about \bar{C} Girder. May be moved laterally in pairs.



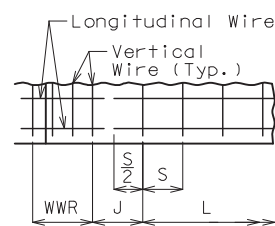
DIMENSIONS



\bar{C} GIRDER STRAND ARRANGEMENT



END BENT STRANDS AT GIRDER ENDS



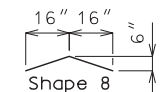
WELDED WIRE PLACEMENT

S = Vertical wire spacing
L = Length of WWR mats
J = Distance between WWR mats

Bill of Reinforcing Steel

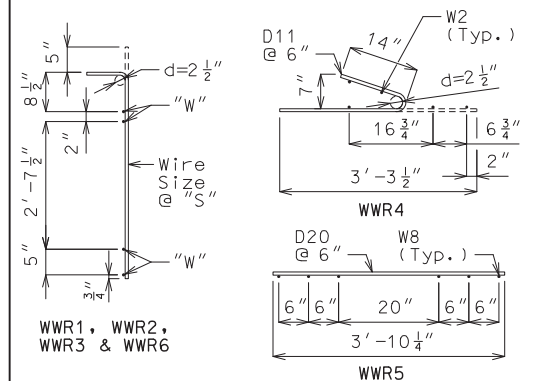
Bars Each Girder				
No.	Size/Mark	Length	Shape	
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2	4 G3	5'-5"	20	
14	4 G6	Varies	20	

Bending Diagrams

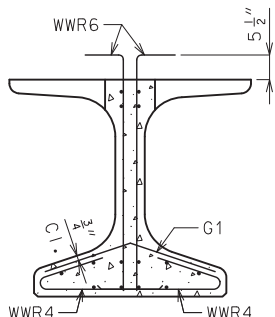


Shape 20

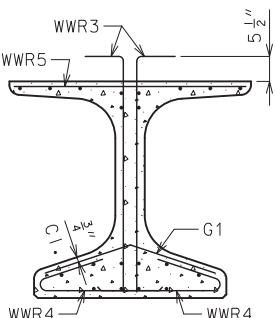
Welded Wire Each Girder					
Mark	Size	S	W	L	J
WWR1	D31	4"	W12	4'-4"	8"
WWR2	D31	8"	W12	22'-8"	12"
WWR3	D31	12"	W12	23'-0"	-
WWR6	D31	2"	W12	16"	2 3/4"



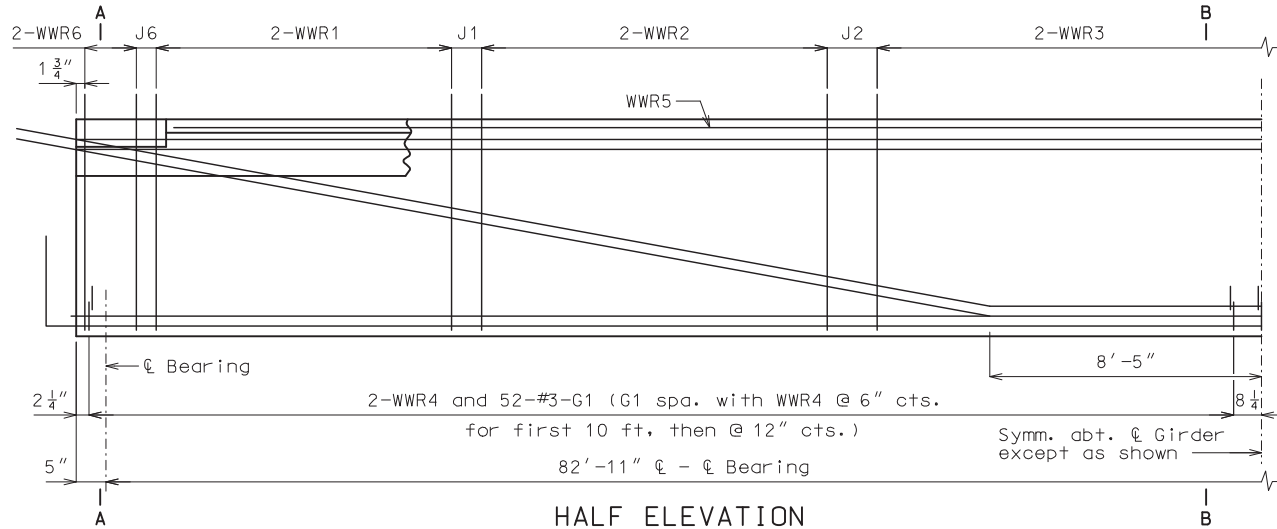
+ Indicates prestressing strand. o Indicates cut & shop bend with 3'-0" projection.



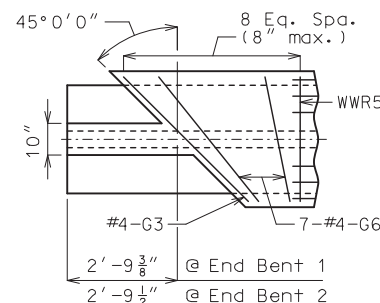
SECTION A-A
Strands not shown for clarity.



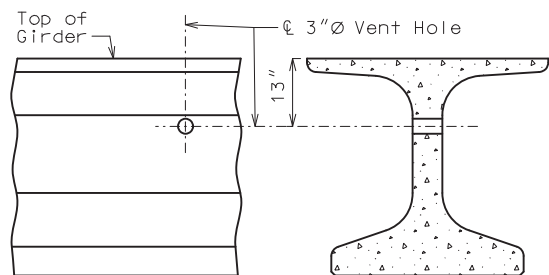
SECTION B-B
Strands not shown for clarity.



HALF ELEVATION
Reinforcement support strands not shown for clarity.

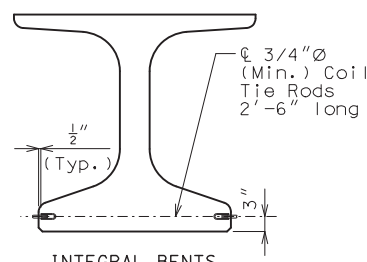


TOP FLANGE BLOCKOUT

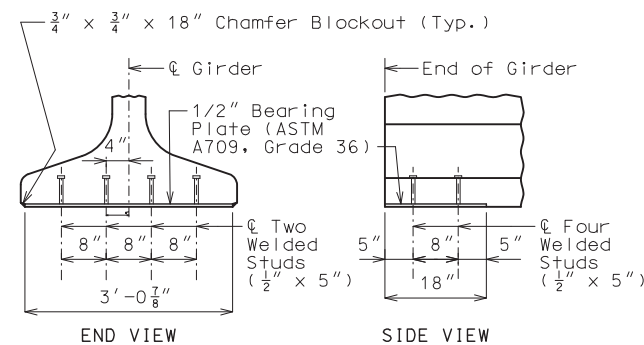


VENT HOLE

Place vent holes at or near upgrade 1/3 point of girders and clear reinforcing steel or strands by 1 1/2" minimum.



INTEGRAL BENTS
COIL TIES



BEARING PLATE

NU-GIRDERS - SPAN (1-2)

All dimensions are out to out.

Hooks and bends shall be in accordance with the CRSI Manual of Standard Practice for Detailing Reinforced Concrete Structures, Stirrup and Tie Dimensions.

Actual bar lengths are measured along centerline of bar to the nearest inch.

Minimum clearance to reinforcing shall be 1", unless otherwise shown.

All bar reinforcement shall be Grade 60.

WWR shall not be epoxy coated.

General Notes:

Concrete for prestressed beams shall be Class A-1 with $f'c = 8000$ psi and $f'ci = 6500$ psi.

Use 30 strands, 0.6" \bar{C} Grade 270, with an initial prestress force of 1318 kips.

Pretensioned members shall be in accordance with Sec 1029.

Fabricator shall be responsible for location and design of lifting devices.

Exterior and interior girders are the same except: application of bond breaker, coil inserts for slab drains.

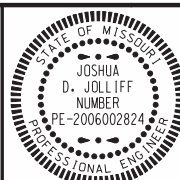
The contractor shall provide bracing necessary for lateral and torsional stability of the girders during construction of the concrete slab and remove the bracing after the slab has attained 75% design strength. Contractor shall not drill holes in the girders.

For Girder Camber Diagram, see Sheet No. 14.

For location of coil inserts at slab drains, see Sheet No. 13.

For location of coil ties at integral bents see Sheets No. 4 and 8.

Alternate bar reinforcing steel details are provided and may be used. The same type of reinforcing steel shall be used for all girders in all spans.



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DATE PREPARED
3/3/2023
ROUTE
67
STATE
MO
DISTRICT
BR
SHEET NO.
10

COUNTY
BUTLER
JOB NO.
J9P3751
CONTRACT ID.

PROJECT NO.

BRIDGE NO.
A9278

DESCRIPTION

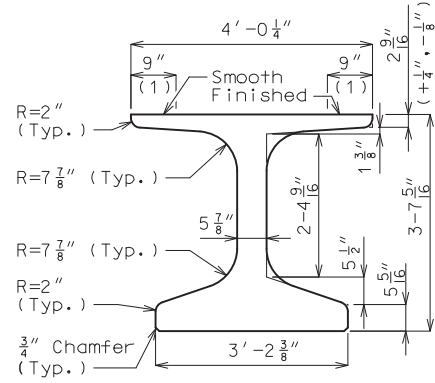
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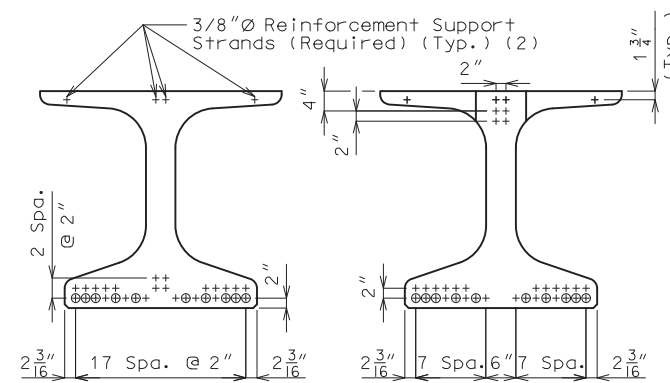
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6630 W. MISSOURI AVENUE
SPRINGFIELD, MO 65807 (417) 869-6009
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(1) Fabricator shall apply a bond breaker to this region excluding where joint filler will be applied.



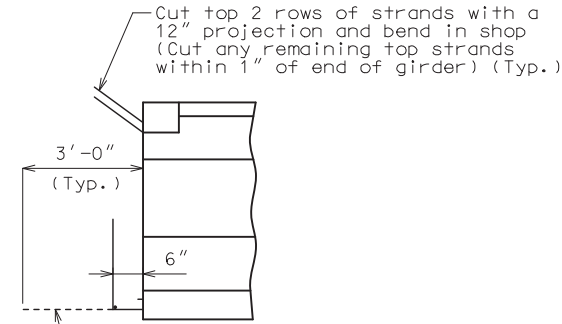
DIMENSIONS

(2) Outer strands tensioned to 2.02 kips/strand and inner strands to 8 kips/strand. Placed symmetrical about \bar{c} Girder. May be moved laterally in pairs.

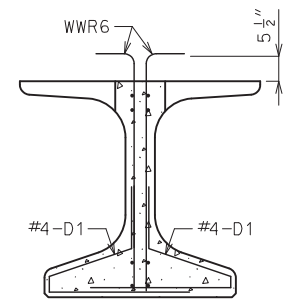


STRAND ARRANGEMENT

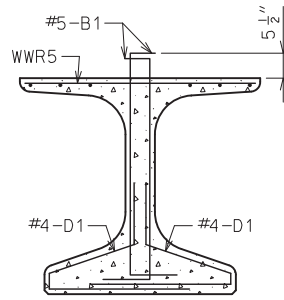
+ Indicates prestressing strand. o Indicates cut & shop bend with 3'-0" projection.



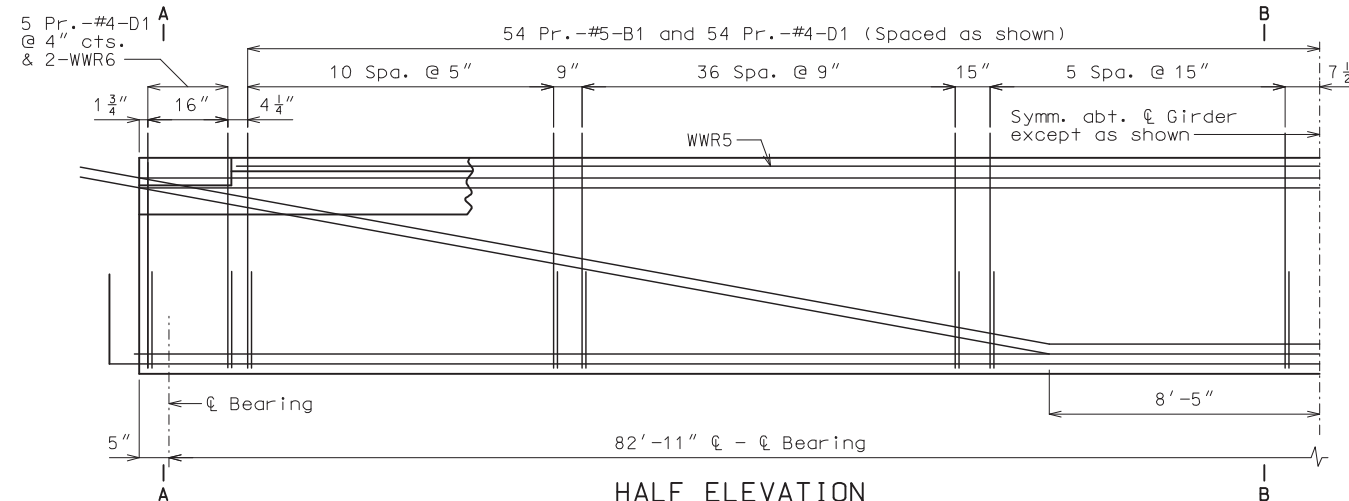
STRANDS AT GIRDER ENDS



SECTION A-A
Strands not shown for clarity.

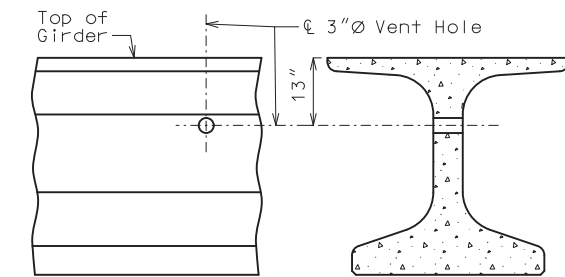


SECTION B-B
Strands not shown for clarity.



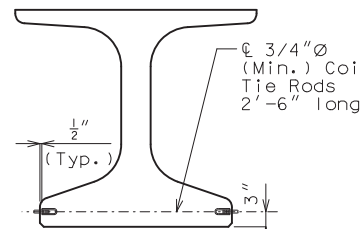
HALF ELEVATION

Reinforcement support strands not shown for clarity.

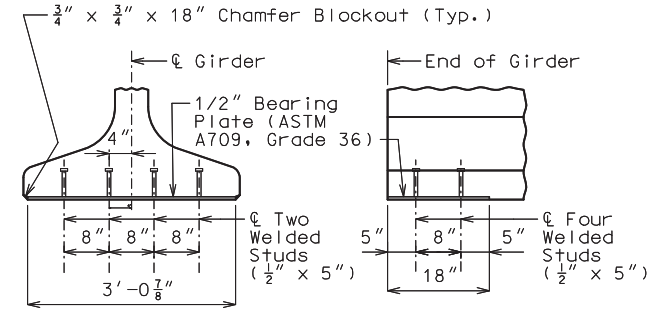


VENT HOLE

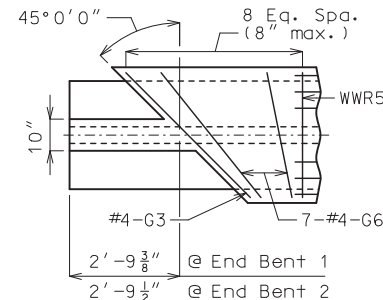
Place vent holes at or near upgrade 1/3 point of girders and clear reinforcing steel or strands by 1 1/2" minimum.



COIL TIES



BEARING PLATE

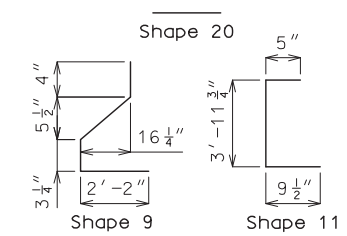


TOP FLANGE BLOCKOUT

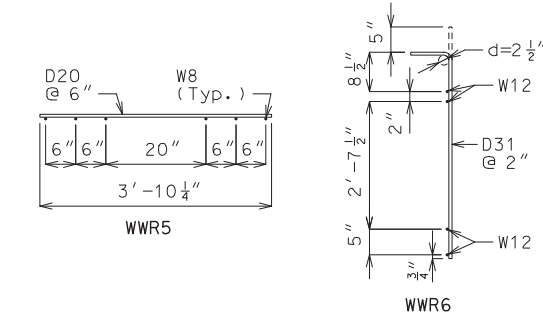
Bill of Reinforcing Steel - Each Girder

No.	Size/Mark	Length	Shape
216	5 B1	5'-0"	11
236	4 D1	4'-0"	9
2	4 G3	5'-5"	20
14	4 G6	Varies	20

Bending Diagrams



Welded Wire Reinforcement - Each Girder



All dimensions are out to out.

Hooks and bends shall be in accordance with the CRSI Manual of Standard Practice for Detailing Reinforced Concrete Structures, Stirrup and Tie Dimensions.

Actual bar lengths are measured along centerline of bar to the nearest inch.

Minimum clearance to reinforcing shall be 1".

All bar reinforcement shall be Grade 60.

The two D1 bars may be furnished as one bar at the fabricator's option.

All B1 bars shall be epoxy coated.

General Notes:

Concrete for prestressed girders shall be Class A-1 with $f'c = 8000$ psi and $f'ci = 6500$ psi.

Use 30 strands, 0.6"Ø Grade 270, with an initial prestress force of 1318 kips.

Prestensioned members shall be in accordance with Sec 1029.

Fabricator shall be responsible for location and design of lifting devices.

Exterior and interior girders are the same except; application of bond breaker, coil inserts for slab drains.

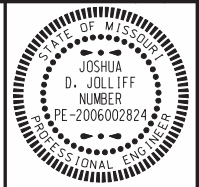
The contractor shall provide bracing necessary for lateral and torsional stability of the girders during construction of the concrete slab and remove the bracing after the slab has attained 75% design strength. Contractor shall not drill holes in the girders.

For Girder Camber Diagram, see Sheet No. 14.

For location of coil inserts at slab drains, see Sheet No. 13.

For location of coil ties at integral bent, see Sheets No. 4 and 8.

Alternate bar reinforcing steel details are provided and may be used. The same type of reinforcing steel shall be used for all girders in all spans.



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DATE PREPARED

3/3/2023

ROUTE 67 STATE MO

DISTRICT BR SHEET NO. 11

COUNTY BUTLER

JOB NO. J9P3751

CONTRACT ID.

PROJECT NO.

BRIDGE NO. A9278

DESCRIPTION

DATE

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

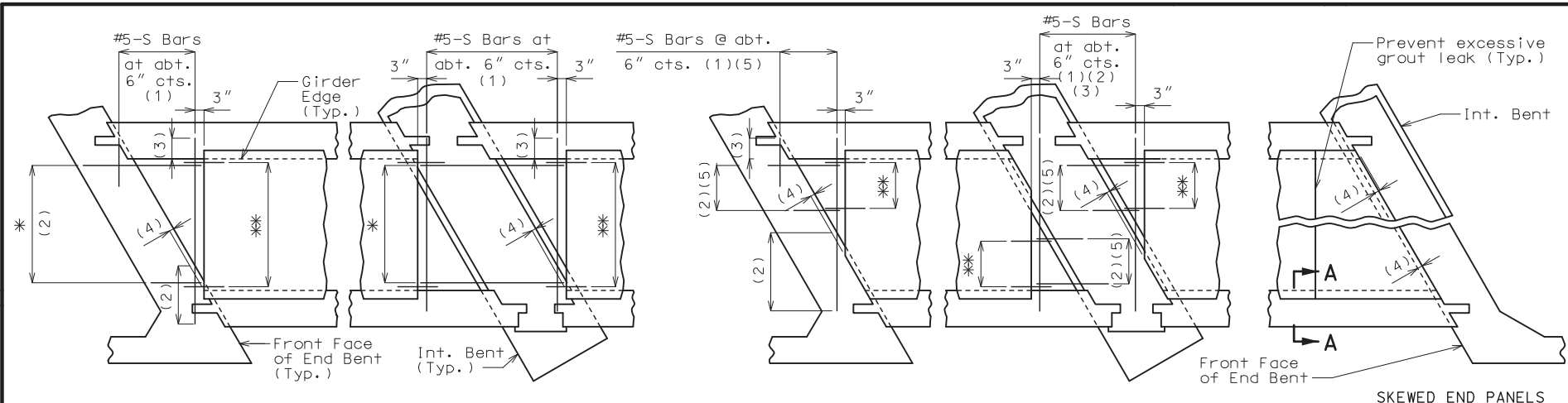


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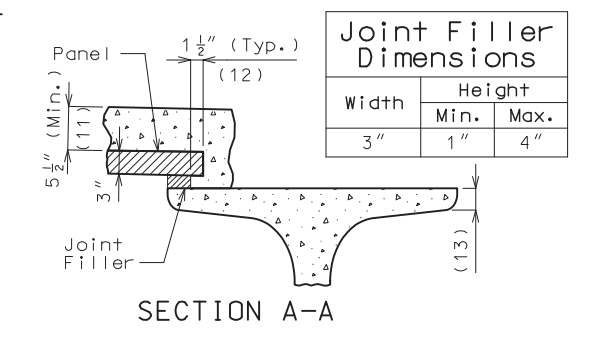
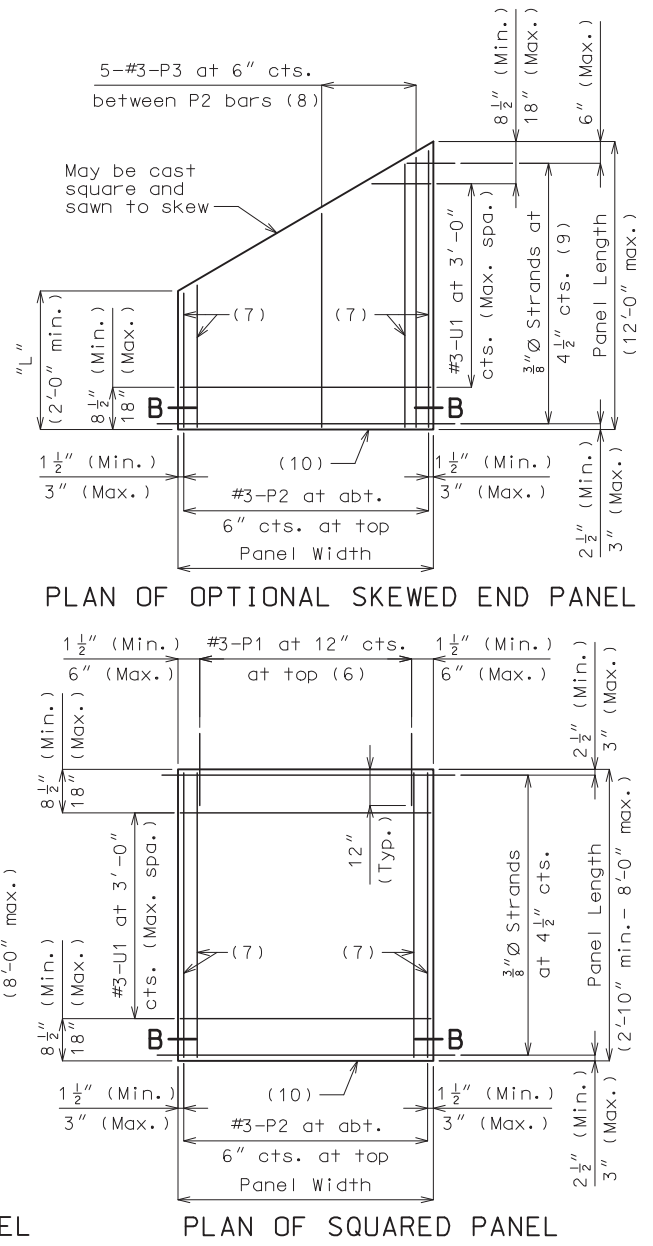
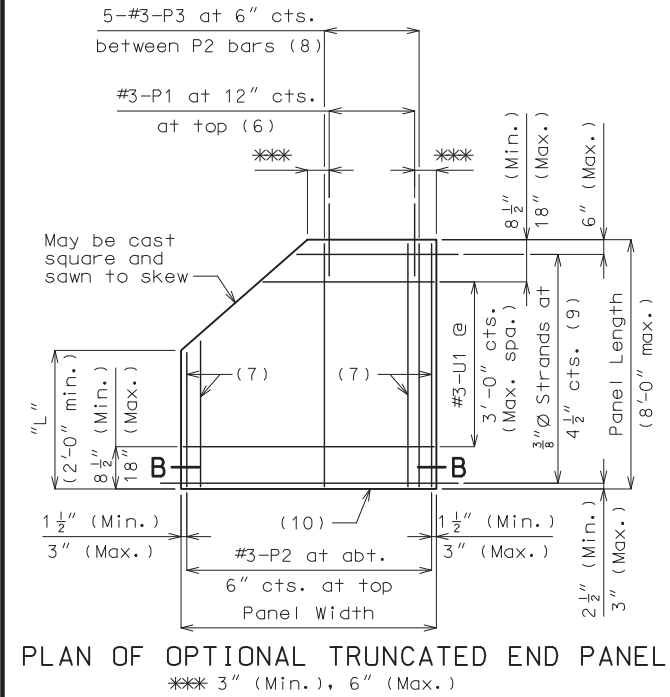
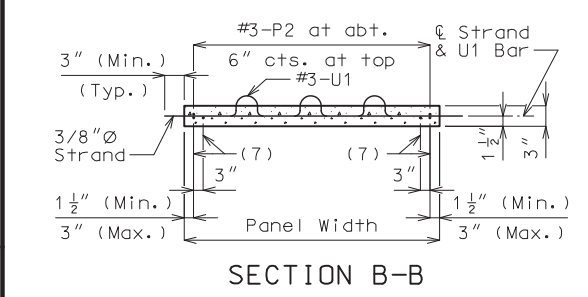
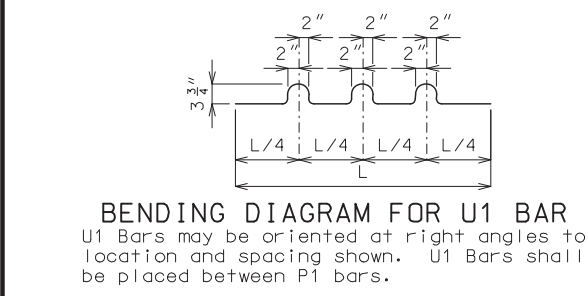


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SQUARED END PANELS OR TRUNCATED END PANELS
 PLAN SHOWING PANELS PLACEMENT



Joint Filler Dimensions		
Width	Height	
	Min.	Max.
3"	1"	4"

- Reference Notes:**
 Plan of Panels Placement:
 (1) S-bars shown are bottom steel in slab between panels and used with squared and truncated end panels only.
 (2) Extend S-bars 18 inches beyond the front face of end bents and int. bents for squared and truncated end panels only.
 (3) Extend S-bars 9 inches beyond edge of girder (Typ.).
 (4) End panels shall be dimensioned 1/2" min. to 1 1/2" max. from the inside face of diaphragm.
 (5) For truncated end panels, use a min. of #5-S bars at 6" crossings in openings, or min. 4x4-W7xW7.
 Plans of Panels:
 (6) For end panels only, P1 bars shall be 2'-0" in length and embedded 12". P1 bars will not be required for panels at squared integral end bents.
 (7) #3-P2 bars near edge of panel at bottom (under strands).
 (8) Use #3-P3 bars if panel is skewed 45° or greater.
 (9) Any strand 2'-0" or shorter shall have a #4 reinforcing bar on each side of it, centered between strands. Strands 2'-0" or shorter may then be debonded at the fabricator's option.
 (10) Optional 1/2" x 45° Chamfer one or both sides at bottom.
 Section A-A:
 (11) Slab thickness over prestressed panels varies due to girder camber. In order to maintain minimum slab thickness, it may be necessary to raise the grade uniformly throughout the structure. No payment will be made for additional labor or materials required for necessary grade adjustment.
 (12) Contractor shall ensure proper consolidation under and between panels.
 (13) At the contractor's option, the variation in slab thickness over prestressed panels may be eliminated or reduced by increasing and varying the girder top flange thickness. Dimensions shall be shown on the shop drawings.

General Notes:

Prestressed Panels:
 Concrete for prestressed panels shall be Class A-1 with $f'c = 6,000$ psi, $f'ci = 4,000$ psi.
 The top surface of all panels shall receive a scored finish with a depth of scoring of 1/8" perpendicular to the prestressing strands in the panels.
 Prestressing tendons shall be high-tensile strength, uncoated, seven-wire, low-relaxation strands for prestressed concrete in accordance with AASHTO M 203 Grade 270, with nominal diameter of strand = 3/8" and nominal area = 0.085 sq.in. and minimum ultimate strength = 22.95 kips (270 ksi). Larger strands may be used with the same spacing and initial tension.
 Initial prestressing force = 17.2 kips/strand.
 The method and sequence of releasing the strands shall be shown on the shop drawings.
 Suitable anchorage devices for lifting panels may be cast in panels, provided the devices are shown on the shop drawings and approved by the engineer. Panel lengths shall be determined by the contractor and shown on the shop drawings.
 When squared end panels are used at skewed bents, the skewed portion shall be cast full depth. No separate payment will be made for additional concrete and reinforcing required.
 Support from diaphragm forms is required under the optional skewed end until cast-in-place concrete has reached 3,000 psi compressive strength.
 Prestressed panels shall be brought to saturated surface-dry (SSD) condition just prior to the deck pour. There shall be no free standing water on the panels or in the area to be cast.
 The prestressed panel quantities are not included in the table of estimated quantities for the slab.

Reinforcing Steel:
 All dimensions are out to out.
 Hooks and bends shall be in accordance with the CRSI Manual of Standard Practice for Detailing Reinforced Concrete Structures, Stirrup and Tie Dimensions.
 Minimum clearance to reinforcing steel shall be 1 1/2", unless otherwise shown.
 If U1 bars interfere with placement of slab steel, U1 loops may be bent over, as necessary, to clear slab steel.
 Deformed welded wire reinforcement (WWR) providing a minimum area of reinforcing perpendicular to strands of 0.22 sq in./ft, with spacing parallel to strands sufficient to ensure proper handling, may be used in lieu of the #3-P2 bars shown. Wire diameter shall not be larger than 0.375 inch. The above alternative reinforcement criteria may be used in lieu of the #3-P3 bars, when required, and placed over a width not less than 2 feet.
 The following reinforcing steel shall be tied securely to the strands with the following maximum spacing in each direction:
 #3-P2 bars at 16 inches.
 WWR at 24 inches.
 The #3-U1 bars shall be tied securely to #3-P2 bars, to WWR or to strands (when placed between P1 bars) at about 3-foot centers.
 Minimum reinforcement steel length shall be 2'-0".
 All reinforcement other than prestressing strands shall be epoxy coated.
 Precast panels may be in contact with stirrup reinforcing in diaphragms.
 S-bars are not listed in the bill of reinforcing.
 Cost of S-bars will be considered completely covered by the contract unit price for the slab.

Joint Filler:
 Joint filler shall be preformed fiber expansion joint material in accordance with Sec 1057 or expanded or extruded polystyrene bedding material in accordance with Sec 1073.
 Use Slab Haunching Diagram on Sheet No. 14 for determining thickness of joint filler within the limits noted in the table of Joint Filler Dimensions.
 Thicker material may be used on one or both sides of the girder to reduce cast-in-place concrete thickness to within tolerances.
 The same thickness of preformed fiber expansion joint material shall be used under any one edge of any panel except at locations where top flange thickness may be stepped. The maximum change in thickness between adjacent panels shall be 1/4 inch. The polystyrene bedding material may be cut with a transition to match haunch height above top of flange.
 Joint filler shall be glued to the girder. When thickness exceeds 1 1/2 inches, the joint filler shall be glued top and bottom. The glue used shall be the type recommended by the joint filler manufacturer.
 Edges of panels shall be uniformly seated on the joint filler before slab reinforcement is placed.

DETAILS OF PRESTRESSED PANELS

Detailed Oct. 2022
 Checked Oct. 2022

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 12 of 24

THIS SHEET HAS BEEN SIGNED, SEALED AND DATED ELECTRONICALLY.
 DATE PREPARED: 3/3/2023

ROUTE	67	STATE	MO
DISTRICT	BR	SHEET NO.	12

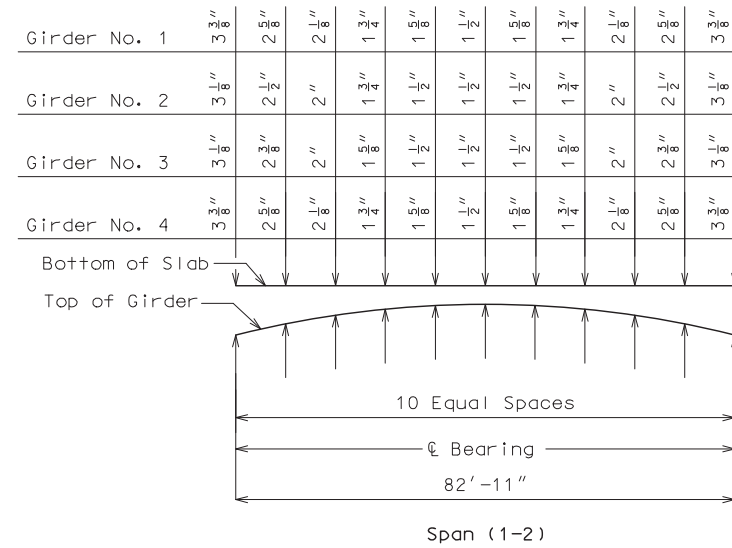
COUNTY: BUTLER
 JOB NO.: J9P3751
 CONTRACT ID.:
 PROJECT NO.:
 BRIDGE NO.: A9278

DESCRIPTION	DATE

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

105 WEST CAPITOL
 JEFFERSON CITY, MO 65102
 1-888-ASK-MODOT (1-888-275-6636)

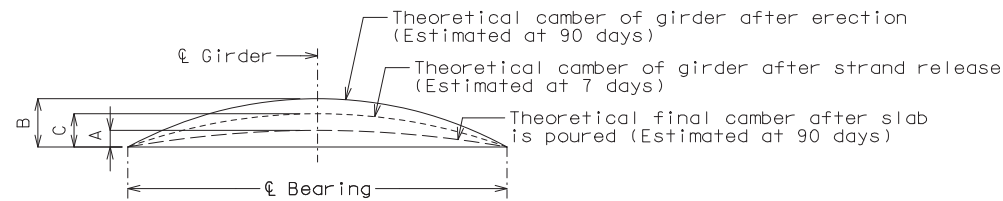
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 6631 WESLEYAN DRIVE, SUITE 300
 SPRINGFIELD, MO 65807 (417) 869-6009
 ENGINEERING CORPORATION - 00631



THEORETICAL SLAB HAUNCHING DIAGRAM

If girder camber is different from that shown in the camber diagram, in order to maintain minimum slab thickness, an adjustment of the slab haunches, an increase in slab thickness or a raise in grade uniformly throughout the structure shall be necessary. No payment will be made for additional labor or materials required for variation in haunching, slab thickness or grade adjustment.

Concrete in the slab haunches is included in the Estimated Quantities for Slab on Concrete NU-Girder.



Girder	Span (1-2)		
	A	B	C
Exterior	1 7/8"	3 1/4"	1 7/8"
Interior	1 5/8"		

GIRDER CAMBER DIAGRAM

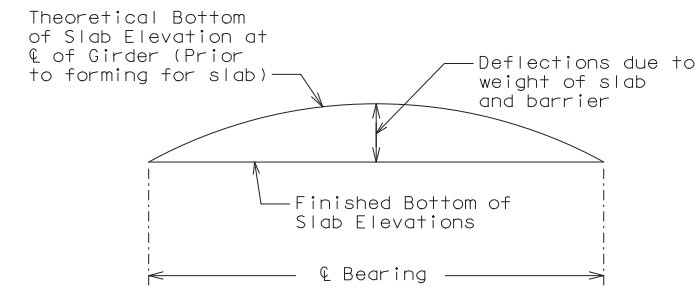
Conversion factors for girder camber (Estimated at 90 days):

- 0.1 pt. = 0.314 x 0.5 pt.
- 0.2 pt. = 0.593 x 0.5 pt.
- 0.3 pt. = 0.813 x 0.5 pt.
- 0.4 pt. = 0.952 x 0.5 pt.

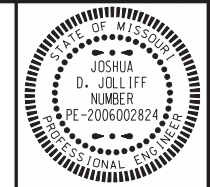
Theoretical Bottom of Slab Elevations at Centerline of Girder (Prior to forming for slab) (Estimated at 90 days)

Girder Number	Span (1-2) (82'-11" ℄ Brg. - ℄ Brg.)										
	℄ Brg.	.10	.20	.30	.40	.50	.60	.70	.80	.90	℄ Brg.
1	308.33	308.35	308.36	308.36	308.35	308.33	308.31	308.27	308.22	308.16	308.10
2	308.52	308.54	308.55	308.56	308.56	308.54	308.51	308.47	308.42	308.35	308.29
3	308.61	308.63	308.64	308.65	308.65	308.63	308.60	308.56	308.50	308.44	308.38
4	308.36	308.37	308.38	308.39	308.38	308.36	308.33	308.29	308.24	308.19	308.13

Elevations are based on a constant slab thickness of 8 1/2" and include allowance for theoretical dead load deflections due to weight of slab (including precast panel) and barrier.



TYPICAL SLAB ELEVATIONS DIAGRAM



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DATE PREPARED

3/3/2023

ROUTE 67 STATE MO

DISTRICT BR SHEET NO. 14

COUNTY BUTLER

JOB NO. J9P3751

CONTRACT ID.

PROJECT NO.

BRIDGE NO. A9278

DESCRIPTION

DATE

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

105 WEST CAPITOL JEFFERSON CITY, MO 65102

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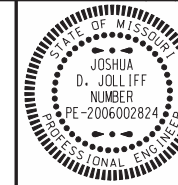
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REV.

TBOS, HAUNCHING AND CAMBER



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DATE PREPARED
3/3/2023

ROUTE 67 STATE MO

DISTRICT BR SHEET NO. 16

COUNTY BUTLER

JOB NO. J9P3751

CONTRACT ID.

PROJECT NO.

BRIDGE NO. A9278

DESCRIPTION

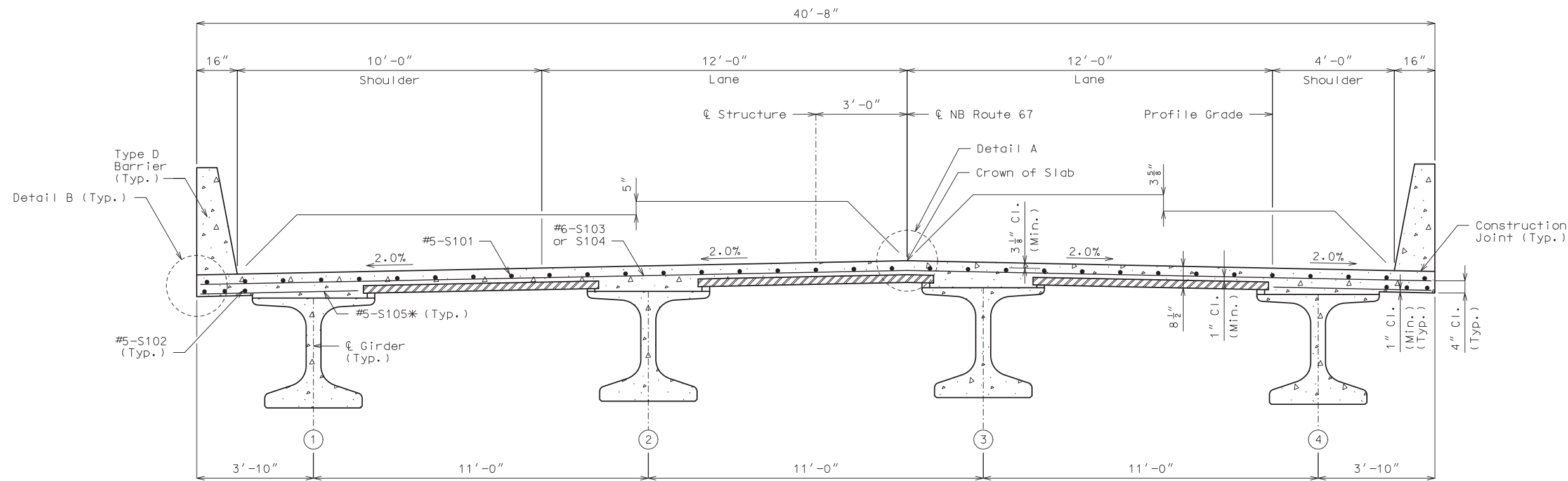
DATE

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION



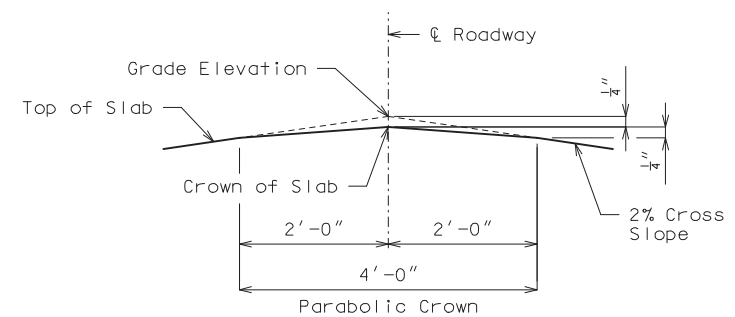
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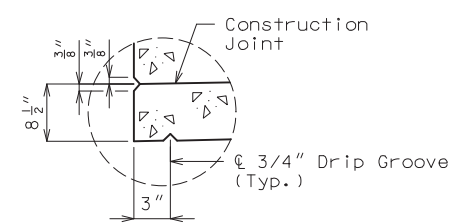


SECTION THRU SLAB

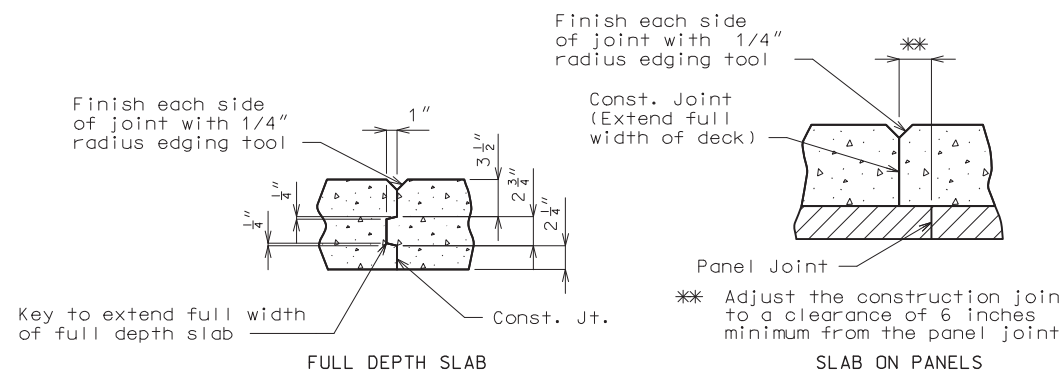
* Alternate bar shape available, see barrier sheets.



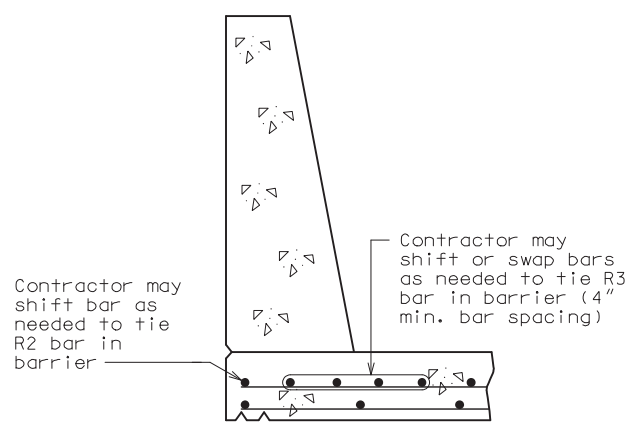
DETAIL A



DETAIL B



SLAB CONSTRUCTION JOINT
(If needed)



OPTIONAL SHIFTING TOP BARS AT BARRIER

DETAILS OF SLAB REINFORCING

General Notes:

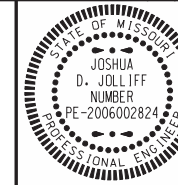
- For details and reinforcement of Type D Barrier not shown, see Sheets No. 17 & 18.
- For details of Precast Prestressed Panels, see Sheet No. 12.
- For Theoretical Bottom of Slab Elevations, Girder Camber Diagram and Theoretical Slab Haunching Diagram, see Sheet No. 14.
- For Plan of Slab Showing Reinforcement, see Sheet No. 15.
- The contractor shall pour and satisfactorily finish the roadway slab at a rate of not less than 25 cubic yards per hour.
- The contractor shall furnish an approved retarder to retard the set of the concrete to 2.5 hours and shall pour and satisfactorily finish the slab pours at the rate given.
- The concrete diaphragm at the integral end bents shall be poured a minimum of 30 minutes and a maximum of 2 hours before the slab is poured.

Detailed Oct. 2022
Checked Oct. 2022

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 16 of 24

REV.



THIS SHEET HAS BEEN SIGNED, SEALED AND DATED ELECTRONICALLY.

DATE PREPARED
3/3/2023

ROUTE 67 STATE MO

DISTRICT BR SHEET NO. 17

COUNTY BUTLER

JOB NO. J9P3751

CONTRACT ID.

PROJECT NO.

BRIDGE NO. A9278

DESCRIPTION

DATE

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

105 WEST CAPITOL JEFFERSON CITY, MO 65102

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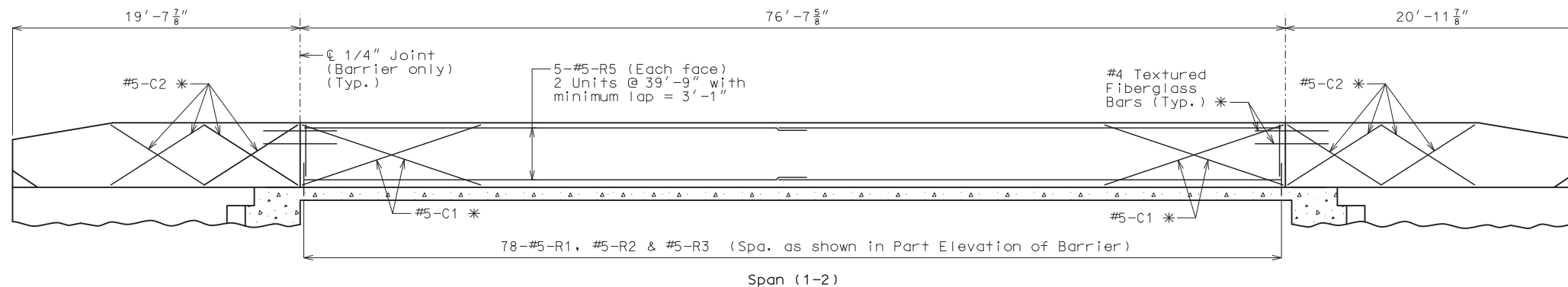
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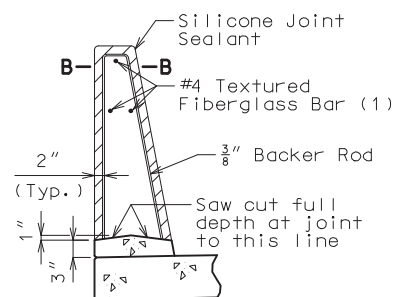
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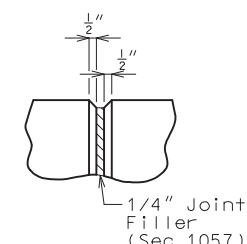


ELEVATION OF BARRIER

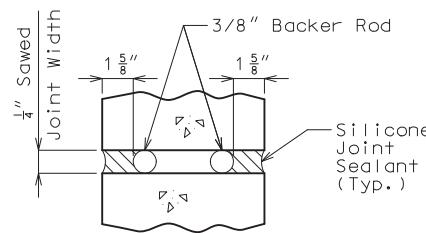
(Left barrier shown, right barrier similar by 180° rotation)
Longitudinal dimensions are horizontal.



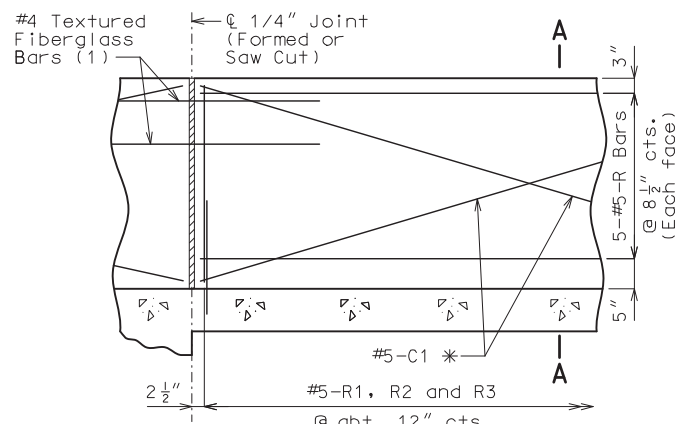
SECTION THRU SAW CUT JOINT



PART ELEVATION AT FORMED JOINT

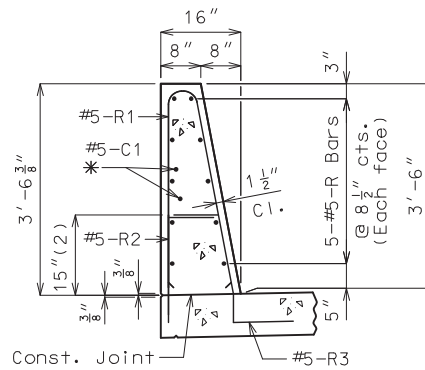


SECTION B-B



PART ELEVATION OF BARRIER

(1) Four feet long, centered on joint, slip-formed option only

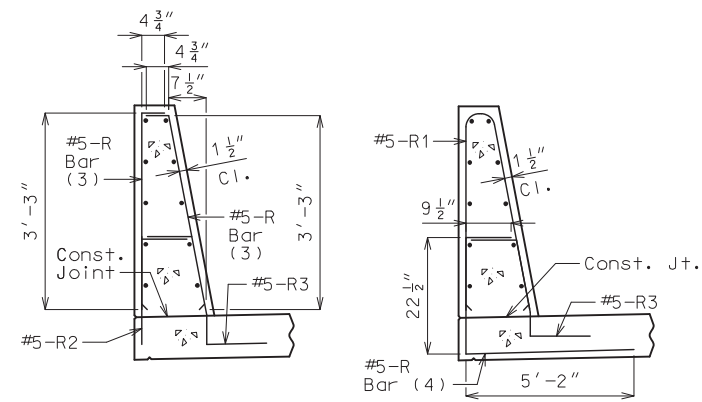


SECTION A-A

Use a minimum lap of 3'-1" for #5 horizontal barrier bars.

The cross-sectional area above the slab is 3.52 square feet.

(2) To top of bar



R-BAR PERMISSIBLE ALTERNATE SHAPE

(3) The R1 bar may be separated into two bars as shown, at the contractor's option, only when slip forming is not used. (All dimensions are out to out.)

(4) The R2 bar and #5 bottom transverse slab bar in cantilever (prestressed panels only) combination may be furnished as one bar as shown, at the contractor's option.

TYPE D BARRIER

General Notes:

* Slip-formed option only.

Conventional forming or slip forming may be used. Saw cut joints may be used with conventional forming.

Top of barrier shall be built parallel to grade and barrier joints (except at end bents) normal to grade.

All exposed edges of barrier shall have either a 1/2-inch radius or a 3/8-inch bevel, unless otherwise noted.

Payment for all concrete and reinforcement, complete in place, will be considered completely covered by the contract unit price for Type D Barrier per linear foot.

Concrete in barrier shall be Class B-1.

Measurement of barrier is to the nearest linear foot for each structure, measured along the outside top of slab from end of wing to end of wing.

Concrete traffic barrier delineators shall be placed on top of the barrier as shown on Missouri Standard Plan 617.10 and in accordance with Sec 617. Delineators on bridges with two-lane, two-way traffic shall have retroreflective sheeting on both sides. Concrete traffic barrier delineators will be considered completely covered by the contract unit price for Type D Barrier.

Joint sealant and backer rods shall be in accordance with Sec 717 for silicone joint sealant for saw cut and formed joints.

For slip-formed option, both sides of barrier shall have a vertically broomed finish and the top shall have a transversely broomed finish.

General Notes:

All concrete for the bridge approach slab and sleeper slab shall be in accordance with Sec 503 ($f'c = 4,000$ psi).
The reinforcing steel in the bridge approach slab and the sleeper slab shall be epoxy coated Grade 60 with $f_y = 60,000$ psi.

Drain pipe may be either 6" diameter corrugated metallic-coated pipe underdrain, 4" diameter corrugated polyvinyl chloride (PVC) drain pipe, or 4" diameter corrugated polyethylene (PE) drain pipe.

Minimum clearance to reinforcing steel shall be 1 1/2", unless otherwise shown.

The reinforcing steel in the bridge approach slab and the sleeper slab shall be continuous. The transverse reinforcing steel may be made continuous by providing a minimum lap splice of 29 inches for #5 bars and 44 inches for #6 bars, or by mechanical bar splice.

Mechanical bar splices shall be in accordance with Sec 710.

All joint filler shall be in accordance with Sec 1057 for preformed fiber expansion joint filler except as noted.

The contractor shall pour and satisfactorily finish the bridge slab before placing the bridge approach slab.

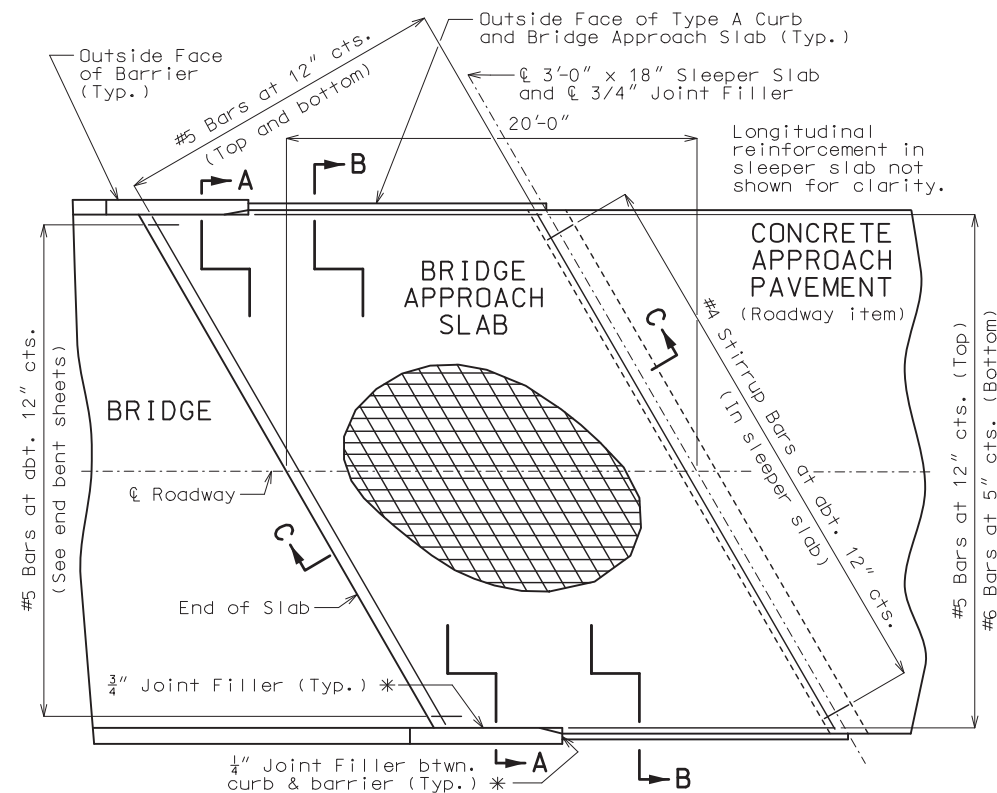
Longitudinal construction joints in approach slab and sleeper slab shall be aligned with longitudinal construction joints in bridge slab.

For concrete approach pavement details, see roadway plans.

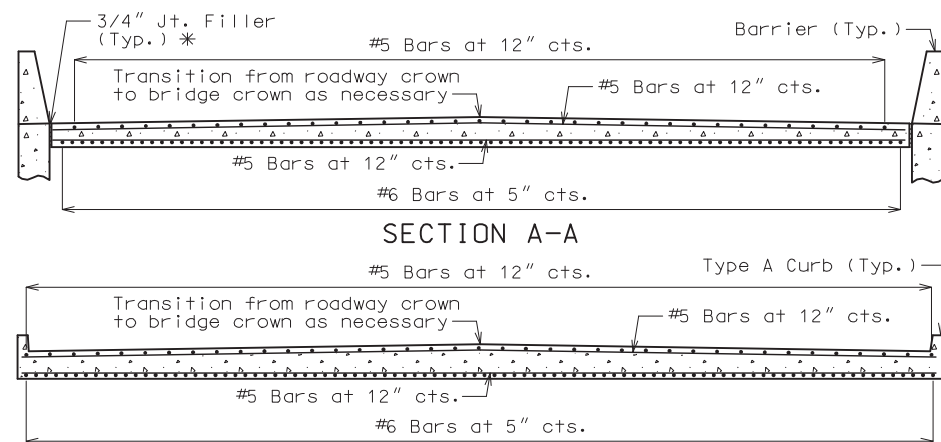
See Missouri Standard Plan 609.00 for details of Type A curb.

Payment for furnishing all materials, labor and excavation necessary to construct the approach slab, including the timber header, sleeper slab, underdrain, Type 5 aggregate base, joint filler and all other appurtenances and incidental work as shown on this sheet, complete in place, will be considered completely covered by the contract unit price for Bridge Approach Slab (Major) per square yard.

* Seal joint between vertical face of approach slab and wing with sealant in accordance with Sec 717 for silicone joint sealant for saw cut and formed joints.

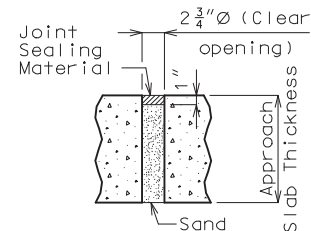


PART PLAN SHOWING REINFORCEMENT

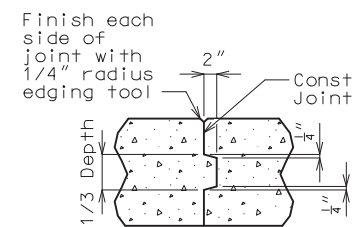


SECTION B-B

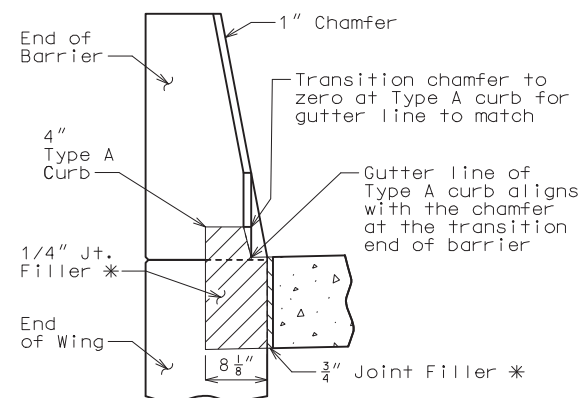
With the approval of the engineer, the contractor may crown the bottom of the approach slab to match the crown of the roadway surface.



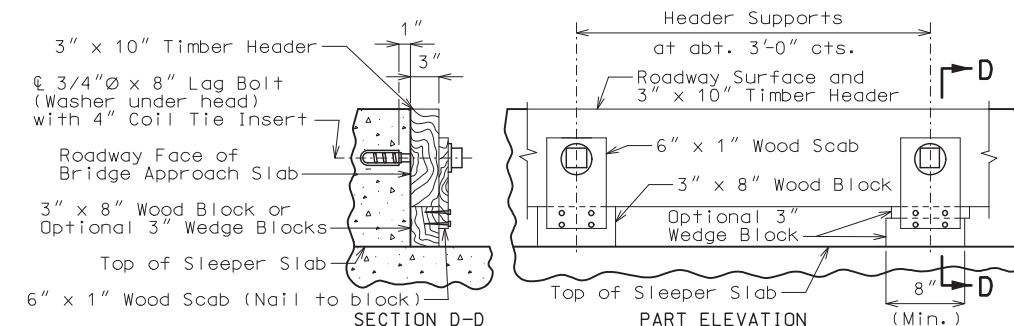
UNDERSEAL ACCESS HOLE DETAIL
(If required)



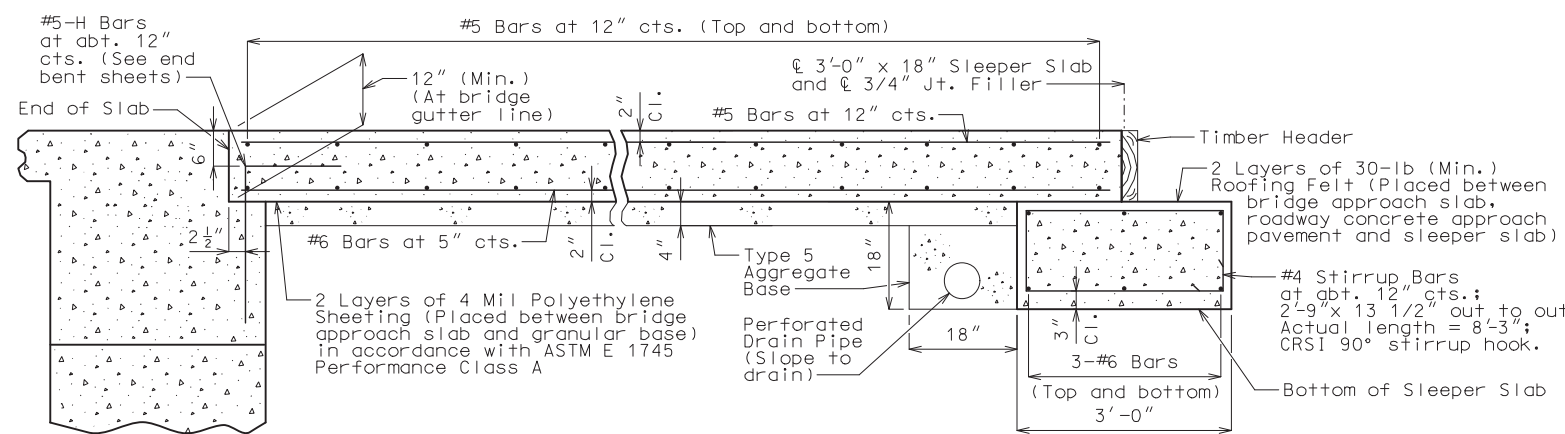
CONSTRUCTION JOINT DETAIL
(If required)



SECTION BETWEEN CURB AND BARRIER

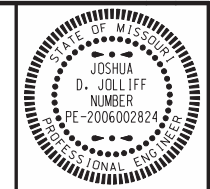


Remove timber header when concrete pavement is placed.



SECTION C-C

BRIDGE APPROACH SLAB (MAJOR)



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DATE PREPARED
3/3/2023

ROUTE 67 STATE MO

DISTRICT BR SHEET NO. 19

COUNTY BUTLER

JOB NO. J9P3751

CONTRACT ID.

PROJECT NO.

BRIDGE NO. A9278

DESCRIPTION

DATE

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

105 WEST CAPITOL JEFFERSON CITY, MO 65102

1-888-ASK-MODOT (1-888-275-6636)

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REV.

BILL OF REINFORCING STEEL

NO. REQ'D.	MARK NO.	LOCATION	EPOXY (E)	SHAPE NO.	STIRRUP (S)	SUBSTR. (X)	VARIES (V)	DIMENSIONS								NOMINAL LENGTH	ACTUAL LENGTH	WEIGHT					
								DIMENSIONS															
								B	C	D	E	F	H	K	NOMINAL LENGTH				ACTUAL LENGTH	WEIGHT			
FT.	IN.	FT.	IN.	FT.	IN.	FT.	IN.	FT.	IN.	FT.	IN.	FT.	IN.	FT.	IN.	LBS.							
		END BENT 1																					
10	6	F100	WING BRACE	E 23				2	3.000	4	6.750	14.000	13.000	5.250	2	1.000	10.250	8	0	7	9	117	
10	6	F101	WING BRACE	E 15				14.000	9	6.000	2	3.000	10.250	2	1.000	13.000	5.250	12	11	12	10	193	
4	6	F102	DIAPHRAGM	E 23				5	2.250	5	1.000				3	8.000	3	8.000	10	4	10	3	62
4	6	F103	DIAPHRAGM	E 21				8	7.500	5	1.000				6	1.250	6	1.250	13	9	13	6	82
38	5	H101	DIAPHRAGM	E 19				2	0.000	15.000								3	3	3	2	126	
4	6	H102	DIAPHRAGM	E 20				57	1.000									57	1	57	1	343	
4	6	H103	DIAPHRAGM	E 20				57	1.000									57	1	57	1	610	
4	5	H104	DIAPHRAGM	E 23				15.250	3	3.000	15.250	10.750	10.750	10.750	10.750	5	10	5	9	24			
9	6	H105	DIAPHRAGM	E 20				14	0.000									14	0	14	0	190	
3	6	H106	DIAPHRAGM	E 20				10	8.000									10	8	10	8	49	
6	6	H107	DIAPHRAGM	E 20				4	5.000									4	5	4	5	40	
2	6	H108	DIAPHRAGM	E 20				2	9.000									2	9	2	9	9	
8	8	H109	BEAM	E 20				57	1.000									57	1	57	1	1220	
4	6	H110	BEAM	E 20				57	1.000									57	1	57	1	343	
4	6	H111	BEAM	E 20				9	3.000									9	3	9	3	56	
16	8	H151	WING WALL	E 20				16	6.000									16	6	16	6	705	
20	6	H152	WING WALL	E 6 S				15	8.000	12.000								16	8	16	7	499	
20	6	H153	WING WALL	E 6 S				15	8.000	12.000								16	8	16	7	499	
71	6	U101	DIAPHRAGM	E 19 S				3	4.000	7	0.000							10	4	10	3	1094	
38	5	U102	DIAPHRAGM	E 10 S				4	1.500	4	7.000							12	10	12	8	503	
38	6	U103	DIAPHRAGM	E 19 S				3	0.000	5	3.000							8	3	8	2	467	
28	5	U104	BEAM	E 10 S				6	3.250	5	3.000							17	10	17	7	514	
24	4	U105	BEAM	E 13 S				5	3.000	2	8.000	2	8.000					16	7	16	4	262	
6	4	U106	BEAM	E 10 S				2	8.000	5	3.000							10	7	10	5	42	
6	4	U107	BEAM	E 13 S				5	3.000	2	11.000	5	3.000	2	11.000			17	1	16	10	68	
4	4	U108	BEAM	E 10 S				2	11.000	5	3.000							11	1	10	11	30	
28	6	V101	DIAPHRAGM	E 20				3	0.000									3	0	3	0	127	
20	5	V102	BEAM	E 20				6	3.000									6	3	6	3	131	
30	6	V251	WING WALL	E 20				7	1.000									7	1	7	1	320	
30	6	V252	WING WALL	E 20				7	1.000									7	1	7	1	320	
		SLAB																					
66	5	S101	SLAB	E 20				45	4.000									45	4	45	4	3121	
18	5	S102	SLAB	E 20				31	3.000									31	3	31	3	587	
95	6	S103	SLAB	E 20				40	5.000									40	5	40	5	5768	
152	6	S104	SLAB	E 20				39	11.500									40	0	40	0		
166	5	S105	SLAB	E 20				5	2.000									5	2	5	2	895	
		INCR. = 0.5'						2	5.500									2	6	2	6	4852	
		TYPE D BARRIER																					
156	5	R1	BARRIER	E 26				3	3.000	5.500	3	0.750						3	3	0.750		1085	
156	5	R2	BARRIER	E 19 S				20.500	9.500									2	6	2	5	394	
156	5	R3	BARRIER	E 27 S				9.500	15.250	5.000	12.000	15.000	3.000	3	6	3	4	543					
40	5	R5	BARRIER	E 20				39	9.000									39	9	39	9	1659	
20	5	K1	BARRIER	E 27 S				3	8.000	9.250	5.250	3	2.750					5.250	1.000	8	2	8	0
94	5	K2	BARRIER	E 27 S				3	8.000	9.250	14.500	2	5.750					14.250	2.750	8	2	8	0
8	5	K3	BARRIER	E 27 S				22.500	9.250	14.750	7.750	12.000	14.500	2.750	5	7	5	3	44				
20	5	K4	BARRIER	E 19 S				4	2.250	10.000								3	3	3	1		
		INCR. = 0.5"						2	6.250	10.000								3	5	3	3	67	
20	5	K5	BARRIER	E 14 S				8.250	9.500	18.500								4.000	18.000	3	12	11	
		INCR. = 0.5"						8.250	9.500	20.500								4.500	20.000	3	3	1	
12	5	K6	BARRIER	E 19 S				4	2	6.750	10.000							3	5	3	4		
		INCR. = 0.5"						2	7.750	10.000								3	6	3	5	43	
12	5	K7	BARRIER	E 21 S				4	2	6.750	10.000							2	6.000	6.250	3	5	4
		INCR. = 0.5"						2	7.750	10.000								2	7.000	6.500	3	6	5
36	5	K8	BARRIER	E 19 S				4	2	8.500	10.000							3	7	3	6		
		INCR. = 0.75"						3	2.500	10.000								4	1	4	0	141	
36	5	K9	BARRIER	E 21 S				4	2	8.500	10.000							2	7.750	6.750	3	7	6
		INCR. = 0.75"						3	2.500	10.000								3	1.750	7.750	4	1	0
54	5	K10	BARRIER	E 19 S				3	3.000	10.000								4	1	4	0	226	
54	5	K11	BARRIER	E 21 S				3	3.000	10.000								3	2.250	7.750	4	1	0
24	5	K12	BARRIER	E 20				19	4.000									19	4	19	4	484	
12	5	K13	BARRIER	E 20				18	7.000									18	7	18	7		
		INCR. = 3'						12	7.000									12	7	12	7	196	
24	5	K14	BARRIER	E 20				20	8.000									20	8	20	8	518	
12	5	K15	BARRIER	E 20				19	11.000									19	11	19	11		
		INCR. = 3'						13	11.000									13	11	13	11	212	

BILL OF REINFORCING STEEL

NO. REQ'D.	MARK NO.	LOCATION	EPOXY (E)	SHAPE NO.	STIRRUP (S)	SUBSTR. (X)	VARIES (V)	DIMENSIONS								NOMINAL LENGTH	ACTUAL LENGTH	WEIGHT				
								DIMENSIONS														
								B	C	D	E	F	H	K	NOMINAL LENGTH				ACTUAL LENGTH	WEIGHT		
FT.	IN.	FT.	IN.	FT.	IN.	FT.	IN.	FT.	IN.	FT.	IN.	FT.	IN.	FT.	IN.	LBS.						
16	8	H251	WING WALL	E 20				16	6.000									16	6	16	6	705
20	6	H252	WING WALL	E 6 S				15	8.000	12.000								16	8	16	7	499
20	6	H253	WING WALL	E 6 S				15	8.000	12.000								16	8	16	7	499
71	6	U201	DIAPHRAGM	E 19 S				3	4.000	7	0.000							10	4	10	3	1094
38	5	U202	DIAPHRAGM	E 10 S				4	1.500	4	7.000							12	10	12	8	503
38	6	U203	DIAPHRAGM	E 19 S				3	0.000	5	3.000							8	3	8	2	467
28	5	U204	BEAM	E 10 S				6	3.000	5	3.000							17	9	17	7	514
24	4	U205	BEAM	E 13 S				5	3.000	2	8.000	2	8.000					16	7	16	4	262
6	4	U206	BEAM	E 10 S				2	8.000	5	3.000							10	7	10	5	42
6	4	U207	BEAM	E 13 S				5	3.000	2	11.000	5	3.000	2	11.000			17	1	16	10	68
4	4	U208	BEAM	E 10 S				2	11.000	5	3.000							11	1	10	11	30
28	6	V201	DIAPHRAGM	E 20				3	0.000									3	0	3	0	127
20	5	V202	BEAM	E 20				6	3.000									6	3	6	3	131
30	6	V251	WING WALL</																			



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DATE PREPARED
3/3/2023

ROUTE 67 STATE MO
DISTRICT BR SHEET NO. 22

COUNTY BUTLER
JOB NO. J9P3751
CONTRACT ID.

PROJECT NO.

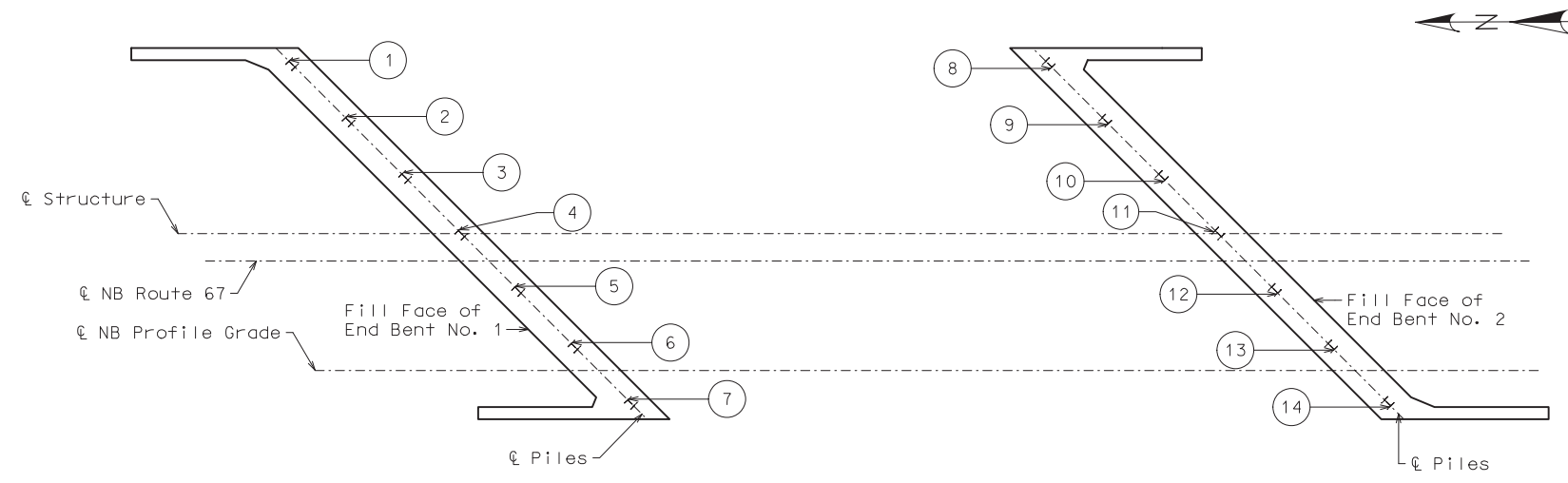
BRIDGE NO. A9278

DATE	DESCRIPTION

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

 105 WEST CAPITOL JEFFERSON CITY, MO 65102
 1-888-ASK-MODOT (1-888-278-6636)

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 ENGINEERING CORPORATION - 000631



PART PLAN SHOWING PILE NUMBERING FOR RECORDING AS-BUILT PILE DATA

As-Built Pile Data			
Pile No.	Length in Place (ft)	Computed Nominal Axial Compressive Resistance (kips)	Remarks
			End Bent No. 1
1			
2			
3			
4			
5			
6			
7			

As-Built Pile Data			
Pile No.	Length in Place (ft)	Computed Nominal Axial Compressive Resistance (kips)	Remarks
			End Bent No. 2
8			
9			
10			
11			
12			
13			
14			

Note:
 Indicate in remarks column:
 A. Pile type and grade
 B. Batter
 C. Driven to practical refusal

This sheet to be completed by MoDOT construction personnel.

AS-BUILT PILE DATA

IF A SEAL IS PRESENT ON THIS SHEET IT HAS BEEN ELECTRONICALLY SEALED AND DATED.

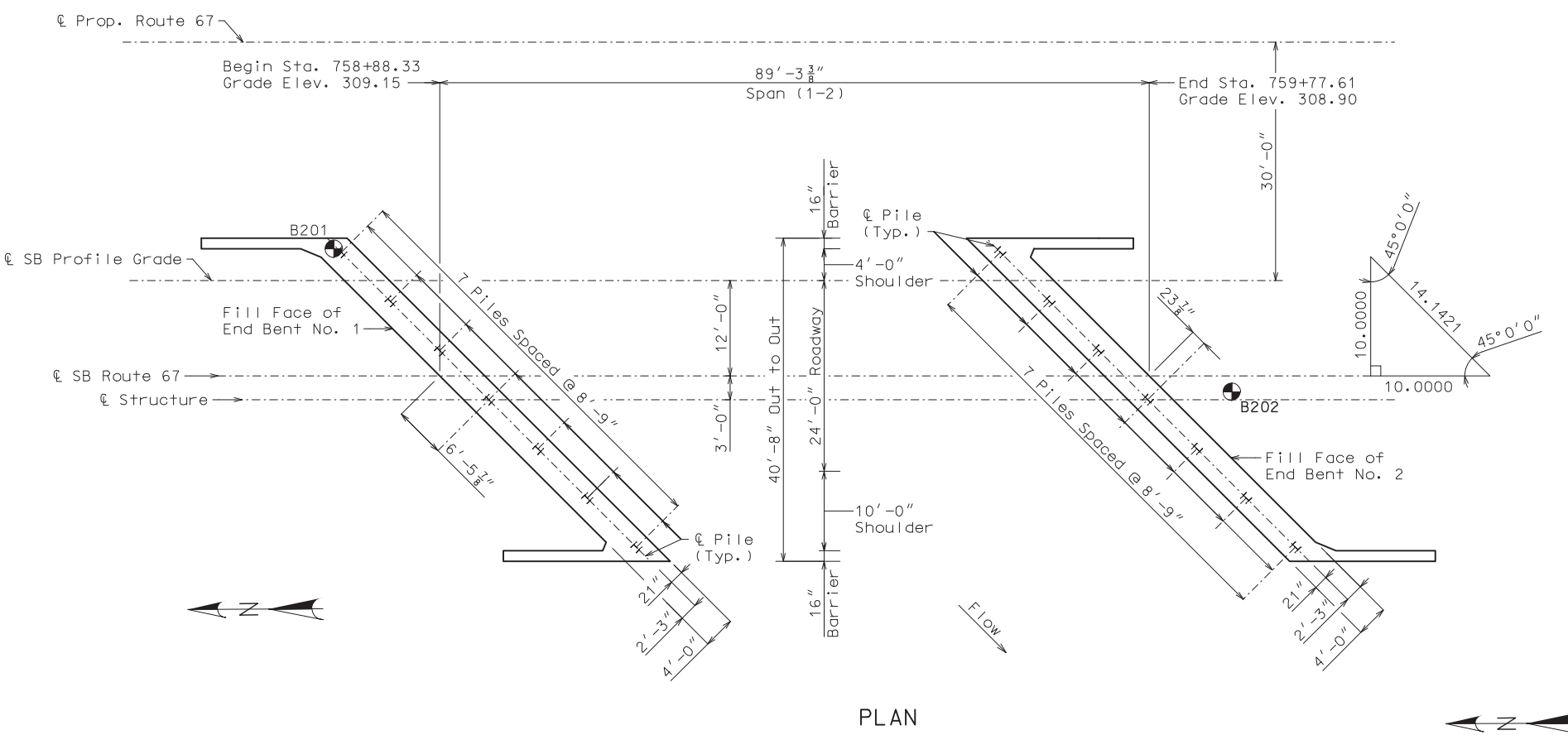
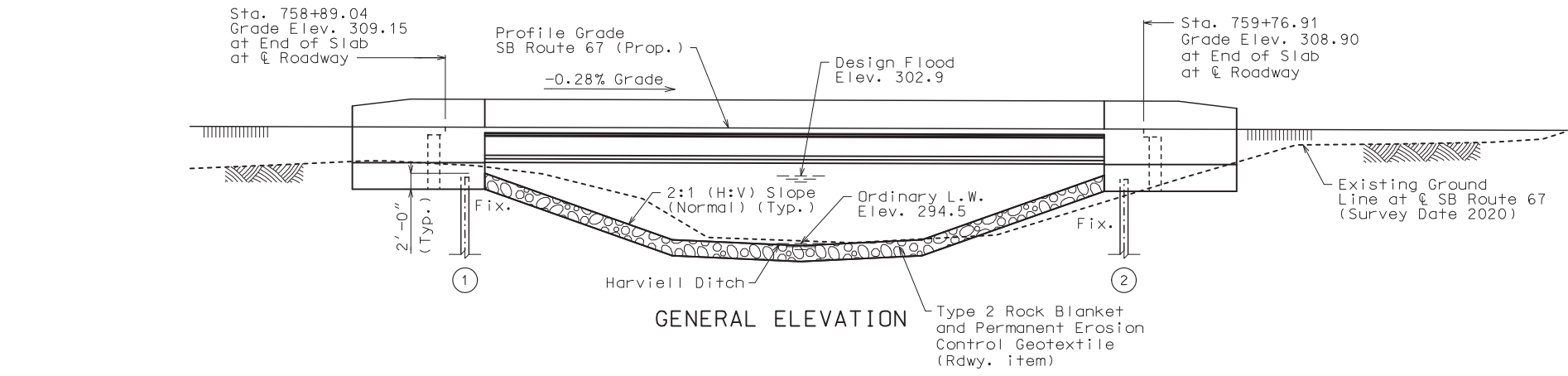
(82.9') PRESTRESSED CONCRETE NU-GIRDER SPAN

Notes:
 Roadway fill shall be completed to the final roadway section and up to the elevation of the bottom of the concrete beam within the limits of the structure and for not less than 25 feet in back of the fill face of the end bents before any piles are driven for any bents falling within the embankment section.

Hydrologic Data	
Drainage Area = 9.3 mi ²	
Design Flood Frequency = 50 years	
Design Flood Discharge = 1,060 cfs	
Design Flood (D.F.) Elevation = 302.9	
Base Flood (100-year)	
Base Flood Elevation = 303.4	
Base Flood Discharge = 1,210 cfs	
Estimated Backwater = 0.1 ft	
Average Velocity thru Opening = 2.5 ft/s	
Freeboard (50-year)	
Freeboard = 1.3 ft	
Roadway Overtopping	
Overtopping Flood Discharge = N/A	
Overtopping Flood Frequency = >500 years	
500 Year Flood Elevation = 304.6	

Indicates location of borings.
Notice and Disclaimer Regarding Boring Log Data
 The locations of all subsurface borings for this structure are shown on the plan sheet(s) for this structure. The boring data for all locations indicated, as well as any other boring logs or other factual records of subsurface data and investigations performed by the department for the design of the project, are shown on Sheet(s) No. 23 & 24 and may be included in the Electronic Bridge Deliverables. They will also be available from the Project Contact upon written request. No greater significance or weight should be given to the boring data depicted on the plan sheets than is given to the subsurface data available from the district or elsewhere.
 The Commission does not represent or warrant that any such boring data accurately depicts the conditions to be encountered in constructing this project. A contractor assumes all risks it may encounter in basing its bid prices, time or schedule of performance on the boring data depicted here or those available from the district, or on any other documentation not expressly warranted, which the contractor may obtain from the Commission.

General Notes:
 Longitudinal dimensions are measured horizontal.
 For General Notes, Estimated Quantities and Foundation Data see Sheet No. 2.



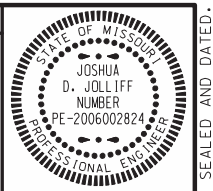
B.M. #32
 N: 282085.74
 E: 814254.75
 Sta. 719+83.75 Exist. Rte 67
 Elev. 308.27
 Chisled "□" on SW corner of west barrier curb on bridge F0665

Control Point #129
 N: 281264.97
 E: 814315.18
 Sta. 728+04.49 Exist. Rte 67
 Elev. 302.52
 5/8" rebar with MoDOT cap

GENERAL PLAN AND ELEVATION

BRIDGE: ROUTE 67 SB OVER HARVIELL DITCH

ROUTE 67 FROM ROUTE 158 TO ROUTE 142
 ABOUT 3.2 MILES NORTH OF ROUTE 142
 BEGINNING STATION 758+88.33 (CL SB ROUTE 67)



THIS SHEET HAS BEEN SIGNED, SEALED AND DATED ELECTRONICALLY.

DATE PREPARED	3/3/2023
ROUTE	67
STATE	MO
DISTRICT	BR
SHEET NO.	1
COUNTY	BUTLER
JOB NO.	J9P3751
CONTRACT ID.	
PROJECT NO.	
BRIDGE NO.	A9279

DESCRIPTION	DATE

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

105 WEST CAPITOL
 JEFFERSON CITY, MO 65102
 1-888-ASK-MODOT (1-888-275-6636)

CMT

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 1651 W. STATE STREET
 SPRINGFIELD, MO 65807 (417) 869-6009
 ENGINEERING CORPORATION - 000631

Designed Sep. 2022
 Detailed Oct. 2022
 Checked Oct. 2022

Note: This drawing is not to scale. Follow dimensions. Sheet No. 1 of 24

General Notes:

Design Specifications:

2020 AASHTO LRFD Bridge Design Specifications (9th Ed.)
 2011 AASHTO Guide Specifications for LRFD Seismic Bridge Design (2nd Ed.)
 and 2014 Interim Revisions (Seismic)
 Seismic Design Category = C
 Design earthquake response spectral acceleration coefficient at
 1.0 second period, $S_{D1} = 0.34$
 Acceleration Coefficient (effective peak ground
 acceleration coefficient), $A_s = 0.37$

Design Loading:

Vehicular = HL-93
 Future Wearing Surface = 35 lb/sf
 Earth = 120 lb/cf
 Equivalent Fluid Pressure = 45 lb/cf (Min.)
 Superstructure: Non-composite for dead load. Composite for live load.

Design Unit Stresses:

Class B Concrete (Substructure) $f'c = 3,000$ psi
 Class B-2 Concrete (Superstructure, except Prestressed
 Girders and Barrier) $f'c = 4,000$ psi
 Class B-1 Concrete (Barrier) $f'c = 4,000$ psi
 Reinforcing Steel (Grade 60) $fy = 60,000$ psi
 Structural Steel HP Pile (ASTM A709 Grade 50S) $fy = 50,000$ psi

For precast prestressed panel stresses, see Sheet No. 12.

For prestressed girder stresses, see Sheets No. 10 & 11.

Neoprene Pads:

Neoprene bearing pads shall be 60 durometer and shall be in
 accordance with Sec 716.

Joint Filler:

All joint filler shall be in accordance with Sec 1057 for preformed sponge
 rubber expansion and partition joint filler, except as noted.

Reinforcing Steel:

Minimum clearance to reinforcing steel shall be 1 1/2", unless otherwise
 shown.

Traffic Handling:

Structure to be closed during construction. Traffic to be maintained on
 other routes during construction. See roadway plans for traffic control.

Miscellaneous:

MoDOT Construction personnel will indicate the type of joint filler option
 used under the precast panels for this structure:

- Constant Joint Filler
- Variable Joint Filler

Estimated Quantities for Slab on Concrete NU-Girder		
Item		Total
Class B-2 Concrete	cu. yard	161
Reinforcing Steel (Epoxy Coated)	pond	33,550

Notes:

The table of Estimated Quantities for Slab on Concrete
 NU-Girder represents the quantities used by the State
 in preparing the cost estimate for concrete slabs. The
 area of the concrete slab will be measured to the
 nearest square yard longitudinally from end of slab to
 end of slab and transversely from out to out of bridge
 slab (or with the horizontal dimensions as shown on
 the plan of slab). Payment for prestressed panels,
 conventional forms, all concrete and epoxy coated
 reinforcing steel will be considered completely
 covered by the contract unit price for the slab.
 Variations may be encountered in the estimated
 quantities but the variations cannot be used for an
 adjustment in the contract unit price.

Method of forming the slab shall be as shown on the
 plans and in accordance with Sec 703. All hardware for
 forming the slab to be left in place as a permanent
 part of the structure shall be coated in accordance
 with ASTM A123 or ASTM B633 with a thickness class SC
 4 and a finish type I, II or III.

The Estimated Quantities for Slab on Concrete
 NU-Girder are based on skewed precast prestressed end
 panels.

Class B-2 Concrete quantity is based on minimum top
 flange thickness and minimum joint material thickness.

The prestressed panel quantities are not included in
 the table of Estimated Quantities for Slab on Concrete
 NU-Girder.

Estimated Quantities				
Item		Substr.	Superstr.	Total
Class 1 Excavation	cu. yard	95		95
Bridge Approach Slab (Major)	sq. yard		171	171
Galvanized Structural Steel Piles (12 in.)	linear foot	1120		1120
Pile Point Reinforcement	each	14		14
Class B Concrete (Substructure)	cu. yard	63.8		63.8
Type D Barrier	linear foot		235	235
Slab on Concrete NU-Girder	sq. yard		398	398
NU 43, Prestressed Concrete NU-Girder	linear foot		335	335
Slab Drain	each		14	14
Vertical Drain at End Bents	each	2		2
Plain Neoprene Bearing Pad	each		8	8

All concrete above the construction joint in the end bents is included in the
 Estimated Quantities for Slab on Concrete NU-Girder.

All reinforcement in the end bents is included in the Estimated Quantities for Slab
 on Concrete NU-Girder.

Cost of L4x4 ASTM A709 Grade 36 HP pile anchors and 3/4-inch diameter ASTM F3125
 Grade A325 Type 1 bolts, complete in place, will be considered completely covered by
 the contract unit price for Galvanized Structural Steel Piles (12 in.).

Foundation Data			
Type	Design Data	Bent Number	
		1	2
Load Bearing Pile	Pile Type and Size	HP 12x53	HP 12x53
	Number	7	7
	Approximate Length Per Each	ft 79	81
	Pile Point Reinforcement	ea All	All
	Min. Galvanized Penetration (Elev.)	ft 281	280
	Pile Driving Verification Method	DF	DF
	Resistance Factor	0.4	0.4
	Minimum Nominal Axial Compressive Resistance	kip 523	523

DF = FHWA-modified Gates Dynamic Pile Formula

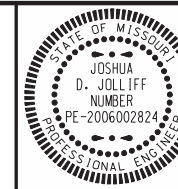
Load Bearing Pile:

$$\text{Minimum Nominal Axial Compressive Resistance} = \frac{\text{Maximum Factored Loads}}{\text{Resistance Factor}}$$

All piles shall be galvanized down to the minimum galvanized penetration
 (elevation).

Pile point reinforcement need not be galvanized. Shop drawings will not
 be required for pile point reinforcement.

The contractor shall make every effort to achieve the minimum galvanized
 penetration (elevation) shown on the plans for all piles. Deviations in
 penetration less than 5 feet of the minimum will be considered acceptable
 provided the contractor makes the necessary corrections to ensure the
 minimum penetration is achieved on subsequent piles.



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DATE PREPARED
 3/3/2023

ROUTE 67 STATE MO

DISTRICT BR SHEET NO. 2

COUNTY BUTLER

JOB NO. J9P3751

CONTRACT ID.

PROJECT NO.

BRIDGE NO. A9279

DESCRIPTION

DATE

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

105 WEST CAPITOL JEFFERSON CITY, MO 65102

1-888-ASK-MODOT (1-888-275-6636)

MoDOT

COMMISSION

105 WEST CAPITOL JEFFERSON CITY, MO 65102

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ESTIMATED QUANTITIES AND GENERAL NOTES

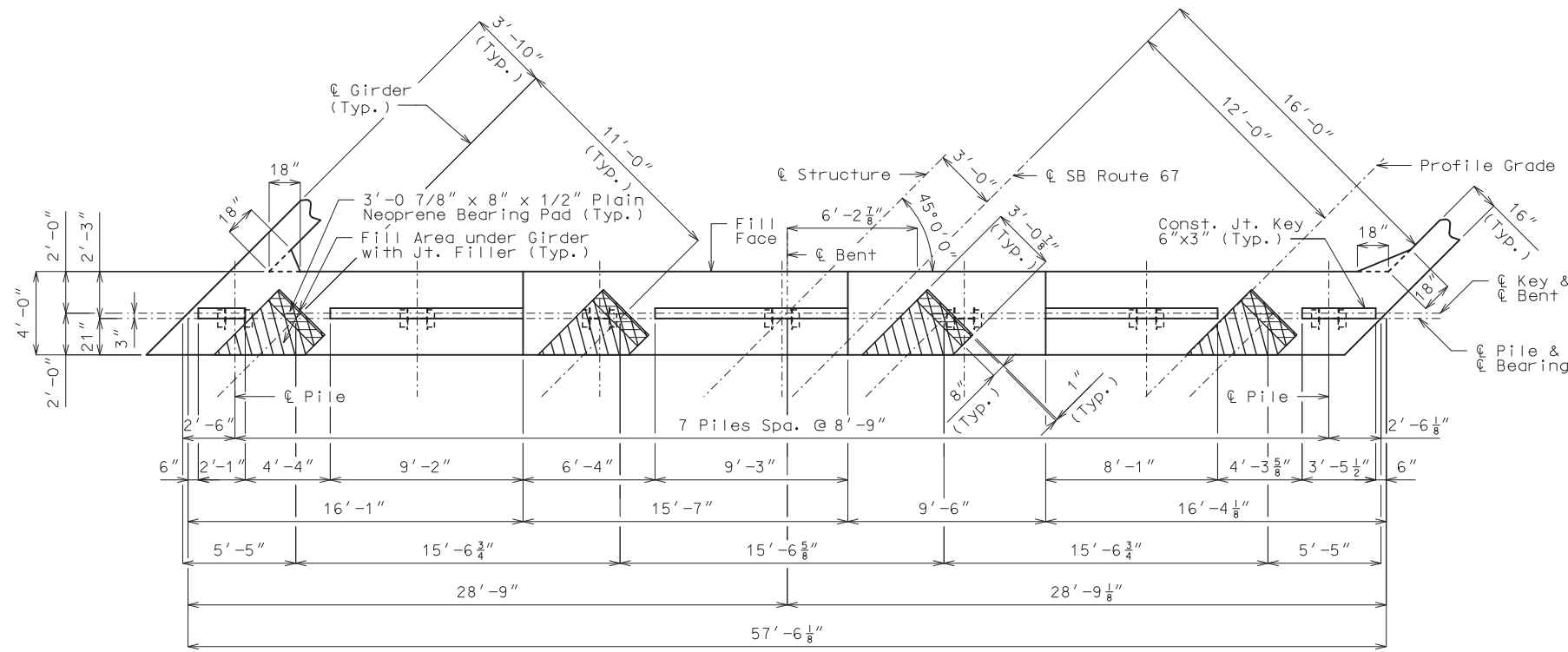
Detailed Oct. 2022
 Checked Oct. 2022

Note: This drawing is not to scale. Follow dimensions.

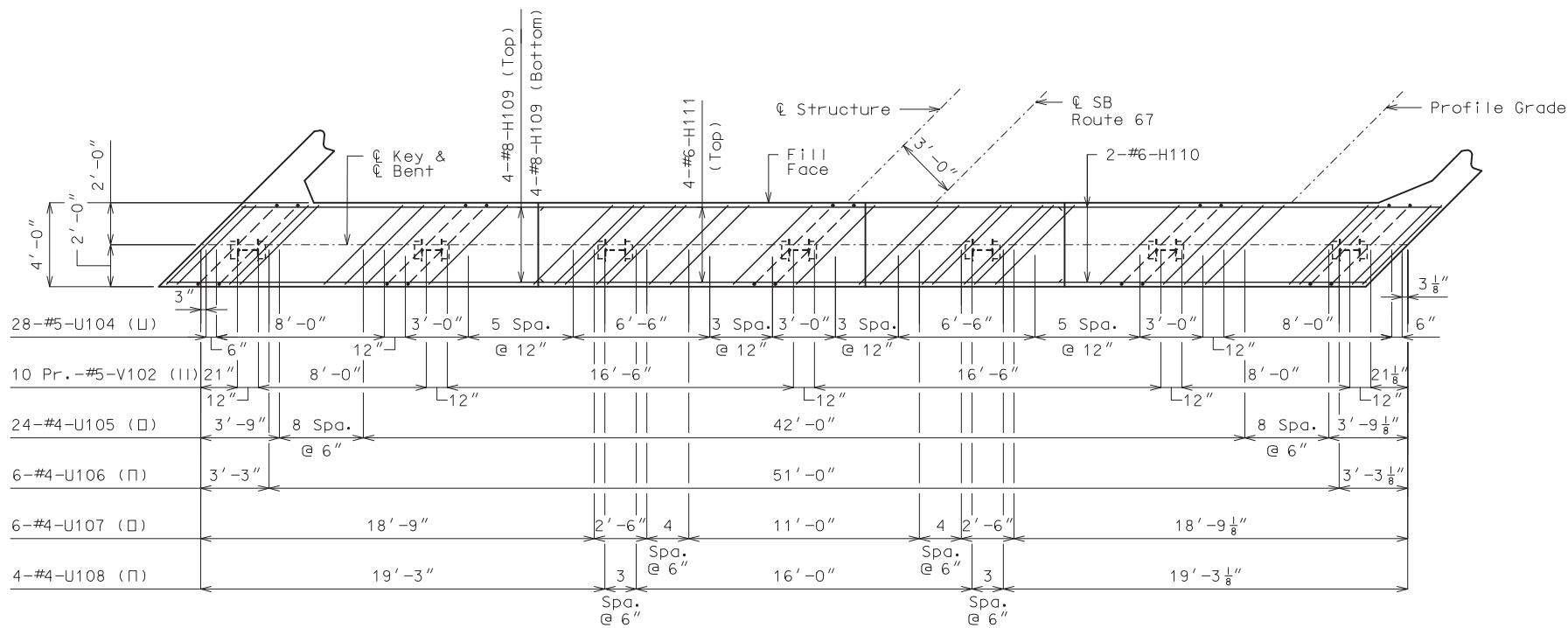
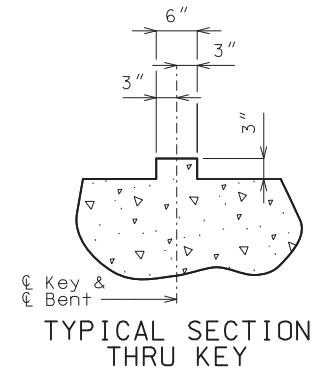
Sheet No. 2 of 24

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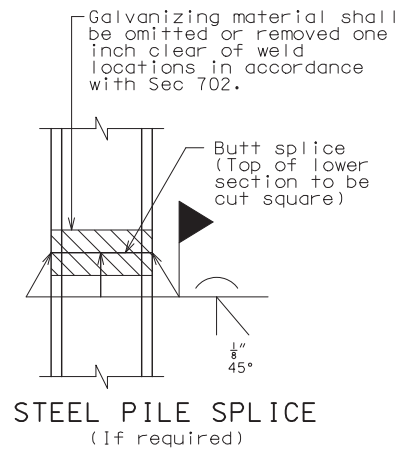


PLAN OF BEAM SHOWING DIMENSIONS



PLAN OF BEAM SHOWING REINFORCEMENT
(Keys not shown for clarity)

DETAILS OF END BENT NO. 1



General Notes:

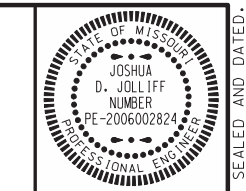
- For details of End Bent No. 1 not shown, see Sheets No. 4 & 5.
- For details of Vertical Drain at End Bents, see Sheet No. 6.
- The U bars and pairs of V bars shall be placed parallel to centerline of roadway.
- Reinforcing steel shall be shifted to clear piles, U bars shall clear piles by at least 1 1/2".

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 3 of 24

Detailed Oct. 2022
Checked Oct. 2022

pw:\cmtengr-pw.bentley.com\cmt-projects\Documents\Projects\MoDOT\20040908\Draw\Structures\Sheets\Harviell Ditch\A9279\B-A9279-003-J9P3751_End_Bent_1_Details.dgn



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DATE PREPARED

3/3/2023

ROUTE 67 STATE MO

DISTRICT BR SHEET NO. 3

COUNTY BUTLER

JOB NO. J9P3751

CONTRACT ID.

PROJECT NO.

BRIDGE NO. A9279

DESCRIPTION

DATE

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

105 WEST CAPITOL JEFFERSON CITY, MO 65102
1-888-ASK-MODOT (1-888-275-6636)

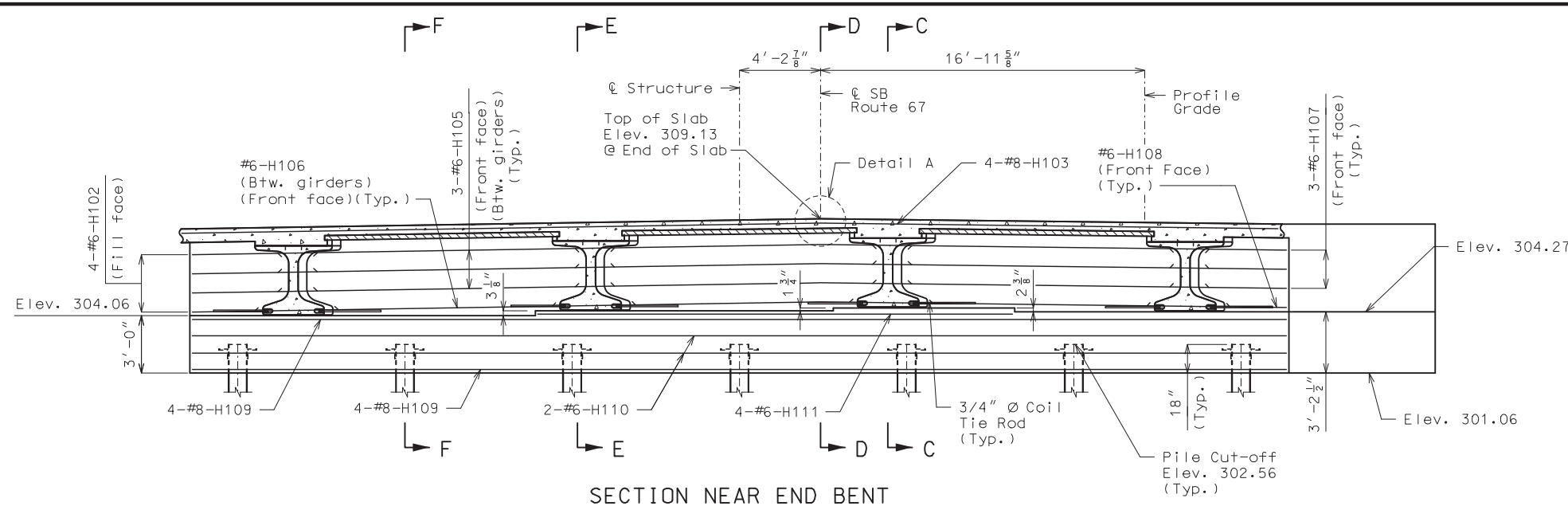


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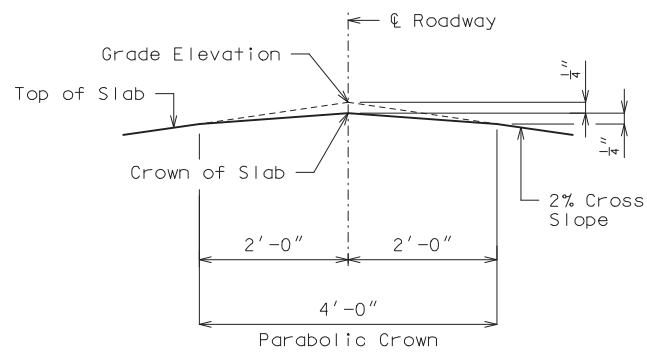
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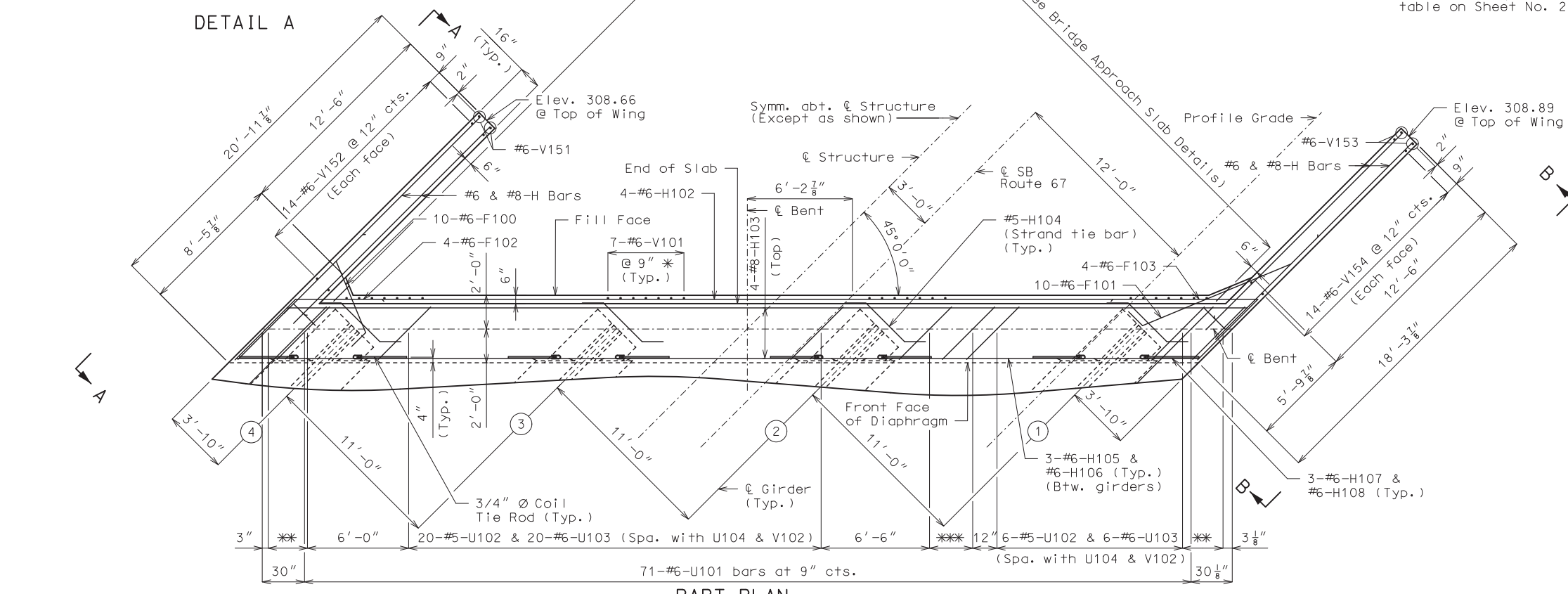
10:08:04 AM 3/3/2023



SECTION NEAR END BENT
Keys not shown for clarity.



DETAIL A



PART PLAN

DETAILS OF END BENT NO. 1

- * Centered behind girders
- ** 4-#5-U109 & 4-#6-U103 (Spa. with U104 & V102)
- *** 4-#5-U109 & 4-#6-U103 (Spa. with U104)

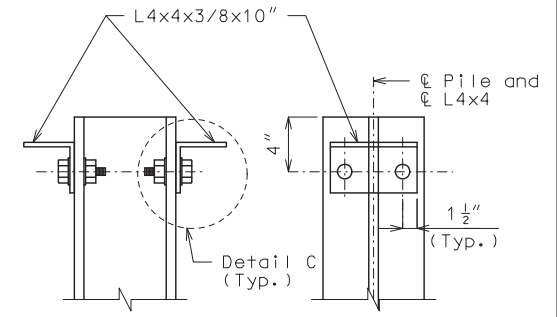
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General Notes:

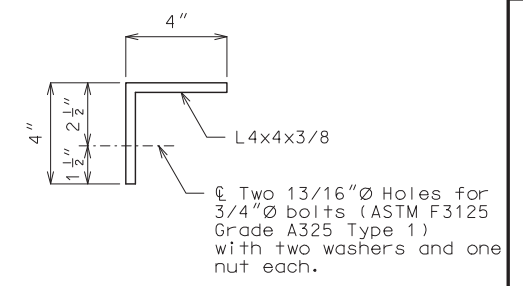
- For details of End Bent No. 1 not shown, see Sheets No. 3 & 5.
- For Sections C-C, D-D, E-E, & F-F, see Sheet No. 5.
- For Elevations A-A & B-B, see Sheet No. 5.
- All concrete in the end bent above top of beam and below top of slab shall be Class B-2.
- For location of Coil Tie Rods and #5-H104 (Strand Tie Bar), see Sheets No. 10 and 11.
- For details of Vertical Drain at End Bents, see Sheet No. 6.
- Reinforcing steel shall be shifted to clear piles, U bars shall clear piles by at least 1 1/2".
- The #6-F100 and #6-F101 bars shall be bent in the field to clear girders.
- The U bars shall be placed parallel to centerline of roadway.
- Strands at end of girders shall be field bent or, if necessary, cut in field to maintain 1 1/2-inch minimum clearance to fill face of end bent.
- For details and reinforcement of the Type D Barrier, see Sheet No. 18.
- For details of Bridge Approach Slab, see Sheet No. 19.

Substructure Quantity Table for Bent No. 1			
Item	Quantity	Unit	Quantity
Class 1 Excavation	75	cu. yard	
Galvanized Structural Steel Piles (12 in.)	553	linear foot	
Pile Point Reinforcement	7	each	
Class B Concrete (Substructure)	31.9	cu. yard	

Note: These quantities are included in the Estimated Quantities table on Sheet No. 2.



DETAILS OF HP PILE ANCHORS

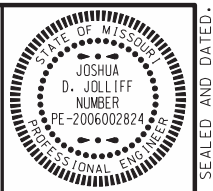


DETAIL C

Angles shall be coated with a minimum of two coats of non-aluminum epoxy mastic primer to provide a dry film thickness of 4 mils minimum, 8 mils maximum, or galvanized in accordance with Sec 1081. Bolts, washers and nuts shall be galvanized in accordance with AASHTO M 232 (ASTM A153), Class C.

Detailed Oct. 2022
Checked Oct. 2022

Sheet No. 4 of 24



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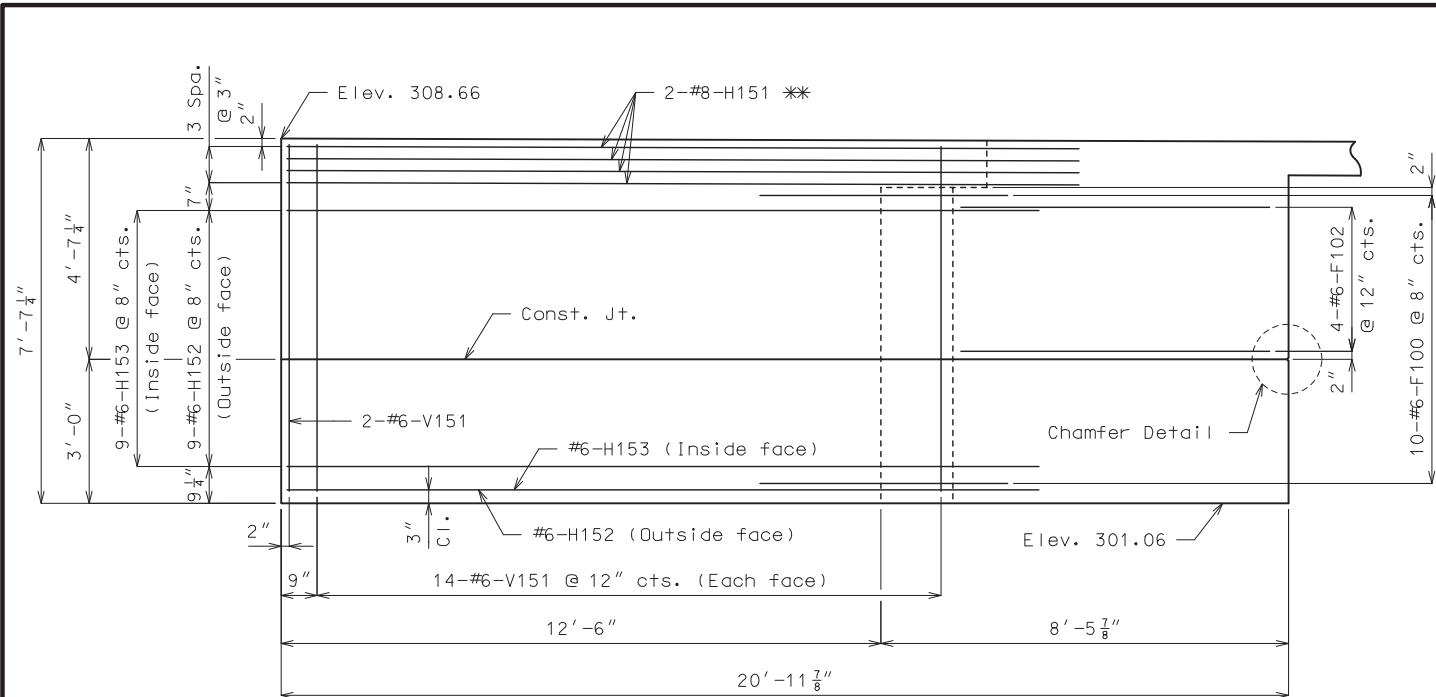
DATE PREPARED	3/3/2023
ROUTE	67
STATE	MO
DISTRICT	BR
SHEET NO.	4
COUNTY	BUTLER
JOB NO.	J9P3751
CONTRACT ID.	
PROJECT NO.	
BRIDGE NO.	A9279

DATE	DESCRIPTION

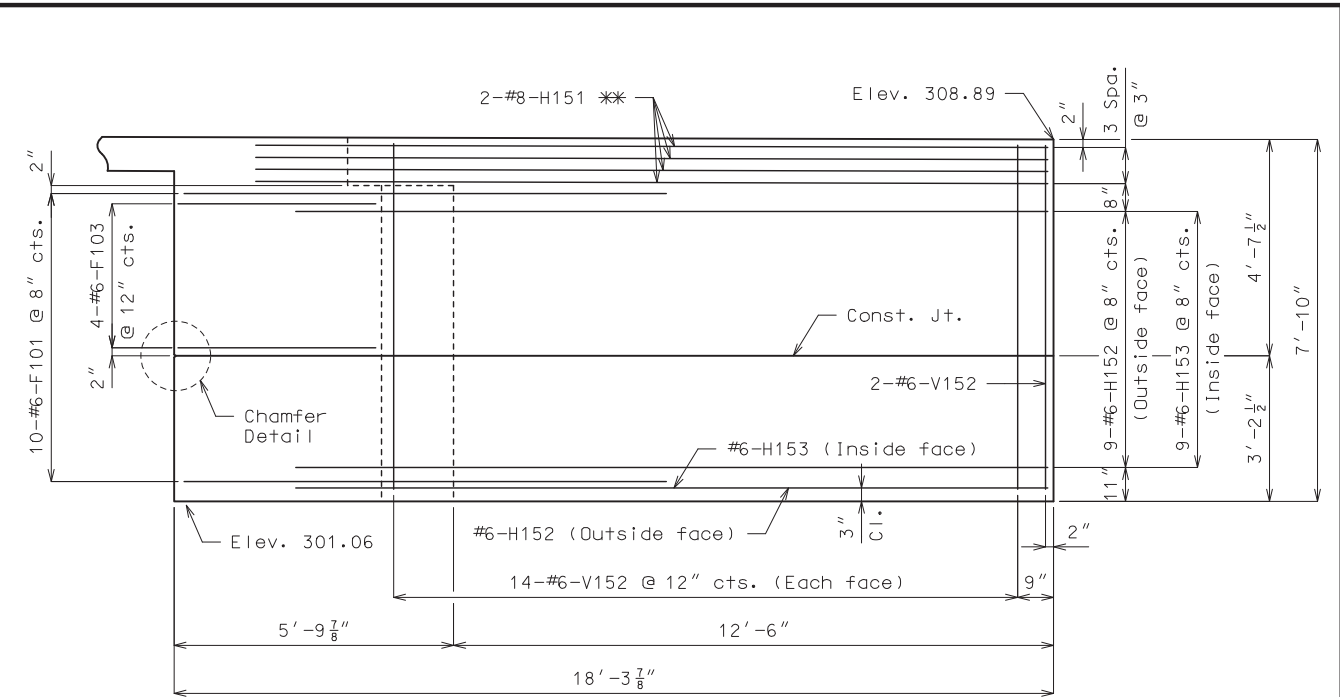
MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION
MoDOT
 105 WEST CAPITOL
 JEFFERSON CITY, MO 65102
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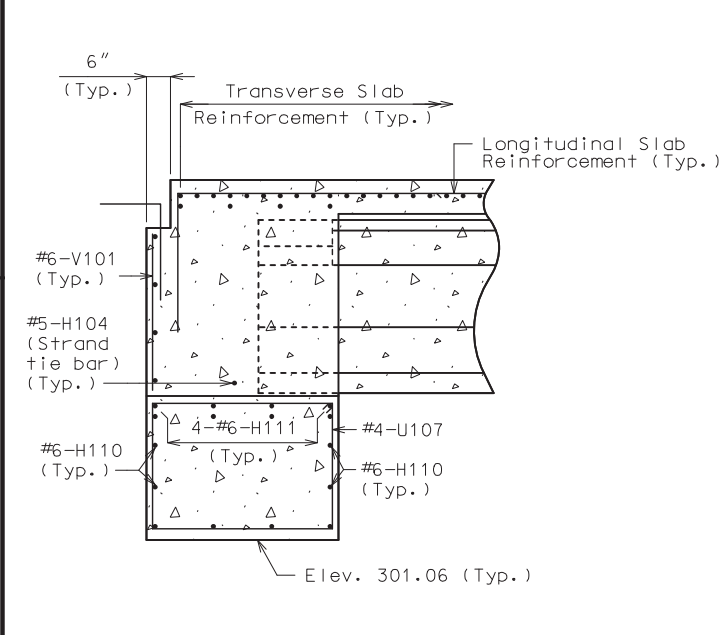
IF A SEAL IS PRESENT ON THIS SHEET IT HAS BEEN ELECTRONICALLY SEALED AND DATED.



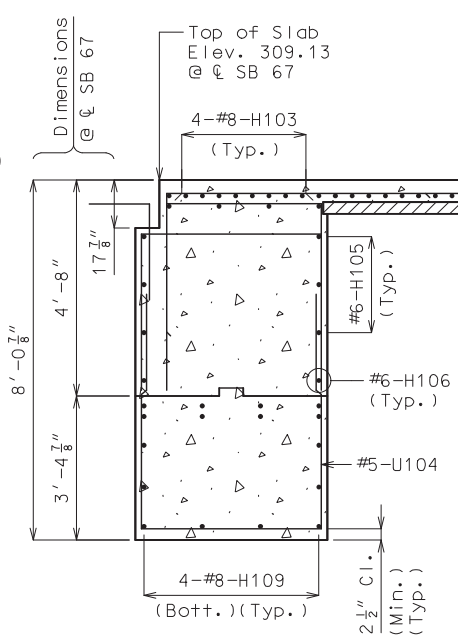
ELEVATION A-A



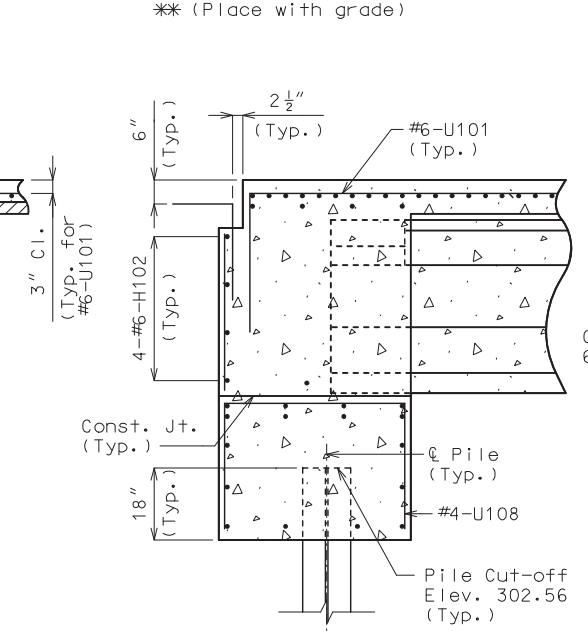
ELEVATION B-B



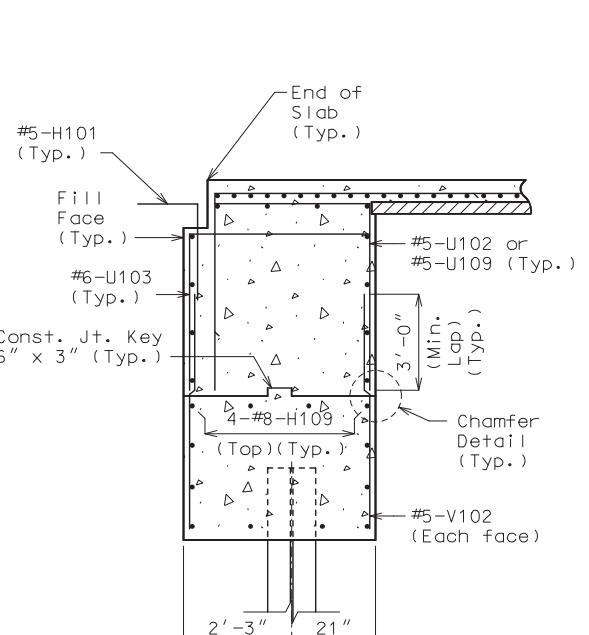
SECTION C-C



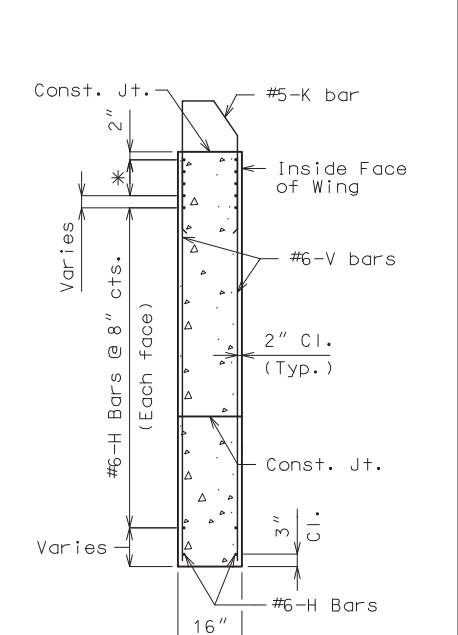
SECTION D-D



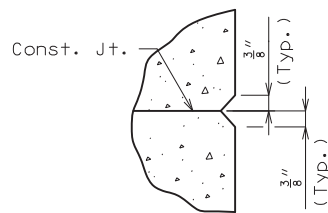
SECTION E-E



SECTION F-F



TYPICAL SECTION THRU WING



CHAMFER DETAIL

DETAILS OF END BENT NO. 1

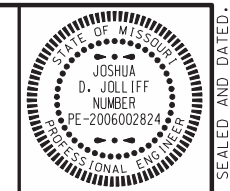
General Notes:

- For details of End Bent No. 1 not shown, see Sheets No. 3 & 4.
- All concrete in the end bent above top of beam and below top of slab shall be Class B-2.
- The #6-F100 and #6-F101 bars shall be bent in field to clear girders.
- For details and reinforcement of the Type D Barrier, see Sheet No. 18.
- For details of Vertical Drain at End Bents, see Sheet No. 6.
- For location of #5-H104 (Strand tie bar), see Sheets No. 10 and 11.
- For location of Sections C-C, D-D, E-E, & F-F, see Sheet No. 4.
- For location of Elevations A-A & B-B, see Sheet No. 4.
- For details of Bridge Approach Slab, see Sheet No. 19.

Detailed Oct. 2022
Checked Oct. 2022

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 5 of 24



THIS SHEET HAS BEEN SIGNED, SEALED AND DATED ELECTRONICALLY.

DATE PREPARED 3/3/2023	
ROUTE 67	STATE MO
DISTRICT BR	SHEET NO. 5
COUNTY BUTLER	
JOB NO. J9P3751	
CONTRACT ID.	
PROJECT NO.	
BRIDGE NO. A9279	

DESCRIPTION	DATE

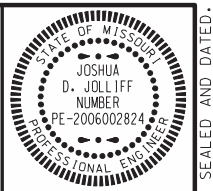
MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

105 WEST CAPITOL
JEFFERSON CITY, MO 65102
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ENGINEERING CORPORATION - 000631

IF A SEAL IS PRESENT ON THIS SHEET IT HAS BEEN ELECTRONICALLY SEALED AND DATED. REV.



THIS SHEET HAS BEEN SIGNED, SEALED AND DATED ELECTRONICALLY.

DATE PREPARED
3/3/2023

ROUTE 67 STATE MO
DISTRICT BR SHEET NO. 6

COUNTY BUTLER
JOB NO. J9P3751
CONTRACT ID.

PROJECT NO.
BRIDGE NO. A9279

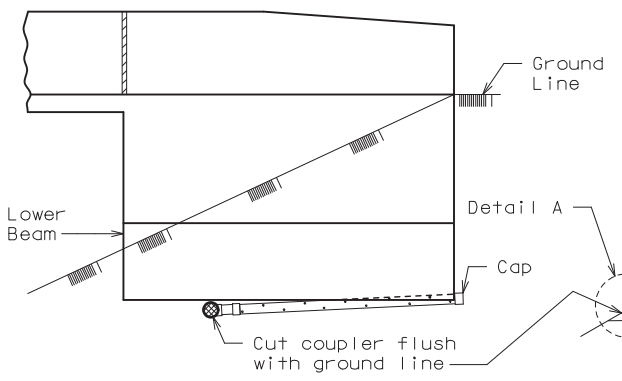
DATE	DESCRIPTION

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

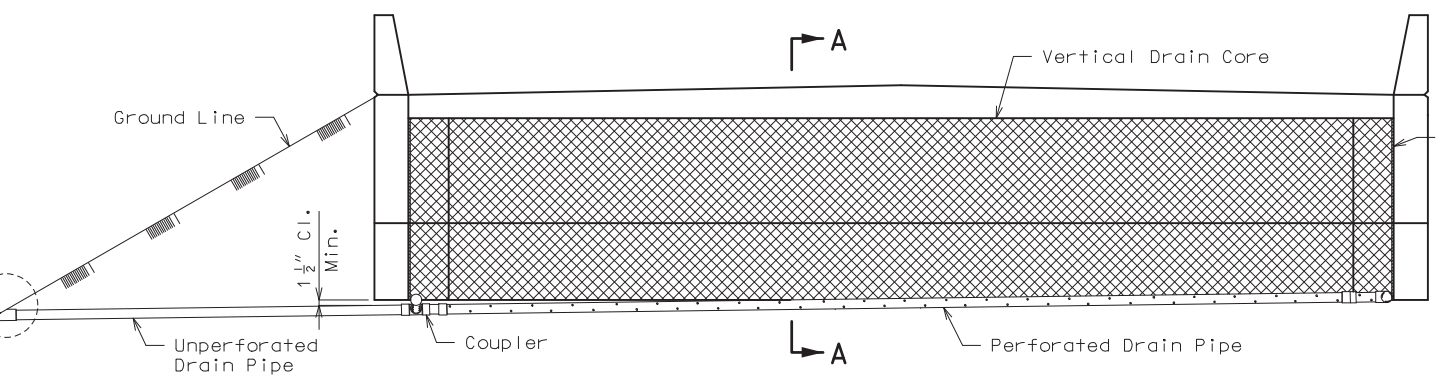
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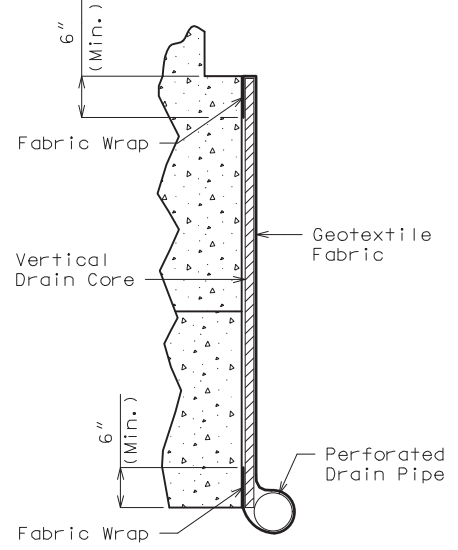
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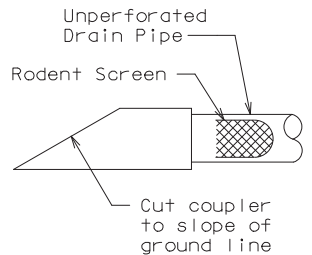
ELEVATION OF WING



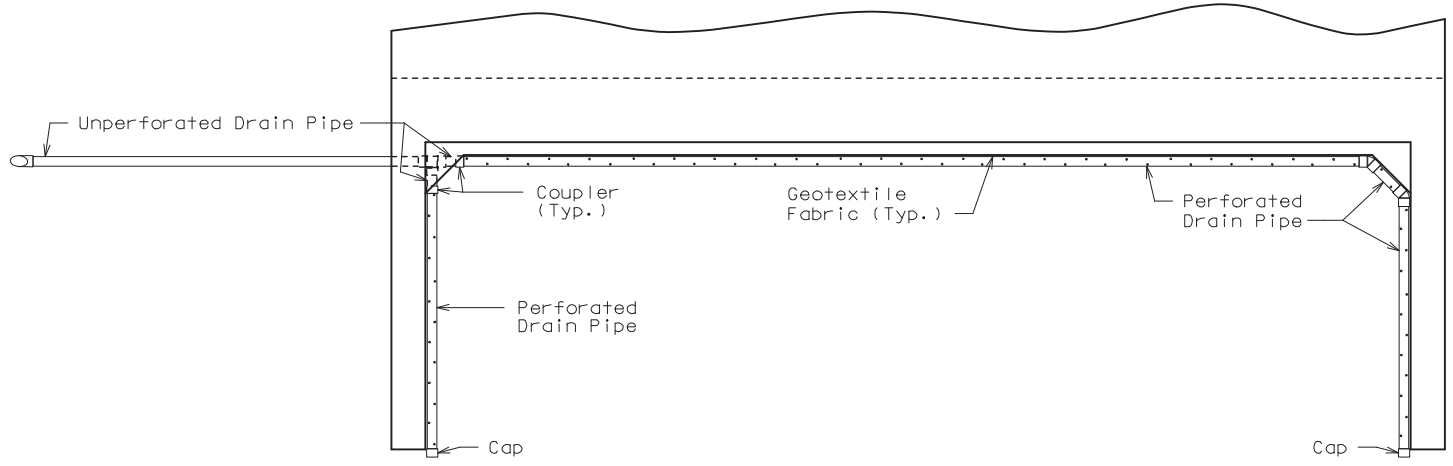
ELEVATION OF END BENT



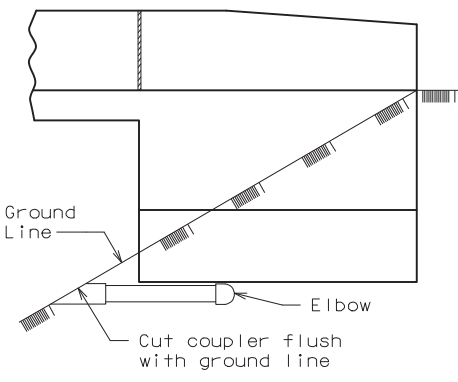
PART SECTION A-A
(Section thru wing similar)



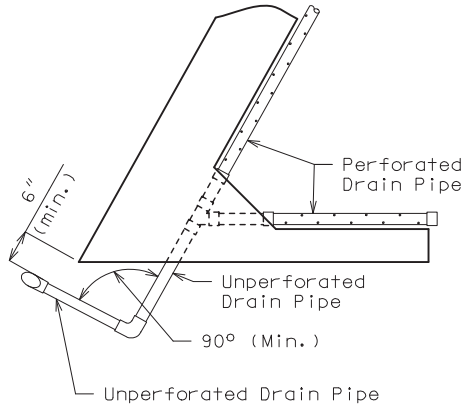
DETAIL A



PLAN OF END BENT



ELEVATION OF WING



PART PLAN

OPTIONAL TURNED DRAIN
(Use only when straight drain is not practical.)

General Notes:

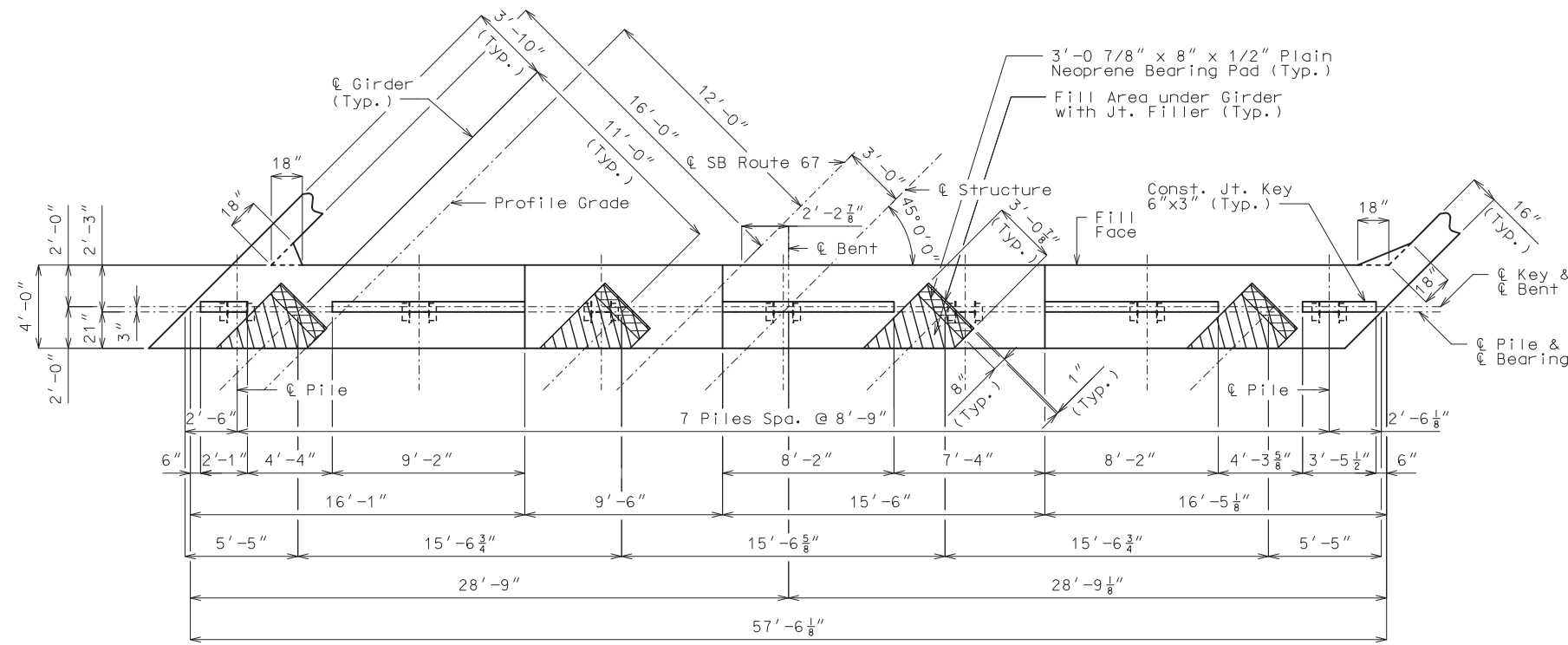
- All drain pipe shall be sloped 1 to 2 percent.
- Drain pipe may be either 6-inch diameter corrugated metallic-coated steel pipe underdrain, 4-inch diameter corrugated polyvinyl chloride (PVC) drain pipe, or 4-inch diameter corrugated polyethylene (PE) drain pipe.
- Drain pipe shall be placed at fill face of end bent and inside face of wings. The pipe shall slope to lowest grade of ground line, also missing the lower beam of end bent by a minimum of 1 1/2 inches.
- Perforated pipe shall be placed at fill face side and inside face of wings at the bottom of end bent and plain pipe shall be used where the vertical drain ends to the exit at ground line.

VERTICAL DRAIN AT END BENTS
(Squared end bent shown, skewed end bent similar)

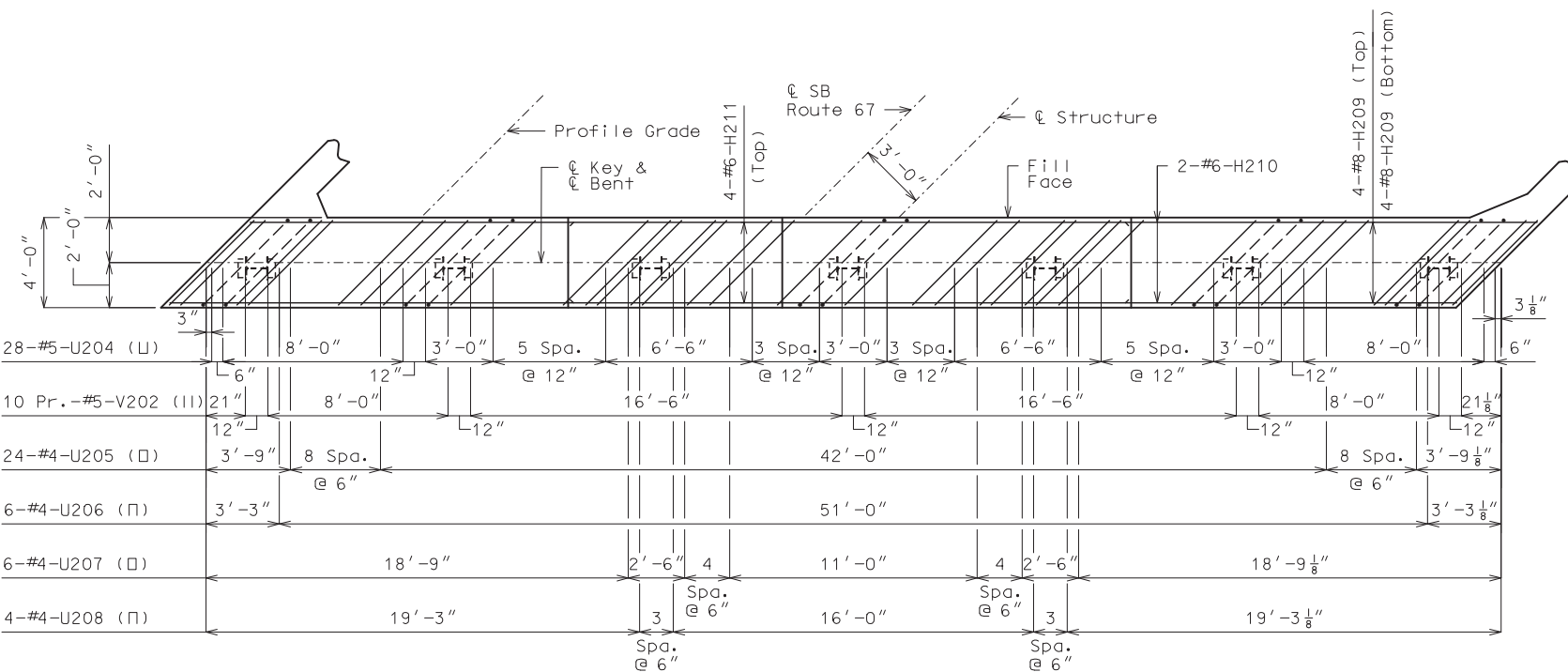
Detailed Oct. 2022
Checked Oct. 2022

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 6 of 24

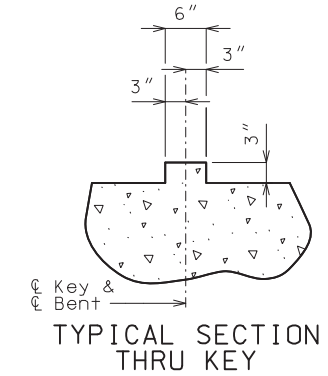


PLAN OF BEAM SHOWING DIMENSIONS

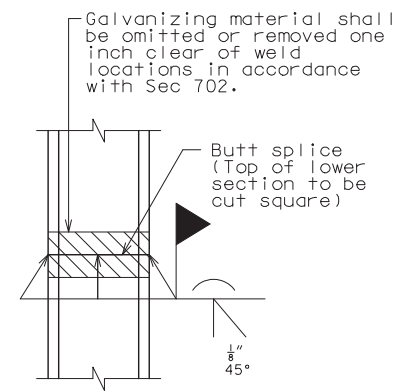


PLAN OF BEAM SHOWING REINFORCEMENT
(Keys not shown for clarity)

DETAILS OF END BENT NO. 2

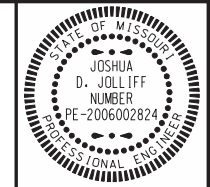


STEEL PILE SPLICE
(If required)



General Notes:

- For details of End Bent No. 2 not shown, see Sheets No. 8 & 9.
- For details of Vertical Drain at End Bents, see Sheet No. 6.
- The U bars and pairs of V bars shall be placed parallel to centerline of roadway.
- Reinforcing steel shall be shifted to clear piles, U bars shall clear piles by at least 1 1/2".



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DATE PREPARED
3/3/2023

ROUTE 67 STATE MO
DISTRICT BR SHEET NO. 7

COUNTY BUTLER
JOB NO. J9P3751
CONTRACT ID.

PROJECT NO.

BRIDGE NO. A9279

DATE	DESCRIPTION

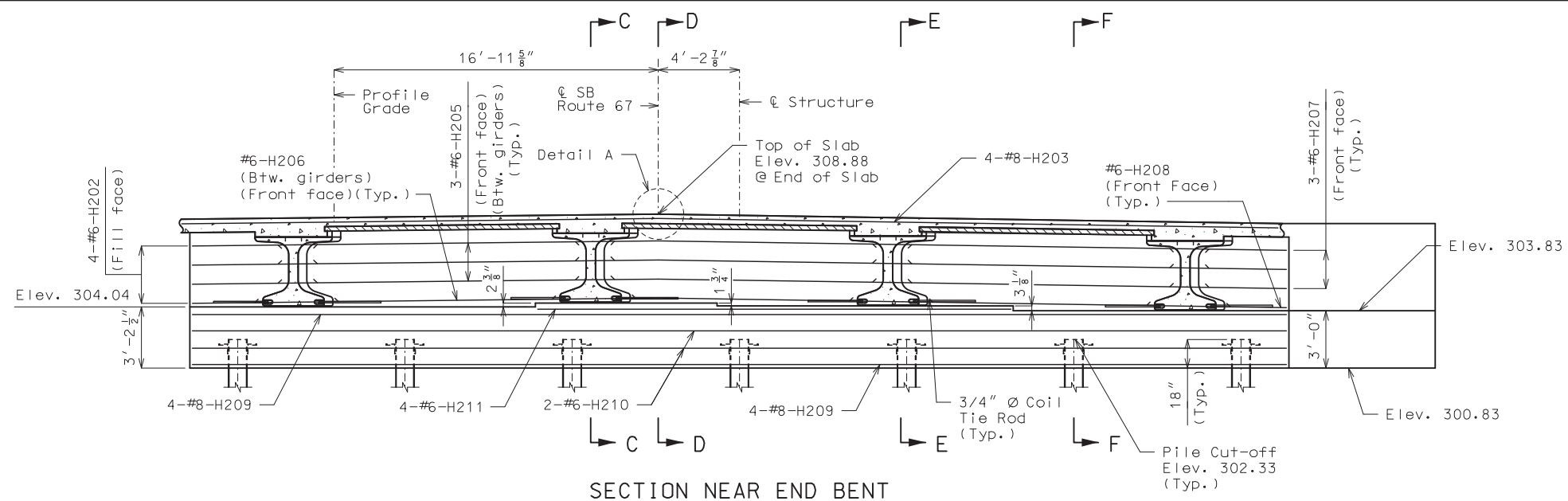
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105 WEST CAPITOL JEFFERSON CITY, MO 65102
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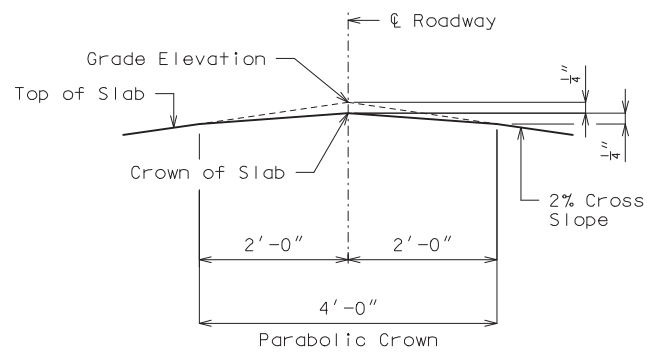
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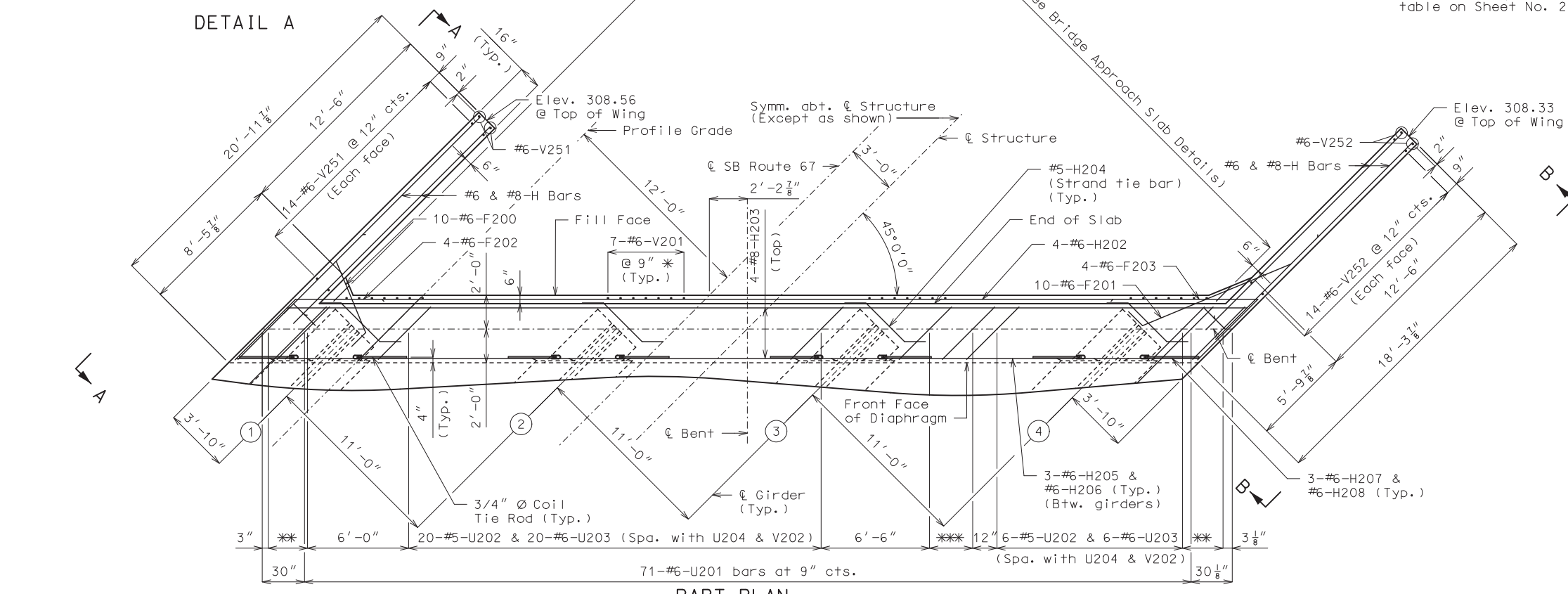
REV.



SECTION NEAR END BENT
Keys not shown for clarity.



DETAIL A



PART PLAN

DETAILS OF END BENT NO. 2

* Centered behind girders
 ** 4-#5-U209 & 4-#6-U203 (Spa. with U204 & V202)
 *** 4-#5-U209 & 4-#6-U203 (Spa. with U204)

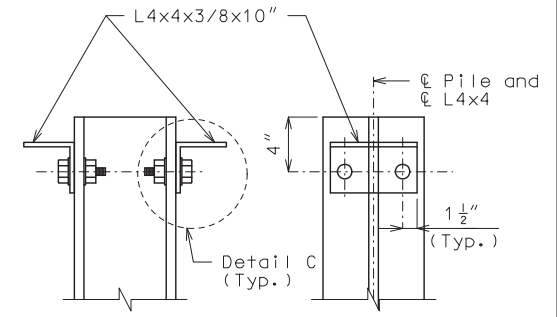
Note: This drawing is not to scale. Follow dimensions.

General Notes:

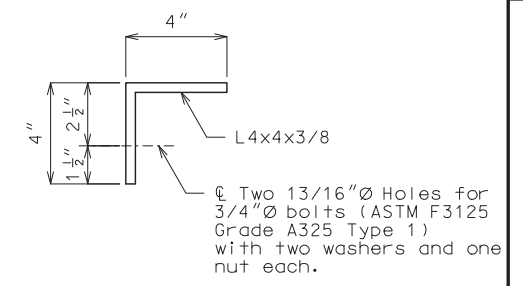
For details of End Bent No. 2 not shown, see Sheets No. 7 & 9.
 For Sections C-C, D-D, E-E, & F-F, see Sheet No. 9.
 For Elevations A-A & B-B, see Sheet No. 9.
 All concrete in the end bent above top of beam and below top of slab shall be Class B-2.
 For location of Coil Tie Rods and #5-H204 (Strand Tie Bar), see Sheets No. 10 and 11.
 For details of Vertical Drain at End Bents, see Sheet No. 6.
 Reinforcing steel shall be shifted to clear piles, U bars shall clear piles by at least 1 1/2\"/>

Item	Quantity
Class 1 Excavation	cu. yard 20
Galvanized Structural Steel Piles (12 in.)	linear foot 567
Pile Point Reinforcement	each 7
Class B Concrete (Substructure)	cu. yard 31.9

Note: These quantities are included in the Estimated Quantities table on Sheet No. 2.

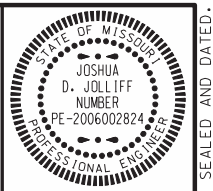


DETAILS OF HP PILE ANCHORS



DETAIL C

Angles shall be coated with a minimum of two coats of non-aluminum epoxy mastic primer to provide a dry film thickness of 4 mils minimum, 8 mils maximum, or galvanized in accordance with Sec 1081. Bolts, washers and nuts shall be galvanized in accordance with AASHTO M 232 (ASTM A153), Class C.



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DATE PREPARED: 3/3/2023
 ROUTE: 67 STATE: MO
 DISTRICT: BR SHEET NO.: 8
 COUNTY: BUTLER
 JOB NO.: J9P3751
 CONTRACT ID.:
 PROJECT NO.:
 BRIDGE NO.: A9279

DATE	DESCRIPTION

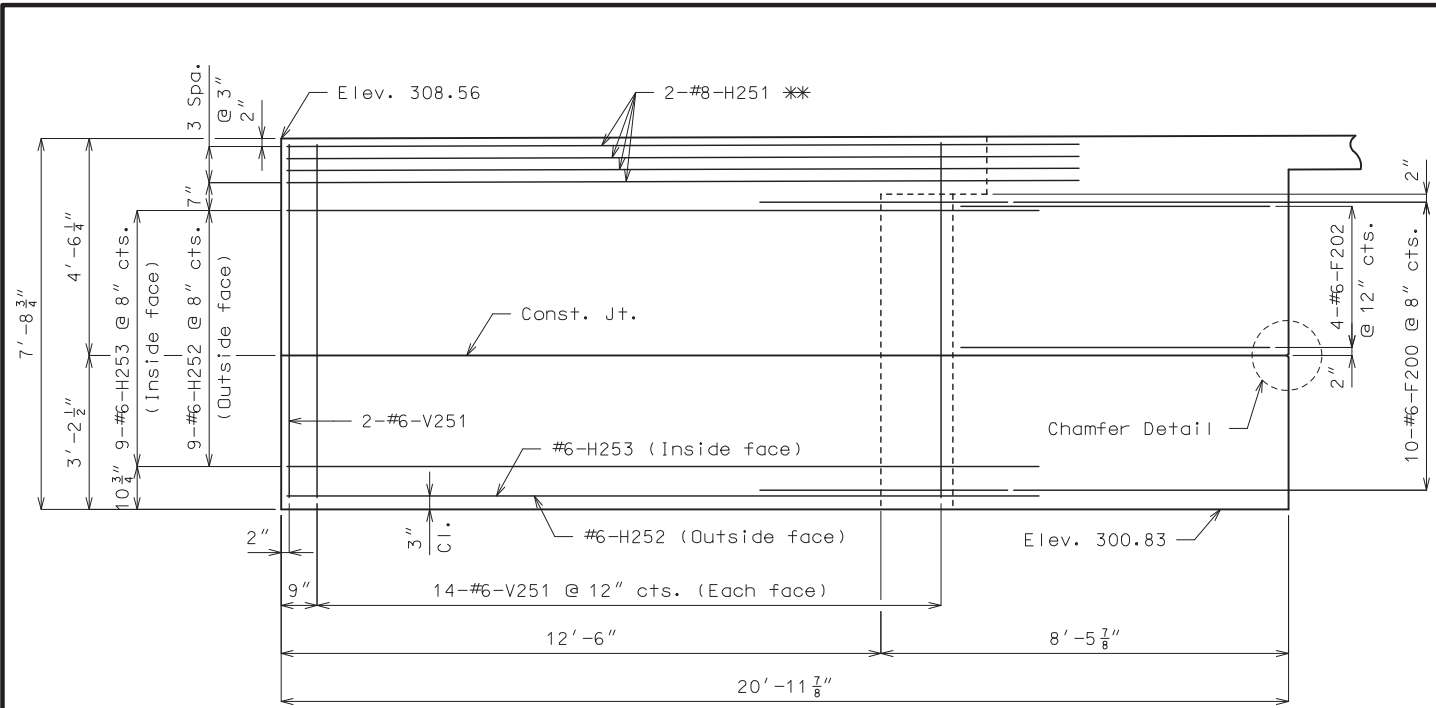
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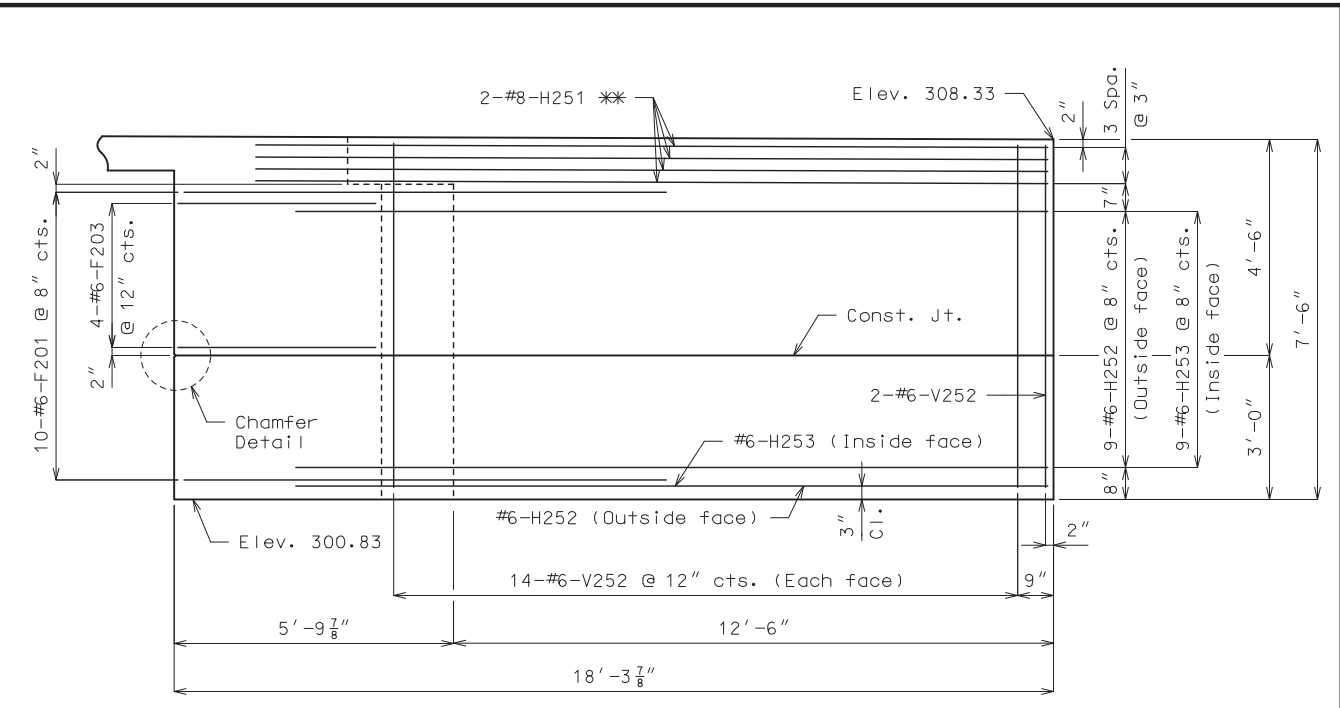
Detailed Oct. 2022
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Sheet No. 8 of 24

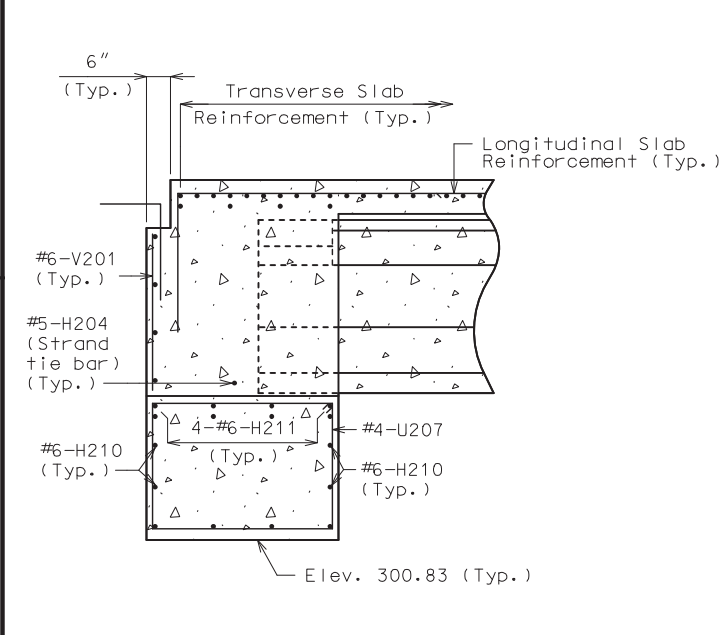
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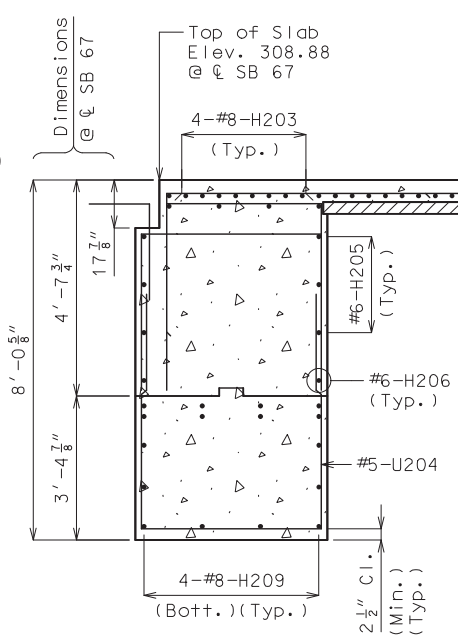
ELEVATION A-A



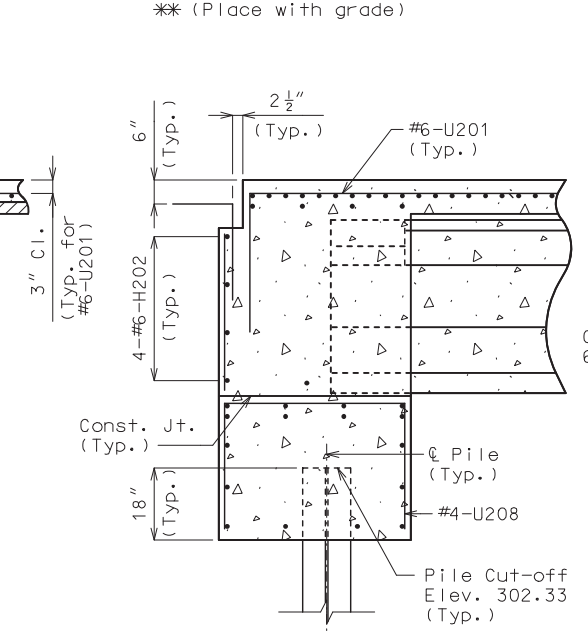
ELEVATION B-B



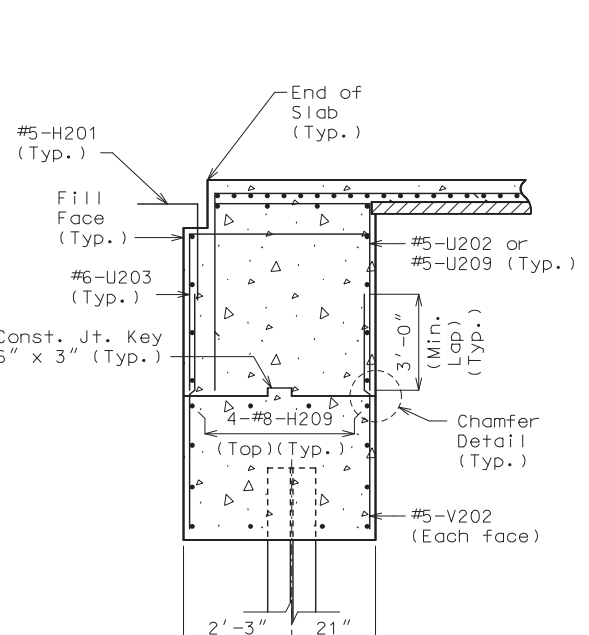
SECTION C-C



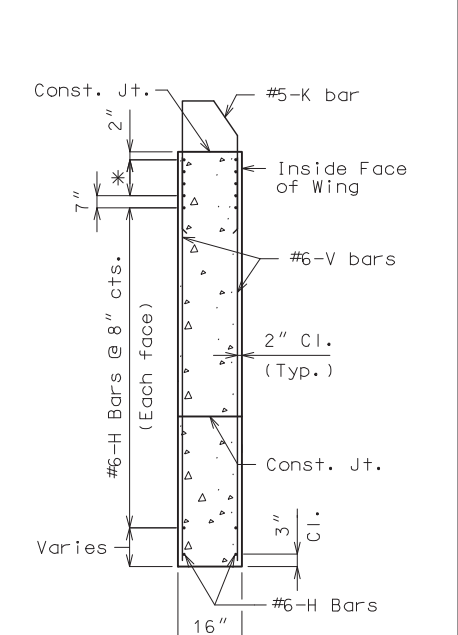
SECTION D-D



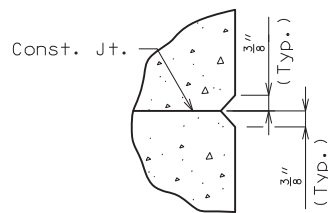
SECTION E-E



SECTION F-F



TYPICAL SECTION THRU WING



CHAMFER DETAIL

DETAILS OF END BENT NO. 2

General Notes:

- For details of End Bent No. 2 not shown, see Sheets No. 7 & 8.
- All concrete in the end bent above top of beam and below top of slab shall be Class B-2.
- The #6-F200 and #6-F201 bars shall be bent in field to clear girders.
- For details and reinforcement of the Type D Barrier, see Sheet No. 18.
- For details of Vertical Drain at End Bents, see Sheet No. 6.
- For location of #5-H204 (Strand tie bar), see Sheets No. 10 and 11.
- For location of Sections C-C, D-D, E-E, & F-F, see Sheet No. 8.
- For location of Elevations A-A & B-B, see Sheet No. 8.
- For details of Bridge Approach Slab, see Sheet No. 19.

Detailed Oct. 2022
Checked Oct. 2022

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 9 of 24

THIS SHEET HAS BEEN SIGNED, SEALED AND DATED ELECTRONICALLY.

DATE PREPARED: 3/3/2023

ROUTE 67, DISTRICT BR, COUNTY BUTLER, JOB NO. J9P3751, CONTRACT ID. PROJECT NO. BRIDGE NO. A9279

DATE	DESCRIPTION

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

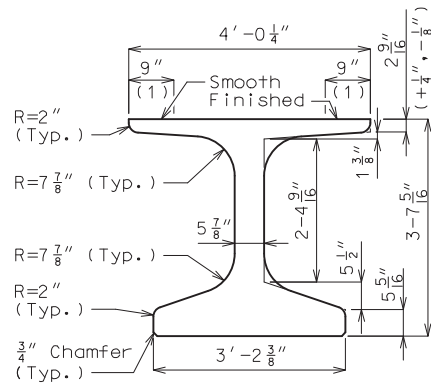
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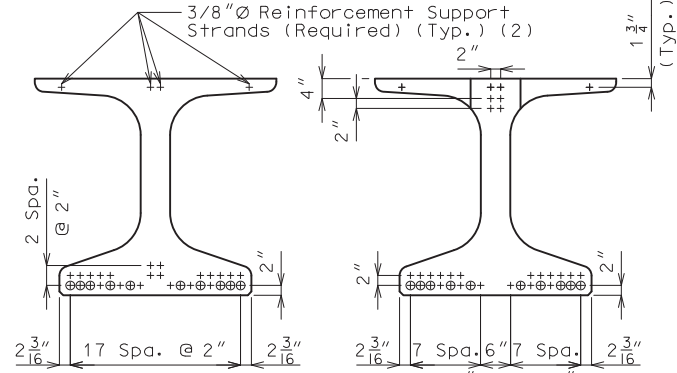
IF A SEAL IS PRESENT ON THIS SHEET IT HAS BEEN ELECTRONICALLY SEALED AND DATED.

(1) Fabricator shall apply a bond breaker to this region excluding where joint filler will be applied.

(2) Outer strands tensioned to 2.02 kips/strand and inner strands to 8 kips/strand. Placed symmetrical about \bar{C} Girder. May be moved laterally in pairs.

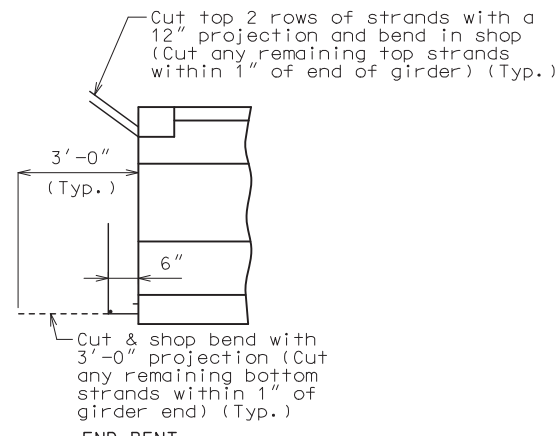


DIMENSIONS

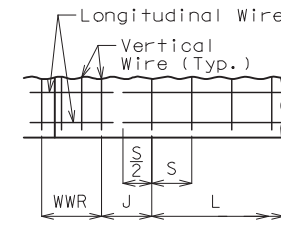


STRAND ARRANGEMENT

+ Indicates prestressing strand. o Indicates cut & shop bend with 3'-0" projection.



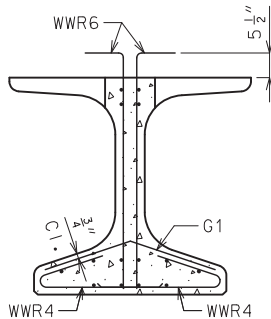
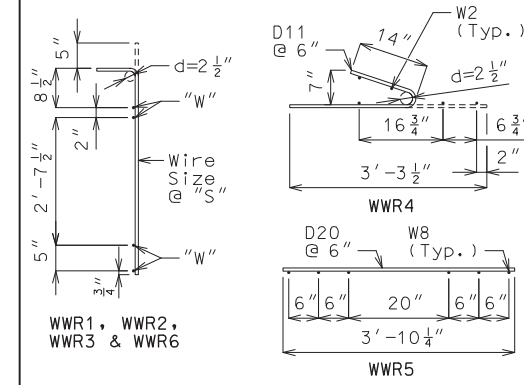
STRANDS AT GIRDER ENDS



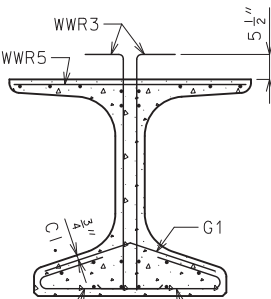
WELDED WIRE PLACEMENT

S = Vertical wire spacing
L = Length of WWR mats
J = Distance between WWR mats

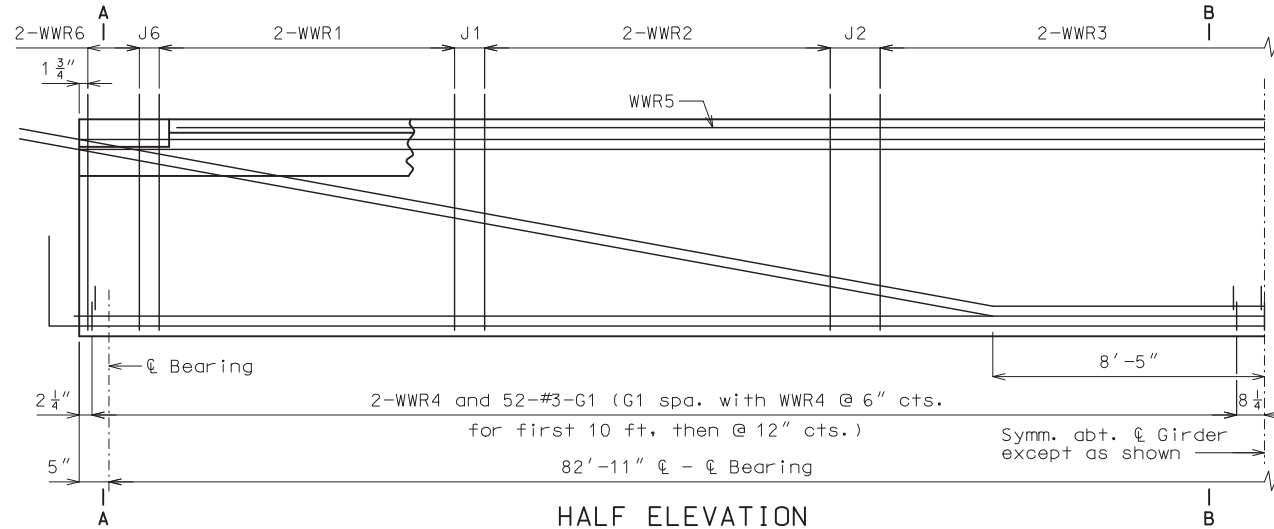
Bill of Reinforcing Steel					
Bars Each Girder					Bending Diagrams
No.	Size/Mark	Length	Shape		
105	3 G1	2'-10"	8		
2	4 G3	5'-5"	20		
14	4 G6	Varies	20		
Welded Wire Each Girder					Shape 20
Mark	Size	S	W	L	
WWR1	D31	4"	W12	4'-4"	8"
WWR2	D31	8"	W12	22'-8"	12"
WWR3	D31	12"	W12	23'-0"	-
WWR6	D31	2"	W12	16"	2 3/4"



SECTION A-A
Strands not shown for clarity.

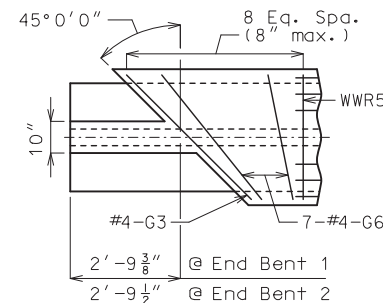


SECTION B-B
Strands not shown for clarity.

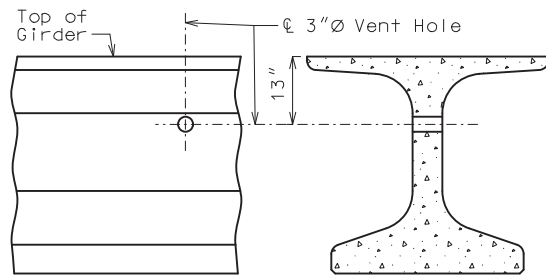


HALF ELEVATION

Reinforcement support strands not shown for clarity.

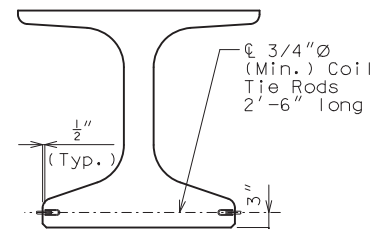


TOP FLANGE BLOCKOUT

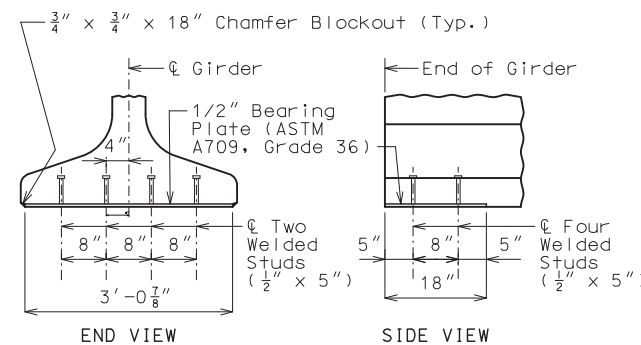


VENT HOLE

Place vent holes at or near upgrade 1/3 point of girders and clear reinforcing steel or strands by 1 1/2" minimum.



**INTEGRAL BENTS
COIL TIES**



BEARING PLATE

NU-GIRDERS - SPAN (1-2)

All dimensions are out to out.

Hooks and bends shall be in accordance with the CRSI Manual of Standard Practice for Detailing Reinforced Concrete Structures, Stirrup and Tie Dimensions.

Actual bar lengths are measured along centerline of bar to the nearest inch.

Minimum clearance to reinforcing shall be 1", unless otherwise shown.

All bar reinforcement shall be Grade 60.

WWR shall not be epoxy coated.

General Notes:

Concrete for prestressed beams shall be Class A-1 with $f'c = 8000$ psi and $f'ci = 6500$ psi.

Use 30 strands, 0.6" $\bar{\phi}$ Grade 270, with an initial prestress force of 1318 kips.

Prestensioned members shall be in accordance with Sec 1029.

Fabricator shall be responsible for location and design of lifting devices.

Exterior and interior girders are the same except: application of bond breaker, coil inserts for slab drains.

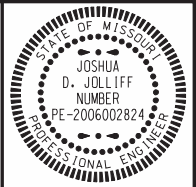
The contractor shall provide bracing necessary for lateral and torsional stability of the girders during construction of the concrete slab and remove the bracing after the slab has attained 75% design strength. Contractor shall not drill holes in the girders.

For Girder Camber Diagram, see Sheet No. 14.

For location of coil inserts at slab drains, see Sheet No. 13.

For location of coil ties at integral bents see Sheets No. 4 and 8.

Alternate bar reinforcing steel details are provided and may be used. The same type of reinforcing steel shall be used for all girders in all spans.



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DATE PREPARED
3/3/2023

ROUTE
67

DISTRICT
BR

COUNTY
BUTLER

JOB NO.
J9P3751

CONTRACT ID.

PROJECT NO.

BRIDGE NO.
A9279

DESCRIPTION

DATE

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

MoDOT

105 WEST CAPITOL
JEFFERSON CITY, MO 65102
1-888-ASK-MODOT (1-888-275-6636)

ENGINEERING CORPORATION - 000631

CMT

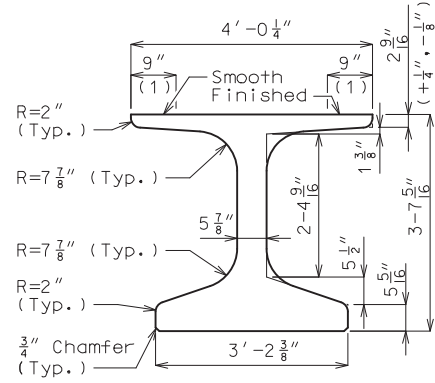
CRAWFORD, MURPHY & TILLY, INC.
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SPRINGFIELD, MO 65807 (417) 869-6009

REVISIONS

IF A SEAL IS PRESENT ON THIS SHEET IT HAS BEEN ELECTRONICALLY SEALED AND DATED.

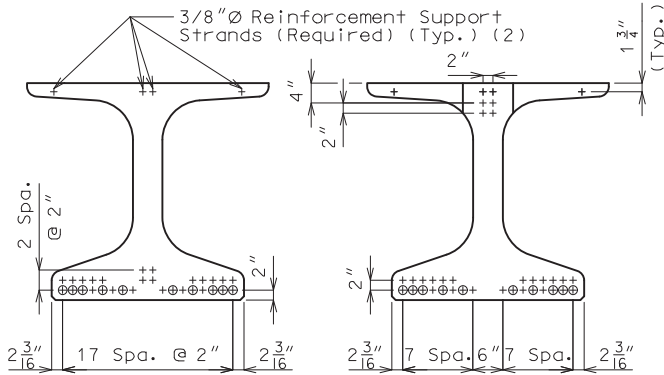
REV.

(1) Fabricator shall apply a bond breaker to this region excluding where joint filler will be applied.



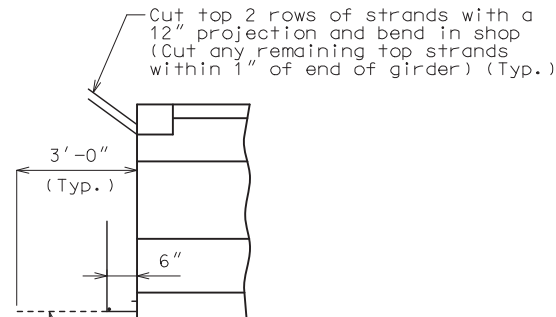
DIMENSIONS

(2) Outer strands tensioned to 2.02 kips/strand and inner strands to 8 kips/strand. Placed symmetrical about \bar{c} Girder. May be moved laterally in pairs.

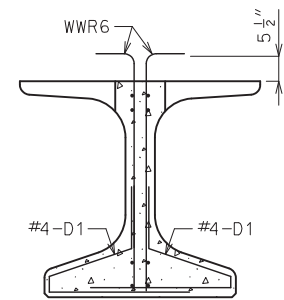


STRAND ARRANGEMENT

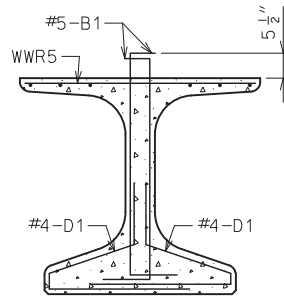
+ Indicates prestressing strand. o Indicates cut & shop bend with 3'-0" projection.



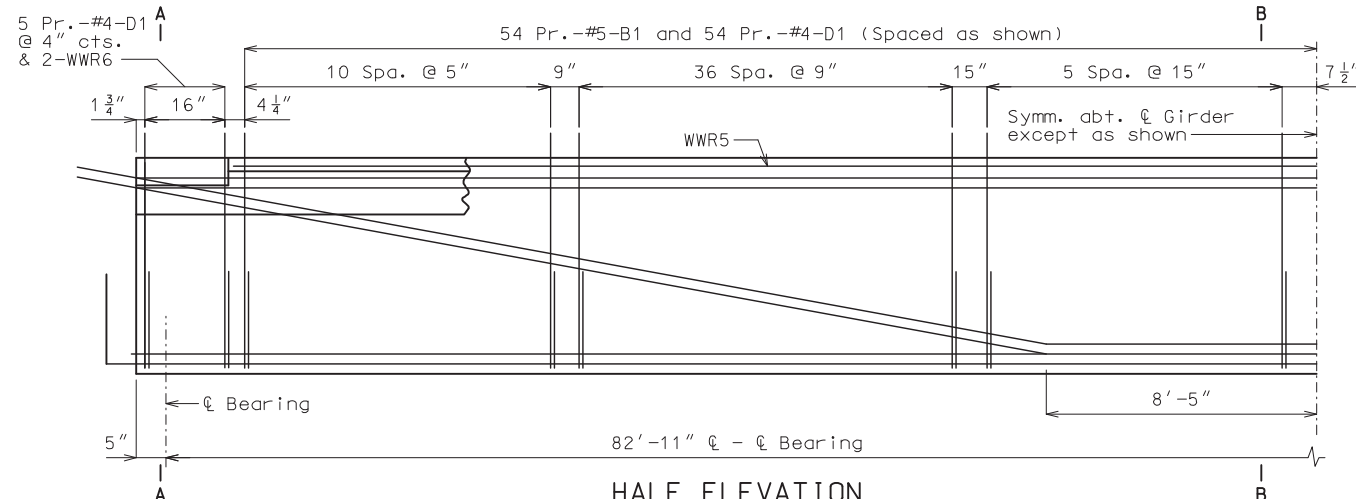
STRANDS AT GIRDER ENDS



SECTION A-A
Strands not shown for clarity.

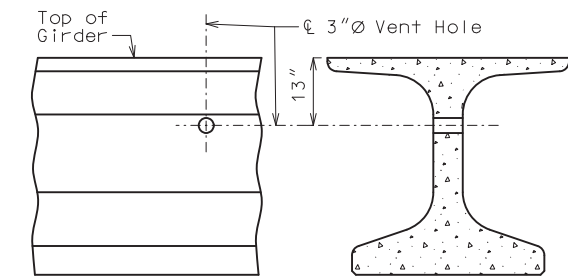


SECTION B-B
Strands not shown for clarity.



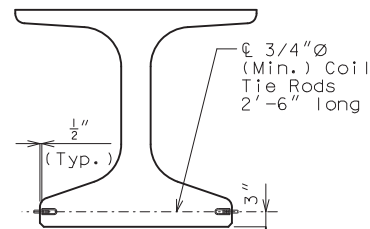
HALF ELEVATION

Reinforcement support strands not shown for clarity.

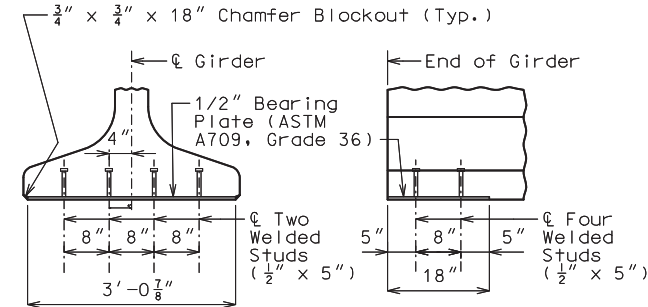


VENT HOLE

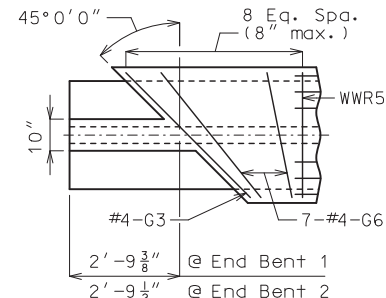
Place vent holes at or near upgrade 1/3 point of girders and clear reinforcing steel or strands by 1 1/2" minimum.



COIL TIES



BEARING PLATE

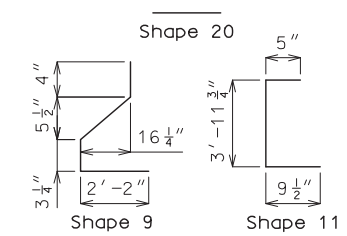


TOP FLANGE BLOCKOUT

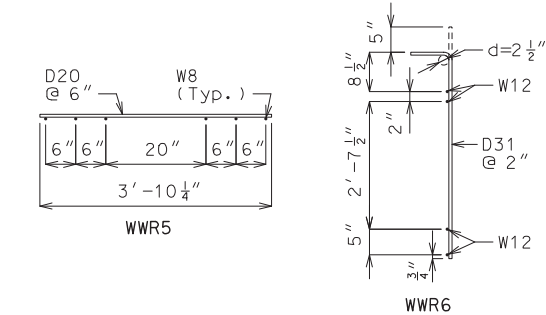
Bill of Reinforcing Steel - Each Girder

No.	Size/Mark	Length	Shape
216	5 B1	5'-0"	11
236	4 D1	4'-0"	9
2	4 G3	5'-5"	20
14	4 G6	Varies	20

Bending Diagrams



Welded Wire Reinforcement - Each Girder



All dimensions are out to out.

Hooks and bends shall be in accordance with the CRSI Manual of Standard Practice for Detailing Reinforced Concrete Structures, Stirrup and Tie Dimensions.

Actual bar lengths are measured along centerline of bar to the nearest inch.

Minimum clearance to reinforcing shall be 1".

All bar reinforcement shall be Grade 60.

The two D1 bars may be furnished as one bar at the fabricator's option.

All B1 bars shall be epoxy coated.

General Notes:

Concrete for prestressed girders shall be Class A-1 with $f'c = 8000$ psi and $f'ci = 6500$ psi.

Use 30 strands, 0.6"Ø Grade 270, with an initial prestress force of 1318 kips.

Prestensioned members shall be in accordance with Sec 1029.

Fabricator shall be responsible for location and design of lifting devices.

Exterior and interior girders are the same except: application of bond breaker, coil inserts for slab drains.

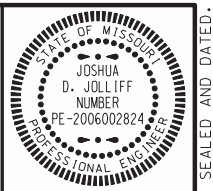
The contractor shall provide bracing necessary for lateral and torsional stability of the girders during construction of the concrete slab and remove the bracing after the slab has attained 75% design strength. Contractor shall not drill holes in the girders.

For Girder Camber Diagram, see Sheet No. 14.

For location of coil inserts at slab drains, see Sheet No. 13.

For location of coil ties at integral bent, see Sheets No. 4 and 8.

Alternate bar reinforcing steel details are provided and may be used. The same type of reinforcing steel shall be used for all girders in all spans.



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DATE PREPARED: 3/3/2023

ROUTE: 67, STATE: MO, DISTRICT: BR, SHEET NO.: 11

COUNTY: BUTLER, JOB NO.: J9P3751, CONTRACT ID.:

PROJECT NO., BRIDGE NO.: A9279

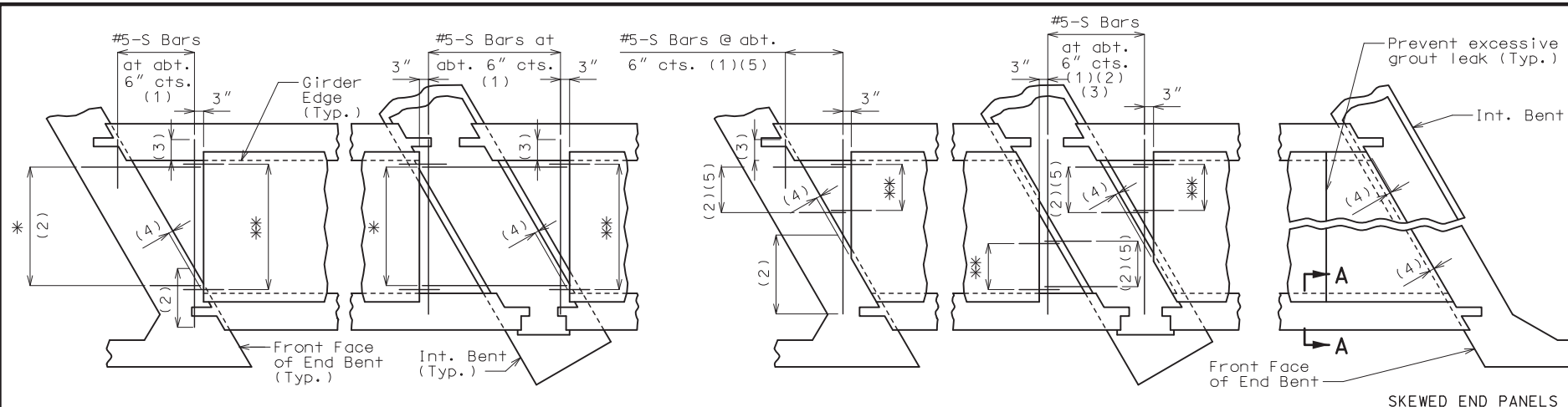
DESCRIPTION	DATE

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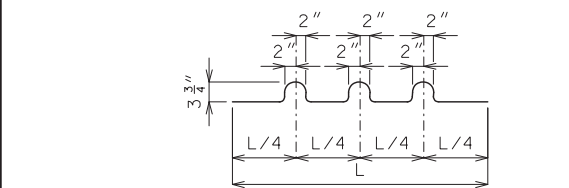


SQUARED END PANELS OR TRUNCATED END PANELS

SKEWED END PANELS

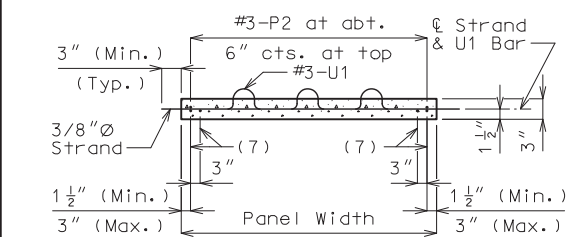
PLAN SHOWING PANELS PLACEMENT

* #5-S Bars at abt. 9" cts. (1)
 ** #3-P1 at 12" cts. (End panels only)

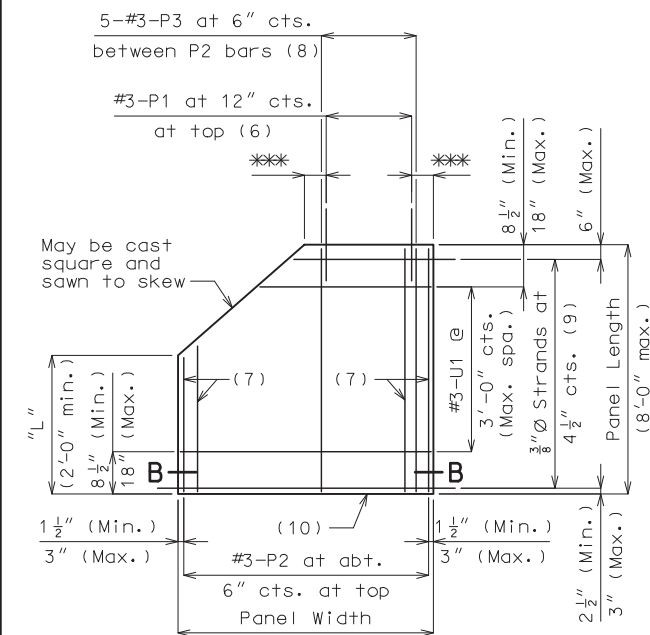


BENDING DIAGRAM FOR U1 BAR

U1 Bars may be oriented at right angles to location and spacing shown. U1 Bars shall be placed between P1 bars.

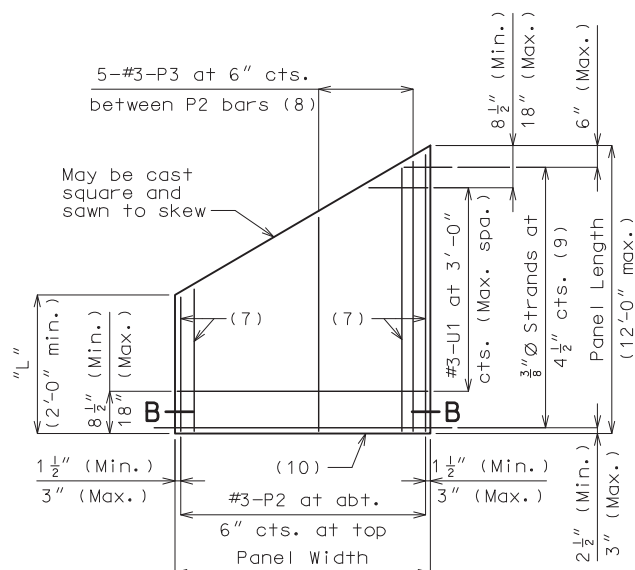


SECTION B-B

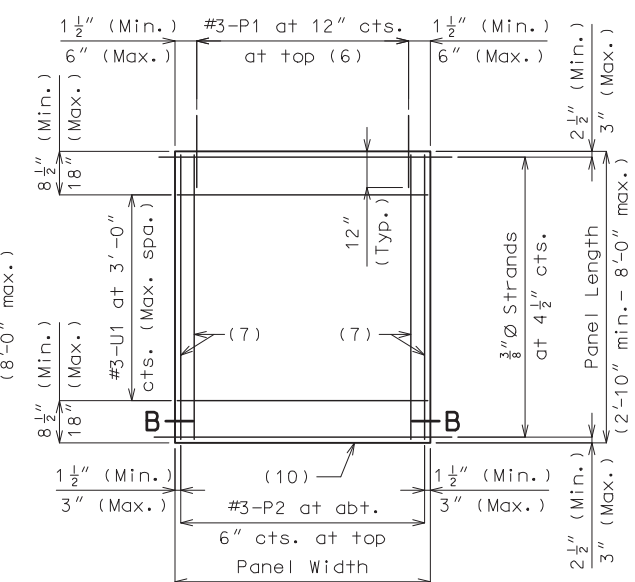


PLAN OF OPTIONAL TRUNCATED END PANEL

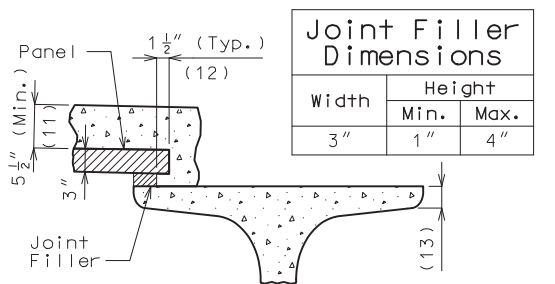
** 3" (Min.), 6" (Max.)



PLAN OF OPTIONAL SKEWED END PANEL



PLAN OF SQUARED PANEL



SECTION A-A

Joint Filler Dimensions

Width	Height	
	Min.	Max.
3"	1"	4"

Reference Notes:

Plan of Panels Placement:

- (1) S-bars shown are bottom steel in slab between panels and used with squared and truncated end panels only.
- (2) Extend S-bars 18 inches beyond the front face of end bents and int. bents for squared and truncated end panels only.
- (3) Extend S-bars 9 inches beyond edge of girder (Typ.).
- (4) End panels shall be dimensioned 1/2" min. to 1 1/2" max. from the inside face of diaphragm.
- (5) For truncated end panels, use a min. of #5-S bars at 6" crossings in openings, or min. 4x4-W7xW7.

Plans of Panels:

- (6) For end panels only, P1 bars shall be 2'-0" in length and embedded 12". P1 bars will not be required for panels at squared integral end bents.
- (7) #3-P2 bars near edge of panel at bottom (under strands).
- (8) Use #3-P3 bars if panel is skewed 45° or greater.
- (9) Any strand 2'-0" or shorter shall have a #4 reinforcing bar on each side of it, centered between strands. Strands 2'-0" or shorter may then be debonded at the fabricator's option.
- (10) Optional 1/2" x 45° Chamfer one or both sides at bottom.

Section A-A:

- (11) Slab thickness over prestressed panels varies due to girder camber. In order to maintain minimum slab thickness, it may be necessary to raise the grade uniformly throughout the structure. No payment will be made for additional labor or materials required for necessary grade adjustment.
- (12) Contractor shall ensure proper consolidation under and between panels.
- (13) At the contractor's option, the variation in slab thickness over prestressed panels may be eliminated or reduced by increasing and varying the girder top flange thickness. Dimensions shall be shown on the shop drawings.

General Notes:

Prestressed Panels:

Concrete for prestressed panels shall be Class A-1 with $f'c = 6,000$ psi, $f'ci = 4,000$ psi.

The top surface of all panels shall receive a scored finish with a depth of scoring of 1/8" perpendicular to the prestressing strands in the panels.

Prestressing tendons shall be high-tensile strength, uncoated, seven-wire, low-relaxation strands for prestressed concrete in accordance with AASHTO M 203 Grade 270, with nominal diameter of strand = 3/8" and nominal area = 0.085 sq.in. and minimum ultimate strength = 22.95 kips (270 ksi). Larger strands may be used with the same spacing and initial tension.

Initial prestressing force = 17.2 kips/strand.

The method and sequence of releasing the strands shall be shown on the shop drawings.

Suitable anchorage devices for lifting panels may be cast in panels, provided the devices are shown on the shop drawings and approved by the engineer. Panel lengths shall be determined by the contractor and shown on the shop drawings.

When squared end panels are used at skewed bents, the skewed portion shall be cast full depth. No separate payment will be made for additional concrete and reinforcing required.

Support from diaphragm forms is required under the optional skewed end until cast-in-place concrete has reached 3,000 psi compressive strength.

Prestressed panels shall be brought to saturated surface-dry (SSD) condition just prior to the deck pour. There shall be no free standing water on the panels or in the area to be cast.

The prestressed panel quantities are not included in the table of estimated quantities for the slab.

Reinforcing Steel:

All dimensions are out to out.

Hooks and bends shall be in accordance with the CRSI Manual of Standard Practice for Detailing Reinforced Concrete Structures, Stirrup and Tie Dimensions.

Minimum clearance to reinforcing steel shall be 1 1/2", unless otherwise shown.

If U1 bars interfere with placement of slab steel, U1 loops may be bent over, as necessary, to clear slab steel.

Deformed welded wire reinforcement (WWR) providing a minimum area of reinforcing perpendicular to strands of 0.22 sq in./ft, with spacing parallel to strands sufficient to ensure proper handling, may be used in lieu of the #3-P2 bars shown. Wire diameter shall not be larger than 0.375 inch. The above alternative reinforcement criteria may be used in lieu of the #3-P3 bars, when required, and placed over a width not less than 2 feet.

The following reinforcing steel shall be tied securely to the strands with the following maximum spacing in each direction:
 #3-P2 bars at 16 inches.
 WWR at 24 inches.

The #3-U1 bars shall be tied securely to #3-P2 bars, to WWR or to strands (when placed between P1 bars) at about 3-foot centers.

Minimum reinforcement steel length shall be 2'-0".

All reinforcement other than prestressing strands shall be epoxy coated.

Precast panels may be in contact with stirrup reinforcing in diaphragms.

S-bars are not listed in the bill of reinforcing.

Cost of S-bars will be considered completely covered by the contract unit price for the slab.

Joint Filler:

Joint filler shall be preformed fiber expansion joint material in accordance with Sec 1057 or expanded or extruded polystyrene bedding material in accordance with Sec 1073.

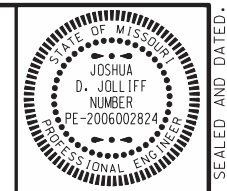
Use Slab Haunching Diagram on Sheet No. 14 for determining thickness of joint filler within the limits noted in the table of Joint Filler Dimensions.

Thicker material may be used on one or both sides of the girder to reduce cast-in-place concrete thickness to within tolerances.

The same thickness of preformed fiber expansion joint material shall be used under any one edge of any panel except at locations where top flange thickness may be stepped. The maximum change in thickness between adjacent panels shall be 1/4 inch. The polystyrene bedding material may be cut with a transition to match haunch height above top of flange.

Joint filler shall be glued to the girder. When thickness exceeds 1 1/2 inches, the joint filler shall be glued top and bottom. The glue used shall be the type recommended by the joint filler manufacturer.

Edges of panels shall be uniformly seated on the joint filler before slab reinforcement is placed.



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DATE PREPARED 3/3/2023

ROUTE 67 MO DISTRICT BR SHEET NO. 12

COUNTY BUTLER JOB NO. J9P3751 CONTRACT ID.

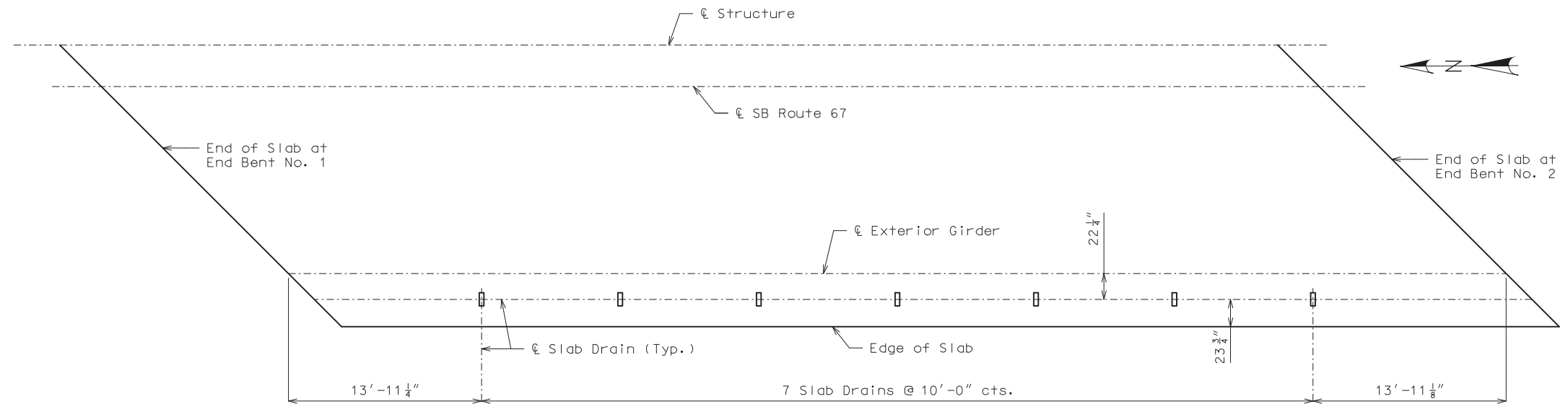
PROJECT NO. BRIDGE NO. A9279

DESCRIPTION	DATE

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION
 105 WEST CAPITOL JEFFERSON CITY, MO 65102
 1-888-ASK-MODOT (1-888-275-6636)

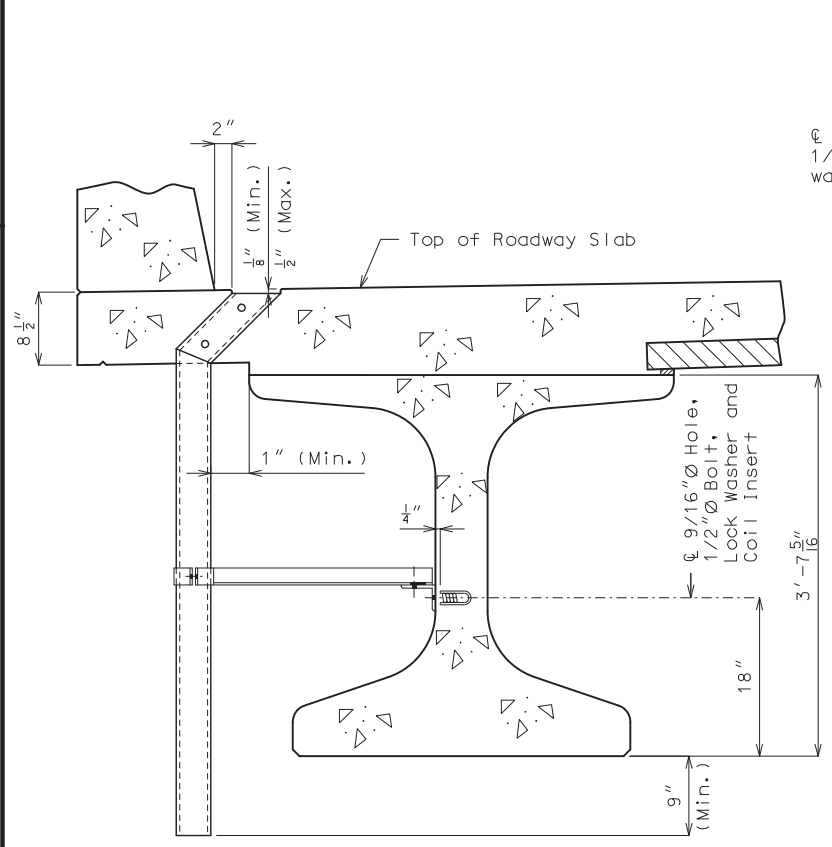


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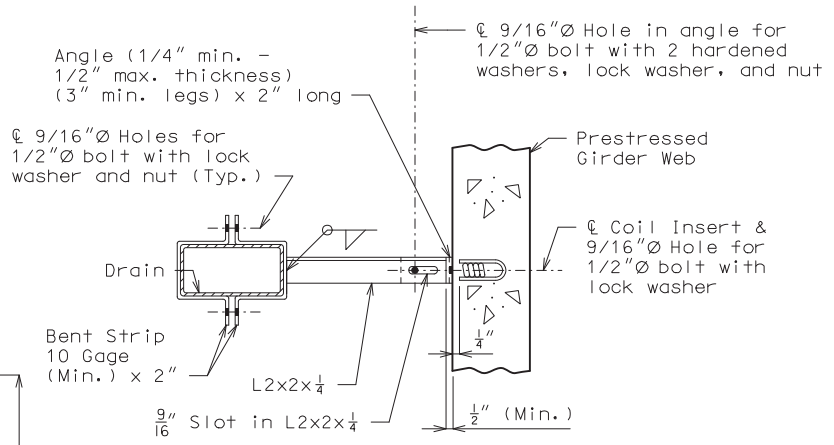


PART PLAN OF SLAB SHOWING SLAB DRAIN LOCATIONS

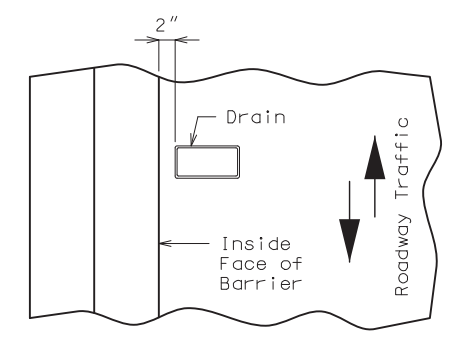
(Right side shown, left side similar)
 Note: Longitudinal dimensions are horizontal.



PART SECTION NEAR DRAIN

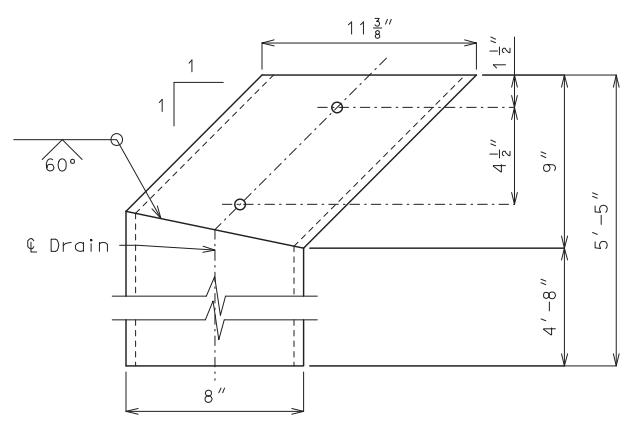


PART SECTION SHOWING BRACKET ASSEMBLY

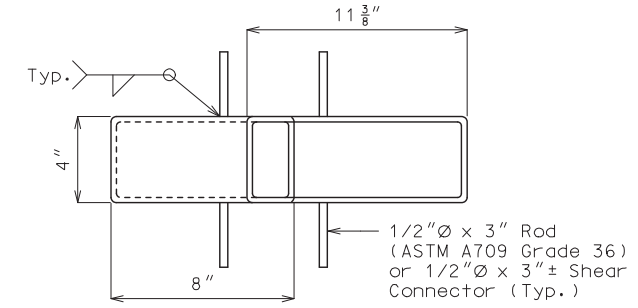


PART PLAN OF SLAB AT DRAIN

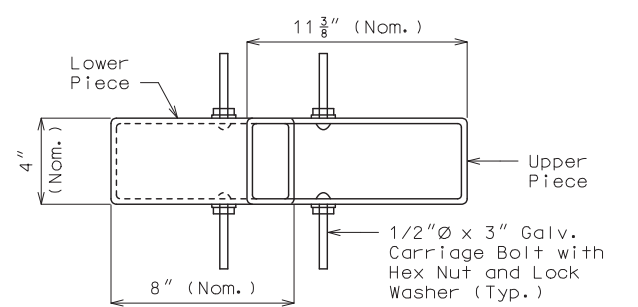
SLAB DRAINS



ELEVATION OF DRAIN



PLAN OF STEEL DRAIN OPTION



PLAN OF FRP DRAIN OPTION

General Notes:

Contractor shall have the option to construct either steel or FRP slab drains. All drains shall be of same type.

Slab drain bracket assembly shall be ASTM A709 Grade 36 steel.

Locate drains in slab by dimensions shown in Part Section Near Drain.

Reinforcing steel shall be shifted to clear drains.

The coil inserts and bracket assembly shall be galvanized in accordance with ASTM A123.

All bolts, hardened washers, lock washers and nuts shall be galvanized in accordance with AASHTO M 232 (ASTM A153), Class C.

All 1/2"Ø bolts shall be ASTM A307.

Shop drawings will not be required for the slab drains and the bracket assembly.

The coil insert required for the bracket assembly attachment shall be located on the prestressed girder shop drawings.

Coil inserts shall have a concrete pull-out strength (ultimate load) of at least 2,500 pounds in 5,000 psi concrete.

The bolt required to attach the slab drain bracket assembly to the prestressed girder web shall be supplied by the prestressed girder fabricator.

Notes for Steel Drain:

Slab drains may be fabricated of either 1/4" welded sheets of ASTM A709 Grade 36 steel or from 1/4" structural steel tubing ASTM A500 or A501.

Outside dimensions of drains are 8" x 4".

The drains shall be galvanized in accordance with ASTM A123.

Notes for FRP Drain:

Drains shall be machine filament-wound thermosetting resin tubing meeting the requirements of ASTM D2996 with the following exceptions:

Shape of drains shall be rectangular with outside nominal dimensions of 8" x 4".

Minimum reinforced wall thickness shall be 1/4 inch.

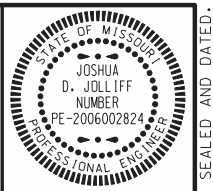
The resin used shall be ultraviolet (UV) resistant and/or have UV inhibitors mixed throughout. Drains may have an exterior coating for additional UV resistance.

The color of the slab drain shall be gray (Federal Standard 26373). The color shall be uniform throughout the resin and any coating used.

The combination of materials used in the manufacture of the drains shall be tested for UV resistance in accordance with ASTM D4329 Cycle A. The representative material shall withstand at least 500 hours of testing with only minor discoloration and without any physical deterioration. The contractor shall furnish the results of the required ultraviolet testing prior to acceptance of the slab drains.

At the contractor's option, drains may be field cut. The method of cutting FRP slab drain shall be as recommended by the manufacturer to ensure a smooth, chip free cut.

Both upper and lower drain pieces shall be rigidly connected to each other. Drain flow shall not be obstructed. Approval of the engineer is required.



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DATE PREPARED
 3/3/2023

ROUTE STATE
 67 MO

DISTRICT SHEET NO.
 BR 13

COUNTY
 BUTLER

JOB NO.
 J9P3751

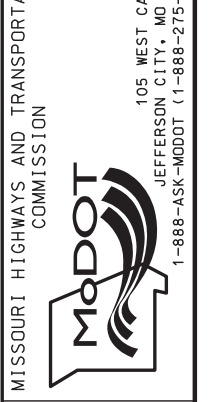
CONTRACT ID.

PROJECT NO.

BRIDGE NO.
 A9279

DESCRIPTION	DATE

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

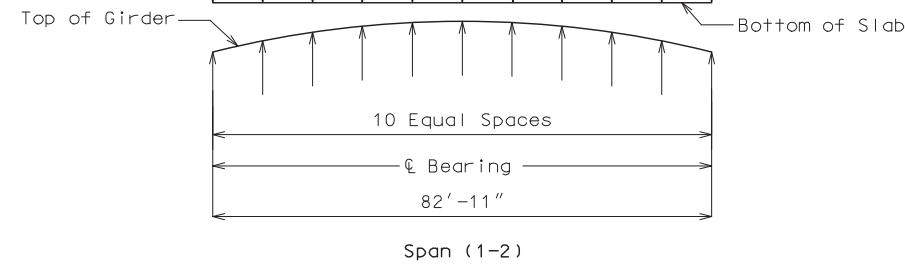


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 ENGINEERING CORPORATION - 000631

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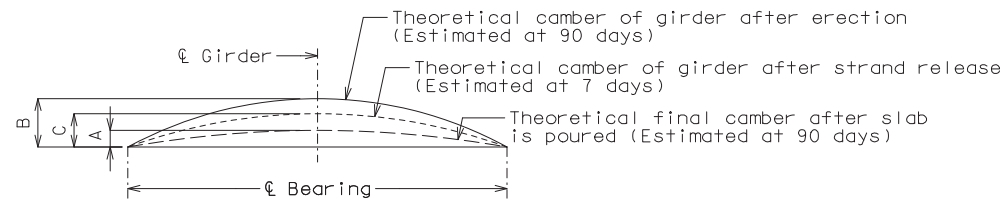
Girder No. 1	3 3/8"	2 1/2"	2 1/2"	1 3/4"	1 1/2"	1 1/2"	1 1/2"	1 1/2"	1 3/4"	2 1/2"	2 1/2"	3 3/8"
Girder No. 2	3 1/4"	2 1/2"	2 1/2"	1 3/4"	1 1/2"	1 1/2"	1 1/2"	1 1/2"	1 3/4"	2 1/2"	2 1/2"	3 1/4"
Girder No. 3	3 1/4"	2 1/2"	2 1/2"	1 3/4"	1 1/2"	1 1/2"	1 1/2"	1 1/2"	1 3/4"	2 1/2"	2 1/2"	3 1/4"
Girder No. 4	3 3/8"	2 1/2"	2 1/2"	1 3/4"	1 1/2"	1 1/2"	1 1/2"	1 1/2"	1 3/4"	2 1/2"	2 1/2"	3 3/8"



THEORETICAL SLAB HAUNCHING DIAGRAM

If girder camber is different from that shown in the camber diagram, in order to maintain minimum slab thickness, an adjustment of the slab haunches, an increase in slab thickness or a raise in grade uniformly throughout the structure shall be necessary. No payment will be made for additional labor or materials required for variation in haunching, slab thickness or grade adjustment.

Concrete in the slab haunches is included in the Estimated Quantities for Slab on Concrete NU-Girder.



Girder	Span (1-2)		
	A	B	C
Exterior	1 7/8"	3 1/4"	1 7/8"
Interior	1 3/8"		

GIRDER CAMBER DIAGRAM

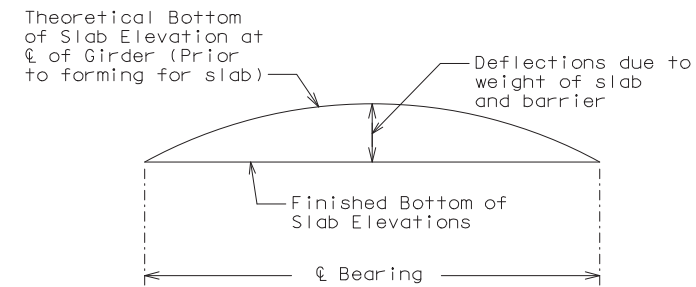
Conversion factors for girder camber (Estimated at 90 days):

- 0.1 pt. = 0.314 x 0.5 pt.
- 0.2 pt. = 0.593 x 0.5 pt.
- 0.3 pt. = 0.813 x 0.5 pt.
- 0.4 pt. = 0.952 x 0.5 pt.

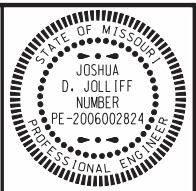
Theoretical Bottom of Slab Elevations at Centerline of Girder (Prior to forming for slab) (Estimated at 90 days)

Girder Number	Span (1-2) (82'-11" ℄ Brg. - ℄ Brg.)										
	℄ Brg.	.10	.20	.30	.40	.50	.60	.70	.80	.90	℄ Brg.
1	307.92	307.96	307.98	307.99	307.98	307.96	307.93	307.89	307.84	307.78	307.69
2	308.12	308.16	308.17	308.17	308.16	308.14	308.12	308.08	308.03	307.97	307.89
3	307.97	308.01	308.02	308.02	308.01	307.99	307.97	307.93	307.88	307.82	307.74
4	307.71	307.75	307.77	307.78	307.77	307.75	307.72	307.68	307.63	307.57	307.48

Elevations are based on a constant slab thickness of 8 1/2" and include allowance for theoretical dead load deflections due to weight of slab (including precast panel) and barrier.



TYPICAL SLAB ELEVATIONS DIAGRAM



THIS SHEET HAS BEEN SIGNED, SEALED AND DATED ELECTRONICALLY.

DATE PREPARED
3/3/2023

ROUTE 67 STATE MO

DISTRICT BR SHEET NO. 14

COUNTY BUTLER

JOB NO. J9P3751

CONTRACT ID.

PROJECT NO.

BRIDGE NO. A9279

DATE	DESCRIPTION

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

MoDOT

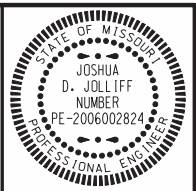
105 WEST CAPITOL JEFFERSON CITY, MO 65102
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ENGINEERING CORPORATION - 000631

TBOS, HAUNCHING AND CAMBER

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DATE PREPARED
3/3/2023

ROUTE STATE
67 MO
DISTRICT SHEET NO.
BR 15

COUNTY
BUTLER
JOB NO.
J9P3751
CONTRACT ID.

PROJECT NO.
BRIDGE NO.
A9279

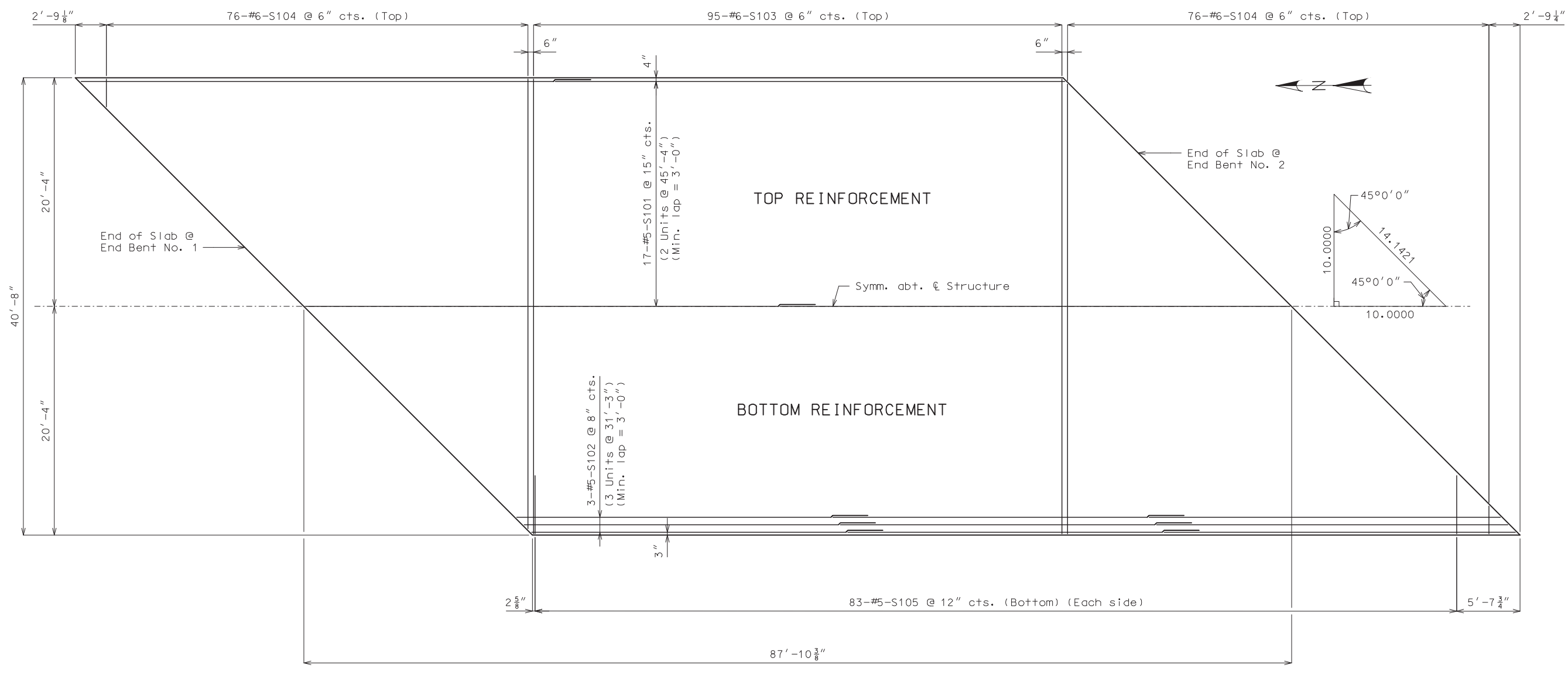
DATE	DESCRIPTION

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

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PLAN OF SLAB SHOWING REINFORCEMENT

SLAB REINFORCEMENT DETAILS

General Notes:

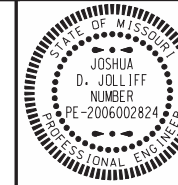
- Longitudinal slab dimensions shown are horizontally.
- For details of Precast Prestressed Panels, see Sheet No. 12.
- For Theoretical Bottom of Slab Elevations, Girder Camber Diagram, and Theoretical Slab Haunching Diagram, see Sheet No. 14.
- For Section Thru Slab, see Sheet No. 16.
- For details and reinforcement of Type D Barrier not shown, see Sheets No. 17 and 18.

Detailed Oct. 2022
Checked Oct. 2022

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 15 of 24

IF A SEAL IS PRESENT ON THIS SHEET IT HAS BEEN ELECTRONICALLY SEALED AND DATED.



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DATE PREPARED
3/3/2023

ROUTE 67 STATE MO

DISTRICT BR SHEET NO. 16

COUNTY BUTLER

JOB NO. J9P3751

CONTRACT ID.

PROJECT NO.

BRIDGE NO. A9279

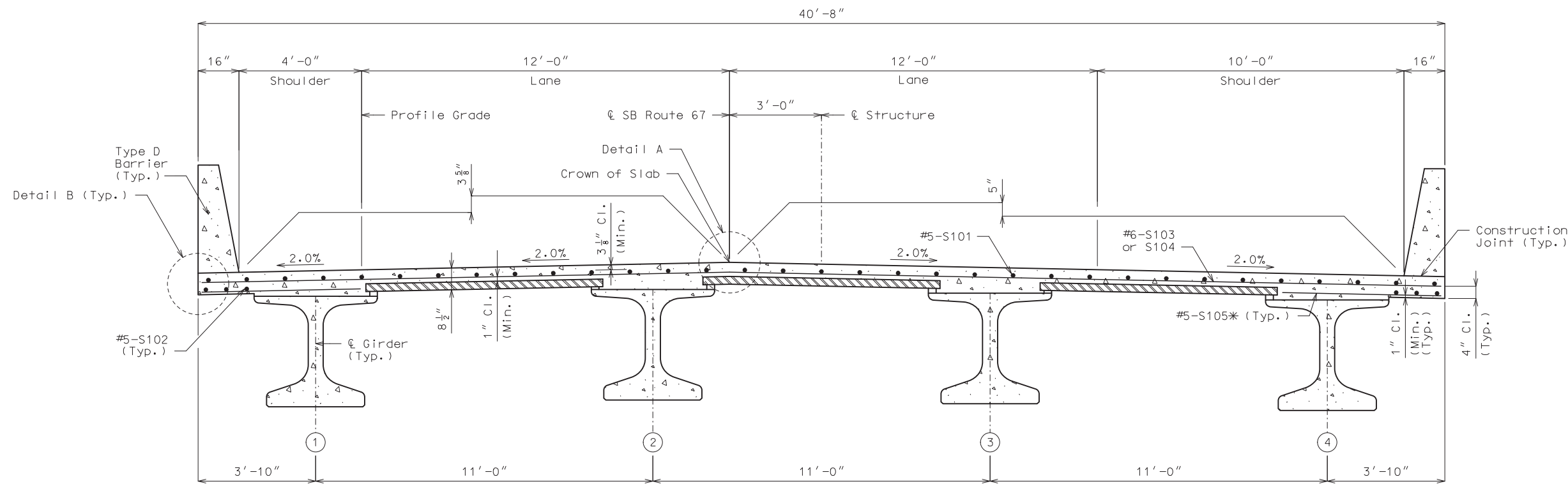
DATE	DESCRIPTION

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION
 105 WEST CAPITOL
 JEFFERSON CITY, MO 65102
 1-888-ASK-MODOT (1-888-275-6636)



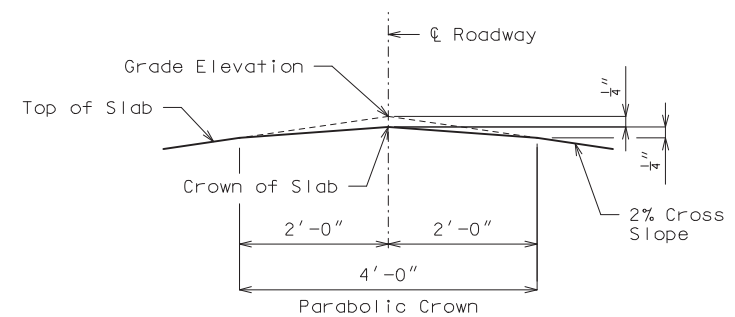
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 ENGINEERING CORPORATION - 000631

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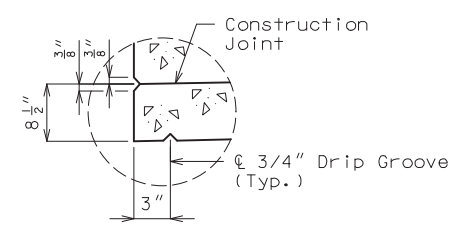


SECTION THRU SLAB

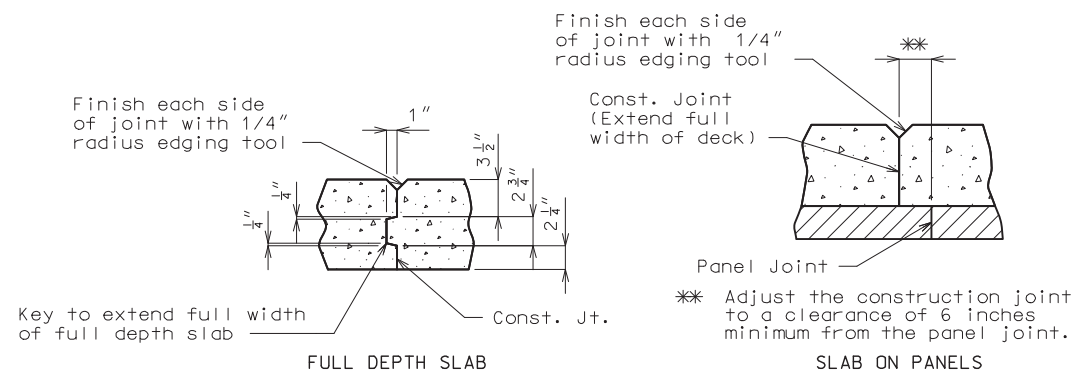
* Alternate bar shape available, see barrier sheets.



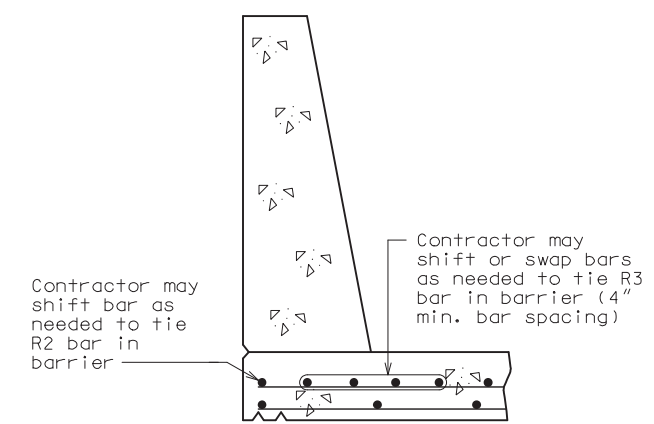
DETAIL A



DETAIL B



SLAB CONSTRUCTION JOINT
(If needed)



OPTIONAL SHIFTING TOP BARS AT BARRIER

General Notes:

- For details and reinforcement of Type D Barrier not shown, see Sheets No. 17 & 18.
- For details of Precast Prestressed Panels, see Sheet No. 12.
- For Theoretical Bottom of Slab Elevations, Girder Camber Diagram and Theoretical Slab Haunching Diagram, see Sheet No. 14.
- For Plan of Slab Showing Reinforcement, see Sheet No. 15.
- The contractor shall pour and satisfactorily finish the roadway slab at a rate of not less than 25 cubic yards per hour.
- The contractor shall furnish an approved retarder to retard the set of the concrete to 2.5 hours and shall pour and satisfactorily finish the slab pours at the rate given.
- The concrete diaphragm at the integral end bents shall be poured a minimum of 30 minutes and a maximum of 2 hours before the slab is poured.

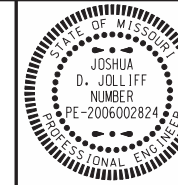
DETAILS OF SLAB REINFORCING

Detailed Oct. 2022
 Checked Oct. 2022

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 16 of 24

REV.



THIS SHEET HAS BEEN SIGNED, SEALED AND DATED ELECTRONICALLY.

DATE PREPARED
3/3/2023

ROUTE 67 STATE MO

DISTRICT BR SHEET NO. 17

COUNTY BUTLER

JOB NO. J9P3751

CONTRACT ID.

PROJECT NO.

BRIDGE NO. A9279

DESCRIPTION

DATE

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

105 WEST CAPITOL JEFFERSON CITY, MO 65102

1-888-ASK-MODOT (1-888-275-6636)

MoDOT

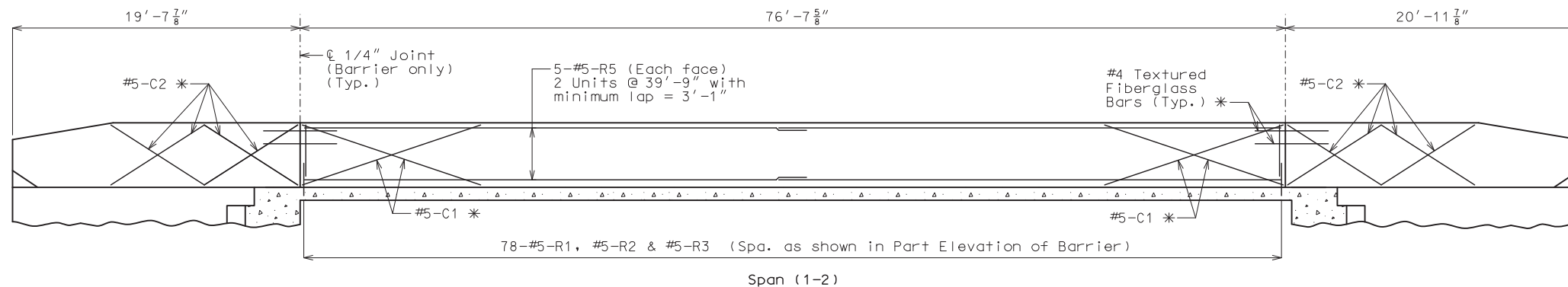
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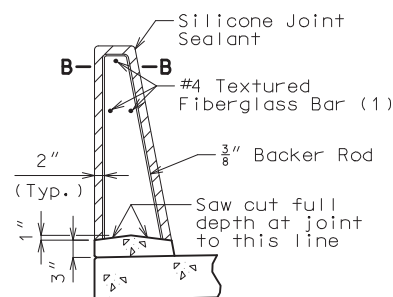
ENGINEERING CORPORATION - 000631

REV.

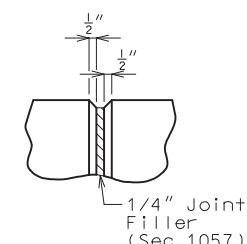


ELEVATION OF BARRIER

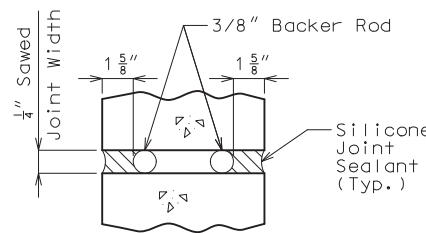
(Left barrier shown, right barrier similar by 180° rotation)
Longitudinal dimensions are horizontal.



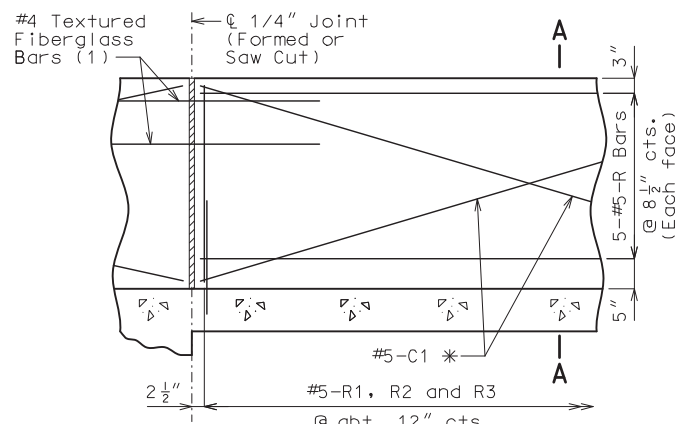
SECTION THRU SAW CUT JOINT



PART ELEVATION AT FORMED JOINT

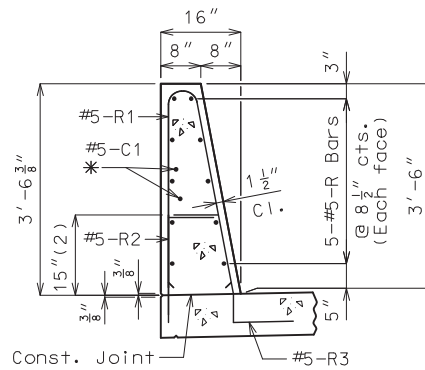


SECTION B-B



PART ELEVATION OF BARRIER

(1) Four feet long, centered on joint, slip-formed option only

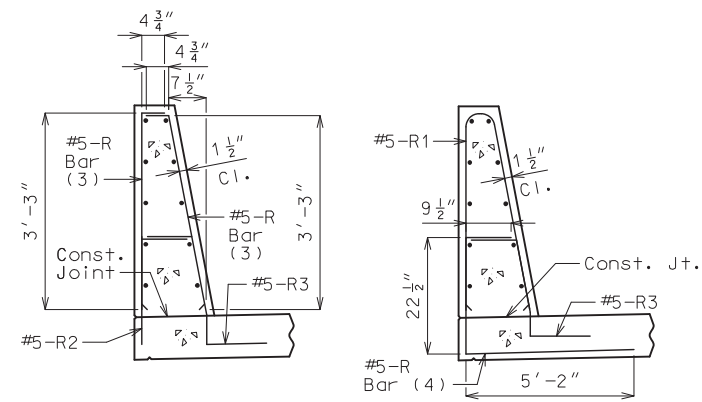


SECTION A-A

Use a minimum lap of 3'-1" for #5 horizontal barrier bars.

The cross-sectional area above the slab is 3.52 square feet.

(2) To top of bar



R-BAR PERMISSIBLE ALTERNATE SHAPE

(3) The R1 bar may be separated into two bars as shown, at the contractor's option, only when slip forming is not used. (All dimensions are out to out.)

(4) The R2 bar and #5 bottom transverse slab bar in cantilever (prestressed panels only) combination may be furnished as one bar as shown, at the contractor's option.

TYPE D BARRIER

General Notes:

* Slip-formed option only.

Conventional forming or slip forming may be used. Saw cut joints may be used with conventional forming.

Top of barrier shall be built parallel to grade and barrier joints (except at end bents) normal to grade.

All exposed edges of barrier shall have either a 1/2-inch radius or a 3/8-inch bevel, unless otherwise noted.

Payment for all concrete and reinforcement, complete in place, will be considered completely covered by the contract unit price for Type D Barrier per linear foot.

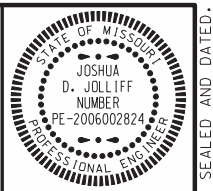
Concrete in barrier shall be Class B-1.

Measurement of barrier is to the nearest linear foot for each structure, measured along the outside top of slab from end of wing to end of wing.

Concrete traffic barrier delineators shall be placed on top of the barrier as shown on Missouri Standard Plan 617.10 and in accordance with Sec 617. Delineators on bridges with two-lane, two-way traffic shall have retroreflective sheeting on both sides. Concrete traffic barrier delineators will be considered completely covered by the contract unit price for Type D Barrier.

Joint sealant and backer rods shall be in accordance with Sec 717 for silicone joint sealant for saw cut and formed joints.

For slip-formed option, both sides of barrier shall have a vertically broomed finish and the top shall have a transversely broomed finish.



THIS SHEET HAS BEEN SIGNED, SEALED AND DATED ELECTRONICALLY.

DATE PREPARED 3/3/2023

ROUTE 67 MO DISTRICT BR SHEET NO. 18

COUNTY BUTLER JOB NO. J9P3751 CONTRACT ID.

PROJECT NO. BRIDGE NO. A9279

DESCRIPTION	DATE

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

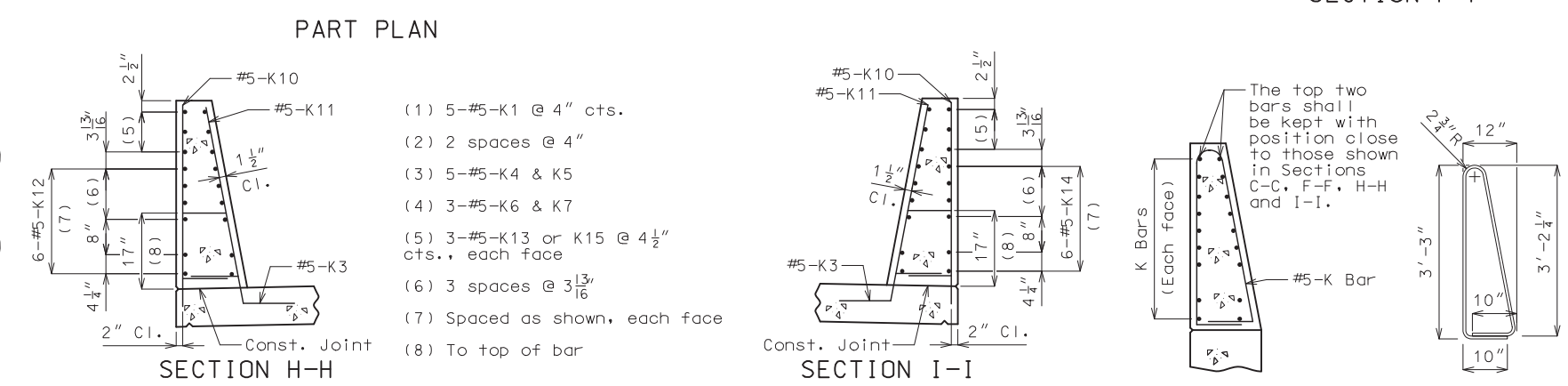
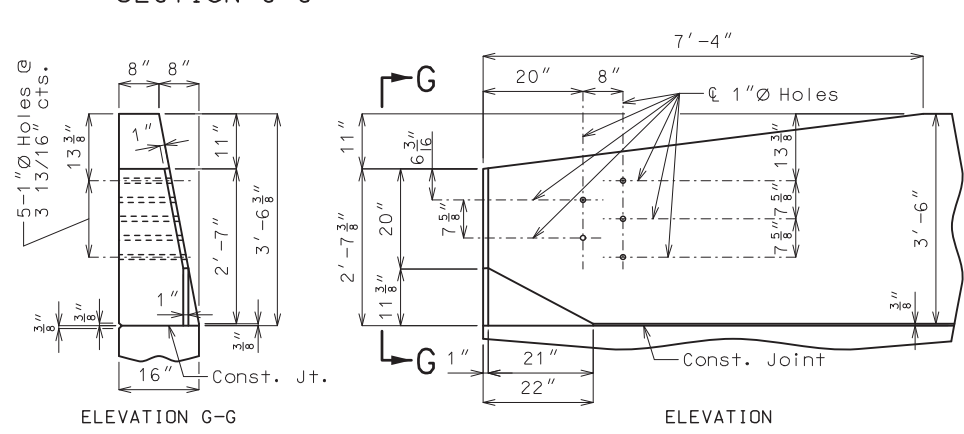
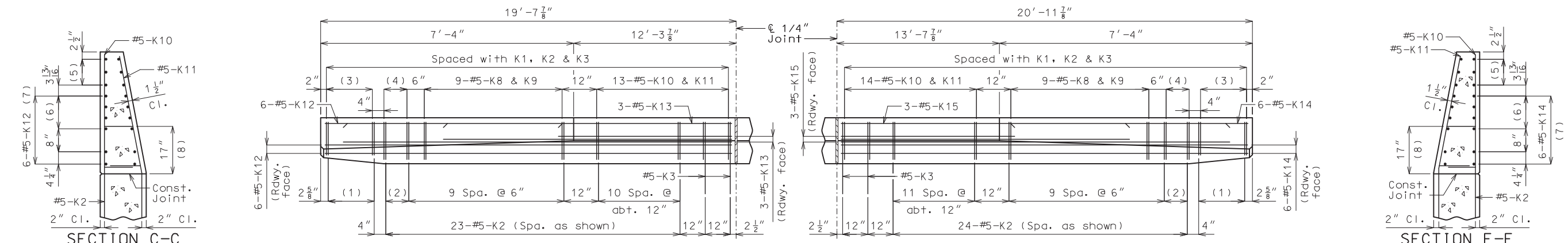
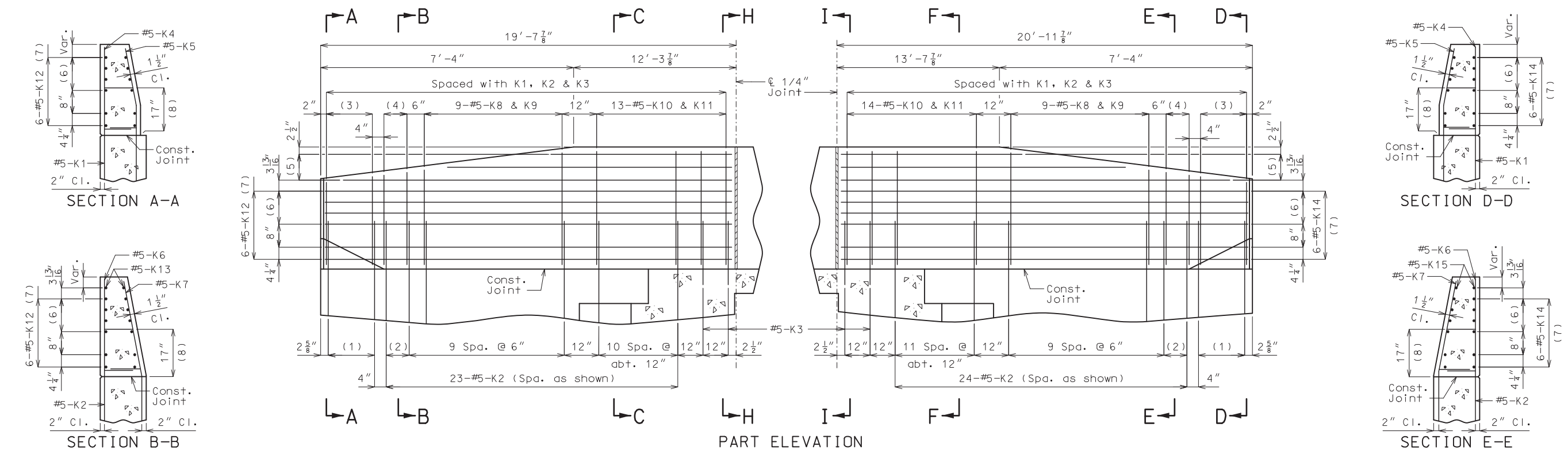
105 WEST CAPITOL JEFFERSON CITY, MO 65102 1-888-ASK-MODOT (1-888-275-6636)



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General Notes:

Concrete traffic barrier delineators shall be placed on top of the barrier as shown on Missouri Standard Plan 617.10 and in accordance with Sec 617. Delineators on bridges with two-lane, two-way traffic shall have retroreflective sheeting on both sides. Concrete traffic barrier delineators will be considered completely covered by the contract unit price for Type D Barrier.

Reinforcing Steel:

Minimum clearance to reinforcing steel shall be 1 1/2\"/>

K10-K11 BAR PERMISSIBLE ALTERNATE SHAPE

(Other K bars not shown for clarity)

The K10-K11 bar combination may be furnished as one bar as shown, at the contractor's option.

All dimensions are out to out.

TYPE D BARRIER AT END BENTS

(Left barrier shown, right barrier similar by 180° rotation)

Detailed Oct. 2022
Checked Oct. 2022

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 18 of 24

General Notes:

All concrete for the bridge approach slab and sleeper slab shall be in accordance with Sec 503 ($f'c = 4,000$ psi).
 The reinforcing steel in the bridge approach slab and the sleeper slab shall be epoxy coated Grade 60 with $f_y = 60,000$ psi.

Drain pipe may be either 6" diameter corrugated metallic-coated pipe underdrain, 4" diameter corrugated polyvinyl chloride (PVC) drain pipe, or 4" diameter corrugated polyethylene (PE) drain pipe.

Minimum clearance to reinforcing steel shall be 1 1/2", unless otherwise shown.

The reinforcing steel in the bridge approach slab and the sleeper slab shall be continuous. The transverse reinforcing steel may be made continuous by providing a minimum lap splice of 29 inches for #5 bars and 44 inches for #6 bars, or by mechanical bar splice.

Mechanical bar splices shall be in accordance with Sec 710.

All joint filler shall be in accordance with Sec 1057 for preformed fiber expansion joint filler except as noted.

The contractor shall pour and satisfactorily finish the bridge slab before placing the bridge approach slab.

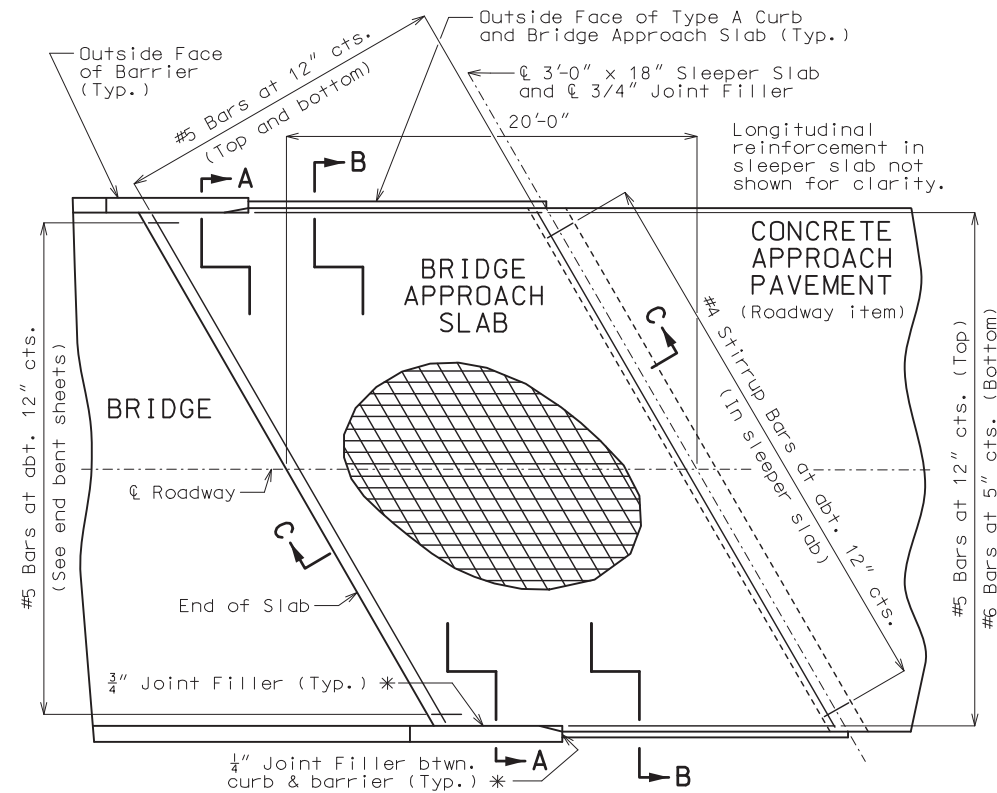
Longitudinal construction joints in approach slab and sleeper slab shall be aligned with longitudinal construction joints in bridge slab.

For concrete approach pavement details, see roadway plans.

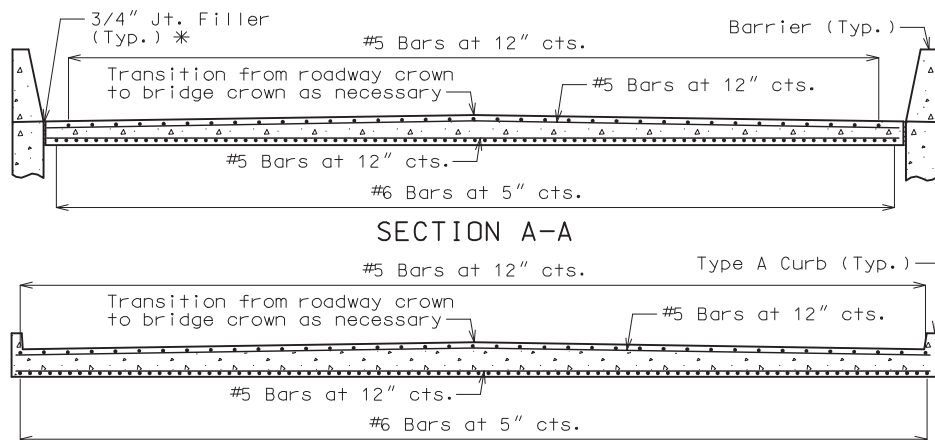
See Missouri Standard Plan 609.00 for details of Type A curb.

Payment for furnishing all materials, labor and excavation necessary to construct the approach slab, including the timber header, sleeper slab, underdrain, Type 5 aggregate base, joint filler and all other appurtenances and incidental work as shown on this sheet, complete in place, will be considered completely covered by the contract unit price for Bridge Approach Slab (Major) per square yard.

* Seal joint between vertical face of approach slab and wing with sealant in accordance with Sec 717 for silicone joint sealant for saw cut and formed joints.



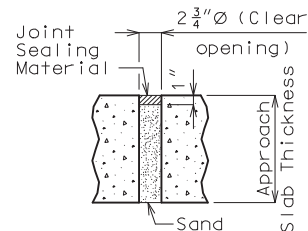
PART PLAN SHOWING REINFORCEMENT



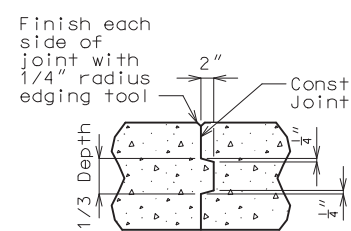
SECTION A-A

SECTION B-B

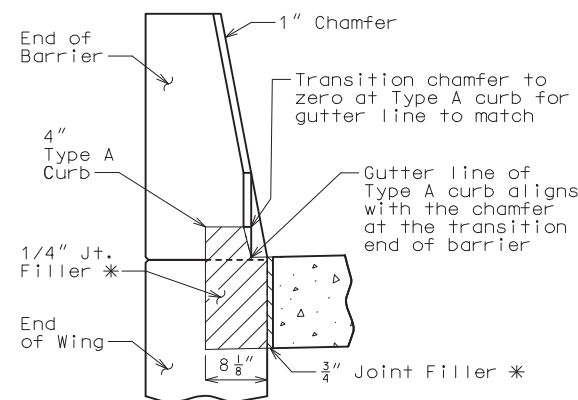
With the approval of the engineer, the contractor may crown the bottom of the approach slab to match the crown of the roadway surface.



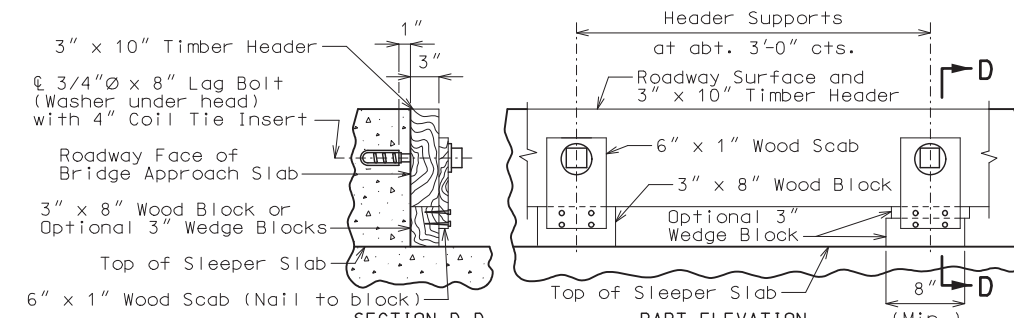
UNDERSEAL ACCESS HOLE DETAIL
(If required)



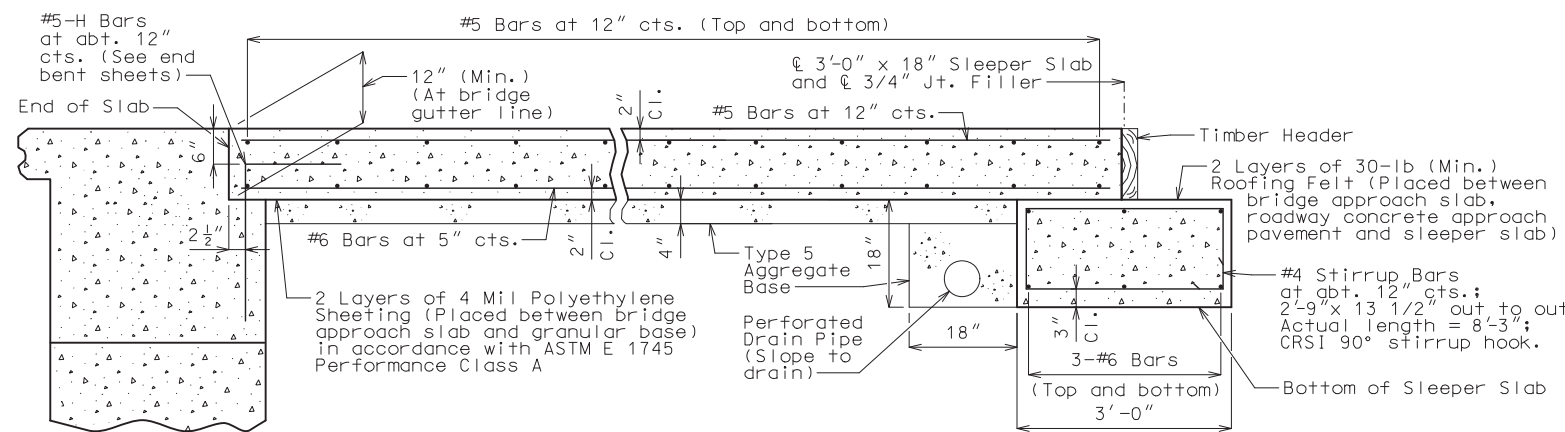
CONSTRUCTION JOINT DETAIL
(If required)



SECTION BETWEEN CURB AND BARRIER



SECTION D-D DETAILS OF TIMBER HEADER



SECTION C-C

BRIDGE APPROACH SLAB (MAJOR)

Detailed Oct. 2022
 Checked Oct. 2022

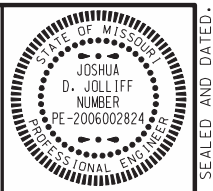
Note: This drawing is not to scale. Follow dimensions.

Sheet No. 19 of 24

pw:\cmtengr-pw.bentley.com\cmt-projects\Documents\Projects\MoDOT\120040908\Draw\Structures\Sheets\Harviell Ditch\A9279\B_A9279_019_J9P3751_Approach_Slab.dgn

10.08.22 AM

3/3/2023



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DATE PREPARED 3/3/2023

ROUTE 67 STATE MO

DISTRICT BR SHEET NO. 19

COUNTY BUTLER

JOB NO. J9P3751

CONTRACT ID.

PROJECT NO.

BRIDGE NO. A9279

DESCRIPTION

DATE

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

105 WEST CAPITOL JEFFERSON CITY, MO 65102

1-888-ASK-MODOT (1-888-275-6636)

MoDOT

ENGINEERING CORPORATION - 000631

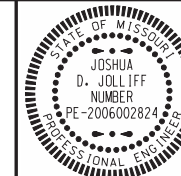
CRAWFORD, MURPHY & TILLY, INC.

6831 WESLEYAN DRIVE

SPRINGFIELD, MO 65807 (417) 869-6009

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DATE PREPARED
3/3/2023

ROUTE STATE
67 MO

DISTRICT SHEET NO.
BR 22

COUNTY

BUTLER

JOB NO.

J9P3751

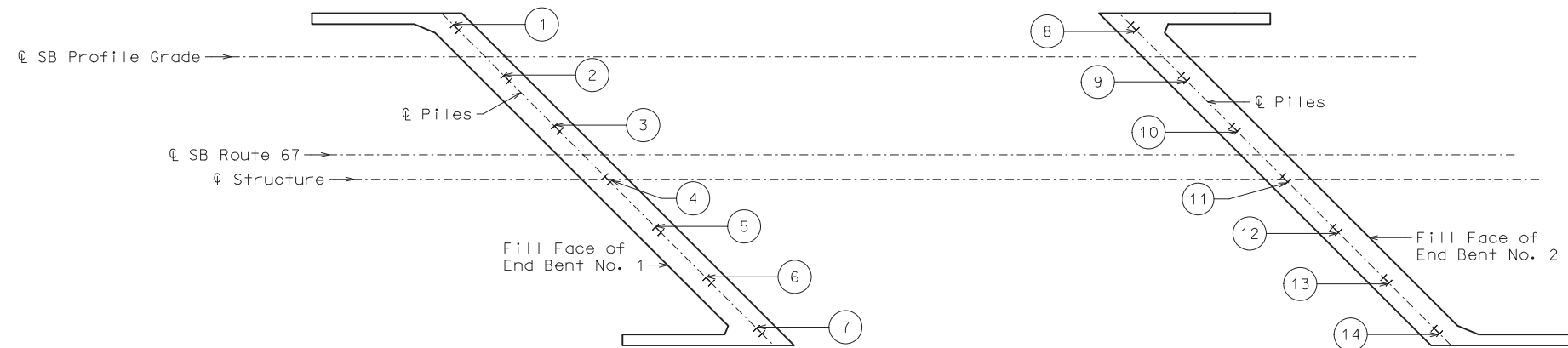
CONTRACT ID.

PROJECT NO.

BRIDGE NO.

A9279

DATE	DESCRIPTION



PART PLAN SHOWING PILE NUMBERING FOR RECORDING AS-BUILT PILE DATA

As-Built Pile Data			
Pile No.	Length in Place (ft)	Computed Nominal Axial Compressive Resistance (kips)	Remarks
End Bent No. 1			
1			
2			
3			
4			
5			
6			
7			

As-Built Pile Data			
Pile No.	Length in Place (ft)	Computed Nominal Axial Compressive Resistance (kips)	Remarks
End Bent No. 2			
8			
9			
10			
11			
12			
13			
14			

Note:
Indicate in remarks column:
A. Pile type and grade
B. Batter
C. Driven to practical refusal

This sheet to be completed by MoDOT construction personnel.

AS-BUILT PILE DATA

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 22 of 24

Detailed Oct. 2022
Checked Oct. 2022



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REV.



Bacon Farmer Workman Engineering & Testing Inc.
500 S 17th St
Paducah, KY 42003
Telephone: 2704431995
Fax: 2704431904

BORING NUMBER B-201
PAGE 1 OF 2

CLIENT MoDOT PROJECT NAME Route 67 over Harviell Ditch
PROJECT NUMBER 20284 PROJECT LOCATION Butler County, Missouri
DATE STARTED 12/7/21 COMPLETED 12/7/21 GROUND ELEVATION 303.7 ft. HOLE SIZE 6.25 inches
DRILLING CONTRACTOR Smith & Co. DRILLED BY Smith & Co GROUND WATER LEVELS:
DRILLING METHOD Hollow Stem Auger and Mud Rotary (CME-750) AT TIME OF DRILLING --
LOGGED BY NB CHECKED BY CM AT END OF DRILLING --
STATION 758+75 OFFSET 26' RT AFTER DRILLING --

DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY % (ROD)	BLOW COUNTS (N VALUE)	POCKET PEN. (tsf)	DRY UNIT WT. (pcf)	MOISTURE CONTENT (%)	ATTERBERG LIMITS			FINES CONTENT (%)
									LIQUID LIMIT	PLASTIC LIMIT	PLASTICITY INDEX	
0		TOPSOIL - 6 inches										
		(CL) SANDY LEAN CLAY: Light gray, moist, stiff to hard	SPT SS1	100	5-6-7 (13)			4				45
			SPT SS2	72	6-11-15 (26)			10				52
			SPT SS3	94	9-17-17 (34)			6				28
			SPT SS4	89	5-6-8 (14)			10				17
		(SP) SAND: Gray to grayish brown, moist, very loose to medium dense	SPT SS5	83	4-7-4 (11)			24				24
			SPT SS6	89	1-2-1 (3)			21				16
			SPT SS7	44	5-2-2 (4)			23				31
			SPT SS8	67	5-10-9 (19)			20				22
			SPT SS9	67	8-9-7 (16)			23				28
			SPT SS10	61	9-12-14 (26)			21				21
			SPT SS11	61	7-7-12 (19)			22				23
			SPT SS12	0	8-5-5 (10)							

(Continued Next Page)

B-201



Bacon Farmer Workman Engineering & Testing Inc.
500 S 17th St
Paducah, KY 42003
Telephone: 2704431995
Fax: 2704431904

BORING NUMBER B-201
PAGE 2 OF 2

CLIENT MoDOT PROJECT NAME Route 67 over Harviell Ditch
PROJECT NUMBER 20284 PROJECT LOCATION Butler County, Missouri

DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY % (ROD)	BLOW COUNTS (N VALUE)	POCKET PEN. (tsf)	DRY UNIT WT. (pcf)	MOISTURE CONTENT (%)	ATTERBERG LIMITS			FINES CONTENT (%)
									LIQUID LIMIT	PLASTIC LIMIT	PLASTICITY INDEX	
		(SP) SAND: Gray to grayish brown, moist, very loose to medium dense (continued)						16				31
		(SP) GRAVELLY SAND: Graysih brown, moist, medium dense	SPT SS13	56	9-8-8 (16)			14				35
			SPT SS14	39	10-11-9 (20)			15				
			SPT SS15	6	13-10-9 (19)			13				
			SPT SS16	39	9-10-11 (21)			20				30
		(SP) SAND: Grayish brown, moist, medium dense	SPT SS17	44	11-11-12 (23)			13				
		(SP) GRAVELLY SAND: Graysih brown, moist, very dense Refusal at 80.1 feet. Bottom of borehole at 80.1 feet.	SPT SS18	83	12-50/1"							

B-201

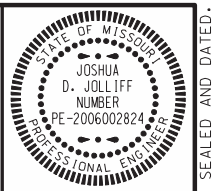
BORING DATA

Note: For locations of borings, see Sheet No. 1.

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 23 of 24

Detailed Oct. 2022
Checked Oct. 2022



THIS SHEET HAS BEEN SIGNED, SEALED AND DATED ELECTRONICALLY.

DATE PREPARED 3/3/2023

ROUTE 67 STATE MO
DISTRICT BR SHEET NO. 23

COUNTY BUTLER
JOB NO. J9P3751
CONTRACT ID.

PROJECT NO.
BRIDGE NO. A9279

DATE	DESCRIPTION

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION
105 WEST CAPITOL JEFFERSON CITY, MO 65102
1-888-ASK-MODOT (1-888-275-6636)

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ENGINEERING CORPORATION - 000631

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Bacon Farmer Workman Engineering & Testing Inc.
500 S 17th St
Paducah, KY 42003
Telephone: 2704431995
Fax: 2704431904

BORING NUMBER B-202
PAGE 1 OF 2

CLIENT MoDOT PROJECT NAME Route 67 over Harviell Ditch
PROJECT NUMBER 20284 PROJECT LOCATION Butler County, Missouri
DATE STARTED 12/14/21 COMPLETED 12/15/21 GROUND ELEVATION 304.8 ft HOLE SIZE 6.25 inches
DRILLING CONTRACTOR Smith & Co. DRILLED BY Smith & Co GROUND WATER LEVELS:
DRILLING METHOD Hollow Stem Auger and Mud Rotary (CME-750) ∇ AT TIME OF DRILLING 19.50 ft / Elev 285.30 ft
LOGGED BY NB CHECKED BY CM AT END OF DRILLING --
STATION 759+88 OFFSET 44' RT AFTER DRILLING --

DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (tsf)	DRY UNIT WT. (pcf)	MOISTURE CONTENT (%)	ATTERBERG LIMITS			FINES CONTENT (%)
									LIQUID LIMIT	PLASTIC LIMIT	PLASTICITY INDEX	
0		TOPSOIL - 6 inches (CL) SANDY LEAN CLAY: Grayish brown, moist, soft to medium stiff	SPT SS1	61	1-2-3 (5)			15				57
			SPT SS2	100	1-1-1 (2)			17				61
			SPT SS3	100	1-3-5 (8)			16				55
10		(SP-SC) SAND WITH CLAY: Light brownish gray to gray, moist, medium dense	SPT SS4	92	3-5-7 (12)			13				38
			SPT SS5	89	5-9-12 (21)			19				39
20			SPT SS6	86	5-8-10 (18)			19				37
		(SP) SAND: Gray, moist, loose to medium dense	SPT SS7	44	1-2-3 (5)			20				60
			SPT SS8	56	5-6-7 (13)			19				43
			SPT SS9	61	8-8-8 (16)			20				51
40		(SP) GRAVELLY SAND: Gray, moist, medium dense to dense	SPT SS10	67	5-8-12 (20)			19				39
			SPT SS11	64	4-3-10 (18)			20				24
50			SPT SS12	58	6-9-8 (17)			19				24

(Continued Next Page)

B-202



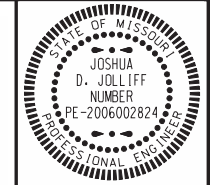
Bacon Farmer Workman Engineering & Testing Inc.
500 S 17th St
Paducah, KY 42003
Telephone: 2704431995
Fax: 2704431904

BORING NUMBER B-202
PAGE 2 OF 2

CLIENT MoDOT PROJECT NAME Route 67 over Harviell Ditch
PROJECT NUMBER 20284 PROJECT LOCATION Butler County, Missouri

DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (tsf)	DRY UNIT WT. (pcf)	MOISTURE CONTENT (%)	ATTERBERG LIMITS			FINES CONTENT (%)
									LIQUID LIMIT	PLASTIC LIMIT	PLASTICITY INDEX	
		(SP) GRAVELLY SAND: Gray, most, medium dense to dense (continued)	SPT SS13	0	4-6-6 (12)			18				21
60			SPT SS14	72	6-9-12 (21)			15				24
			SPT SS15	100	5-10-11 (21)			19				19
70			SPT SS16	61	8-13-12 (25)			17				24
			SPT SS17	72	8-9-15 (24)			20				23
80			SPT SS18	64	11-15-17 (32)			17				21
		Refusal at 83.0 feet. Bottom of borehole at 83.0 feet.										

B-202



THIS SHEET HAS BEEN SIGNED, SEALED AND DATED ELECTRONICALLY.

DATE PREPARED 3/3/2023
ROUTE 67 STATE MO
DISTRICT BR SHEET NO. 24
COUNTY BUTLER
JOB NO. J9P3751
CONTRACT ID.
PROJECT NO.
BRIDGE NO. A9279

DATE	DESCRIPTION

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

 105 WEST CAPITOL JEFFERSON CITY, MO 65102
 1-888-ASK-MODOT (1-888-275-6636)

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 ENGINEERING CORPORATION - 000631

Detailed Oct. 2022
Checked Oct. 2022

Note: This drawing is not to scale. Follow dimensions.

BORING DATA
Note: For locations of borings, see Sheet No. 1.

Sheet No. 24 of 24

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REV.

(82.9') PRESTRESSED CONCRETE NU-GIRDER SPAN

Notes:

Roadway fill shall be completed to the final roadway section and up to the elevation of the bottom of the concrete beam within the limits of the structure and for not less than 25 feet in back of the fill face of the end bents before any piles are driven for any bents falling within the embankment section.

Hydrologic Data	
Drainage Area = 9.3 mi ²	
Design Flood Frequency = 50 years	
Design Flood Discharge = 1,060 cfs	
Design Flood (D.F.) Elevation = 302.8	
Base Flood (100-year)	
Base Flood Elevation = 303.3	
Base Flood Discharge = 1,210 cfs	
Estimated Backwater = 0.1 ft	
Average Velocity thru Opening = 2.8 ft/s	
Freeboard (50-year)	
Freeboard = 1.2 ft	
Roadway Overtopping	
Overtopping Flood Discharge = N/A	
Overtopping Flood Frequency = >500 years	
500 Year Flood Elevation = 304.5	

⊙ Indicates location of borings.

Notice and Disclaimer Regarding Boring Log Data

The locations of all subsurface borings for this structure are shown on the plan sheet(s) for this structure. The boring data for all locations indicated, as well as any other boring logs or other factual records of subsurface data and investigations performed by the department for the design of the project, are shown on Sheet(s) No. 23 & 24 and may be included in the Electronic Bridge Deliverables. They will also be available from the Project Contact upon written request. No greater significance or weight should be given to the boring data depicted on the plan sheets than is given to the subsurface data available from the district or elsewhere.

The Commission does not represent or warrant that any such boring data accurately depicts the conditions to be encountered in constructing this project. A contractor assumes all risks it may encounter in basing its bid prices, time or schedule of performance on the boring data depicted here or those available from the district, or on any other documentation not expressly warranted, which the contractor may obtain from the Commission.

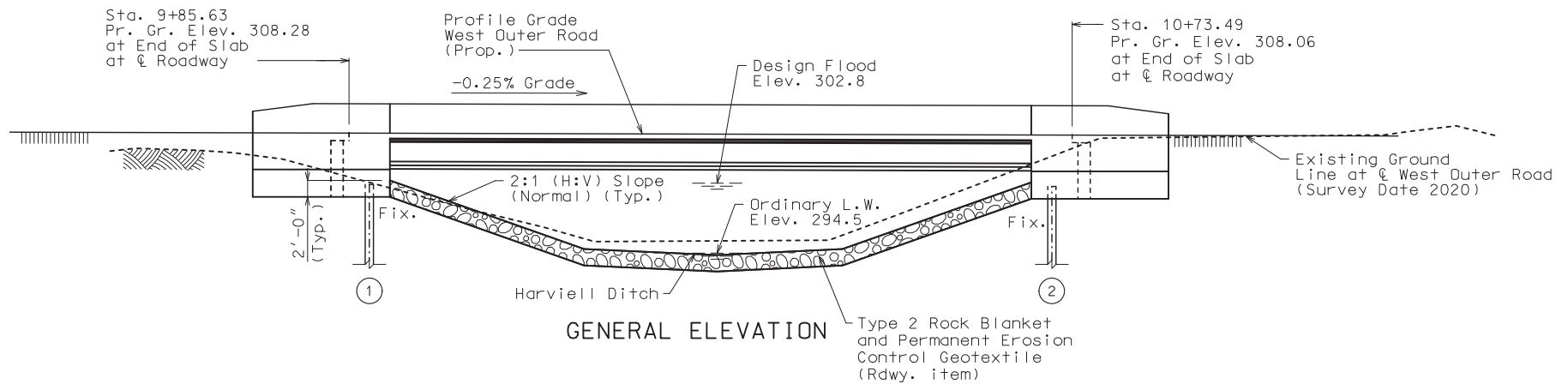
General Notes:

Longitudinal dimensions are measured horizontal.

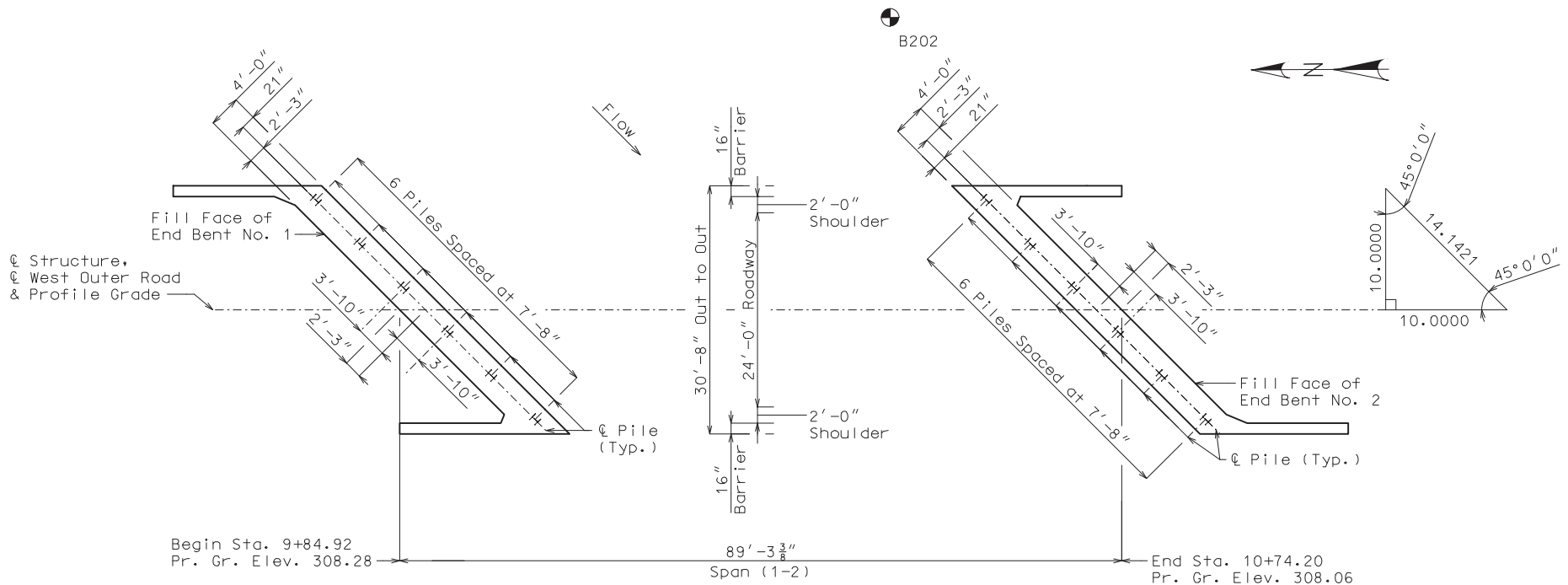
For General Notes, Estimated Quantities and Foundation Data see Sheet No. 2.

B.M. #32
 N: 282085.74
 E: 814254.75
 Sta. 719+83.75 Exist. Rte 67
 Elev. 308.27
 Chisled "□" on SW corner of west barrier curb on bridge F0665

Control Point #129
 N: 281264.97
 E: 814315.18
 Sta. 728+04.49 Exist. Rte 67
 Elev. 302.52
 5/8" rebar with MoDOT cap

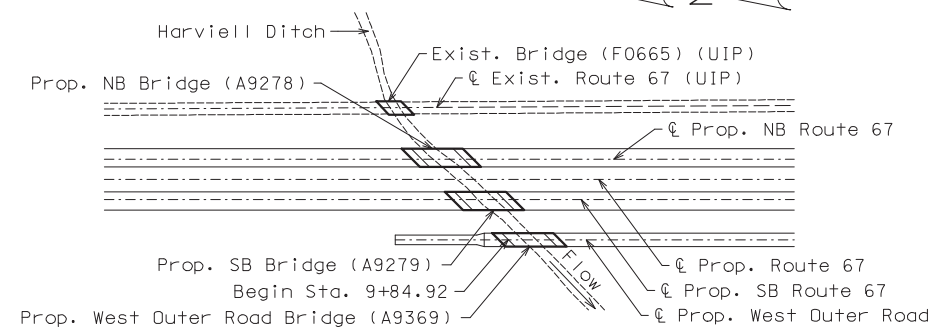


⊙ B201



⊙ B202

PLAN



LOCATION SKETCH

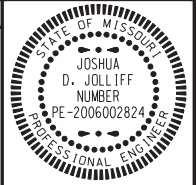
BRIDGE: ROUTE 67 WEST OUTER ROAD OVER HARVIELL DITCH

ROUTE 67 WEST OUTER ROAD FROM COUNTY ROAD 352 ABOUT 3.2 MILES NORTH OF ROUTE 142 BEGINNING STATION 9+84.92 (⊙ WEST OUTER ROAD ROUTE 67)

GENERAL PLAN AND ELEVATION

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 1 of 24



THIS SHEET HAS BEEN SIGNED, SEALED AND DATED ELECTRONICALLY.

DATE PREPARED 3/3/2023

ROUTE 67 STATE MO

DISTRICT BR SHEET NO. 1

COUNTY BUTLER

JOB NO. J9P3751

CONTRACT ID.

PROJECT NO.

BRIDGE NO. A9369

DESCRIPTION	DATE

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION
 MoDOT
 105 WEST CAPITOL JEFFERSON CITY, MO 65102
 1-888-ASK-MODOT (1-888-275-6636)

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General Notes:

Design Specifications:

2020 AASHTO LRFD Bridge Design Specifications (9th Ed.)
 2011 AASHTO Guide Specifications for LRFD Seismic Bridge Design (2nd Ed.)
 and 2014 Interim Revisions (Seismic Details)

Seismic Design Category = C

Design earthquake response spectral acceleration coefficient at 1.0 second period, $S_{D1} = 0.34$

Acceleration Coefficient (effective peak ground acceleration coefficient), $A_s = 0.37$

Design Loading:

Vehicular = HL-93

Future Wearing Surface = 35 lb/sf

Earth = 120 lb/cf

Equivalent Fluid Pressure = 45 lb/cf (Min.)

Superstructure: Non-composite for dead load. Composite for live load.

Design Unit Stresses:

Class B Concrete (Substructure) $f'c = 3,000$ psi

Class B-2 Concrete (Superstructure, except Prestressed Girders and Barrier) $f'c = 4,000$ psi

Class B-1 Concrete (Barrier) $f'c = 4,000$ psi

Reinforcing Steel (Grade 60) $fy = 60,000$ psi

Structural Steel HP Pile (ASTM A709 Grade 50S) $fy = 50,000$ psi

For precast prestressed panel stresses, see Sheet No. 12.

For prestressed girder stresses, see Sheets No. 10 & 11.

Neoprene Pads:

Neoprene bearing pads shall be 60 durometer and shall be in accordance with Sec 716.

Joint Filler:

All joint filler shall be in accordance with Sec 1057 for preformed sponge rubber expansion and partition joint filler, except as noted.

Reinforcing Steel:

Minimum clearance to reinforcing steel shall be 1 1/2", unless otherwise shown.

Traffic Handling:

Structure to be closed during construction. Traffic to be maintained on other routes during construction. See roadway plans for traffic control.

Miscellaneous:

MoDOT Construction personnel will indicate the type of joint filler option used under the precast panels for this structure:

- Constant Joint Filler
- Variable Joint Filler

Estimated Quantities for Slab on Concrete NU-Girder		
Item		Total
Class B-2 Concrete	cu. yard	126
Reinforcing Steel (Epoxy Coated)	pond	25,590

Notes:

The table of Estimated Quantities for Slab on Concrete NU-Girder represents the quantities used by the State in preparing the cost estimate for concrete slabs. The area of the concrete slab will be measured to the nearest square yard longitudinally from end of slab to end of slab and transversely from out to out of bridge slab (or with the horizontal dimensions as shown on the plan of slab). Payment for prestressed panels, conventional forms, all concrete and epoxy coated reinforcing steel will be considered completely covered by the contract unit price for the slab. Variations may be encountered in the estimated quantities but the variations cannot be used for an adjustment in the contract unit price.

Method of forming the slab shall be as shown on the plans and in accordance with Sec 703. All hardware for forming the slab to be left in place as a permanent part of the structure shall be coated in accordance with ASTM A123 or ASTM B633 with a thickness class SC 4 and a finish type I, II or III.

The Estimated Quantities for Slab on Concrete NU-Girder are based on skewed precast prestressed end panels.

Class B-2 Concrete quantity is based on minimum top flange thickness and minimum joint material thickness.

The prestressed panel quantities are not included in the table of Estimated Quantities for Slab on Concrete NU-Girder.

Estimated Quantities				
Item		Substr.	Superstr.	Total
Class 1 Excavation	cu. yard	160		160
Bridge Approach Slab (Minor)	sq. yard		127	127
Galvanized Structural Steel Piles (12 in.)	linear foot	960		960
Pile Point Reinforcement	each	12		12
Class B Concrete (Substructure)	cu. yard	48.8		48.8
Type D Barrier	linear foot		235	235
Slab on Concrete NU-Girder	sq. yard		300	300
NU 43, Prestressed Concrete NU-Girder	linear foot		335	335
Slab Drain	each		14	14
Vertical Drain at End Bents	each	2		2
Plain Neoprene Bearing Pad	each		8	8

All concrete above the construction joint in the end bents is included in the Estimated Quantities for Slab on Concrete NU-Girder.

All reinforcement in the end bents is included in the Estimated Quantities for Slab on Concrete NU-Girder.

Cost of L4x4 ASTM A709 Grade 36 HP pile anchors and 3/4-inch diameter ASTM F3125 Grade A325 Type 1 bolts, complete in place, will be considered completely covered by the contract unit price for Galvanized Structural Steel Piles (12 in.).

Foundation Data			
Type	Design Data	Bent Number	
		1	2
Load Bearing Pile	Pile Type and Size	HP 12x53	HP 12x53
	Number	ea 6	ea 6
	Approximate Length Per Each	ft 79	ft 81
	Pile Point Reinforcement	ea All	ea All
	Min. Galvanized Penetration (Elev.)	ft 281	ft 280
	Pile Driving Verification Method	DF	DF
	Resistance Factor	0.4	0.4
Minimum Nominal Axial Compressive Resistance	kip	500	500

DF = FHWA-modified Gates Dynamic Pile Formula

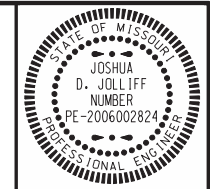
Load Bearing Pile:

Minimum Nominal Axial Compressive Resistance = $\frac{\text{Maximum Factored Loads}}{\text{Resistance Factor}}$

All piles shall be galvanized down to the minimum galvanized penetration (elevation).

Pile point reinforcement need not be galvanized. Shop drawings will not be required for pile point reinforcement.

The contractor shall make every effort to achieve the minimum galvanized penetration (elevation) shown on the plans for all piles. Deviations in penetration less than 5 feet of the minimum will be considered acceptable provided the contractor makes the necessary corrections to ensure the minimum penetration is achieved on subsequent piles.



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DATE PREPARED 3/3/2023	
ROUTE 67	STATE MO
DISTRICT BR	SHEET NO. 2
COUNTY BUTLER	
JOB NO. J9P3751	
CONTRACT ID.	

PROJECT NO.

BRIDGE NO.
A9369

DATE	DESCRIPTION

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

105 WEST CAPITOL
JEFFERSON CITY, MO 65102
1-888-ASK-MODOT (1-888-275-6636)

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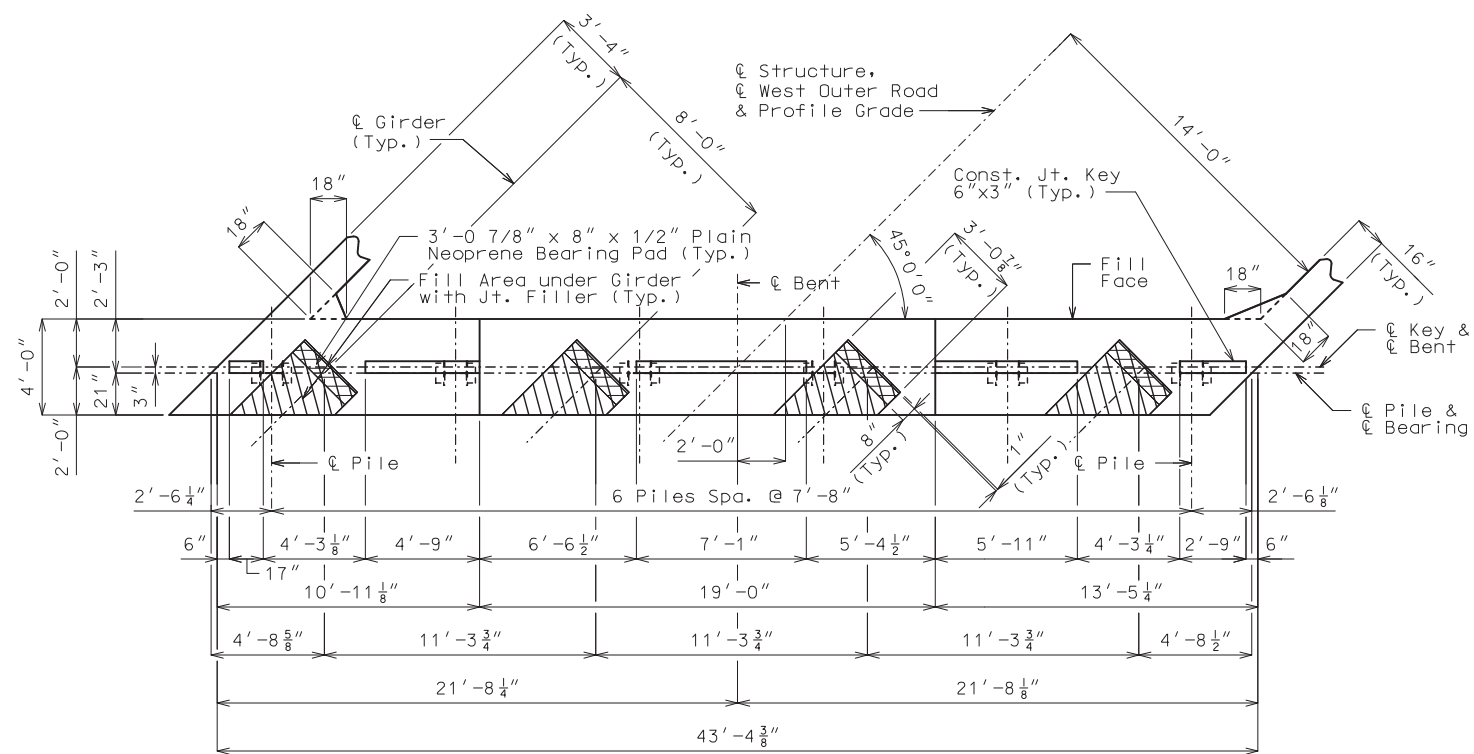
ESTIMATED QUANTITIES AND GENERAL NOTES

Detailed Jan. 2023
 Checked Jan. 2023

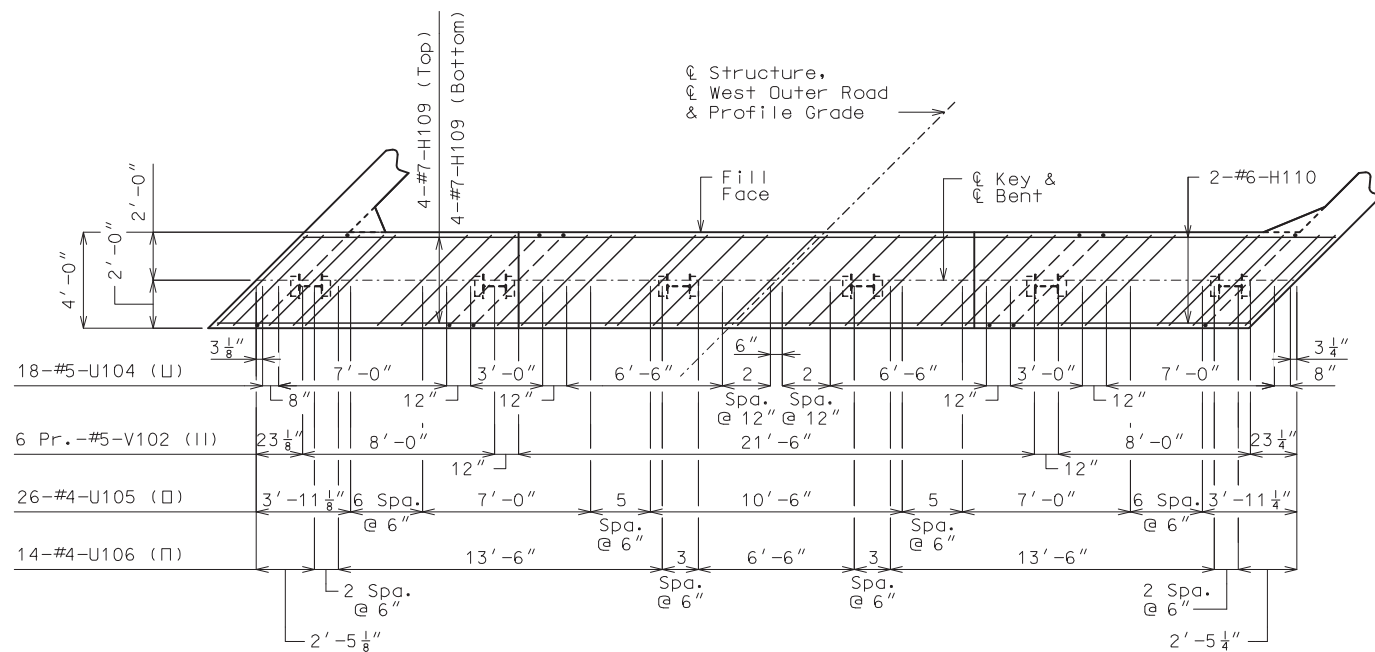
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Sheet No. 2 of 24

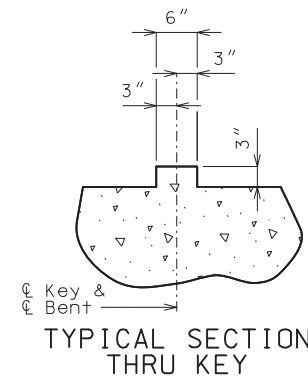
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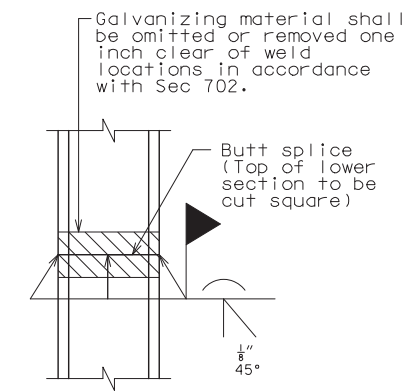
PLAN OF BEAM SHOWING DIMENSIONS



PLAN OF BEAM SHOWING REINFORCEMENT
(Keys not shown for clarity)



STEEL PILE SPLICE
(If required)



General Notes:

- For details of End Bent No. 1 not shown, see Sheets No. 4 & 5.
- For details of Vertical Drain at End Bents, see Sheet No. 6.
- The U bars and pairs of V bars shall be placed parallel to centerline of roadway.
- Reinforcing steel shall be shifted to clear piles, U bars shall clear piles by at least 1 1/2".

DETAILS OF END BENT NO. 1

Detailed Jan. 2023
Checked Jan. 2023

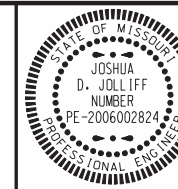
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Sheet No. 3 of 24

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3/3/2023



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DATE PREPARED
3/3/2023

ROUTE 67 STATE MO
DISTRICT BR SHEET NO. 3

COUNTY BUTLER
JOB NO. J9P3751
CONTRACT ID.

PROJECT NO.

BRIDGE NO. A9369

DATE	DESCRIPTION

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

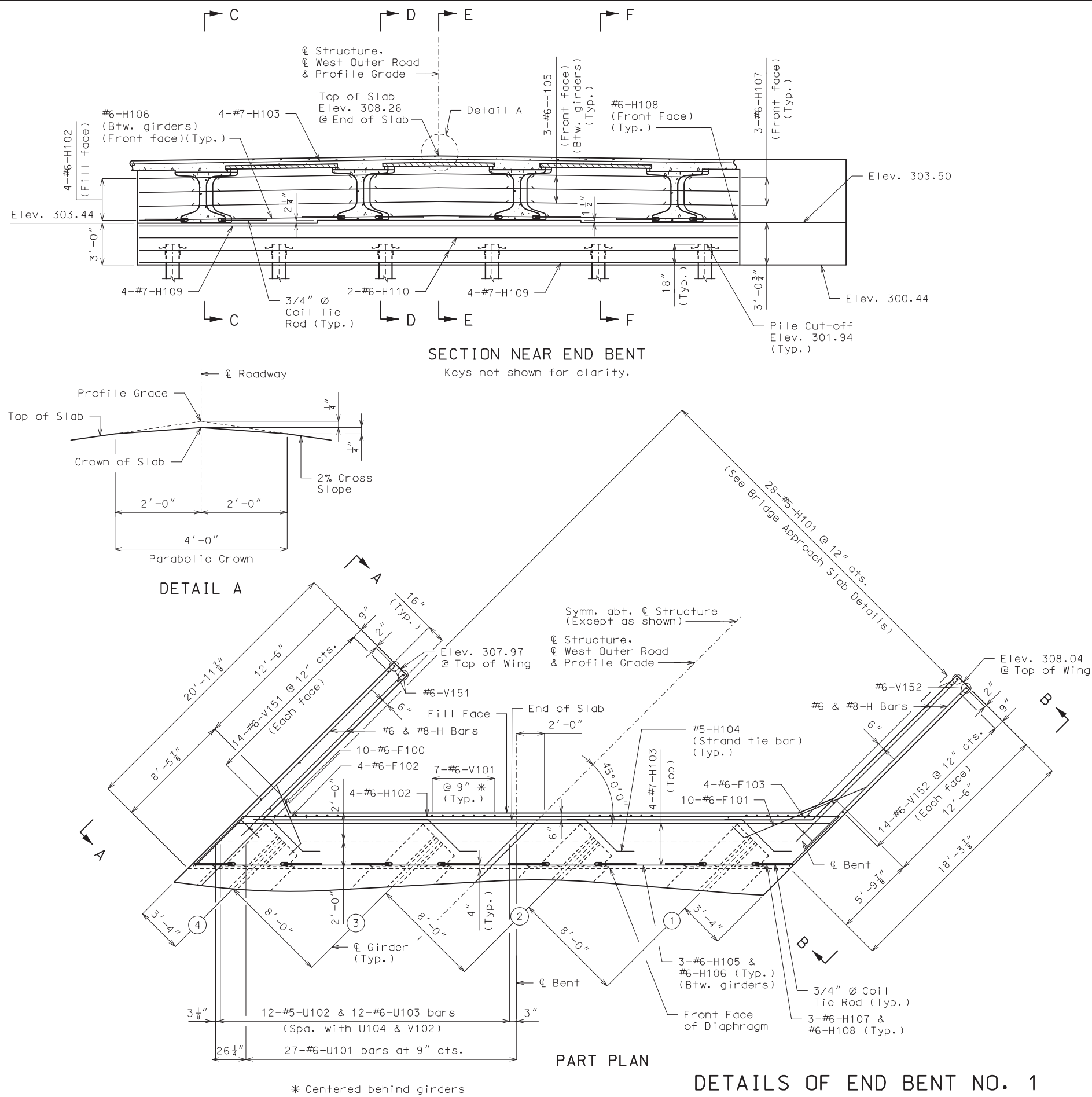


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REV.



General Notes:

For details of End Bent No. 1 not shown, see Sheets No. 3 & 5.
For Sections C-C, D-D, E-E, & F-F, see Sheet No. 5.
For Elevations A-A & B-B, see Sheet No. 5.

All concrete in the end bent above top of beam and below top of slab shall be Class B-2.

For location of Coil Tie Rods and #5-H104 (Strand Tie Bar), see Sheets No. 10 and 11.

For details of Vertical Drain at End Bents, see Sheet No. 6.

Reinforcing steel shall be shifted to clear piles. U bars shall clear piles by at least 1 1/2".

The #6-F100 and #6-F101 bars shall be bent in the field to clear girders.

The U bars shall be placed parallel to centerline of roadway.

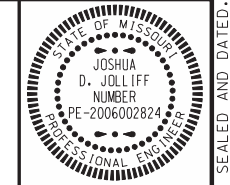
Strands at end of girders shall be field bent or, if necessary, cut in field to maintain 1 1/2-inch minimum clearance to fill face of end bent.

For details and reinforcement of the Type D Barrier, see Sheet No. 18.

For details of Bridge Approach Slab, see Sheet No. 19.

Item	Quantity	Unit
Class 1 Excavation	45	cu. yard
Galvanized Structural Steel Piles (12 in.)	474	linear foot
Pile Point Reinforcement	6	each
Class B Concrete (Substructure)	24.4	cu. yard

Note: These quantities are included in the Estimated Quantities table on Sheet No. 2.



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DATE PREPARED
3/3/2023

ROUTE 67 STATE MO
DISTRICT BR SHEET NO. 4

COUNTY BUTLER
JOB NO. J9P3751
CONTRACT ID.

PROJECT NO.
BRIDGE NO. A9369

DATE	DESCRIPTION

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION
MoDOT
105 WEST CAPITOL
JEFFERSON CITY, MO 65102
1-888-ASK-MODOT (1-888-278-6636)

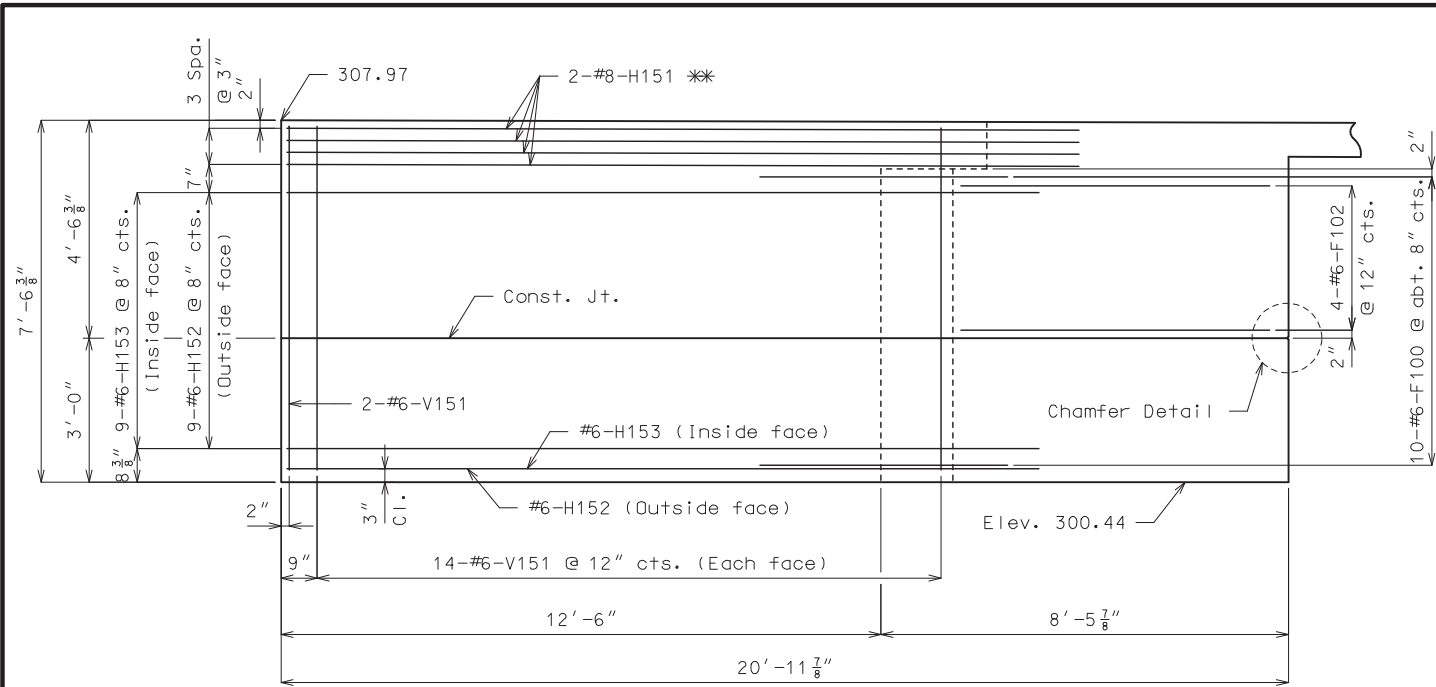
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665 WEST CENTRAL EXPRESSWAY
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ENGINEERING CORPORATION - 000631

Detailed Jan. 2023
Checked Jan. 2023

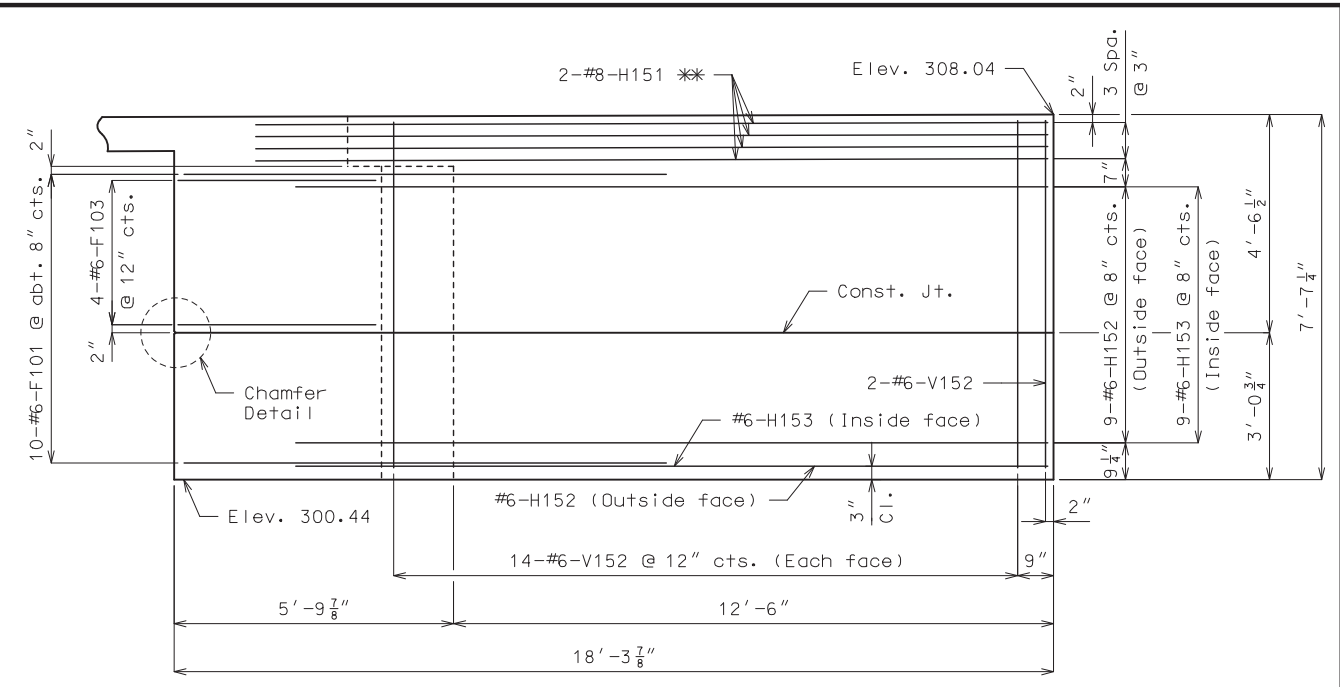
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Sheet No. 4 of 24

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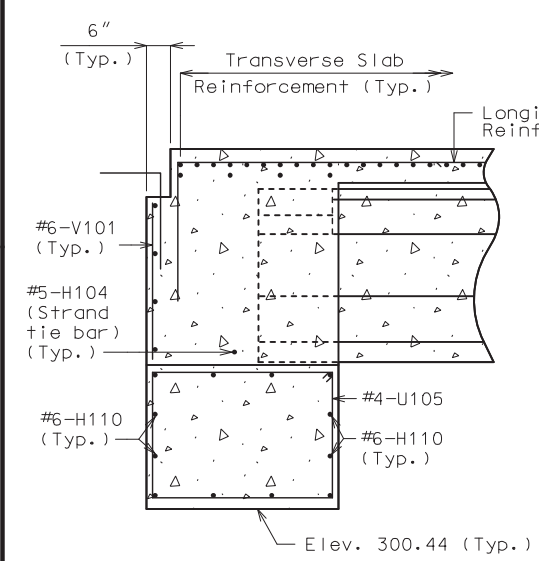


ELEVATION A-A

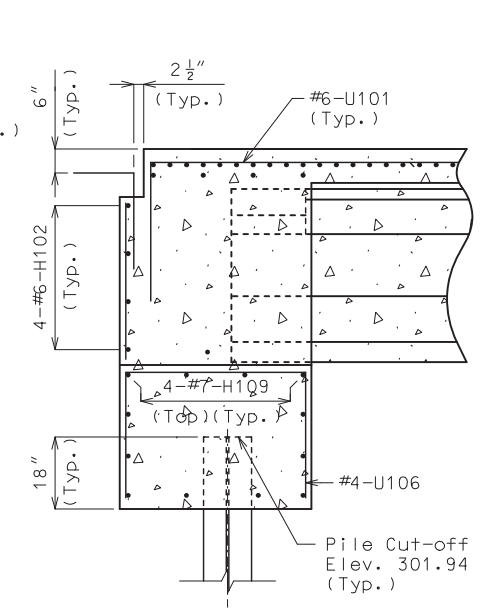


ELEVATION B-B

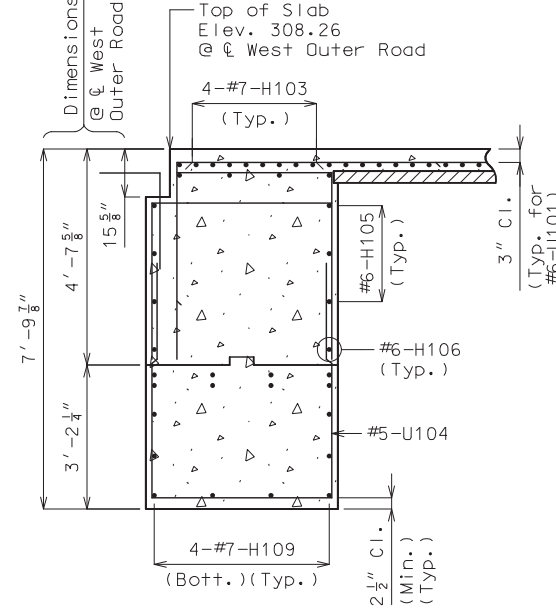
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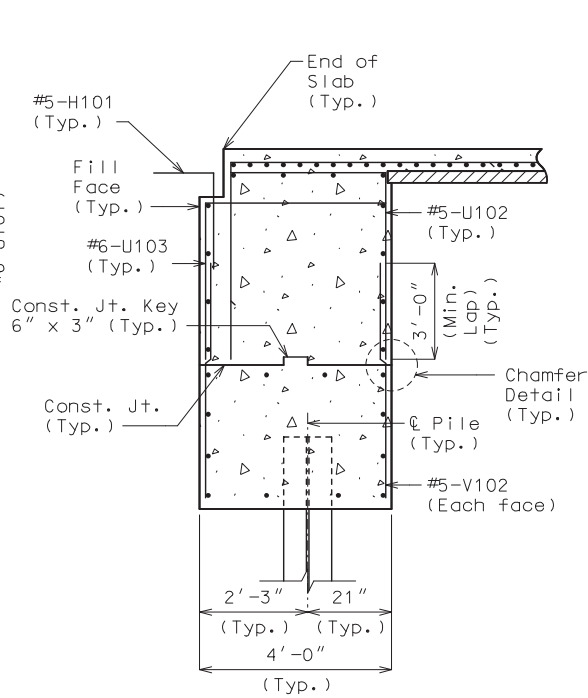
SECTION C-C



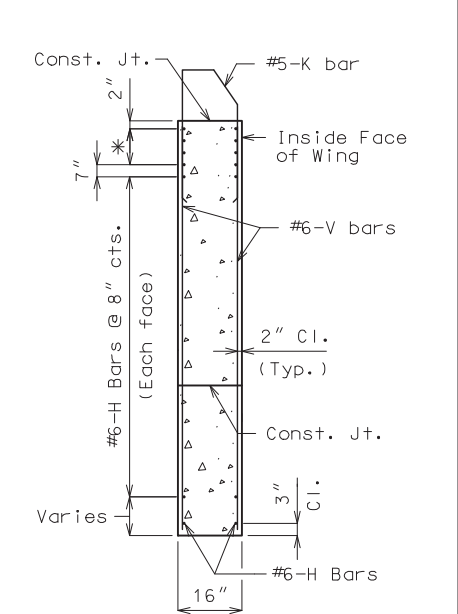
SECTION D-D



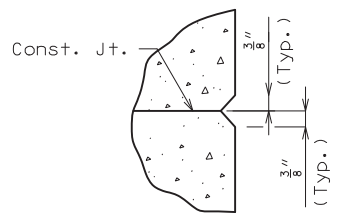
SECTION E-E



SECTION F-F



TYPICAL SECTION THRU WING



CHAMFER DETAIL

DETAILS OF END BENT NO. 1

General Notes:

- For details of End Bent No. 1 not shown, see Sheets No. 3 & 4.
- All concrete in the end bent above top of beam and below top of slab shall be Class B-2.
- The #6-F100 and #6-F101 bars shall be bent in field to clear girders.
- For details and reinforcement of the Type D Barrier, see Sheet No. 18.
- For details of Vertical Drain at End Bents, see Sheet No. 6.
- For location of #5-H104 (Strand tie bar), see Sheets No. 10 and 11.
- For location of Sections C-C, D-D, E-E, & F-F, see Sheet No. 4.
- For location of Elevations A-A & B-B, see Sheet No. 4.
- For details of Bridge Approach Slab, see Sheet No. 19.

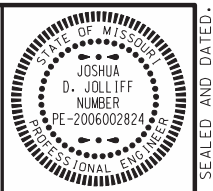
Detailed Jan. 2023
Checked Jan. 2023

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 5 of 24

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10:25:08 AM 3/3/2023



THIS SHEET HAS BEEN SIGNED, SEALED AND DATED ELECTRONICALLY.

DATE PREPARED
3/3/2023
ROUTE 67 STATE MO
DISTRICT BR SHEET NO. 5
COUNTY BUTLER
JOB NO. J9P3751
CONTRACT ID.

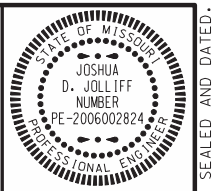
PROJECT NO.
BRIDGE NO. A9369

DESCRIPTION	DATE

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION
MoDOT
105 WEST CAPITOL
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DATE PREPARED
3/3/2023

ROUTE 67 STATE MO

DISTRICT BR SHEET NO. 6

COUNTY BUTLER

JOB NO. J9P3751

CONTRACT ID.

PROJECT NO.

BRIDGE NO. A9369

DATE	DESCRIPTION

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

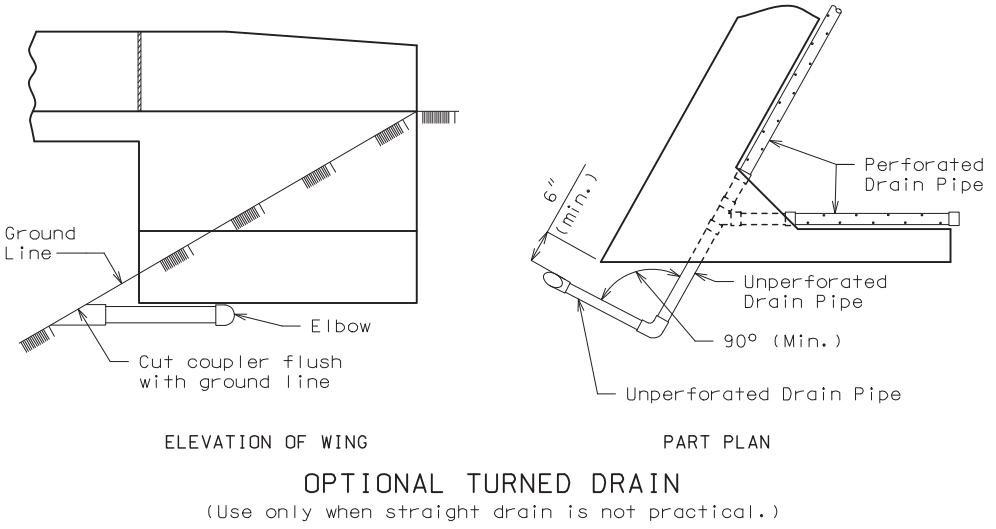
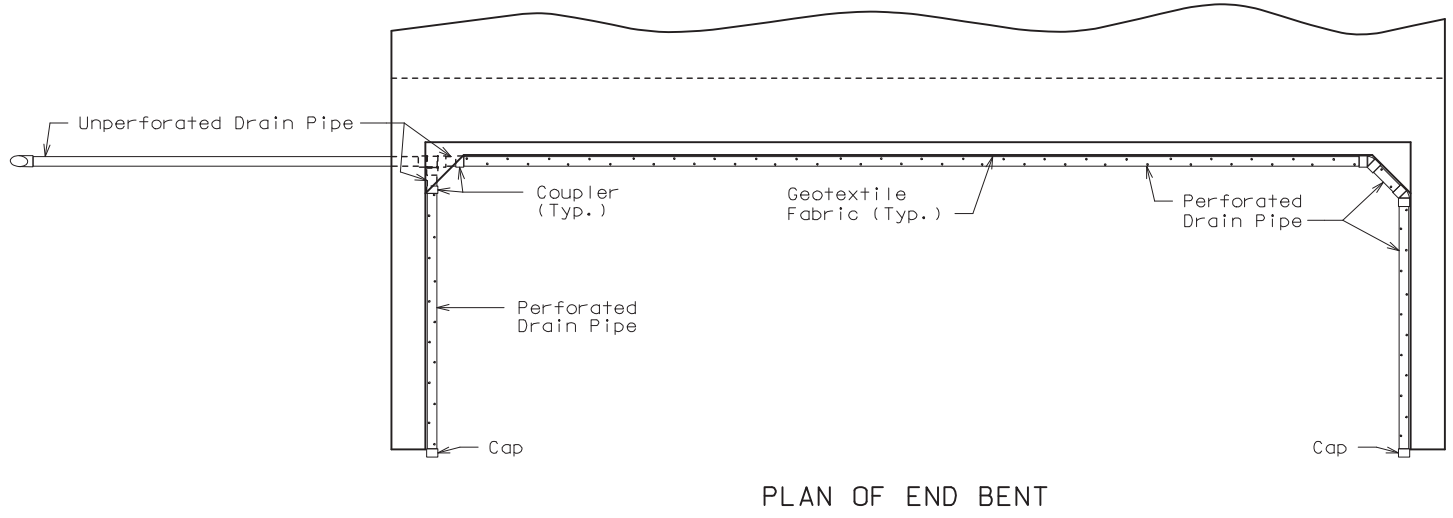
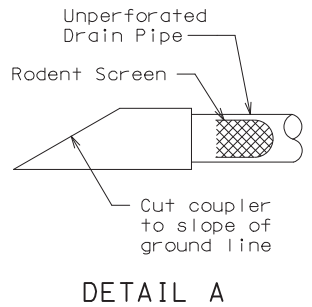
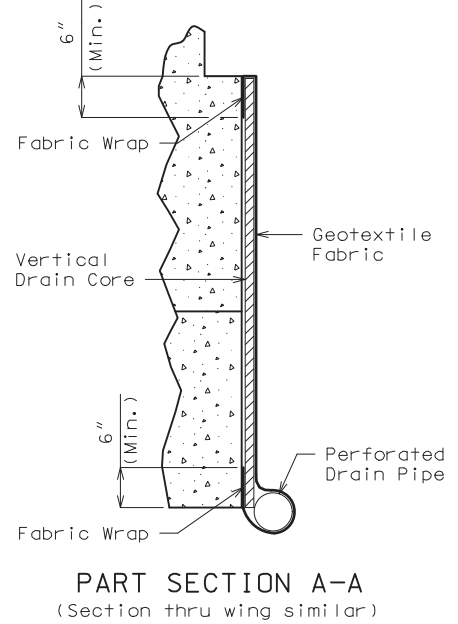
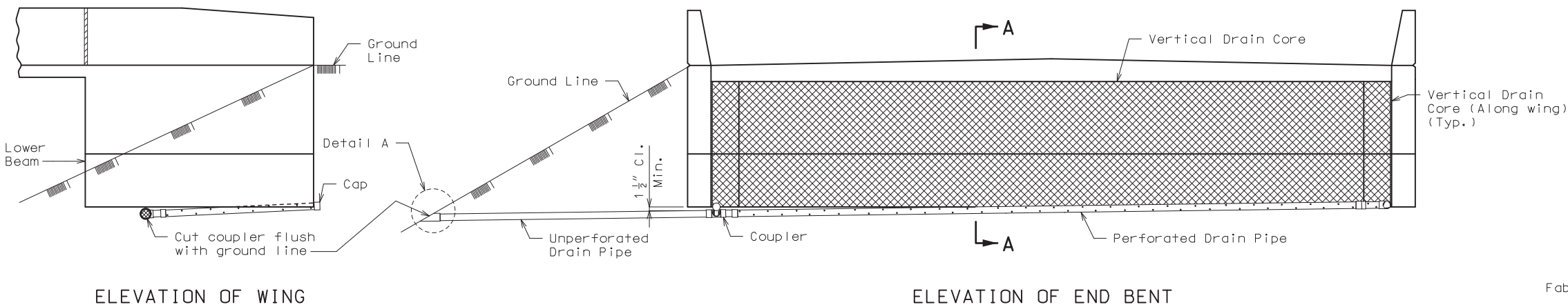
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General Notes:

All drain pipe shall be sloped 1 to 2 percent.

Drain pipe may be either 6-inch diameter corrugated metallic-coated steel pipe underdrain, 4-inch diameter corrugated polyvinyl chloride (PVC) drain pipe, or 4-inch diameter corrugated polyethylene (PE) drain pipe.

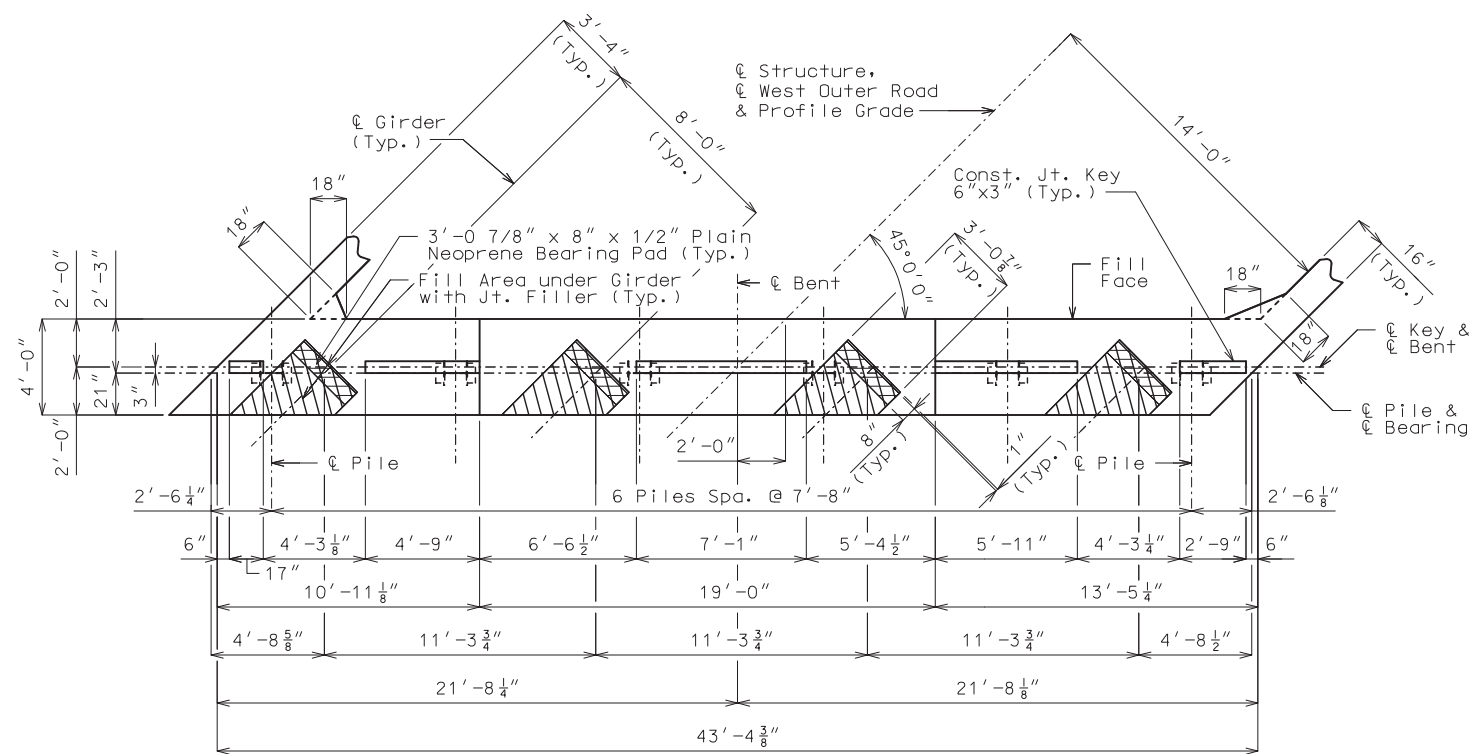
Drain pipe shall be placed at fill face of end bent and inside face of wings. The pipe shall slope to lowest grade of ground line, also missing the lower beam of end bent by a minimum of 1 1/2 inches.

Perforated pipe shall be placed at fill face side and inside face of wings at the bottom of end bent and plain pipe shall be used where the vertical drain ends to the exit at ground line.

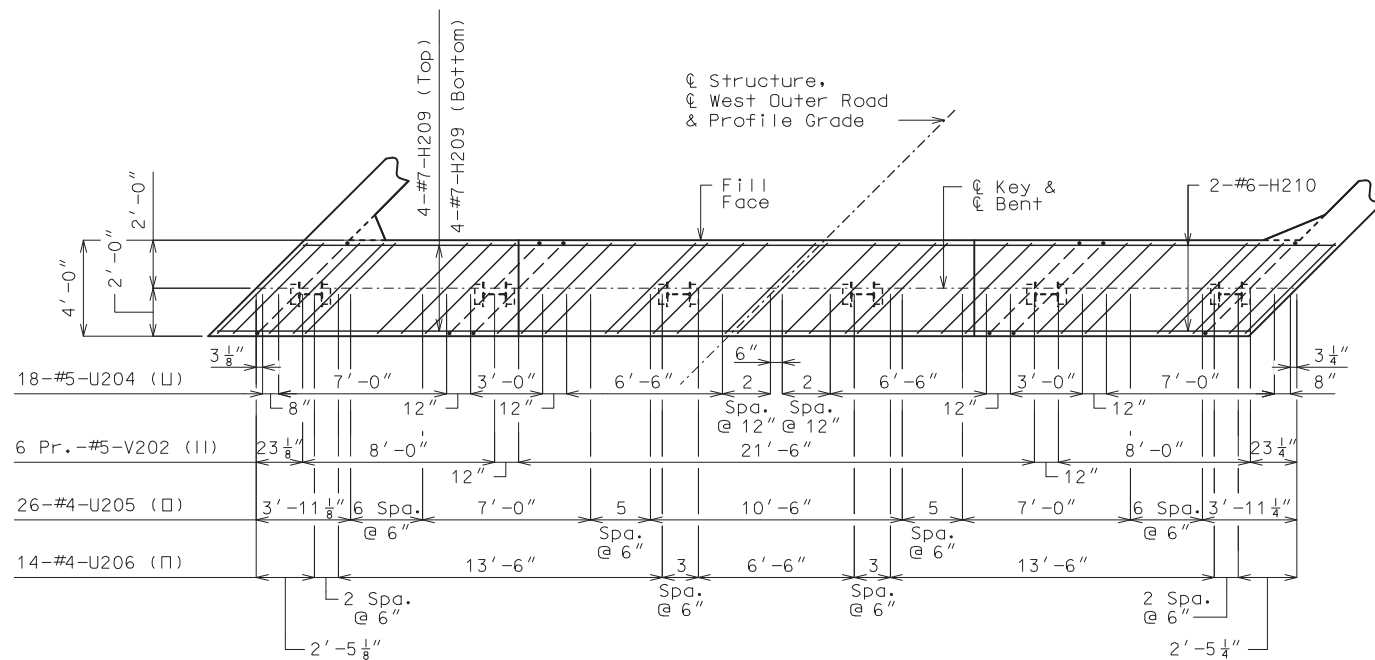
VERTICAL DRAIN AT END BENTS
(Squared end bent shown, skewed end bent similar)

Detailed Jan. 2023
Checked Jan. 2023

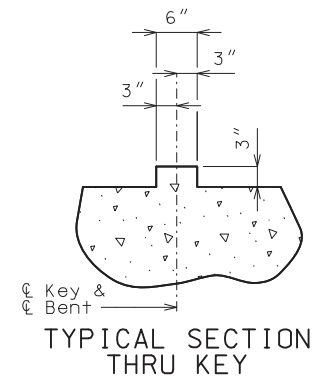
Note: This drawing is not to scale. Follow dimensions. Sheet No. 6 of 24



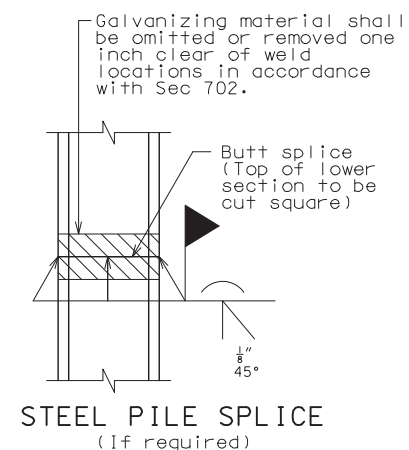
PLAN OF BEAM SHOWING DIMENSIONS



PLAN OF BEAM SHOWING REINFORCEMENT
(Keys not shown for clarity)



TYPICAL SECTION THRU KEY



STEEL PILE SPLICE
(If required)

General Notes:

- For details of End Bent No. 2 not shown, see Sheets No. 8 & 9.
- For details of Vertical Drain at End Bents, see Sheet No. 6.
- The U bars and pairs of V bars shall be placed parallel to centerline of roadway.
- Reinforcing steel shall be shifted to clear piles, U bars shall clear piles by at least 1 1/2".

DETAILS OF END BENT NO. 2

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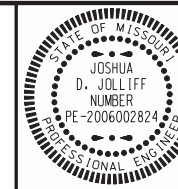
Note: This drawing is not to scale. Follow dimensions.

Sheet No. 7 of 24

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3/3/2023



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DATE PREPARED
3/3/2023

ROUTE 67 STATE MO

DISTRICT BR SHEET NO. 7

COUNTY BUTLER

JOB NO. J9P3751

CONTRACT ID.

PROJECT NO.

BRIDGE NO. A9369

DATE	DESCRIPTION

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

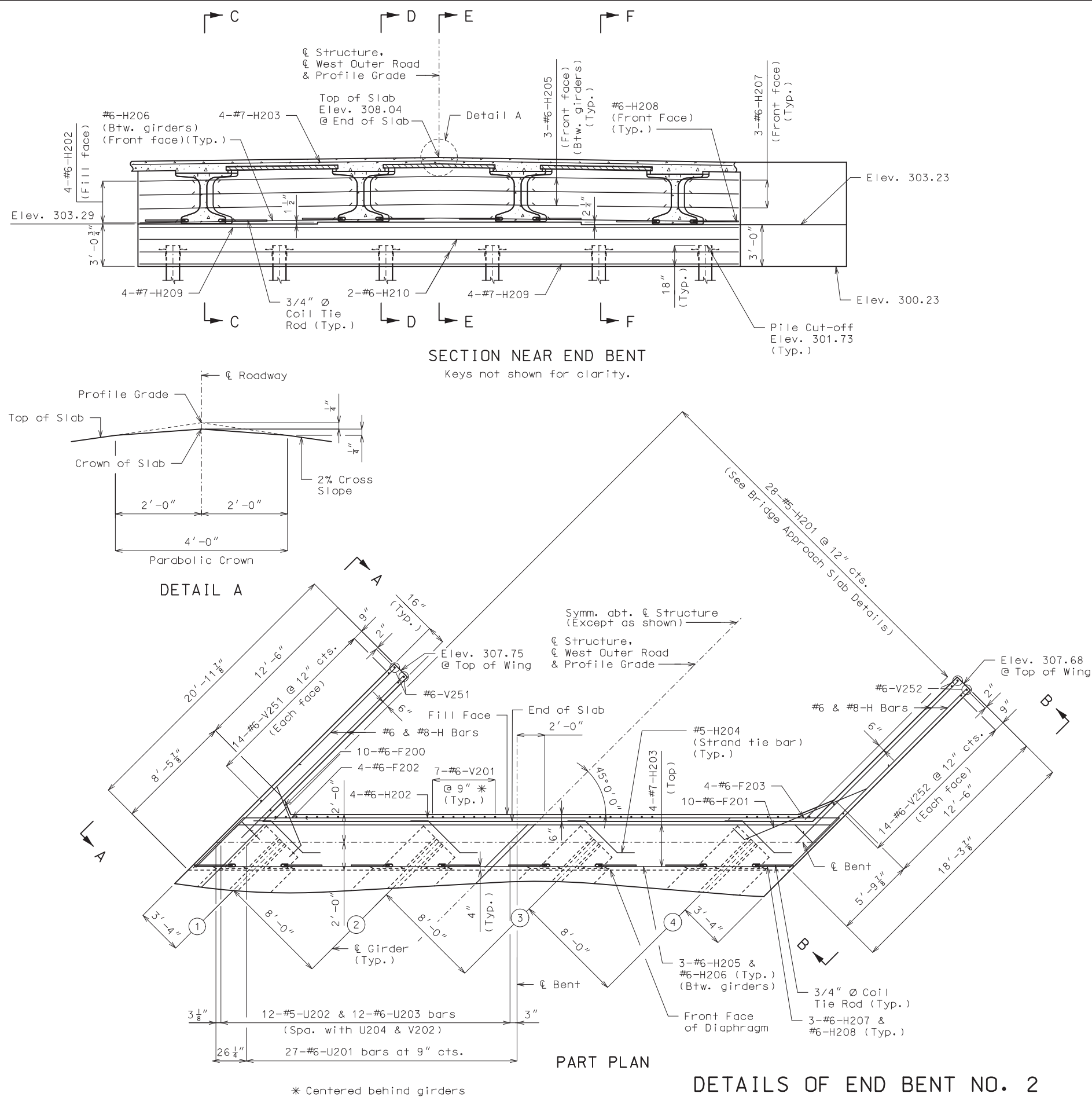
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General Notes:

For details of End Bent No. 2 not shown, see Sheets No. 7 & 9.

For Sections C-C, D-D, E-E, & F-F, see Sheet No. 9.

For Elevations A-A & B-B, see Sheet No. 9.

All concrete in the end bent above top of beam and below top of slab shall be Class B-2.

For location of Coil Tie Rods and #5-H204 (Strand Tie Bar), see Sheets No. 10 and 11.

For details of Vertical Drain at End Bents, see Sheet No. 6.

Reinforcing steel shall be shifted to clear piles, U bars shall clear piles by at least 1 1/2".

The #6-F200 and #6-F201 bars shall be bent in the field to clear girders.

The U bars shall be placed parallel to centerline of roadway.

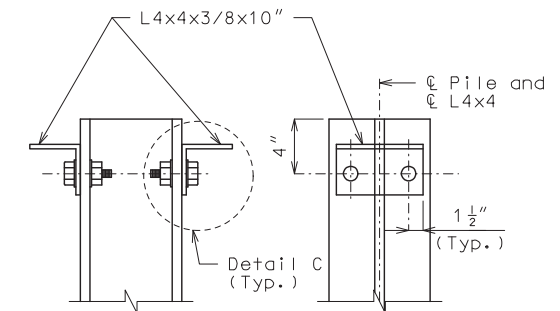
Strands at end of girders shall be field bent or, if necessary, cut in field to maintain 1 1/2-inch minimum clearance to fill face of end bent.

For details and reinforcement of the Type D Barrier, see Sheet No. 18.

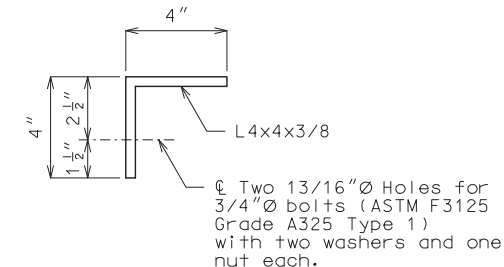
For details of Bridge Approach Slab, see Sheet No. 19.

Item	Quantity
Class 1 Excavation	cu. yard 115
Galvanized Structural Steel Piles (12 in.)	linear foot 486
Pile Point Reinforcement	each 6
Class B Concrete (Substructure)	cu. yard 24.4

Note: These quantities are included in the Estimated Quantities table on Sheet No. 2.

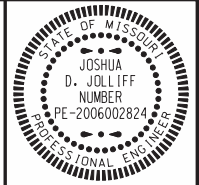


DETAILS OF HP PILE ANCHORS



DETAIL C

Angles shall be coated with a minimum of two coats of non-aluminum epoxy mastic primer to provide a dry film thickness of 4 mils minimum, 8 mils maximum, or galvanized in accordance with Sec 1081. Bolts, washers and nuts shall be galvanized in accordance with AASHTO M 232 (ASTM A153), Class C.



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DATE PREPARED
3/3/2023

ROUTE 67 STATE MO

DISTRICT BR SHEET NO. 8

COUNTY BUTLER

JOB NO. J9P3751

CONTRACT ID.

PROJECT NO.

BRIDGE NO. A9369

DESCRIPTION

DATE

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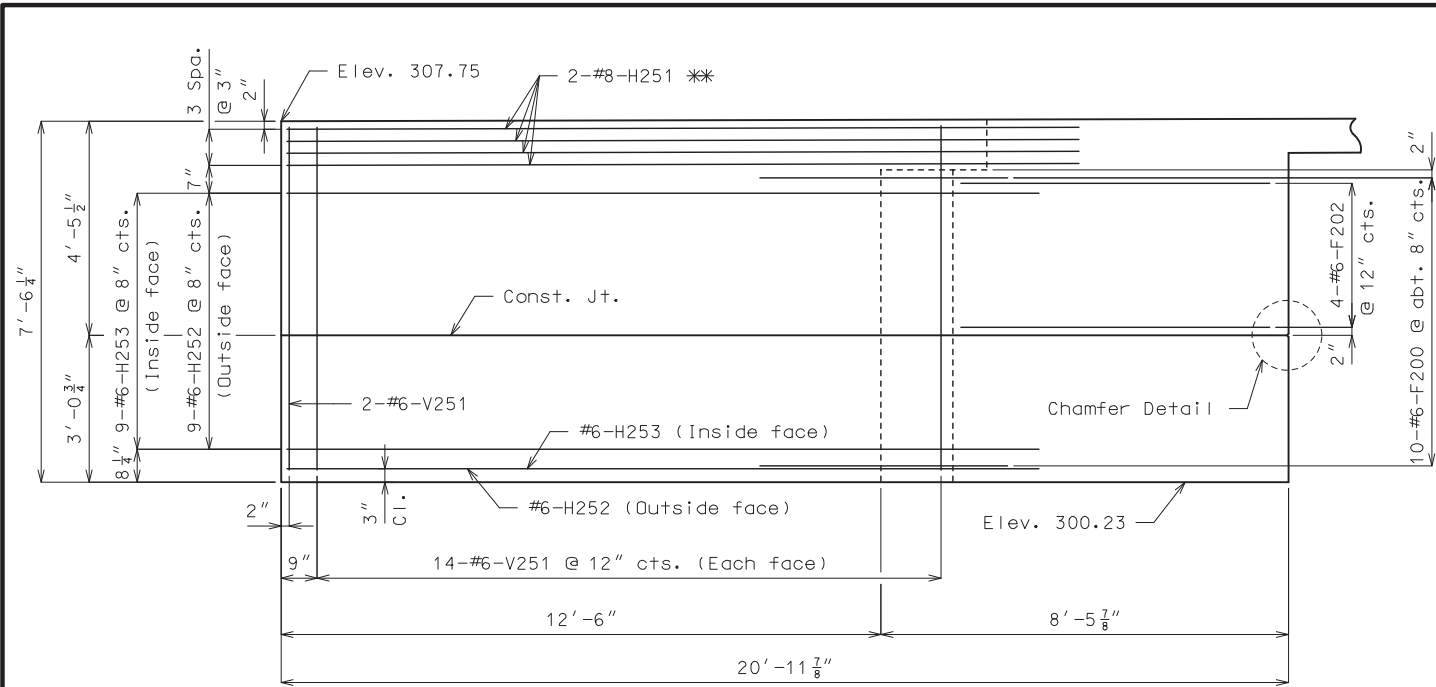
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6651 WEST LINCOLN ST. #300

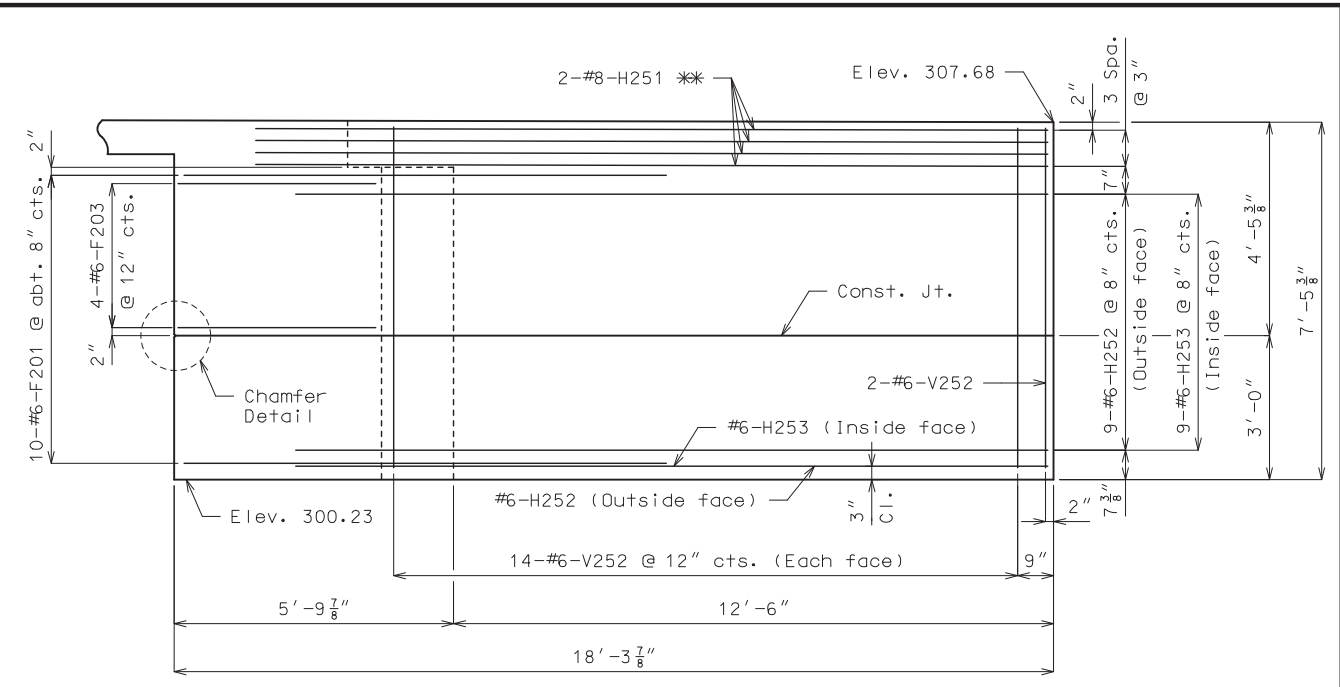
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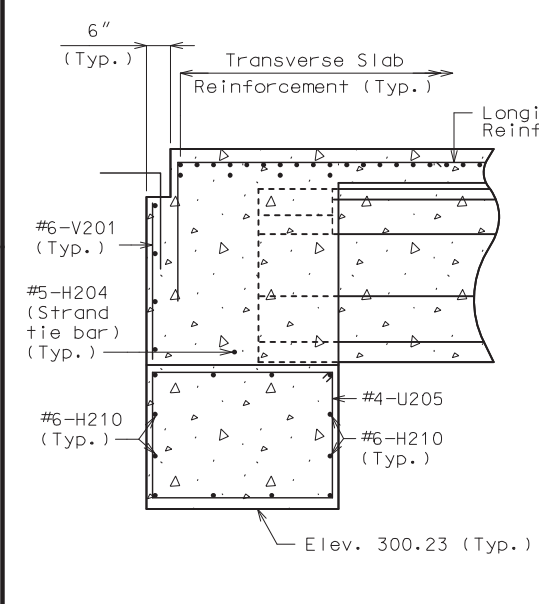


ELEVATION A-A

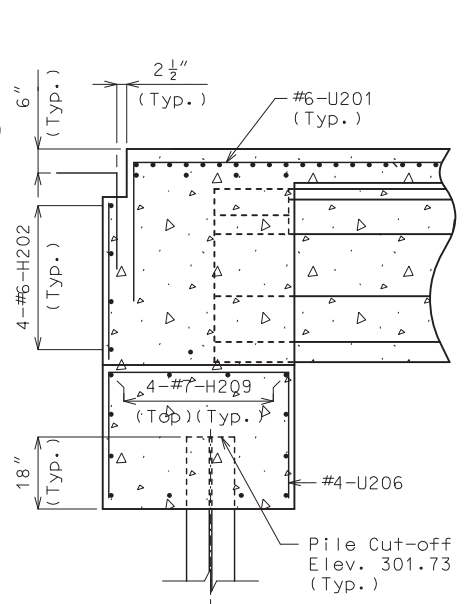


ELEVATION B-B

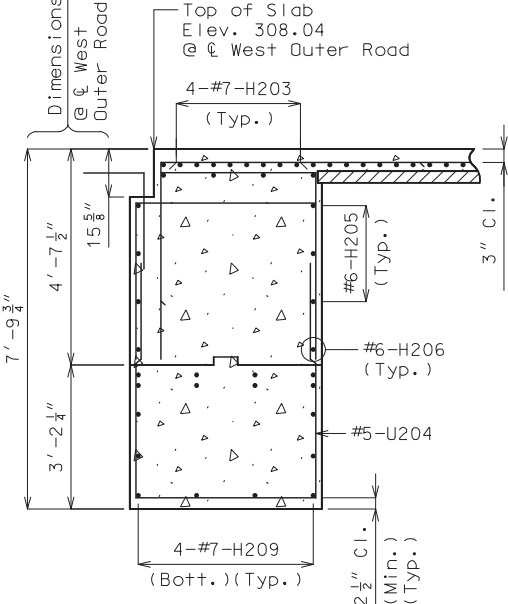
※ (Place with grade)



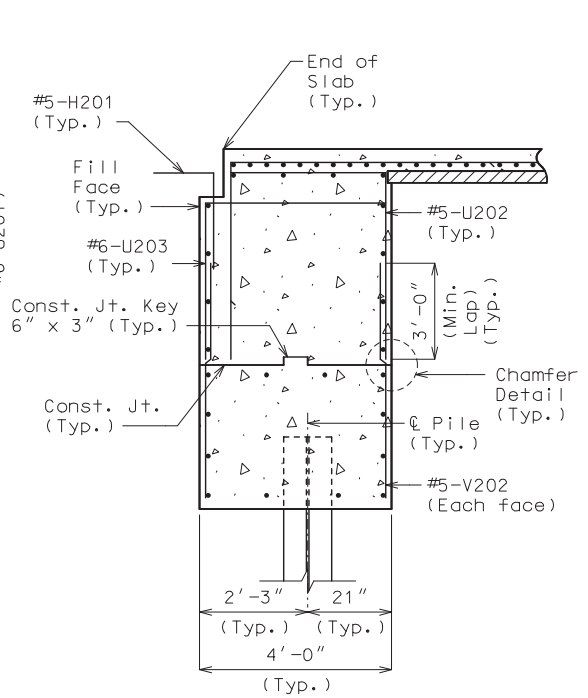
SECTION C-C



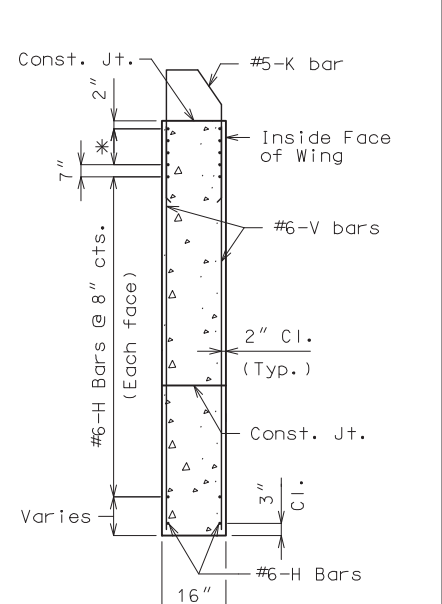
SECTION D-D



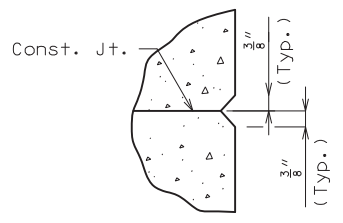
SECTION E-E



SECTION F-F



TYPICAL SECTION THRU WING



CHAMFER DETAIL

DETAILS OF END BENT NO. 2

General Notes:

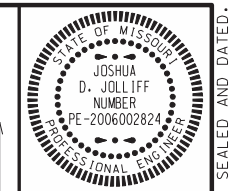
- For details of End Bent No. 2 not shown, see Sheets No. 7 & 8.
- All concrete in the end bent above top of beam and below top of slab shall be Class B-2.
- The #6-F200 and #6-F201 bars shall be bent in field to clear girders.
- For details and reinforcement of the Type D Barrier, see Sheet No. 18.
- For details of Vertical Drain at End Bents, see Sheet No. 6.
- For location of #5-H204 (Strand tie bar), see Sheets No. 10 and 11.
- For location of Sections C-C, D-D, E-E, & F-F, see Sheet No. 8.
- For location of Elevations A-A & B-B, see Sheet No. 8.
- For details of Bridge Approach Slab, see Sheet No. 19.

Detailed Jan. 2023
Checked Jan. 2023

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 9 of 24

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DATE PREPARED 3/3/2023	
ROUTE 67	STATE MO
DISTRICT BR	SHEET NO. 9
COUNTY BUTLER	
JOB NO. J9P3751	
CONTRACT ID.	
PROJECT NO.	
BRIDGE NO. A9369	

DESCRIPTION	DATE

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

105 WEST CAPITOL
JEFFERSON CITY, MO 65102
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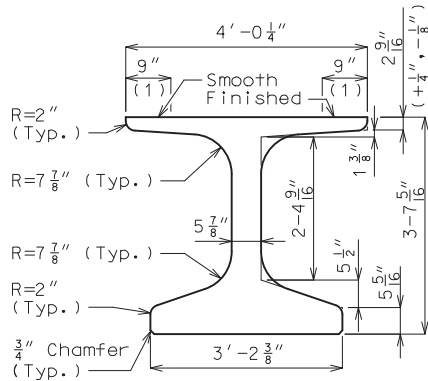
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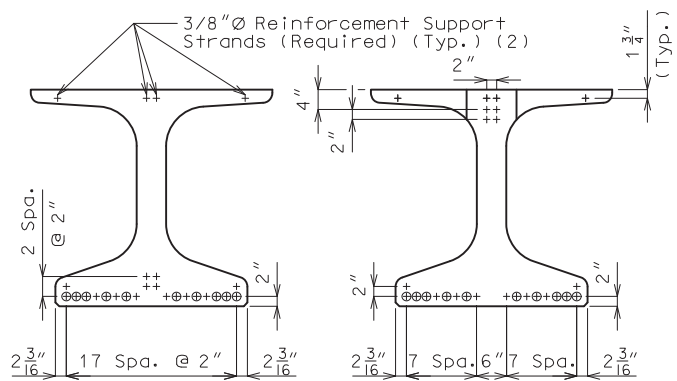
REV.

(1) Fabricator shall apply a bond breaker to this region excluding where joint filler will be applied.

(2) Outer strands tensioned to 2.02 kips/strand and inner strands to 8 kips/strand. Placed symmetrical about \bar{c} Girder. May be moved laterally in pairs.

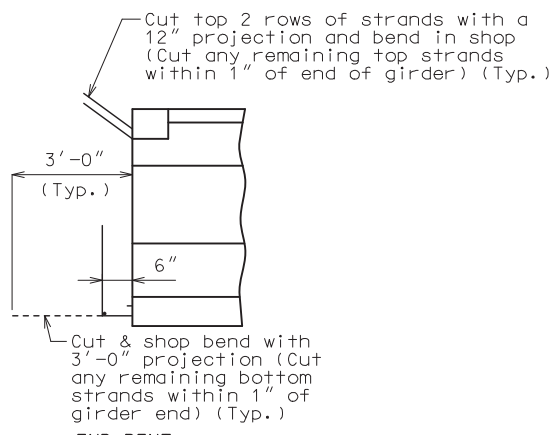


DIMENSIONS

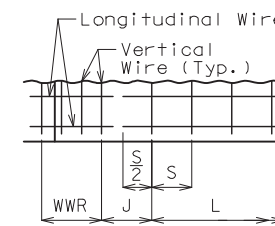


\bar{c} GIRDER STRAND ARRANGEMENT

+ Indicates prestressing strand. \circ Indicates cut & shop bend with 3'-0" projection.



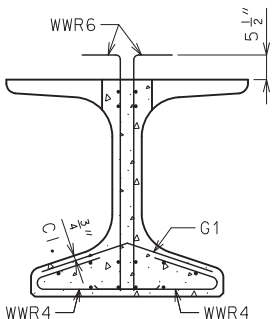
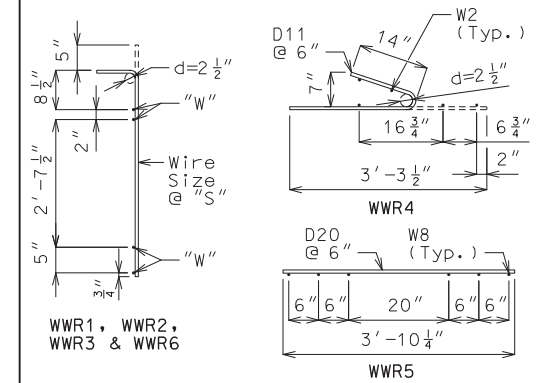
STRANDS AT GIRDER ENDS



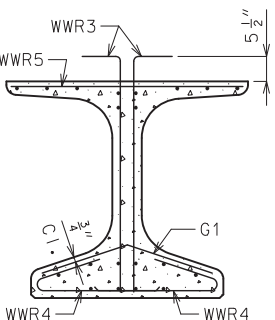
WELDED WIRE PLACEMENT

S = Vertical wire spacing
L = Length of WWR mats
J = Distance between WWR mats

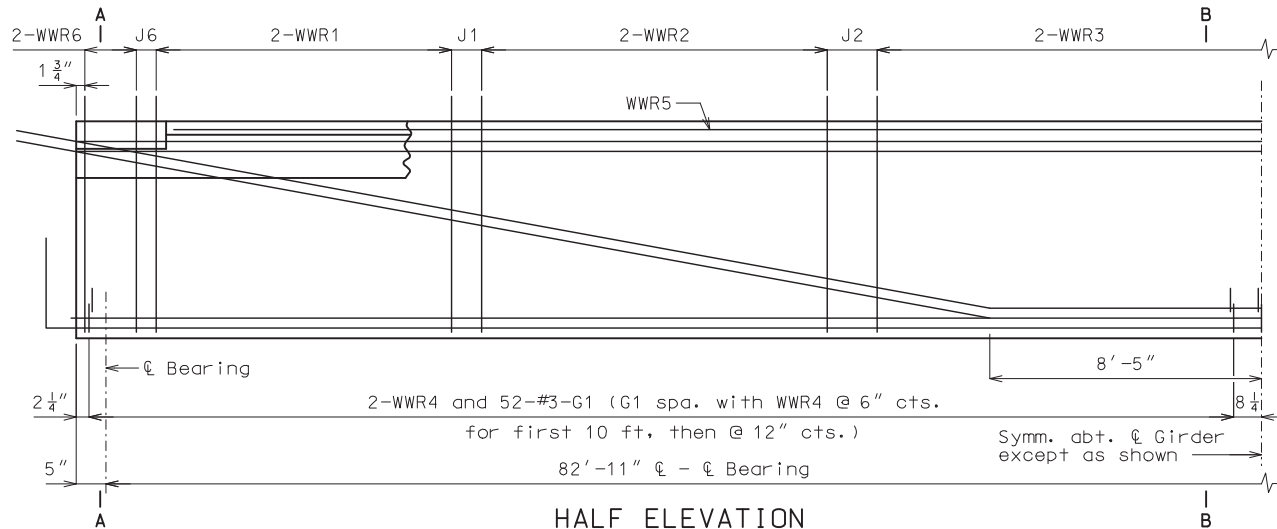
Bill of Reinforcing Steel					
Bars Each Girder					
No.	Size/Mark	Length	Shape	Bending Diagrams	
105	3 G1	2'-10"	8	Shape 8	
2	4 G3	5'-5"	20		
14	4 G6	Varies	20	Shape 20	
Welded Wire Each Girder					
Mark	Size	S	W	L	J
WWR1	D31	4"	W12	4'-4"	8"
WWR2	D31	8"	W12	22'-8"	12"
WWR3	D31	12"	W12	23'-0"	-
WWR6	D31	2"	W12	16"	2 3/4"



SECTION A-A
Strands not shown for clarity.

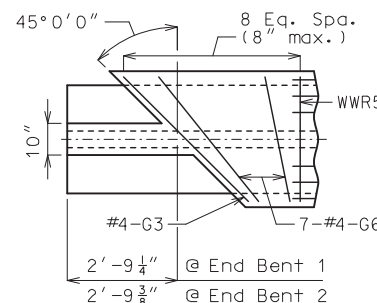


SECTION B-B
Strands not shown for clarity.

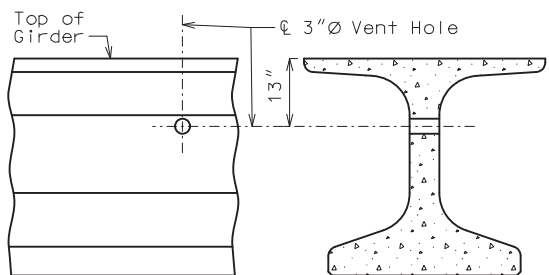


HALF ELEVATION

Reinforcement support strands not shown for clarity.

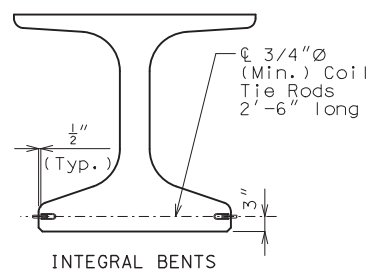


TOP FLANGE BLOCKOUT

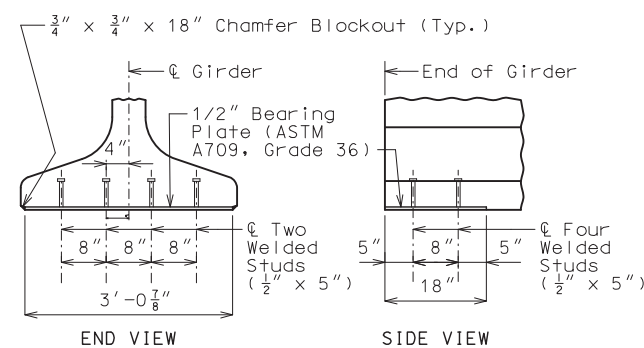


VENT HOLE

Place vent holes at or near upgrade 1/3 point of girders and clear reinforcing steel or strands by 1 1/2" minimum.



INTEGRAL BENTS
COIL TIES



BEARING PLATE

NU-GIRDERS - SPAN (1-2)

All dimensions are out to out.

Hooks and bends shall be in accordance with the CRSI Manual of Standard Practice for Detailing Reinforced Concrete Structures, Stirrup and Tie Dimensions.

Actual bar lengths are measured along centerline of bar to the nearest inch.

Minimum clearance to reinforcing shall be 1", unless otherwise shown.

All bar reinforcement shall be Grade 60.

WWR shall not be epoxy coated.

General Notes:

Concrete for prestressed beams shall be Class A-1 with $f'c = 8000$ psi and $f'ci = 6500$ psi.

Use 22 strands, 0.6" \bar{c} Grade 270, with an initial prestress force of 967 Kips.

Pretensioned members shall be in accordance with Sec 1029.

Fabricator shall be responsible for location and design of lifting devices.

Exterior and interior girders are the same except: application of bond breaker, coil inserts for slab drains.

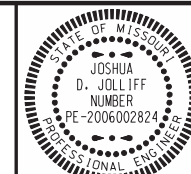
The contractor shall provide bracing necessary for lateral and torsional stability of the girders during construction of the concrete slab and remove the bracing after the slab has attained 75% design strength. Contractor shall not drill holes in the girders.

For Girder Camber Diagram, see Sheet No. 14.

For location of coil inserts at slab drains, see Sheet No. 13.

For location of coil ties at integral bents see Sheets No. 4 and 8.

Alternate bar reinforcing steel details are provided and may be used. The same type of reinforcing steel shall be used for all girders in all spans.



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DATE PREPARED
3/3/2023

ROUTE
67

DISTRICT
BR

COUNTY
BUTLER

JOB NO.
J9P3751

CONTRACT ID.

PROJECT NO.

BRIDGE NO.
A9369

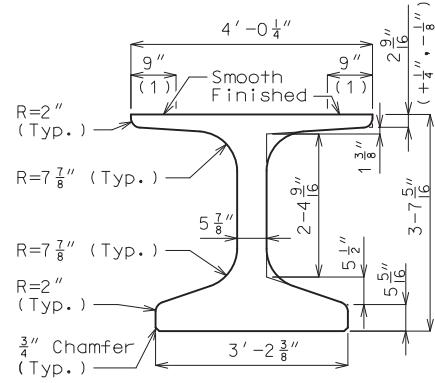
DESCRIPTION	DATE

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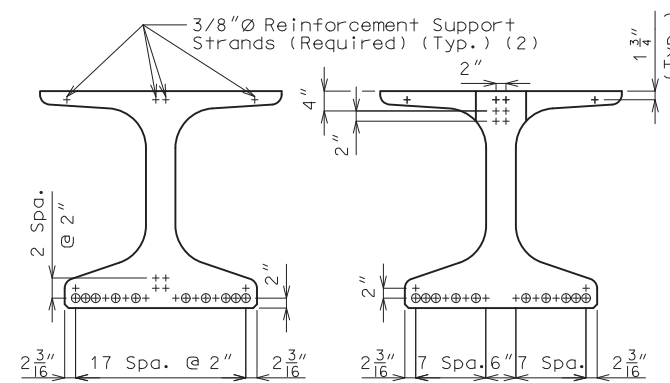
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(1) Fabricator shall apply a bond breaker to this region excluding where joint filler will be applied.



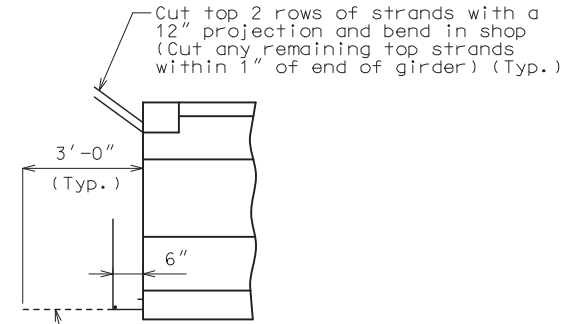
DIMENSIONS

(2) Outer strands tensioned to 2.02 kips/strand and inner strands to 8 kips/strand. Placed symmetrical about \bar{c} Girder. May be moved laterally in pairs.

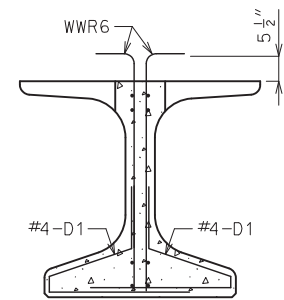


G GIRDER STRAND ARRANGEMENT

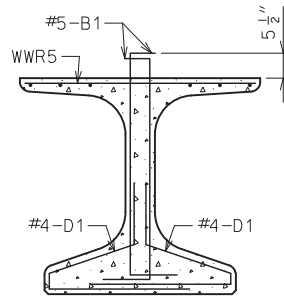
+ Indicates prestressing strand. o Indicates cut & shop bend with 3'-0" projection.



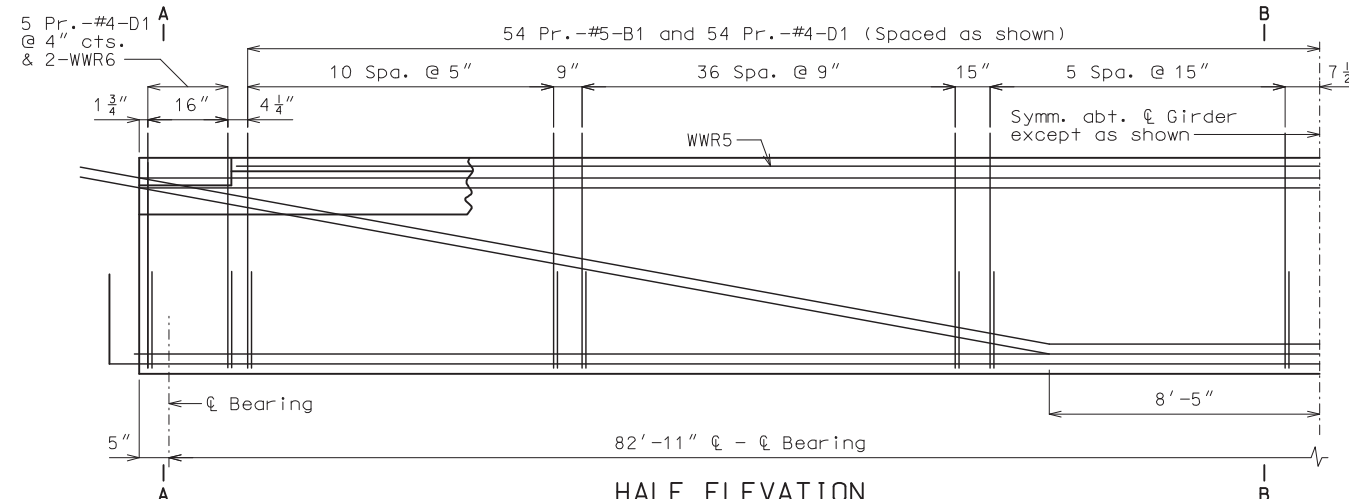
STRANDS AT GIRDER ENDS



SECTION A-A
Strands not shown for clarity.

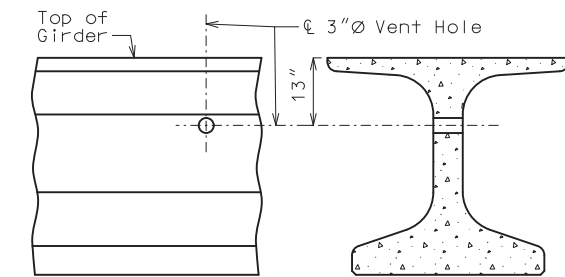


SECTION B-B
Strands not shown for clarity.



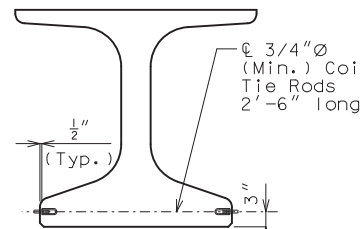
HALF ELEVATION

Reinforcement support strands not shown for clarity.

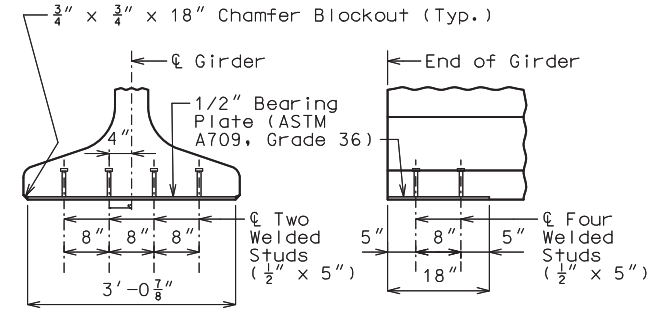


VENT HOLE

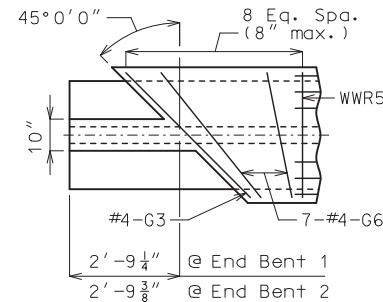
Place vent holes at or near upgrade 1/3 point of girders and clear reinforcing steel or strands by 1 1/2" minimum.



INTEGRAL BENTS COIL TIES



BEARING PLATE

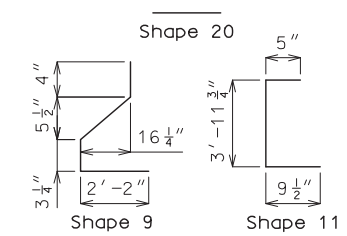


TOP FLANGE BLOCKOUT

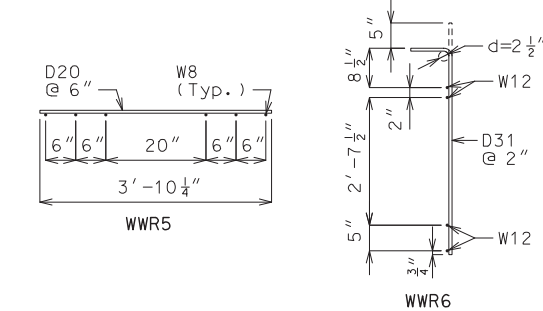
Bill of Reinforcing Steel - Each Girder

No.	Size/Mark	Length	Shape
216	5 B1	5'-0"	11
236	4 D1	4'-0"	9
2	4 G3	5'-5"	20
14	4 G6	Varies	20

Bending Diagrams



Welded Wire Reinforcement - Each Girder



All dimensions are out to out.

Hooks and bends shall be in accordance with the CRSI Manual of Standard Practice for Detailing Reinforced Concrete Structures, Stirrup and Tie Dimensions.

Actual bar lengths are measured along centerline of bar to the nearest inch.

Minimum clearance to reinforcing shall be 1".

All bar reinforcement shall be Grade 60.

The two D1 bars may be furnished as one bar at the fabricator's option.

All B1 bars shall be epoxy coated.

General Notes:

Concrete for prestressed girders shall be Class A-1 with $f'c = 8000$ psi and $f'ci = 6500$ psi.

Use 22 strands, 0.6"Ø Grade 270, with an initial prestress force of 967 kips.

Pretensioned members shall be in accordance with Sec 1029.

Fabricator shall be responsible for location and design of lifting devices.

Exterior and interior girders are the same except: application of bond breaker, coil inserts for slab drains.

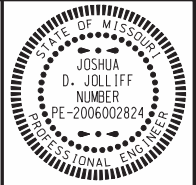
The contractor shall provide bracing necessary for lateral and torsional stability of the girders during construction of the concrete slab and remove the bracing after the slab has attained 75% design strength. Contractor shall not drill holes in the girders.

For Girder Camber Diagram, see Sheet No. 14.

For location of coil inserts at slab drains, see Sheet No. 13.

For location of coil ties at integral bent, see Sheets No. 4 and 8.

Alternate bar reinforcing steel details are provided and may be used. The same type of reinforcing steel shall be used for all girders in all spans.



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ROUTE 67 STATE MO

DISTRICT BR SHEET NO. 11

COUNTY BUTLER

JOB NO. J9P3751

CONTRACT ID.

PROJECT NO.

BRIDGE NO. A9369

DESCRIPTION

DATE

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

105 WEST CAPITOL JEFFERSON CITY, MO 65102

1-888-ASK-MODOT (1-888-275-6636)

MoDOT

ENGINEERING CORPORATION - 000631

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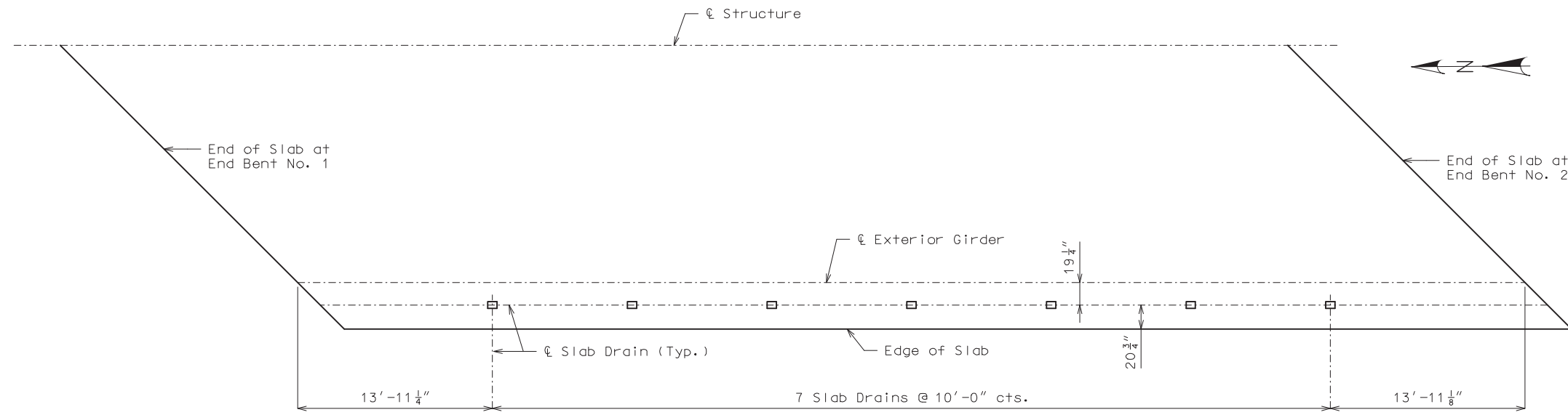
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REVISIONS



PART PLAN OF SLAB SHOWING SLAB DRAIN LOCATIONS
 (Right side shown, left side similar)
 Note: Longitudinal dimensions are horizontal.

General Notes:
 Contractor shall have the option to construct either steel or FRP slab drains. All drains shall be of same type.

Slab drain bracket assembly shall be ASTM A709 Grade 36 steel.

Locate drains in slab by dimensions shown in Part Section Near Drain.

Reinforcing steel shall be shifted to clear drains.

The coil inserts and bracket assembly shall be galvanized in accordance with ASTM A123.

All bolts, hardened washers, lock washers and nuts shall be galvanized in accordance with AASHTO M 232 (ASTM A153), Class C.

All 1/2"Ø bolts shall be ASTM A307.

Shop drawings will not be required for the slab drains and the bracket assembly.

The coil insert required for the bracket assembly attachment shall be located on the prestressed girder shop drawings.

Coil inserts shall have a concrete pull-out strength (ultimate load) of at least 2,500 pounds in 5,000 psi concrete.

The bolt required to attach the slab drain bracket assembly to the prestressed girder web shall be supplied by the prestressed girder fabricator.

Notes for Steel Drain:
 Slab drains may be fabricated of either 1/4" welded sheets of ASTM A709 Grade 36 steel or from 1/4" structural steel tubing ASTM A500 or A501.

Outside dimensions of drains are 8" x 4".

The drains shall be galvanized in accordance with ASTM A123.

Notes for FRP Drain:
 Drains shall be machine filament-wound thermosetting resin tubing meeting the requirements of ASTM D2996 with the following exceptions:

Shape of drains shall be rectangular with outside nominal dimensions of 8" x 4".

Minimum reinforced wall thickness shall be 1/4 inch.

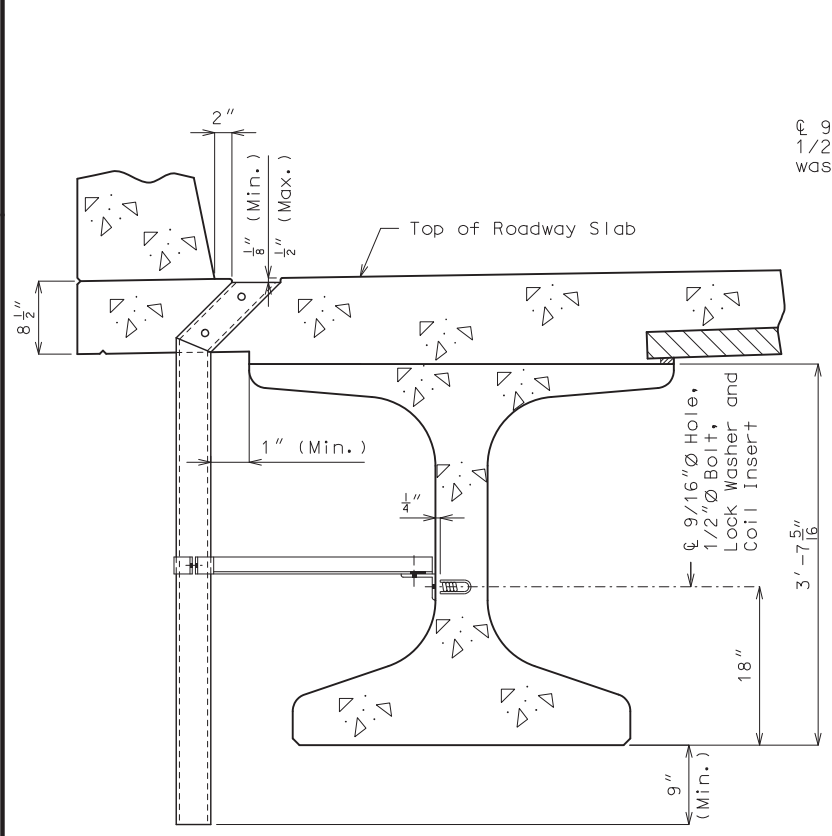
The resin used shall be ultraviolet (UV) resistant and/or have UV inhibitors mixed throughout. Drains may have an exterior coating for additional UV resistance.

The color of the slab drain shall be gray (Federal Standard 26373). The color shall be uniform throughout the resin and any coating used.

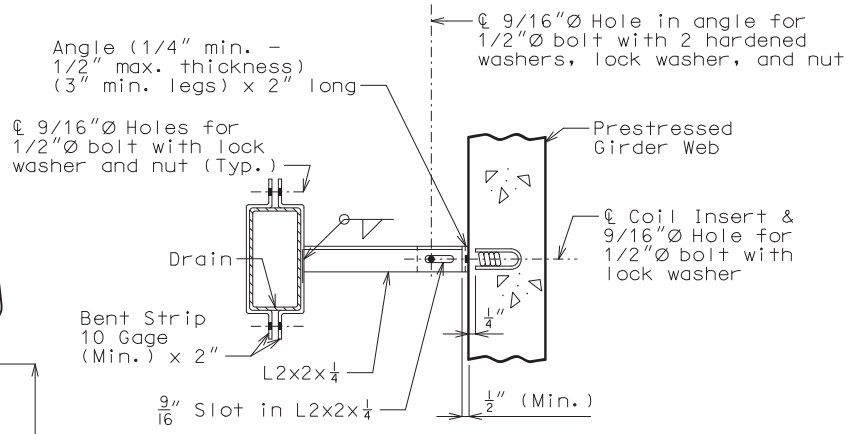
The combination of materials used in the manufacture of the drains shall be tested for UV resistance in accordance with ASTM D4329 Cycle A. The representative material shall withstand at least 500 hours of testing with only minor discoloration and without any physical deterioration. The contractor shall furnish the results of the required ultraviolet testing prior to acceptance of the slab drains.

At the contractor's option, drains may be field cut. The method of cutting FRP slab drain shall be as recommended by the manufacturer to ensure a smooth, chip free cut.

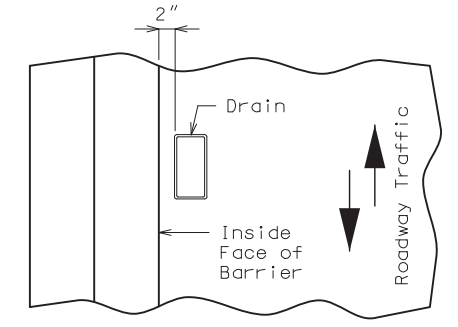
Both upper and lower drain pieces shall be rigidly connected to each other. Drain flow shall not be obstructed. Approval of the engineer is required.



PART SECTION NEAR DRAIN

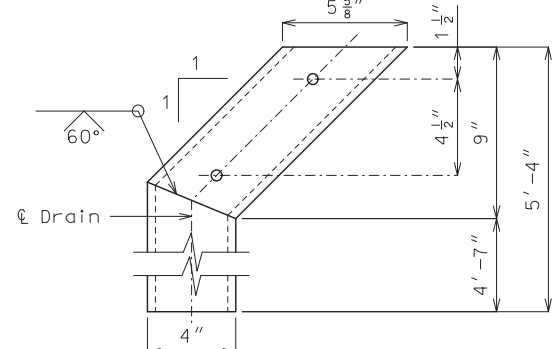


PART SECTION SHOWING BRACKET ASSEMBLY

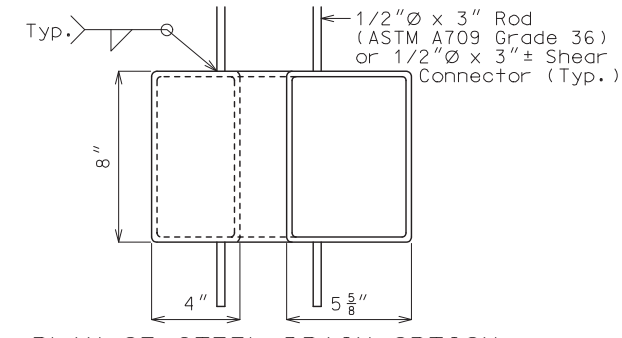


PART PLAN OF SLAB AT DRAIN

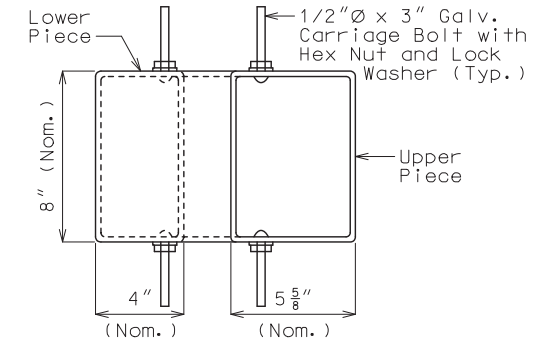
SLAB DRAINS



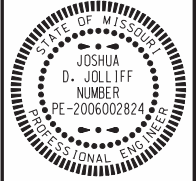
ELEVATION OF DRAIN



PLAN OF STEEL DRAIN OPTION



PLAN OF FRP DRAIN OPTION



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DATE PREPARED: 3/3/2023
 ROUTE: 67 STATE: MO
 DISTRICT: BR SHEET NO.: 13

COUNTY: BUTLER
 JOB NO.: J9P3751
 CONTRACT ID.

PROJECT NO.
 BRIDGE NO.: A9369

DESCRIPTION	DATE

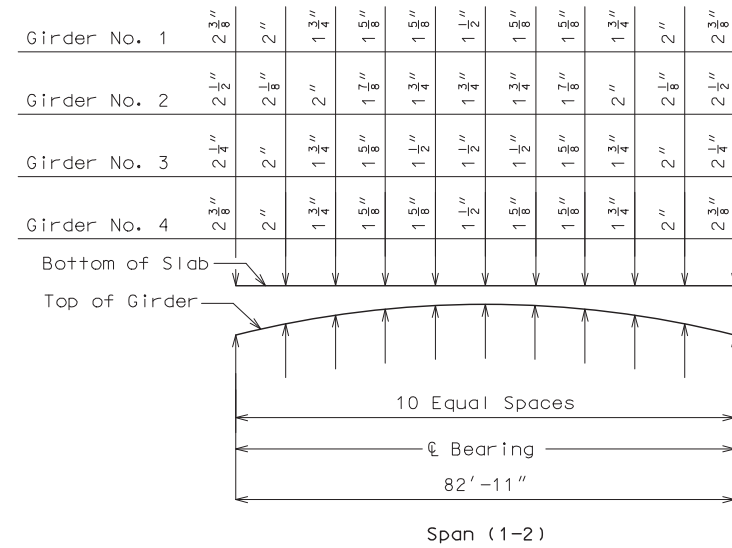
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Detailed Jan. 2023
 Checked Jan. 2023

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 13 of 24



THEORETICAL SLAB HAUNCHING DIAGRAM

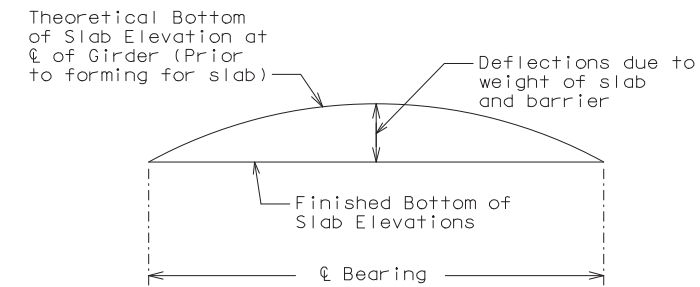
If girder camber is different from that shown in the camber diagram, in order to maintain minimum slab thickness, an adjustment of the slab haunches, an increase in slab thickness or a raise in grade uniformly throughout the structure shall be necessary. No payment will be made for additional labor or materials required for variation in haunching, slab thickness or grade adjustment.

Concrete in the slab haunches is included in the Estimated Quantities for Slab on Concrete NU-Girder.

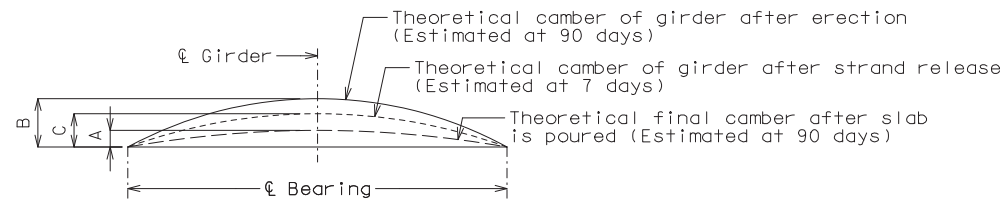
Theoretical Bottom of Slab Elevations at Centerline of Girder (Prior to forming for slab) (Estimated at 90 days)

Girder Number	Span (1-2) (82'-11" C Brg. - C Brg.)										
	C Brg.	.10	.20	.30	.40	.50	.60	.70	.80	.90	C Brg.
1	307.35	307.36	307.37	307.37	307.36	307.35	307.32	307.29	307.24	307.20	307.15
2	307.49	307.50	307.51	307.52	307.51	307.49	307.47	307.43	307.39	307.34	307.29
3	307.47	307.48	307.49	307.50	307.49	307.47	307.45	307.41	307.37	307.32	307.27
4	307.29	307.30	307.31	307.31	307.30	307.29	307.26	307.23	307.18	307.14	307.09

Elevations are based on a constant slab thickness of 8 1/2" and include allowance for theoretical dead load deflections due to weight of slab (including precast panel) and barrier.



TYPICAL SLAB ELEVATIONS DIAGRAM



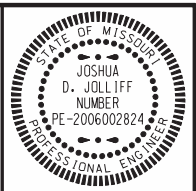
Girder	Span (1-2)		
	A	B	C
Exterior	7/8"	2"	1 1/8"
Interior	3/4"		

GIRDER CAMBER DIAGRAM

Conversion factors for girder camber (Estimated at 90 days):

- 0.1 pt. = 0.314 x 0.5 pt.
- 0.2 pt. = 0.593 x 0.5 pt.
- 0.3 pt. = 0.813 x 0.5 pt.
- 0.4 pt. = 0.952 x 0.5 pt.

TBOS, HAUNCHING AND CAMBER



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3/3/2023

ROUTE 67 STATE MO

DISTRICT BR SHEET NO. 14

COUNTY BUTLER

JOB NO. J9P3751

CONTRACT ID.

PROJECT NO.

BRIDGE NO. A9369

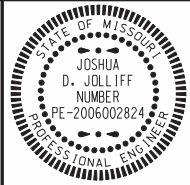
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DISTRICT BR SHEET NO. 15

COUNTY BUTLER
JOB NO. J9P3751
CONTRACT ID.
PROJECT NO.
BRIDGE NO. A9369

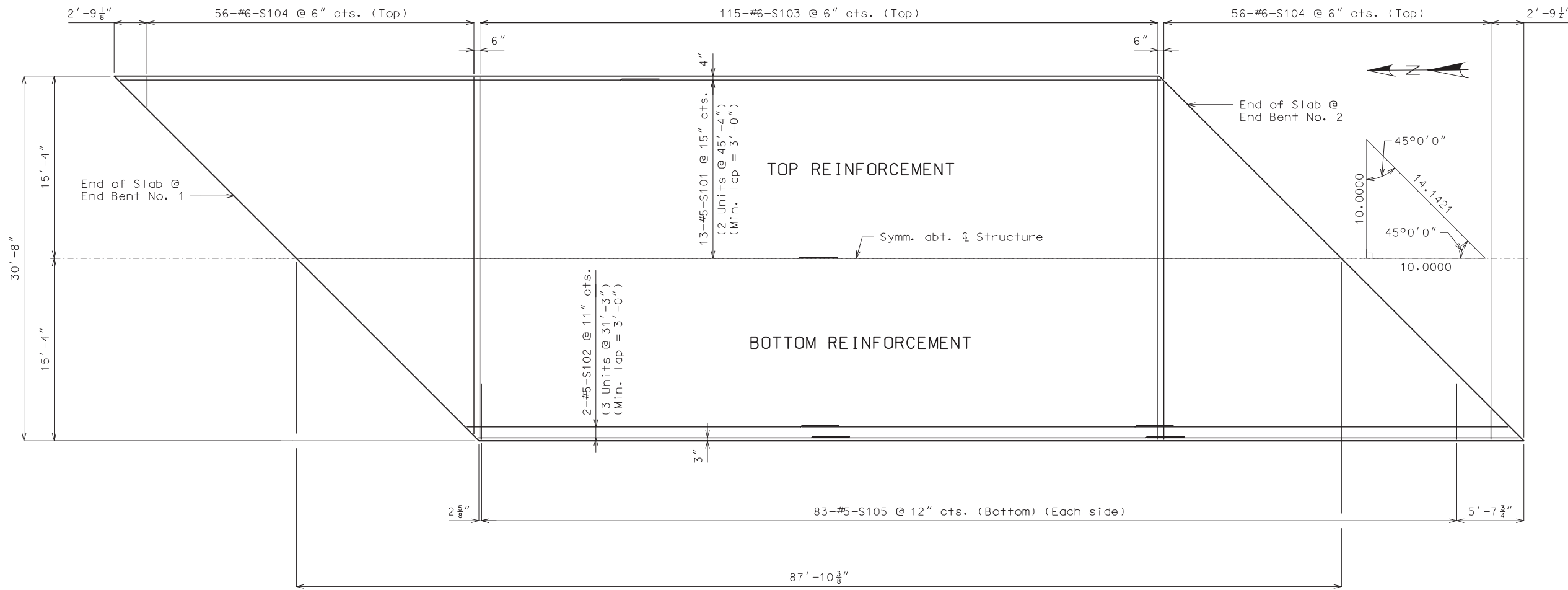
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PLAN OF SLAB SHOWING REINFORCEMENT

SLAB REINFORCEMENT DETAILS

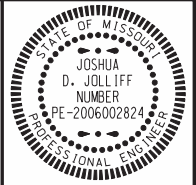
- General Notes:**
- Longitudinal slab dimensions shown are horizontally.
 - For details of Precast Prestressed Panels, see Sheet No. 12.
 - For Theoretical Bottom of Slab Elevations, Girder Camber Diagram, and Theoretical Slab Haunching Diagram, see Sheet No. 14.
 - For Section Thru Slab, see Sheet No. 16.
 - For details and reinforcement of Type D Barrier not shown, see Sheets No. 17 and 18.

Detailed Jan. 2023
Checked Jan. 2023

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 15 of 24

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ROUTE 67 STATE MO
DISTRICT BR SHEET NO. 16

COUNTY BUTLER
JOB NO. J9P3751
CONTRACT ID.

PROJECT NO.
BRIDGE NO. A9369

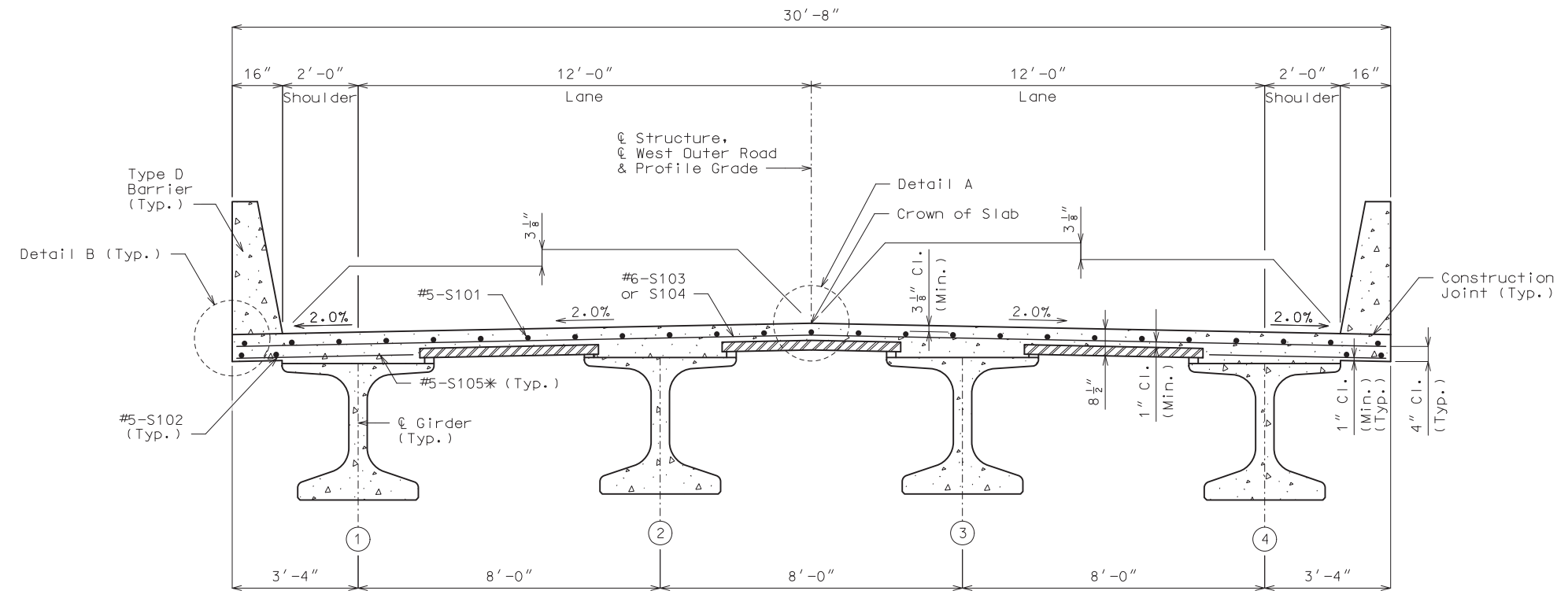
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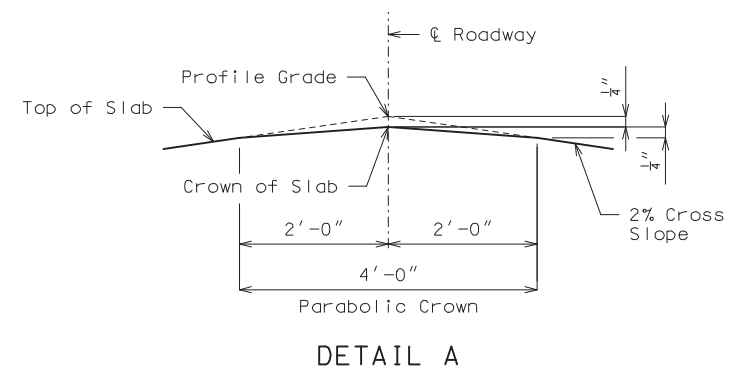
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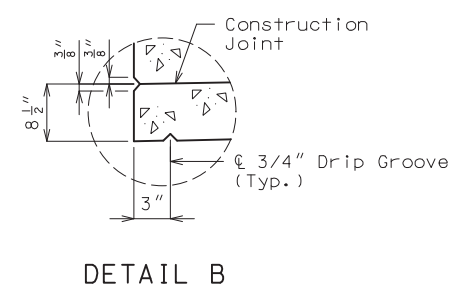


SECTION THRU SLAB

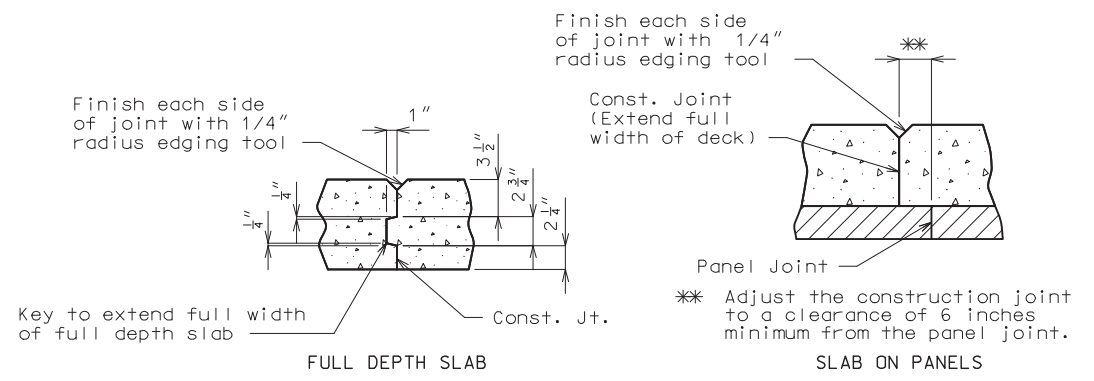
* Alternate bar shape available, see barrier sheets.



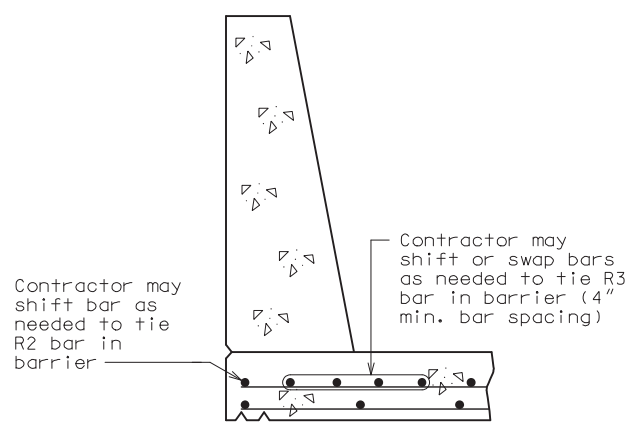
DETAIL A



DETAIL B



SLAB CONSTRUCTION JOINT
(If needed)



OPTIONAL SHIFTING TOP BARS AT BARRIER

General Notes:

- For details and reinforcement of Type D Barrier not shown, see Sheets No. 17 & 18.
- For details of Precast Prestressed Panels, see Sheet No. 12.
- For Theoretical Bottom of Slab Elevations, Girder Camber Diagram and Theoretical Slab Haunching Diagram, see Sheet No. 14.
- For Plan of Slab Showing Reinforcement, see Sheet No. 15.
- The contractor shall pour and satisfactorily finish the roadway slab at a rate of not less than 25 cubic yards per hour.
- The contractor shall furnish an approved retarder to retard the set of the concrete to 2.5 hours and shall pour and satisfactorily finish the slab pours at the rate given.
- The concrete diaphragm at the integral end bents shall be poured a minimum of 30 minutes and a maximum of 2 hours before the slab is poured.

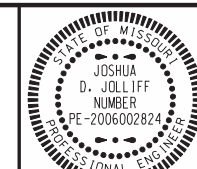
DETAILS OF SLAB REINFORCING

Detailed Jan. 2023
Checked Jan. 2023

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 16 of 24

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3/3/2023

ROUTE 67 STATE MO
DISTRICT BR SHEET NO. 17

COUNTY BUTLER
JOB NO. J9P3751
CONTRACT ID.

PROJECT NO.
BRIDGE NO. A9369

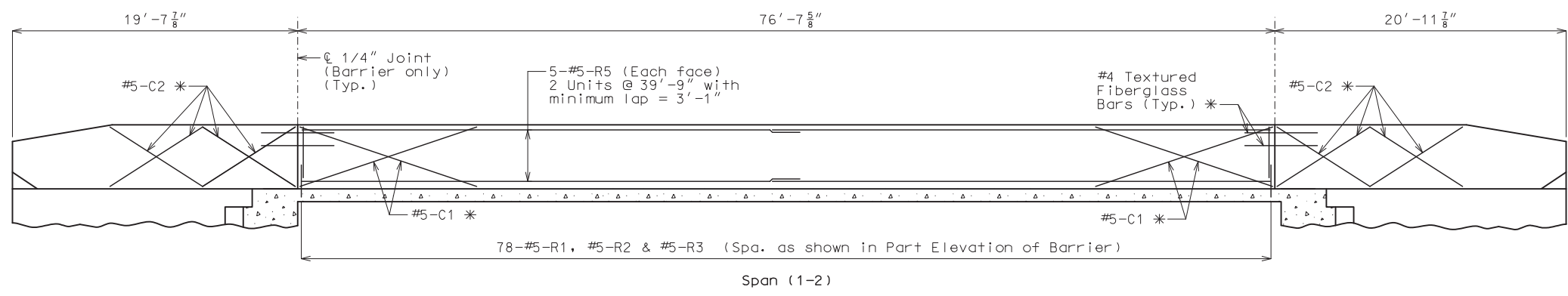
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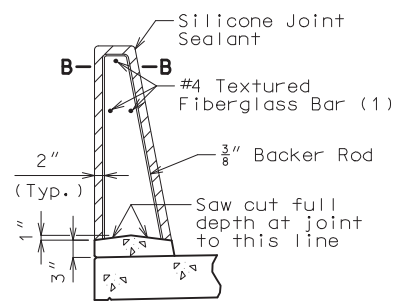
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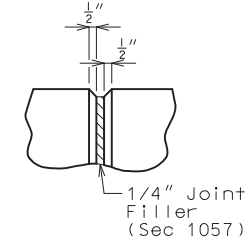
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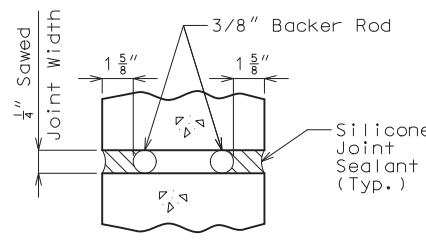
ELEVATION OF BARRIER
 (Left barrier shown, right barrier similar by 180° rotation)
 Longitudinal dimensions are horizontal.



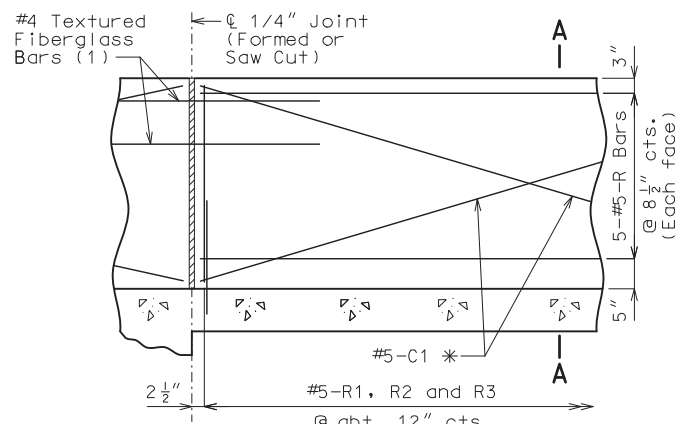
SECTION THRU SAW CUT JOINT



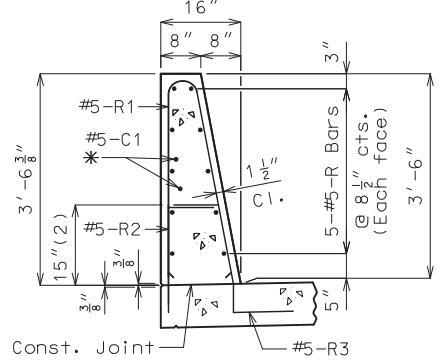
PART ELEVATION AT FORMED JOINT



SECTION B-B

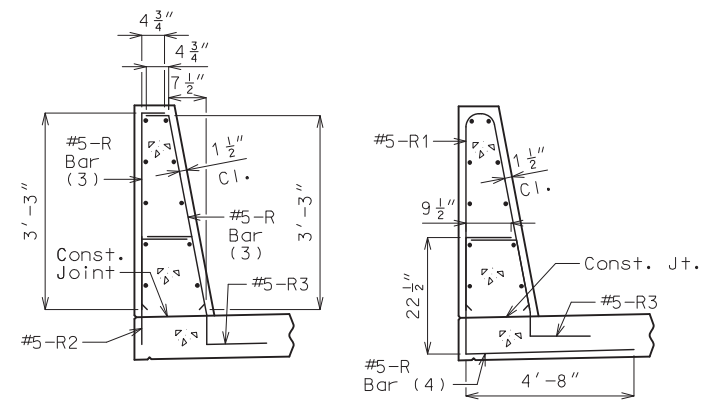


PART ELEVATION OF BARRIER
 (1) Four feet long, centered on joint, slip-formed option only



SECTION A-A

Use a minimum lap of 3'-1" for #5 horizontal barrier bars.
 The cross-sectional area above the slab is 3.52 square feet.
 (2) To top of bar



R-BAR PERMISSIBLE ALTERNATE SHAPE

(3) The R1 bar may be separated into two bars as shown, at the contractor's option, only when slip forming is not used. (All dimensions are out to out.)
 (4) The R2 bar and #5 bottom transverse slab bar in cantilever (prestressed panels only) combination may be furnished as one bar as shown, at the contractor's option.

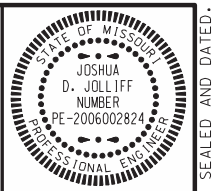
TYPE D BARRIER

General Notes:

- * Slip-formed option only.
- Conventional forming or slip forming may be used. Saw cut joints may be used with conventional forming.
- Top of barrier shall be built parallel to grade and barrier joints (except at end bents) normal to grade.
- All exposed edges of barrier shall have either a 1/2-inch radius or a 3/8-inch bevel, unless otherwise noted.
- Payment for all concrete and reinforcement, complete in place, will be considered completely covered by the contract unit price for Type D Barrier per linear foot.
- Concrete in barrier shall be Class B-1.
- Measurement of barrier is to the nearest linear foot for each structure, measured along the outside top of slab from end of wing to end of wing.
- Concrete traffic barrier delineators shall be placed on top of the barrier as shown on Missouri Standard Plan 617.10 and in accordance with Sec 617. Delineators on bridges with two-lane, two-way traffic shall have retroreflective sheeting on both sides. Concrete traffic barrier delineators will be considered completely covered by the contract unit price for Type D Barrier.

Joint sealant and backer rods shall be in accordance with Sec 717 for silicone joint sealant for saw cut and formed joints.

For slip-formed option, both sides of barrier shall have a vertically broomed finish and the top shall have a transversely broomed finish.



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DATE PREPARED 3/3/2023

ROUTE 67 STATE MO

DISTRICT BR SHEET NO. 18

COUNTY BUTLER

JOB NO. J9P3751

CONTRACT ID.

PROJECT NO.

BRIDGE NO. A9369

DESCRIPTION

DATE

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

105 WEST CAPITOL JEFFERSON CITY, MO 65102

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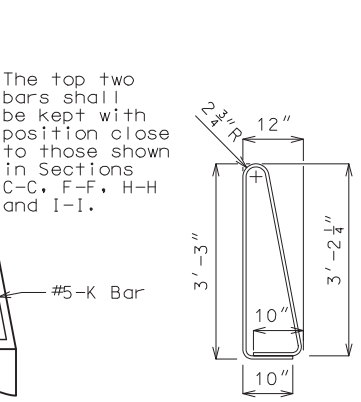
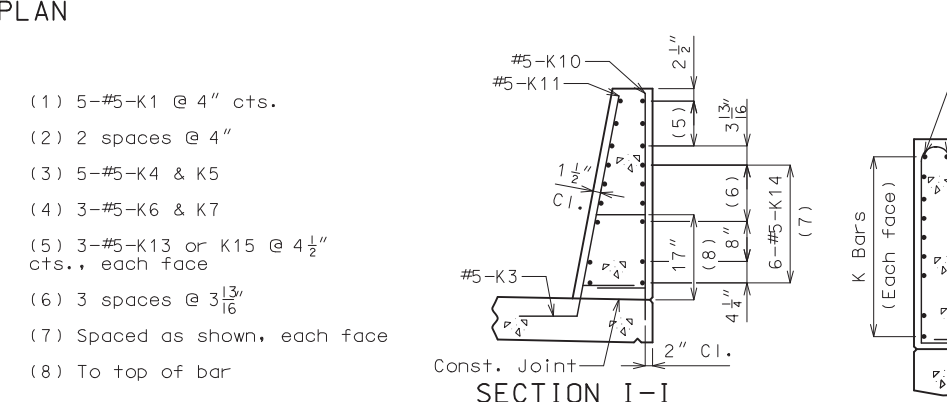
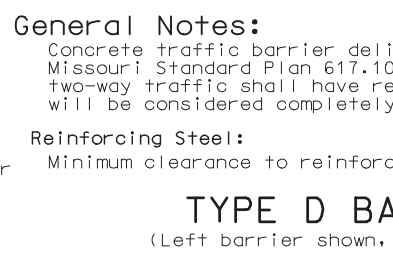
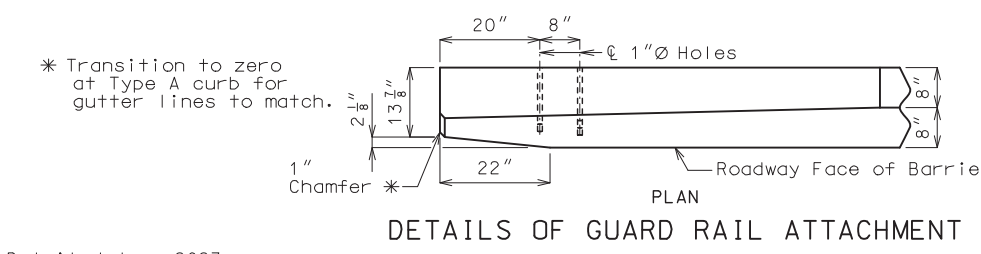
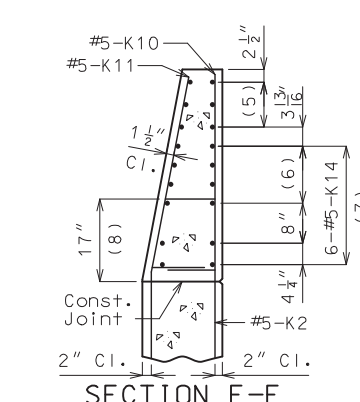
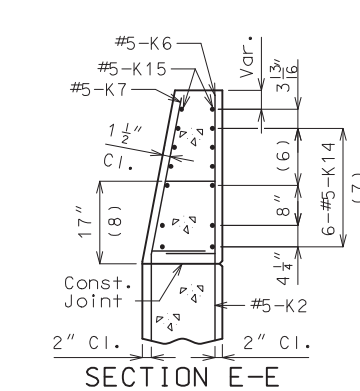
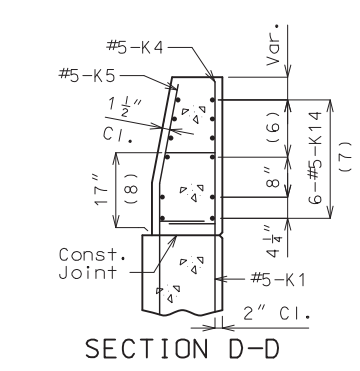
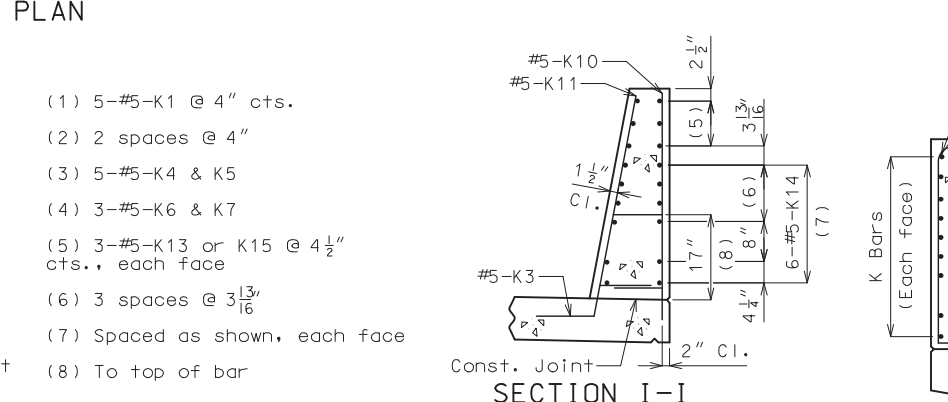
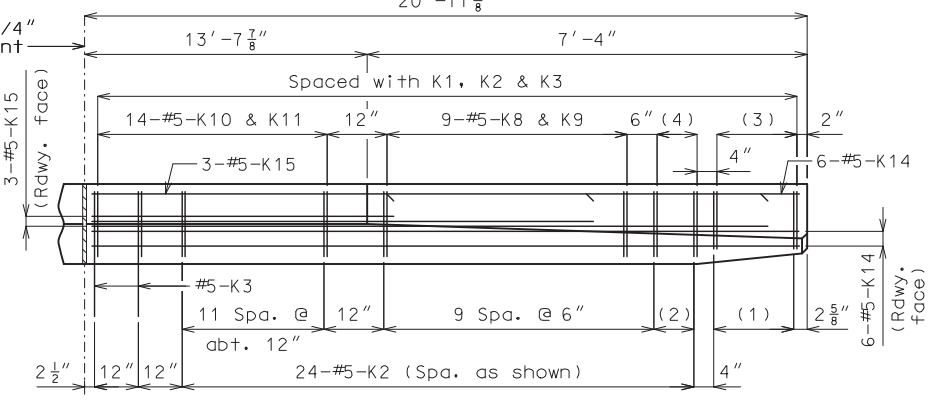
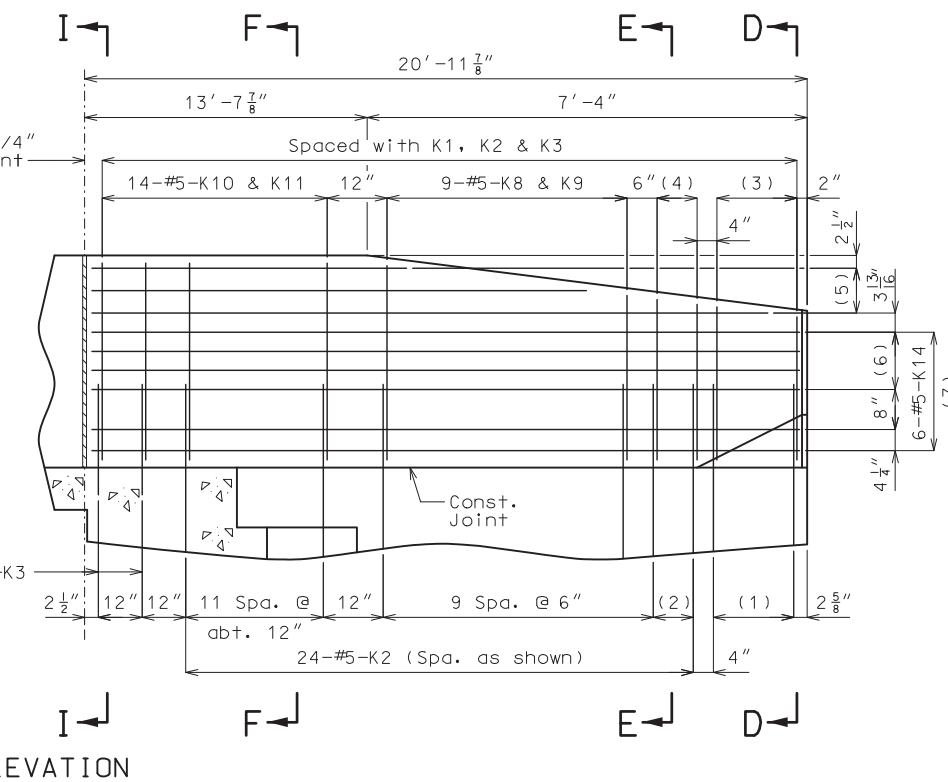
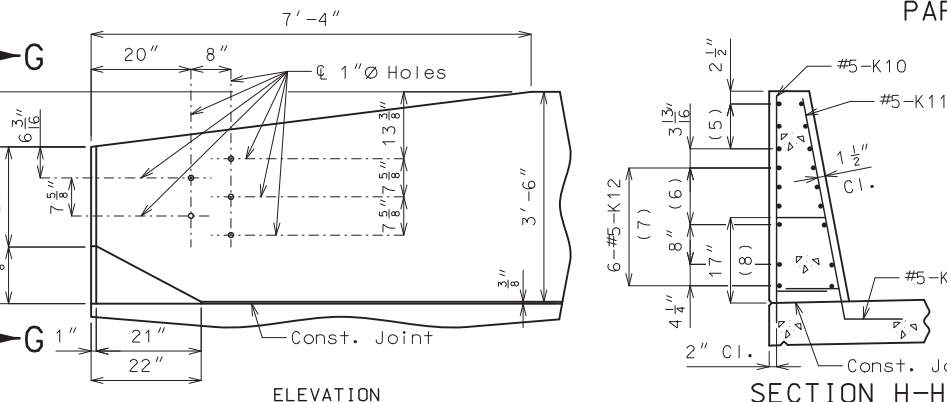
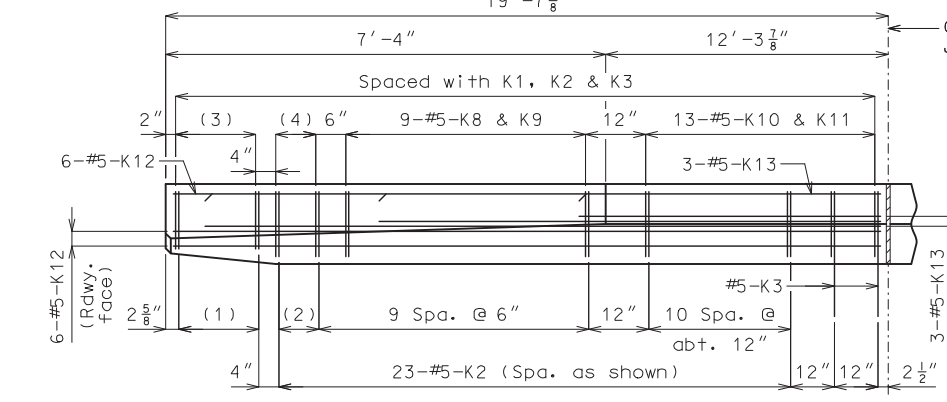
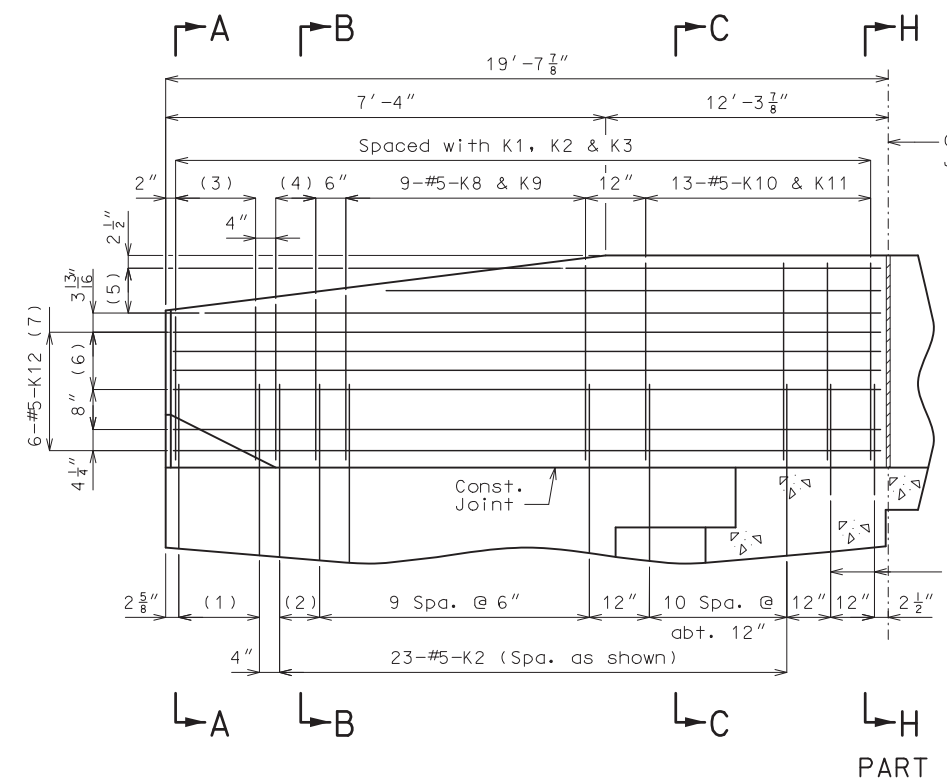
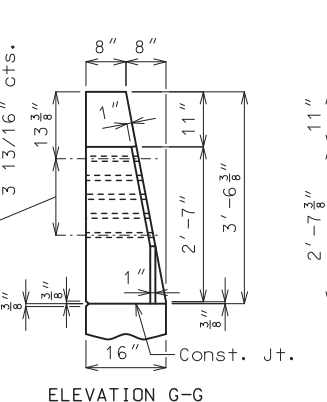
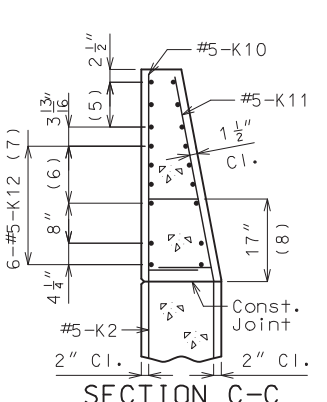
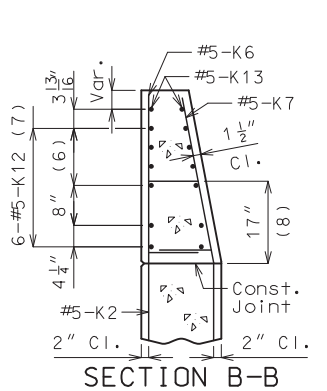
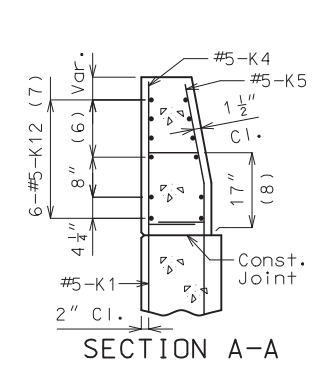
CRAWFORD, MURPHY & TILLY, INC.

6651 WESLEYAN DRIVE, SPRINGFIELD, MO 65807 (417) 869-6009

ENGINEERING CORPORATION - 000631

REV.

IF A SEAL IS PRESENT ON THIS SHEET IT HAS BEEN ELECTRONICALLY SEALED AND DATED.



General Notes:
Concrete traffic barrier delineators shall be placed on top of the barrier as shown on Missouri Standard Plan 617.10 and in accordance with Sec 617. Delineators on bridges with two-lane, two-way traffic shall have retroreflective sheeting on both sides. Concrete traffic barrier delineators will be considered completely covered by the contract unit price for Type D Barrier.

Reinforcing Steel:
Minimum clearance to reinforcing steel shall be 1 1/2" except as shown for bars embedded into end bent.

- (1) 5-#5-K1 @ 4" cts.
- (2) 2 spaces @ 4"
- (3) 5-#5-K4 & K5
- (4) 3-#5-K6 & K7
- (5) 3-#5-K13 or K15 @ 4 1/2" cts., each face
- (6) 3 spaces @ 3 13/16"
- (7) Spaced as shown, each face
- (8) To top of bar

The top two bars shall be kept with position close to those shown in Sections C-C, F-F, H-H and I-I.

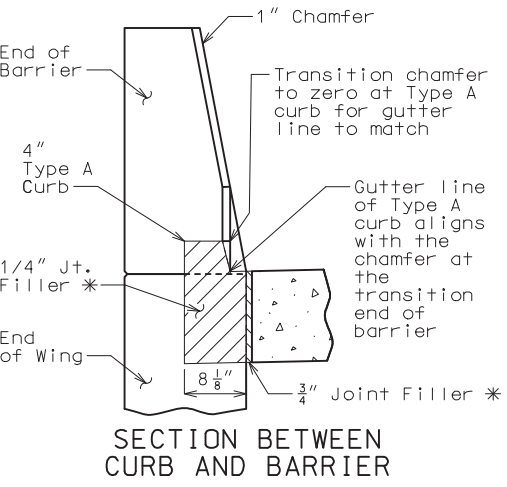
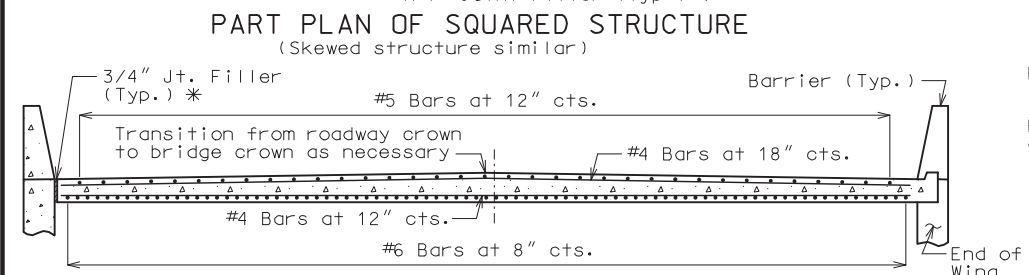
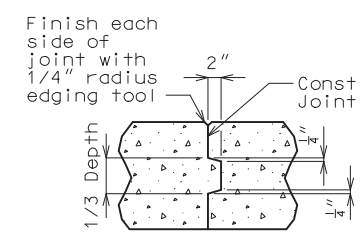
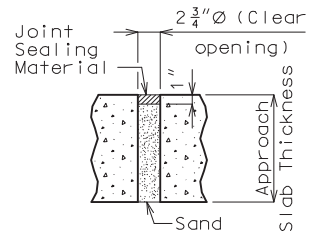
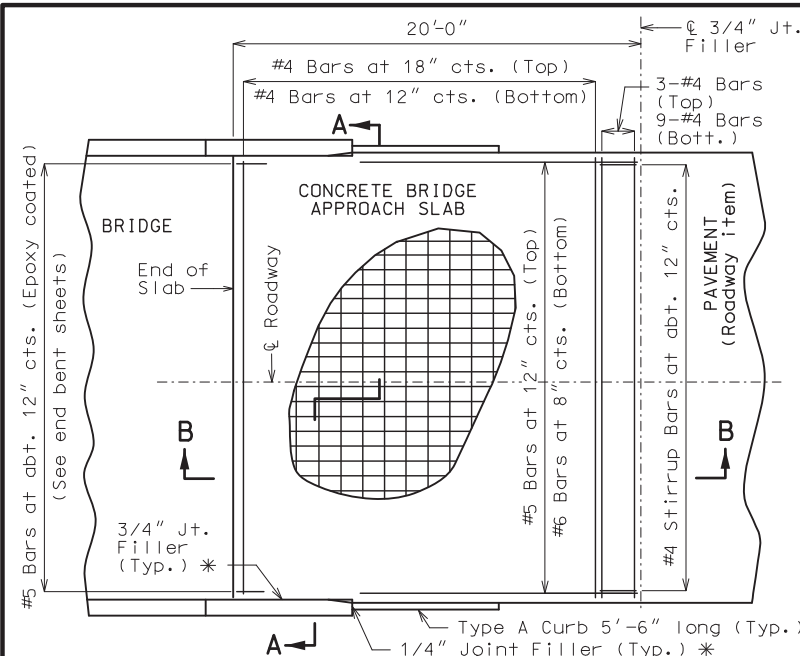
K Bars (Each face)
#5-K Bar

TYPE D BARRIER AT END BENTS
(Left barrier shown, right barrier similar by 180° rotation)

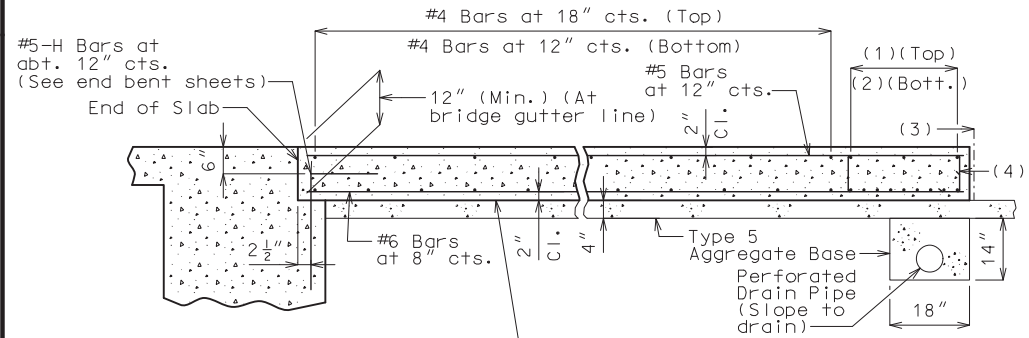
Detailed Jan. 2023
Checked Jan. 2023

Note: This drawing is not to scale. Follow dimensions.

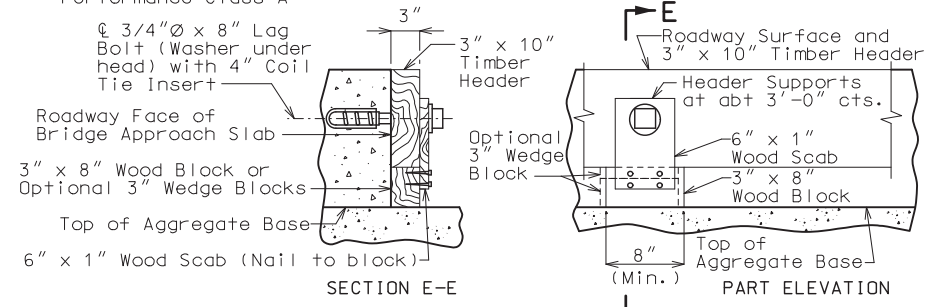
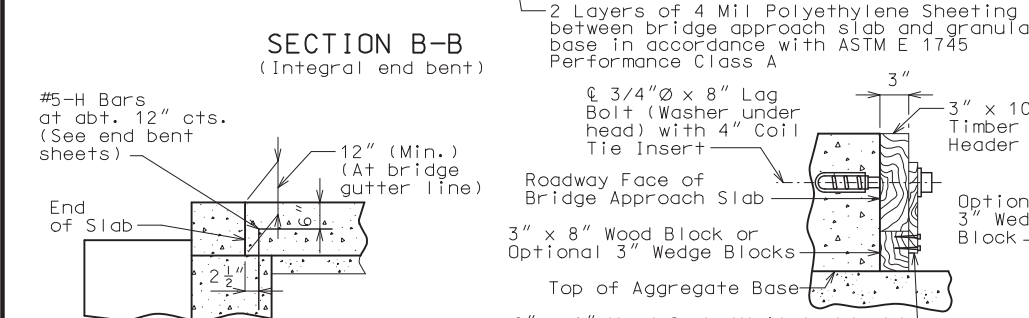
Sheet No. 18 of 24



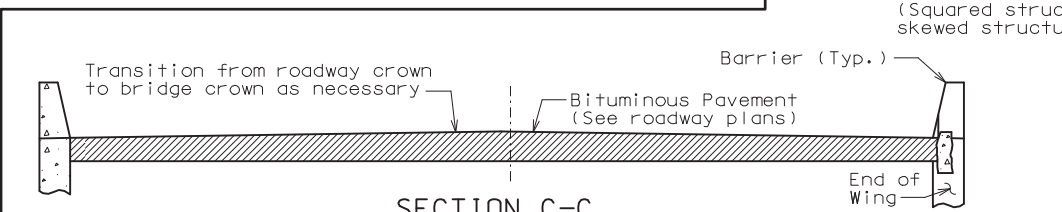
With the approval of the engineer, the contractor may crown the bottom of the approach slab to match the crown of the roadway surface.



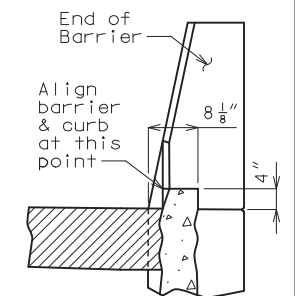
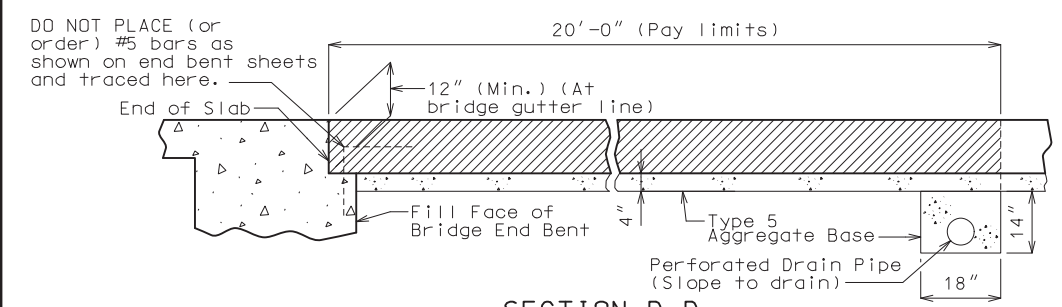
- (1) 3-#4 Bars
- (2) 9-#4 Bars
- (3) 3/4" Jt. Filler
- (4) #4 Stirrup Bars at abt. 12" cts.; 2'-0" x 8" (Min.) out to out; Actual length = 5'-10" (Min.); 90° stirrup hook at bottom; Stirrup height (8") and actual length vary due to crown.



Remove timber header when concrete pavement is placed.
OPTIONAL CONCRETE SLAB



With the approval of the engineer, the contractor may crown the bottom of the approach slab to match the crown of the roadway surface.

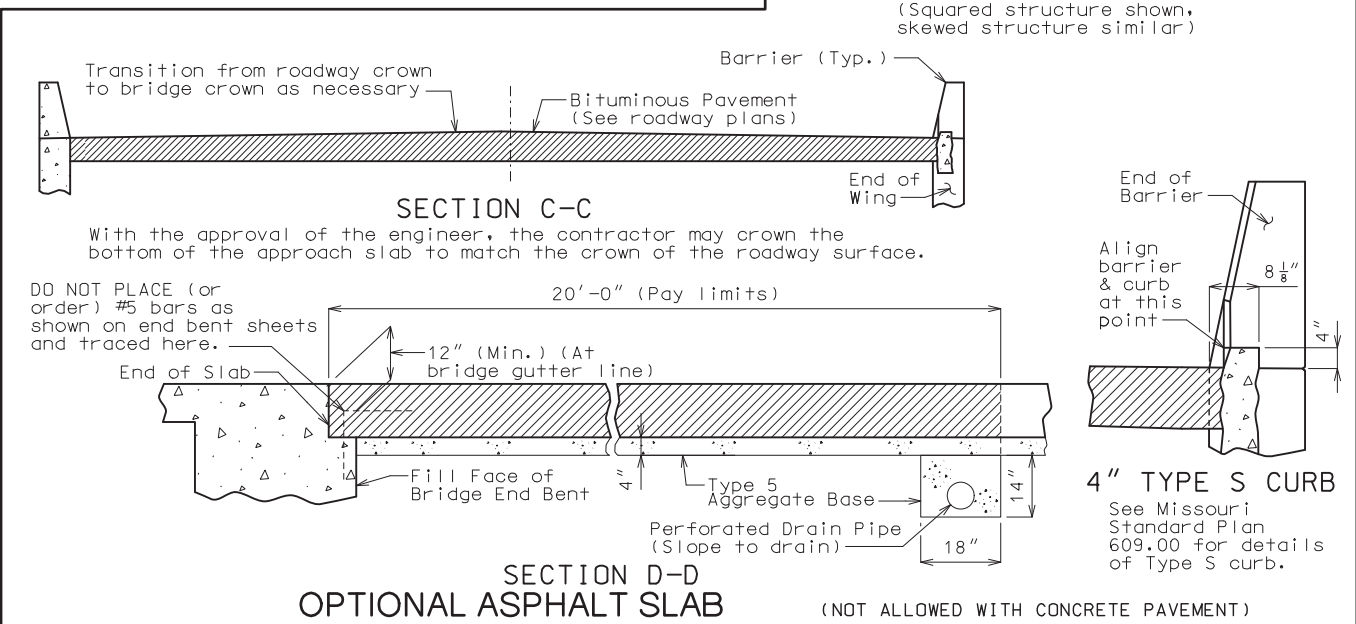
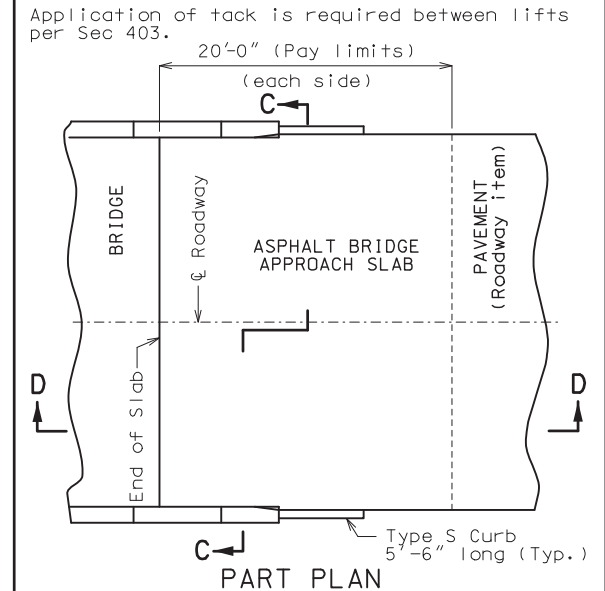


See Missouri Standard Plan 609.00 for details of Type S curb.

Notes For Concrete Slab Only:
 All concrete for the bridge approach slab shall be in accordance with Sec 503 ($f'c = 4,000$ psi).
 The reinforcing steel in the bridge approach slab shall be epoxy coated Grade 60 with $f_y = 60,000$ psi.
 Longitudinal construction joints in bridge approach slab shall be aligned with longitudinal construction joints in bridge slab.
 Minimum clearance to reinforcing steel shall be 1 1/2", unless otherwise shown.
 The reinforcing steel in the bridge approach slab shall be continuous. The transverse reinforcing steel may be made continuous by providing a minimum lap splice of 23 inches for #4 bars, or by mechanical bar splice.
 Mechanical bar splices shall be in accordance with Sec 710.
 All joint filler shall be in accordance with Sec 1057 for preformed fiber expansion joint filler except as noted.
 Payment for furnishing all materials, labor and excavation necessary to construct the concrete bridge approach slab, including the timber header, underdrain, Type 5 aggregate base, joint filler, and all other appurtenances and incidental work as shown on this sheet, complete in place, will be considered completely covered by the contract unit price for Bridge Approach Slab (Minor) per square yard.
 * Seal joint between vertical face of approach slab and wing with sealant in accordance with Sec 717 for silicone joint sealant for saw cut and formed joints.

General Notes:
 Contractor shall have the option to construct either slab except as noted.
 The contractor shall pour and satisfactorily finish the bridge slab before placing the bridge approach slab.
 Drain pipe may be either 6" diameter corrugated metallic-coated pipe underdrain, 4" diameter corrugated polyvinyl chloride (PVC) drain pipe, or 4" diameter corrugated polyethylene (PE) drain pipe.
 MoDOT Construction personnel will indicate the bridge approach slab used for this structure:
 Concrete Bridge Approach Slab
 Asphalt Bridge Approach Slab

Notes For Asphalt Slab Only:
 Payment for furnishing all materials, labor and excavation necessary to construct the asphalt bridge approach slab, including tack, curb, underdrain and Type 5 aggregate base within the pay limits shown, complete in place, will be considered completely covered by the contract unit price for Bridge Approach Slab (Minor) per square yard.
 Application of tack is required between lifts per Sec 403.



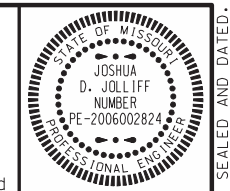
DO NOT PLACE (or order) #5 bars as shown on end bent sheets and traced here.
 With the approval of the engineer, the contractor may crown the bottom of the approach slab to match the crown of the roadway surface.

BRIDGE APPROACH SLAB (MINOR)
 Integral end bents shown, non-integral end bent similar.

Detailed Jan. 2023
 Checked Jan. 2023

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 19 of 24



THIS SHEET HAS BEEN SIGNED, SEALED AND DATED ELECTRONICALLY.

DATE PREPARED: 3/3/2023

ROUTE	STATE
67	MO
DISTRICT	SHEET NO.
BR	19

COUNTY: BUTLER
 JOB NO.: J9P3751
 CONTRACT ID.:

PROJECT NO.:

BRIDGE NO.: A9369

DATE	DESCRIPTION

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

MoDOT

105 WEST CAPITOL
 JEFFERSON CITY, MO 65102
 1-888-ASK-MODOT (1-888-275-6636)

CMT

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 CRAWFORD, MURPHY & TILLY, INC.
 6651 WESLEYAN DRIVE, SUITE 300
 SPRINGFIELD, MO 65807 (417) 869-6009
 ENGINEERING CORPORATION - 000631

IF A SEAL IS PRESENT ON THIS SHEET IT HAS BEEN ELECTRONICALLY SEALED AND DATED.

BILL OF REINFORCING STEEL

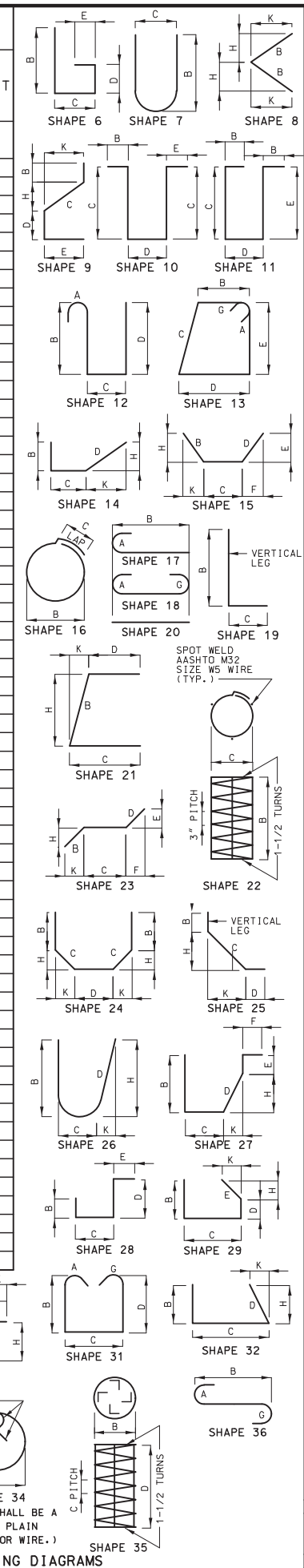
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									B		C		D		E					F		H		K	
									FT.	IN.	FT.	IN.	FT.	IN.	FT.	IN.				FT.	IN.	FT.	IN.	FT.	IN.
		END BENT 1																							
10	6 F100	WING BRACE	E 23						2	3.000	4	6.750	14.000	13.000	5.250	2	1.000	10.250	8	0	7	9	117		
10	6 F101	WING BRACE	E 15							14.000	9	6.000	2	3.000	10.250	2	1.000	13.000	5.250	12	11	12	10	193	
4	6 F102	DIAPHRAGM	E 23						4	6.000	5	1.000				3	2.250	3	2.250	9	7	9	6	58	
4	6 F103	DIAPHRAGM	E 21						7	10.750	5	1.000				5	7.000	5	7.000	13	0	12	9	77	
28	5 H101	DIAPHRAGM	E 19						2	0.000	15.000								3	3	3	2	93		
4	6 H102	DIAPHRAGM	E 20						43	0.000									43	0	43	0	259		
4	7 H103	DIAPHRAGM	E 20						43	0.000									43	0	43	0	352		
4	5 H104	DIAPHRAGM	E 23						15.250	3	3.000	15.250	10.750	10.750	10.750	10.750	10.750	5	10	5	9	24			
9	6 H105	DIAPHRAGM	E 20						9	9.000									9	9	9	9	132		
3	6 H106	DIAPHRAGM	E 20						6	5.000									6	5	6	5	29		
6	6 H107	DIAPHRAGM	E 20						3	9.000									3	9	3	9	34		
2	6 H108	DIAPHRAGM	E 20						2	1.000									2	1	2	1	7		
8	7 H109	BEAM	E 20						43	0.000									43	0	43	0	704		
4	6 H110	BEAM	E 20						43	0.000									43	0	43	0	259		
16	8 H151	WING WALL	E 20						16	6.000									16	6	16	6	705		
20	6 H152	WING WALL	E 6 S						15	8.000	12.000								16	8	16	7	499		
20	6 H153	WING WALL	E 6 S						15	8.000	12.000								16	8	16	7	499		
53	6 U101	DIAPHRAGM	E 19 S						3	3.000	7	0.000							10	3	10	2	810		
24	5 U102	DIAPHRAGM	E 10 S						4	1.000	4	7.000							12	9	12	7	315		
24	6 U103	DIAPHRAGM	E 19 S						3	0.250	5	3.000							8	4	8	2	295		
18	5 U104	BEAM	E 10 S						6	2.000	5	3.000							17	7	17	5	327		
26	4 U105	BEAM	E 13 S						5	3.000	2	8.000	5	3.000	2	8.000			16	7	16	4	284		
14	4 U106	BEAM	E 10 S						2	8.000	5	3.000							10	7	10	5	98		
28	6 V201	DIAPHRAGM	E 20						3	0.000									3	0	3	0	127		
12	5 V202	BEAM	E 20						6	2.000									6	2	6	2	78		
30	6 V251	WING WALL	E 20						7	1.000									7	1	7	1	320		
30	6 V252	WING WALL	E 20						7	0.000									7	0	7	0	316		
		SLAB																							
50	5 S101	SLAB	E 20						45	4.000									45	4	45	4	2365		
12	5 S102	SLAB	E 20						31	3.000									31	3	31	3	392		
115	6 S103	SLAB	E 20						30	5.000									30	5	30	5	5254		
112	6 S104	SLAB	E 20						V 2	29	11.500								30	0	30	0			
		INCR. = 0.5'									2	5.500							2	6	2	6	2734		
166	5 S105	SLAB	E 20						4	8.000									4	8	4	8	808		
		TYPE D BARRIER																							
156	5 R1	BARRIER	E 26						3	3.000	5.500	3	0.750			3	3.000	6.750	6	11	6	8	1085		
156	5 R2	BARRIER	E 19 S							20.500	9.500								2	6	2	5	394		
156	5 R3	BARRIER	E 27 S							9.500	15.250	5.000	12.000	15.000	3.000	3	6	3	4	543					
40	5 R5	BARRIER	E 20						39	9.000									39	9	39	9	1659		
20	5 K1	BARRIER	E 27 S						3	8.000	9.250	5.250	3	2.750			5.250	1.000	8	2	8	0	167		
94	5 K2	BARRIER	E 27 S						3	8.000	9.250	14.500	2	5.750			14.250	2.750	8	2	8	0	785		
8	5 K3	BARRIER	E 27 S							22.500	9.250	14.750	7.750	12.000	14.500	2.750	5	7	5	3	44				
20	5 K4	BARRIER	E 19 S						V 4	2	4.250	10.000							3	3	3	1			
		INCR. = 0.5"								2	6.250	10.000							3	5	3	3	67		
20	5 K5	BARRIER	E 14 S						V 4	8.250	9.500	18.500			4.000	18.000	3	1	2	11					
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12	5 K6	BARRIER	E 19 S						V 4	2	6.750	10.000							3	5	3	4			
		INCR. = 0.5"								2	7.750	10.000							3	6	3	5	43		
12	5 K7	BARRIER	E 21 S						V 4	2	6.750	10.000			2	6.000	6.250	3	5	3	4				
		INCR. = 0.5"								2	7.750	10.000			2	7.000	6.500	3	6	3	5	43			
36	5 K8	BARRIER	E 19 S						V 4	2	8.500	10.000							3	7	3	6			
		INCR. = 0.75"								3	2.500	10.000							4	1	4	0	141		
36	5 K9	BARRIER	E 21 S						V 4	2	8.500	10.000			2	7.750	6.750	3	7	3	6				
		INCR. = 0.75"								3	2.500	10.000			3	1.750	7.750	4	1	4	0	141			
54	5 K10	BARRIER	E 19 S						3	3.000	10.000								4	1	4	0	226		
54	5 K11	BARRIER	E 21 S						3	3.000	10.000			3	2.250	7.750	4	1	4	0	226				
24	5 K12	BARRIER	E 20						19	4.000									19	4	19	4	484		
12	5 K13	BARRIER	E 20						V 4	18	7.000								18	7	18	7			
		INCR. = 3'								12	7.000								12	7	12	7	196		
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12	5 K15	BARRIER	E 20						V 4	19	11.000								19	11	19	11			
		INCR. = 3'								13	11.000								13	11	13	11	212		
8	5 C1	SLIP FORM	E 20						12	0.000									12	0	12	0	101		
16	5 C2	SLIP FORM	E 20						8	0.000									8	0	8	0	134		

BILL OF REINFORCING STEEL

NO. REQ'D.	MARK NO.	LOCATION	EPOXY (E)	SHAPE NO.	STIRRUP (S)	SUBSTR. (X)	VARIES (V)	NO. EACH	DIMENSIONS								NOMINAL LENGTH	ACTUAL LENGTH	WEIGHT						
									B		C		D		E					F		H		K	
									FT.	IN.	FT.	IN.	FT.	IN.	FT.	IN.				FT.	IN.	FT.	IN.	FT.	IN.
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24	5 U202	DIAPHRAGM	E 10 S						4	1.000	4	7.000							12	9	12	7	315		
24	6 U203	DIAPHRAGM	E 19 S						3	0.250	5	3.000							8	4	8	2	295		
18	5 U204	BEAM	E 10 S						6	2.000	5	3.000							17	7	17	5	327		
26	4 U205	BEAM	E 13 S						5	3.000	2	8.000	5	3.000	2	8.000			16	7	16	4	284		
14	4 U206	BEAM	E 10 S						2	8.000	5	3.000							10	7	10	5	98		
28	6 V201	DIAPHRAGM	E 20						3	0.000									3	0	3	0	127		
12	5 V202	BEAM	E 20						6	2.000									6	2	6	2	78		
30	6 V251	WING WALL	E 20						7	1.000									7	1	7	1	320		
30	6 V252	WING WALL	E 20						7	0.000									7	0	7	0	316		
		SLAB																							
50	5 S101	SLAB	E 20						45	4.000									45	4	45	4	2365		
12	5 S102	SLAB	E 20						31	3.000									31	3	31	3	392		
115	6 S103	SLAB	E 20						30	5.000									30	5	30	5	5254		
112	6 S104	SLAB	E 20																						

BILL OF REINFORCING STEEL																						
NO. REQ'D.	MARK NO.	LOCATION	EPOXY (E)	SHAPE NO.	STIRRUP (S)	SUBSTR. (X)	VARIES (V)	NO. EACH	DIMENSIONS								NOMINAL LENGTH	ACTUAL LENGTH	WEIGHT			
									B	C	D	E	F	H	K	FT.				IN.	FT.	IN.
									FT.	IN.	FT.	IN.	FT.	IN.	FT.							
		TOTALS																				
4				E													764					
5				E													12511					
6				E													16060					
7				E													2112					
8				E													1410					
		TOTAL		E													32857					
SLAB ON CONCRETE NU-GIRDER																						
4				E													764					
5				E													5239					
6				E													16060					
7				E													2112					
8				E													1410					
		TOTAL		E													25585					
TYPE D BARRIER																						
5				E													7037					
		TOTAL		E													7037					
SLIP FORM OPTION																						
5				E													235					
		TOTAL		E													235					

BILL OF REINFORCING STEEL																						
NO. REQ'D.	MARK NO.	LOCATION	EPOXY (E)	SHAPE NO.	STIRRUP (S)	SUBSTR. (X)	VARIES (V)	NO. EACH	DIMENSIONS								NOMINAL LENGTH	ACTUAL LENGTH	WEIGHT			
									B	C	D	E	F	H	K	FT.				IN.	FT.	IN.
									FT.	IN.	FT.	IN.	FT.	IN.	FT.							



STATE OF MISSOURI
JOSHUA D. JOLLIFF
NUMBER
PE-2006002824
PROFESSIONAL ENGINEER

THIS SHEET HAS BEEN
SIGNED, SEALED AND DATED
ELECTRONICALLY.

DATE PREPARED
3/3/2023

ROUTE
67 STATE
MO

DISTRICT SHEET NO.
BR 21

COUNTY
BUTLER

JOB NO.
J9P3751

CONTRACT ID.

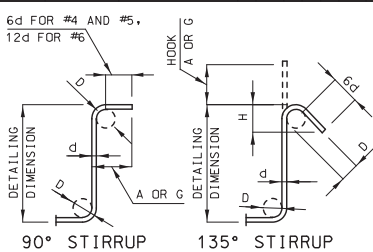
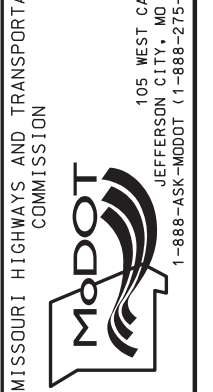
PROJECT NO.

BRIDGE NO.
A9369

DATE	DESCRIPTION

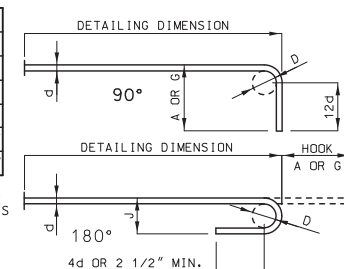
MISSOURI HIGHWAYS AND TRANSPORTATION
COMMISSION

105 WEST CAPITOL
JEFFERSON CITY, MO 65102
1-888-ASK-MODOT (1-888-278-6636)



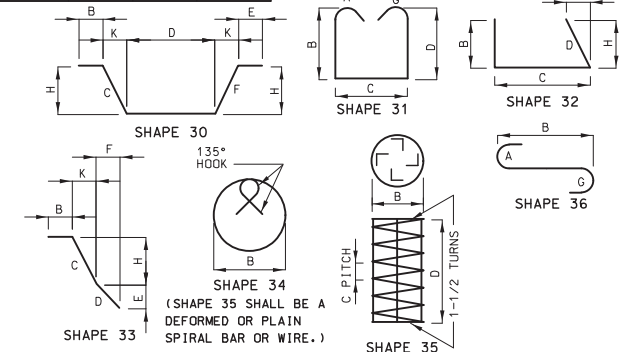
STIRRUP HOOK DIMENSIONS					
GRADES 40 - 50 - 60 KSI					
BAR SIZE	D (IN.)	90° HOOK		135° HOOK	
		HOK A OR G	HOK A OR G	APPROX. H	
#4	2"	4 1/2"	4 1/2"	3"	
#5	2 1/2"	6"	5 1/2"	3 3/4"	
#6	4 1/2"	12"	8"	4 1/2"	

NOTE: UNLESS OTHERWISE NOTED, DIAMETER "D" IS THE SAME FOR ALL BENDS AND HOOKS ON A BAR.



BAR SIZE	D (IN.)	END HOOK DIMENSIONS			
		ALL GRADES			
		180° HOOKS		90° HOOKS	
		A OR G	J	A OR G	
#3	2 1/4"	5"	3"	6"	
#4	3"	6"	4"	8"	
#5	3 3/4"	7"	5"	10"	
#6	4 1/2"	8"	6"	12"	
#7	5 1/4"	10"	7"	14"	
#8	6"	11"	8"	16"	
#9	9 1/2"	15"	11 3/4"	19"	
#10	10 3/4"	17"	13 1/4"	22"	
#11	12"	19"	14 3/4"	2'-0"	
#14	18 1/4"	2'-3"	21 3/4"	2'-7"	

NOTE:
ALL STANDARD HOOKS AND BENDS OTHER THAN 180 DEGREE ARE TO BE BENT WITH SAME PROCEDURE AS FOR 90 DEGREE STANDARD HOOKS.
HOOKS AND BENDS SHALL BE IN ACCORDANCE WITH THE PROCEDURES AS SHOWN ON THIS SHEET.
E = EPOXY COATED REINFORCEMENT.
S = STIRRUP.
X = BAR IS INCLUDED IN SUBSTRUCTURE QUANTITIES.
V = BAR DIMENSIONS VARY IN EQUAL INCREMENTS BETWEEN DIMENSIONS SHOWN ON THIS LINE AND THE FOLLOWING LINE.
NO. EA. = NUMBER OF BARS OF EACH LENGTH.
NOMINAL LENGTHS ARE BASED ON OUT TO OUT DIMENSIONS SHOWN IN BENDING DIAGRAMS AND ARE LISTED FOR FABRICATORS USE. (NEAREST INCH)
ACTUAL LENGTHS ARE MEASURED ALONG CENTERLINE BAR TO THE NEAREST INCH.
PAYWEIGHTS ARE BASED ON ACTUAL LENGTHS.
FOUR ANGLE OR CHANNEL SPACERS ARE REQUIRED FOR EACH COLUMN SPIRAL. SPACERS ARE TO BE PLACED ON INSIDE OF SPIRALS. LENGTH AND WEIGHT OF COLUMN SPIRALS DO NOT INCLUDE SPLICES OR SPACERS.
REINFORCING STEEL (GRADE 60) F_y = 60,000 PSI.

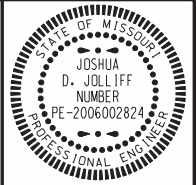


BENDING DIAGRAMS

Detailed Jan. 2023
Checked Jan. 2023

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 21 of 24



THIS SHEET HAS BEEN SIGNED, SEALED AND DATED ELECTRONICALLY.

DATE PREPARED
3/3/2023

ROUTE 67	STATE MO
DISTRICT BR	SHEET NO. 22

COUNTY
BUTLER
JOB NO.
J9P3751
CONTRACT ID.

PROJECT NO.
BRIDGE NO.
A9369

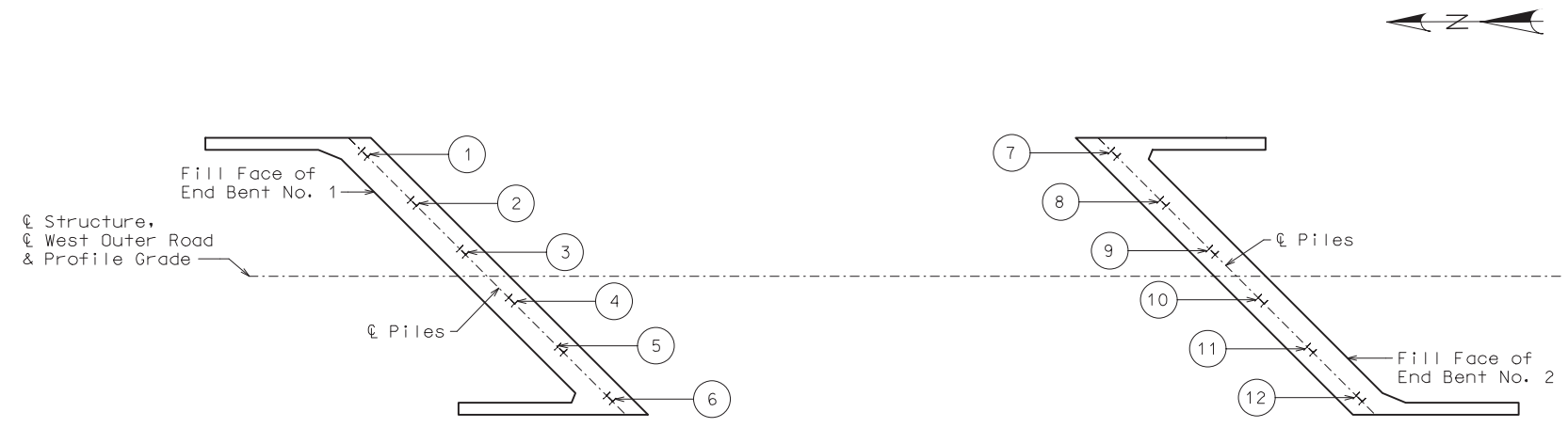
DESCRIPTION	DATE

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

105 WEST CAPITOL JEFFERSON CITY, MO 65102
1-888-ASK-MODOT (1-888-278-6636)

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SPRINGFIELD, MO 65807 (417) 869-6009
ENGINEERING CORPORATION - 000631

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PART PLAN SHOWING PILE NUMBERING FOR RECORDING AS-BUILT PILE DATA

As-Built Pile Data			
Pile No.	Length in Place (ft)	Computed Nominal Axial Compressive Resistance (kips)	Remarks
End Bent No. 1			
1			
2			
3			
4			
5			
6			

As-Built Pile Data			
Pile No.	Length in Place (ft)	Computed Nominal Axial Compressive Resistance (kips)	Remarks
End Bent No. 2			
7			
8			
9			
10			
11			
12			

Note:
Indicate in remarks column:
A. Pile type and grade
B. Batter
C. Driven to practical refusal

This sheet to be completed by MoDOT construction personnel.

AS-BUILT PILE DATA



Bacon Farmer Workman Engineering & Testing Inc.
500 S 17th St
Paducah, KY 42003
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Fax: 2704431904

BORING NUMBER B-201

PAGE 1 OF 2

CLIENT MoDOT PROJECT NAME Route 67 over Harviell Ditch
PROJECT NUMBER 20284 PROJECT LOCATION Butler County, Missouri
DATE STARTED 12/7/21 COMPLETED 12/7/21 GROUND ELEVATION 303.7 ft HOLE SIZE 6.25 inches
DRILLING CONTRACTOR Smith & Co. DRILLED BY Smith & Co GROUND WATER LEVELS:
DRILLING METHOD Hollow Stem Auger and Mud Rotary (CME-750) AT TIME OF DRILLING --
LOGGED BY NB CHECKED BY CM AT END OF DRILLING --
STATION 758+75 OFFSET 26' RT AFTER DRILLING --

DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (tsf)	DRY UNIT WT. (pcf)	MOISTURE CONTENT (%)	ATTERBERG LIMITS			FINES CONTENT (%)
									LIQUID LIMIT	PLASTIC LIMIT	PLASTICITY INDEX	
0		TOPSOIL - 6 inches (CL) SANDY LEAN CLAY: Light gray, moist, stiff to hard										
4			SPT SS1	100	5-6-7 (13)			4				45
8			SPT SS2	72	6-11-15 (26)			10				52
12			SPT SS3	94	9-17-17 (34)			6				28
16			SPT SS4	89	5-6-8 (14)			10				17
20		(SP) SAND: Gray to grayish brown, moist, very loose to medium dense	SPT SS5	83	4-7-4 (11)			24				24
24			SPT SS6	89	1-2-1 (3)			21				16
28			SPT SS7	44	5-2-2 (4)			23				31
32			SPT SS8	67	5-10-9 (19)			20				22
36			SPT SS9	67	8-9-7 (16)			23				28
40			SPT SS10	61	9-12-14 (26)			21				21
44			SPT SS11	61	7-7-12 (19)			22				23
48			SPT SS12	0	8-5-5 (10)							

(Continued Next Page)

B-201



Bacon Farmer Workman Engineering & Testing Inc.
500 S 17th St
Paducah, KY 42003
Telephone: 2704431995
Fax: 2704431904

BORING NUMBER B-201

PAGE 2 OF 2

CLIENT MoDOT PROJECT NAME Route 67 over Harviell Ditch
PROJECT NUMBER 20284 PROJECT LOCATION Butler County, Missouri

DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (tsf)	DRY UNIT WT. (pcf)	MOISTURE CONTENT (%)	ATTERBERG LIMITS			FINES CONTENT (%)
									LIQUID LIMIT	PLASTIC LIMIT	PLASTICITY INDEX	
0		(SP) SAND: Gray to grayish brown, moist, very loose to medium dense (continued)										
60		(SP) GRAVELLY SAND: Grayish brown, moist, medium dense	SPT SS13	56	9-8-8 (16)			16				31
64			SPT SS14	39	10-11-9 (20)			14				35
68			SPT SS15	6	13-10-9 (19)			15				
72			SPT SS16	39	9-10-11 (21)			13				
76		(SP) SAND: Grayish brown, moist, medium dense	SPT SS17	44	11-11-12 (23)			20				30
80		(SP) GRAVELLY SAND: Grayish brown, moist, very dense Refusal at 80.1 feet. Bottom of borehole at 80.1 feet.	SPT SS18	83	12-50/1"			13				

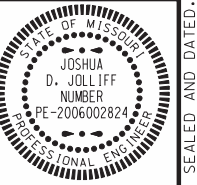
B-201

BORING DATA

Note: For locations of borings, see Sheet No. 1.

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 23 of 24



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DATE PREPARED 3/3/2023

ROUTE 67 STATE MO

DISTRICT BR SHEET NO. 23

COUNTY BUTLER

JOB NO. J9P3751

CONTRACT ID.

PROJECT NO.

BRIDGE NO. A9369

DATE

DESCRIPTION	DATE



Detailed Jan. 2023
Checked Jan. 2023

REV.



Bacon Farmer Workman Engineering & Testing Inc.
 500 S 17th St
 Paducah, KY 42003
 Telephone: 2704431995
 Fax: 2704431904

BORING NUMBER B-202

PAGE 1 OF 2

CLIENT MoDOT PROJECT NAME Route 67 over Harviell Ditch
 PROJECT NUMBER 20284 PROJECT LOCATION Butler County, Missouri
 DATE STARTED 12/14/21 COMPLETED 12/15/21 GROUND ELEVATION 304.8 ft HOLE SIZE 6.25 inches
 DRILLING CONTRACTOR Smith & Co. DRILLED BY Smith & Co GROUND WATER LEVELS:
 DRILLING METHOD Hollow Stem Auger and Mud Rotary (CME-750) ∇ AT TIME OF DRILLING 19.50 ft / Elev 285.30 ft
 LOGGED BY NB CHECKED BY CM AT END OF DRILLING --
 STATION 759+88 OFFSET 44' RT AFTER DRILLING --

DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (tsf)	DRY UNIT WT. (pcf)	MOISTURE CONTENT (%)	ATTERBERG LIMITS			FINES CONTENT (%)
									LIQUID LIMIT	PLASTIC LIMIT	PLASTICITY INDEX	
0		TOPSOIL - 6 inches (CL) SANDY LEAN CLAY: Grayish brown, moist, soft to medium stiff										
5			SPT SS1	61	1-2-3 (5)			15				57
6			SPT SS2	100	1-1-1 (2)			17				61
7			SPT SS3	100	1-3-5 (8)			16				55
10		(SP-SC) SAND WITH CLAY: Light brownish gray to gray, moist, medium dense	SPT SS4	92	3-5-7 (12)			13				38
15			SPT SS5	89	5-9-12 (21)			19				39
20			SPT SS6	86	5-8-10 (18)			19				37
25		(SP) SAND: Gray, moist, loose to medium dense	SPT SS7	44	1-2-3 (5)			20				60
30			SPT SS8	56	5-6-7 (13)			19				43
35			SPT SS9	61	8-8-8 (16)			20				51
40		(SP) GRAVELLY SAND: Gray, moist, medium dense to dense	SPT SS10	67	5-8-12 (20)			19				39
45			SPT SS11	64	4-3-10 (18)			20				24
50			SPT SS12	58	6-9-8 (17)			19				24

(Continued Next Page)

B-202



Bacon Farmer Workman Engineering & Testing Inc.
 500 S 17th St
 Paducah, KY 42003
 Telephone: 2704431995
 Fax: 2704431904

BORING NUMBER B-202

PAGE 2 OF 2

CLIENT MoDOT PROJECT NAME Route 67 over Harviell Ditch
 PROJECT NUMBER 20284 PROJECT LOCATION Butler County, Missouri

DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (tsf)	DRY UNIT WT. (pcf)	MOISTURE CONTENT (%)	ATTERBERG LIMITS			FINES CONTENT (%)
									LIQUID LIMIT	PLASTIC LIMIT	PLASTICITY INDEX	
60		(SP) GRAVELLY SAND: Gray, most, medium dense to dense (continued)	SPT SS13	0	4-6-6 (12)			18				21
65			SPT SS14	72	6-9-12 (21)			15				24
70			SPT SS15	100	5-10-11 (21)			19				19
75			SPT SS16	61	8-13-12 (25)			17				24
80			SPT SS17	72	8-9-15 (24)			20				23
85			SPT SS18	64	11-15-17 (32)			17				21

Refusal at 83.0 feet.
 Bottom of borehole at 83.0 feet.

B-202

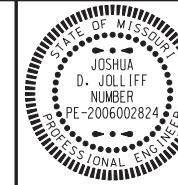
BORING DATA

Note: For locations of borings, see Sheet No. 1.

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 24 of 24

Detailed Jan. 2023
 Checked Jan. 2023



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DATE PREPARED

3/3/2023

ROUTE 67 STATE MO

DISTRICT BR SHEET NO. 24

COUNTY BUTLER

JOB NO. J9P3751

CONTRACT ID.

PROJECT NO.

BRIDGE NO. A9369

DESCRIPTION

DATE

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

105 WEST CAPITOL JEFFERSON CITY, MO 65102

1-888-ASK-MODOT (1-888-275-6636)

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