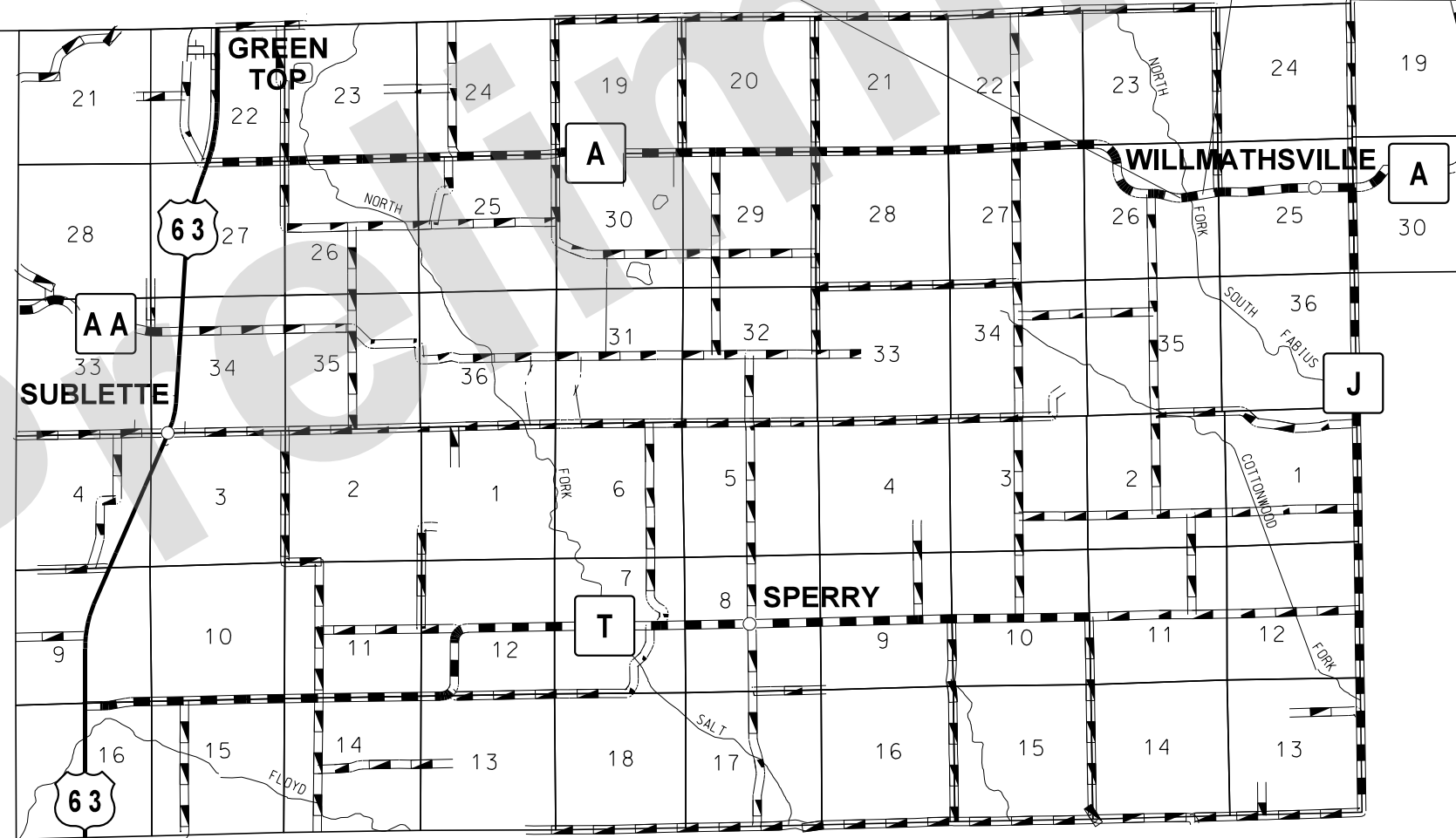


A.A.D.T. - 2025 = 403
A.A.D.T. - 2045 = 445
PEAK HOUR = 9.81%
T = 20.23%
V = 55 M.P.H.
DIRECTIONAL DISTRIBUTION = 49%E/51%W
FUNCTIONAL CLASSIFICATION- MAJOR COLLECTOR

KEY MAP
LOCATION OF ADAIR COUNTY




NOTE: DASHED OR OPEN SYMBOLS INDICATE
EXISTING FEATURES

	EXISTING	NEW
BUILDINGS AND STRUCTURES		
GUARD RAIL		
GUARD CABLE		
CONCRETE RIGHT-OF-WAY MARKER		
STEEL RIGHT-OF-WAY MARKER		
LOCATION SURVEY MARKER		
UTILITIES		
FIBER OPTICS	- FO -	- FO -
OVERHEAD CABLE TV	- OTV -	- OTV -
UNDERGROUND CABLE TV	- UTV -	- UTV -
OVERHEAD TELEPHONE	- OT -	- OT -
UNDERGROUND TELEPHONE	- UT -	- UT -
OVERHEAD POWER	- OE -	- OE -
UNDERGROUND POWER	- UE -	- UE -
SANITARY SEWER	- S -	- S -
STORM SEWER	- SS -	- SS -
GAS	- G -	- G -
WATER	- W -	- W -
MANHOLE		
FIRE HYDRANT		
WATER VALVE		
WATER METER		
DROP INLET		
DITCH BLOCK		
GROUND MOUNTED SIGN		
LIGHT POLE		
H-FRAME POWER POLE		
TELEPHONE PEDESTAL		
FENCE		
CHAIN LINK		- V -
WOVEN WIRE		- X -
GATE POST		
BENCHMARK		

DESCRIPTION	SHEET NUMBER
TITLE SHEET -----	1
TYPICAL SECTIONS (TS) (1 SHEET)--	2
QUANTITIES (QU) (1 SHEET)-----	3
PLAN-PROFILE (PP)-----	4
COORDINATE POINTS (CP)-----	5
SPECIAL SHEETS (SS)-----	6
TRAFFIC CONTROL SHEETS (TC)-----	7
EROSION CONTROL SHEETS (EC)-----	8
BRIDGE DRAWINGS (B)	
A9442-----	1-xx
CROSS SECTIONS (XS)-----	1-4

PROJECT NO.
BRIDGE NO.

[illegible]

MISSOURI HIGHWAYS AND TRANSPORTATION
COMMISSION

105 WEST CAPITOL
JEFFERSON CITY, MO 65102
1-888-ASK-MODOT (1-888-275-6636)

BEGINNING OF PROJECT	STA.	398+02.00
END OF PROJECT	STA.	403+35.00

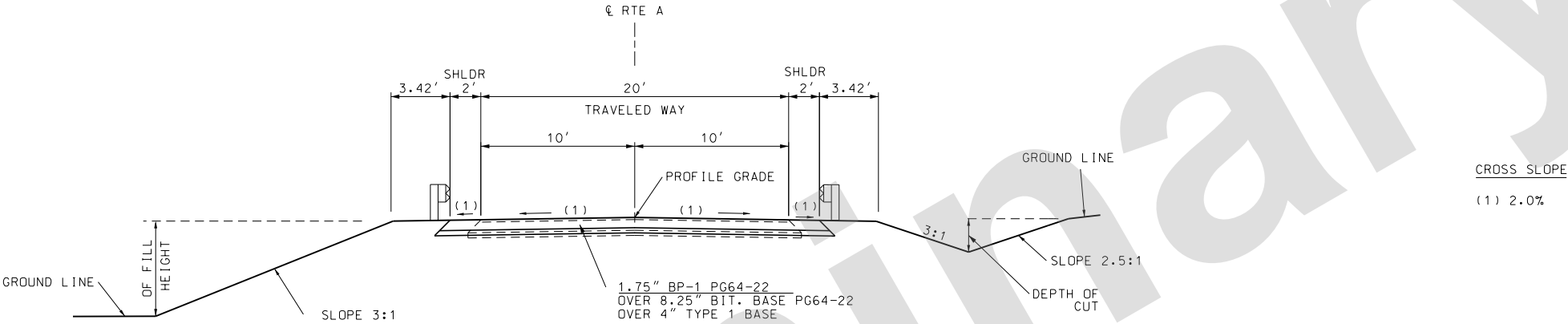
APPARENT LENGTH	533.00 FEET
-----------------	-------------

EQUATIONS AND EXCEPTIONS:

TOTAL CORRECTIONS	0 FEET
NET LENGTH OF PROJECT	533.00 FEET
STATE LENGTH	0.101 MILES

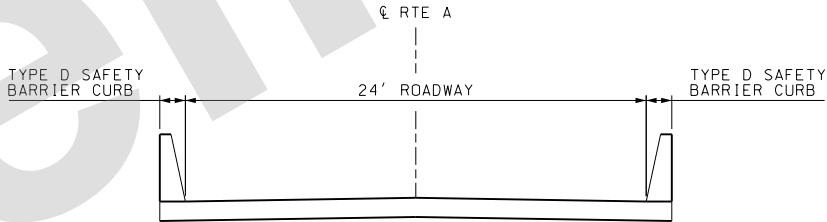
FOR INFORMATION ONLY		
ESTIMATED DISTURBED ACRES	0.90	ACRES

TITLE SHEET



TYPICAL SECTION RTE. A
STA. 398+19.00 TO STA. 402+66.40

* PAVEMENT LIMITS STA. 398+19.00 TO STA. 399+15.62
STA. 401+32.40 TO STA. 402+66.40
20' BRIDGE APPROACH SLAB EACH END OF BRIDGE (BRIDGE ITEM)



PROPOSED TYPICAL SECTION BRIDGE #A9942
STA. 399+35.07 TO STA. 401+12.95

DATE PREPARED 6/4/2024	
ROUTE A	STATE MO
DISTRICT NE	SHEET NO. 2
COUNTY ADAIR	
JOB NO. JNE0140	
CONTRACT ID.	

PROJECT NO.
BRIDGE NO.

DESCRIPTION	DATE

MISSOURI HIGHWAYS AND TRANSPORTATION
COMMISSION

105 WEST CAPITOL
JEFFERSON CITY, MO 65102
1-888-ASK-MODOT (1-888-273-6636)

MoDOT

TYPICAL SECTION SHEET
SHEET 1 OF 1

ALL PROJECT COORDINATES HAVE BEEN PROJECTED FROM THE MISSOURI STATE PLANE COORDINATE (SPC) SYSTEM OF 1983 USING AN AVERAGE PROJECT PROJECTION (GRID TO GROUND) FACTOR. TO GET BACK TO STATE PLANE COORDINATES MULTIPLY THE PROJECT COORDINATES BY THE AVERAGE GRID FACTOR AS SHOWN IN THE "REFERENCE CONTROL INFORMATION" PORTION OF THIS TABLE.

PROJECT COORDINATE INFORMATION

COORDINATE SYSTEM	MODIFIED STATE PLANE (GROUND)
HORIZONTAL DATUM	NAD 83 (2011) EPOCH 2010
VERTICAL DATUM	NAVD88 GNSS DERIVED
GEOID MODEL	I8 US
ELEVATIONS DETERMINED BY	DIFFERENTIAL LEVELING/ GPS - MOVRS

PROJECT PROJECTION FACTOR	1.00006143
---------------------------	------------

REFERENCE CONTROL INFORMATION

COORDINATE SYSTEM	MO COORDINATE SYSTEM OF 1983
CONTROL STATION	MISSOURI CORS
DESIGNATION	MODOT EDINA COR ARP
CORS_ID	MOED
PID	DM4674
LATITUDE	40 11 11.65651
LONGITUDE	92 10 30.28768
NORTHING (M)	483213.886
EASTING (M)	527668.699
ZONE	CENTRAL

PROJECT AVERAGE GRID FACTOR	0.99993857
-----------------------------	------------

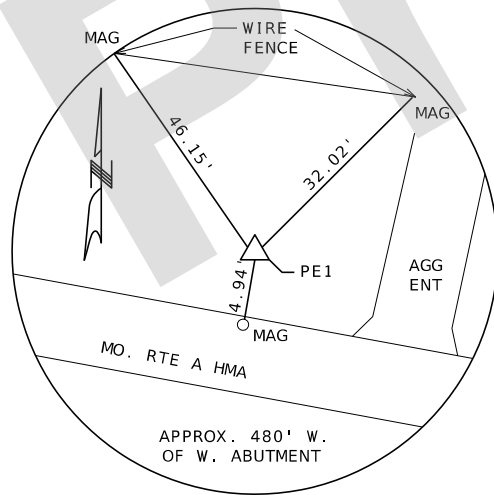
EXAMPLE OF PROJECT COORDINATE TO S.P.C.

```
PROJECT NORTHING X AVERAGE GRID FACTOR
= STATE PLANE NORTHING
PROJECT EASTING X AVERAGE GRID FACTOR
= STATE PLANE EASTING
```

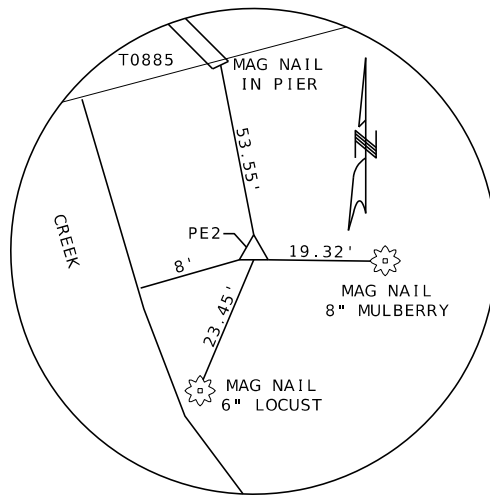
EXAMPLE: CONTROL POINT #
N 1536182.2077 X 0.99993857 = N 1536087.839
E 1545732.8741 X 0.99993857 = E 1545637.920

LINEAR UNIT CONVERSION

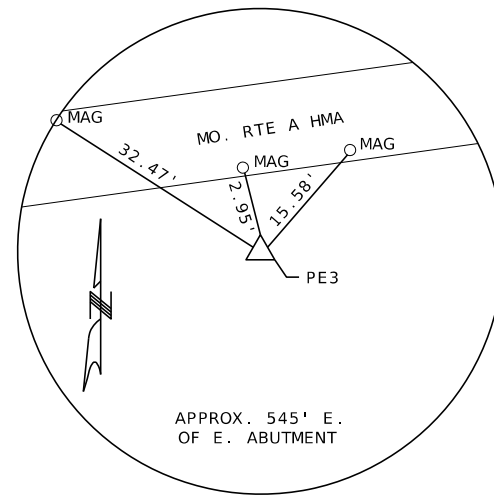
1 METER = 3.280833333 US SURVEY FEET (USFT)



CONTROL POINT PE1
SET 5/8" REBAR
N: 1635794.944
E: 1660342.648
EL: 852.16



CONTROL POINT PE2
SET 5/8" REBAR
N: 1635764.698
E: 1660947.922
EL: 842.92



CONTROL POINT PE3
SET 5/8" REBAR
N: 1635917.365
E: 1661535.858
EL: 863.13

COORDINATE POINT LISTING

[illegible]

ALIGNMENTS

[illegible]

DATE PREPARED	
6/4/2024	
ROUTE	STATE
A	MO
DISTRICT	SHEET NO.
NE	5
COUNTY	
ADAIR	
JOB NO.	
JNE0140	
CONTRACT ID.	

PROJECT NO.
BRIDGE NO.

[illegible]MISSOURI HIGHWAYS AND TRANSPORTATION
COMMISSION

JEFFERSON CITY, MO 65102
1-888-ASK-MODOT (1-888-275-6636)

SHEET 1 OF 1



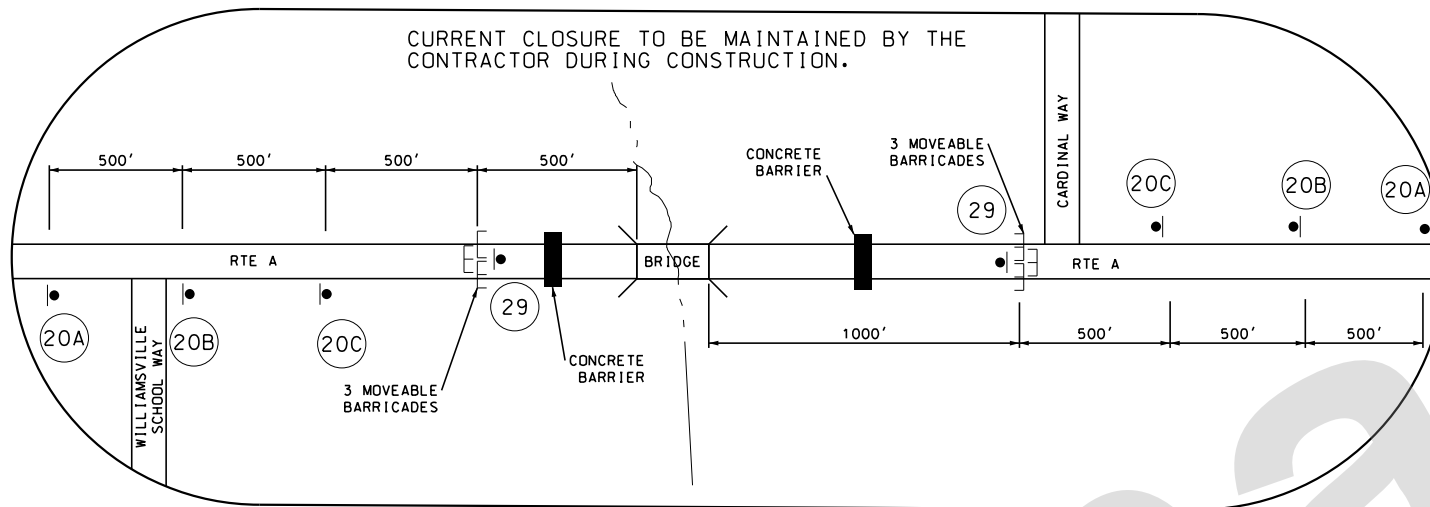
W020-3 (20A) W020-3 (20C) W020-3 (20B)

ROAD CLOSED
7 MILES AHEAD
LOCAL TRAFFIC ONLY

R11-3a (55A) ADVANCED ROAD CLOSURE SIGN

ROAD
CLOSED

R11-2 (29)



NOTES:

TRAFFIC CONTROL SIGNS AND DEVICES TO CLOSE THE ROAD HAVE BEEN PROVIDED AND INSTALLED BY THE COMMISSION.

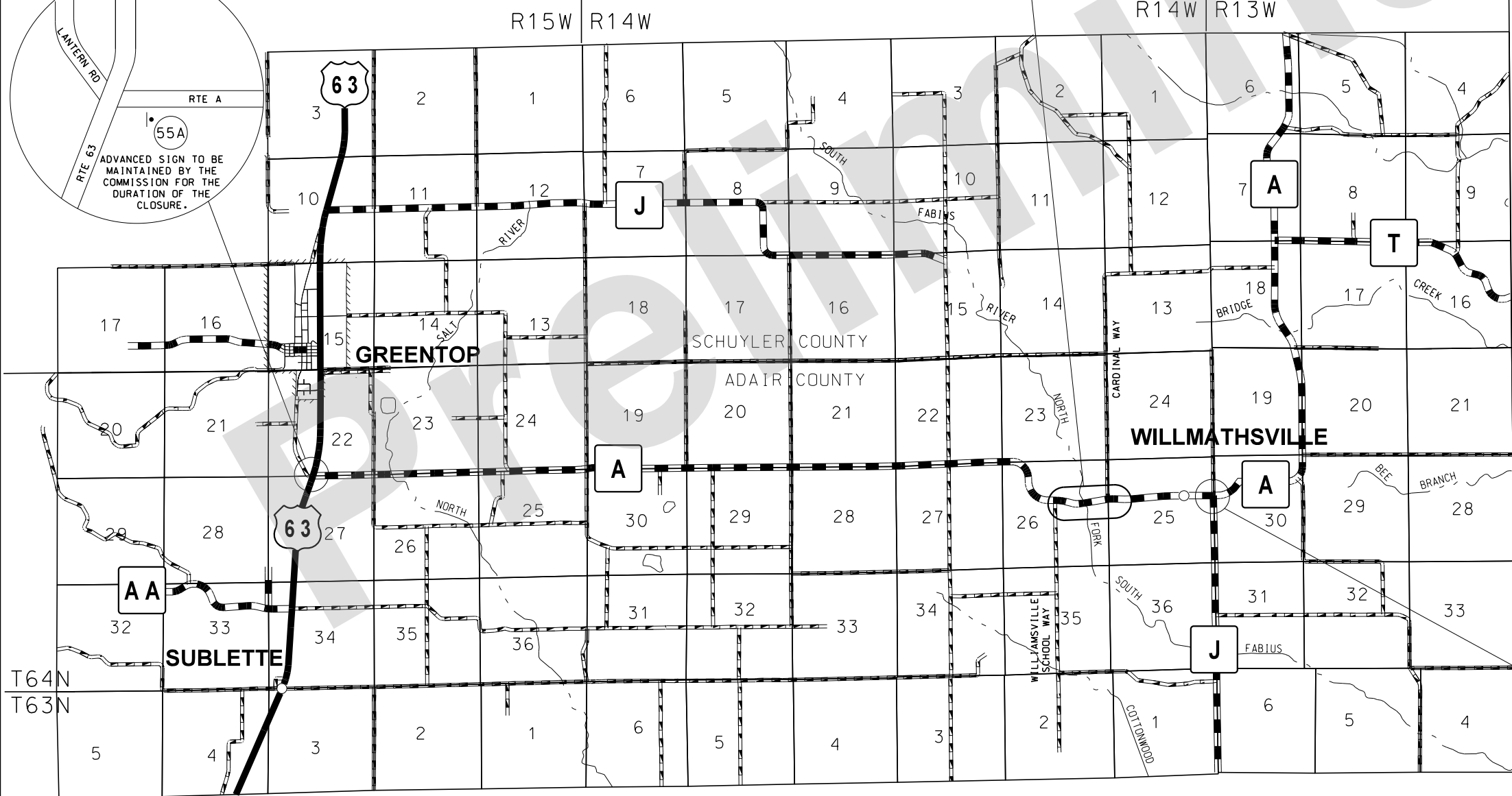
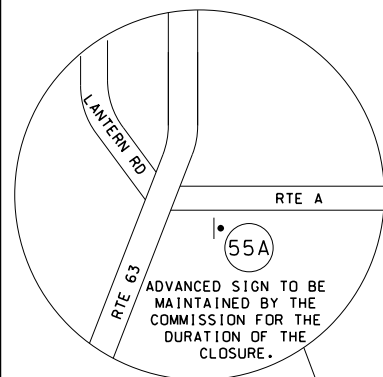
THE CONTRACTOR WILL BE RESPONSIBLE FOR MAINTAINING SIGNS, BARRICADES AND CONCRETE BARRIER ADJACENT TO THE BRIDGE DURING BRIDGE REMOVAL ACTIVITIES.

ADVANCED ROAD CLOSURE SIGNS WILL BE MAINTAINED BY THE COMMISSION.

THE CONTRACTOR MAY ADJUST THE ROAD CLOSURE SIGNS, BARRICADES AND CONCRETE BARRIER WITH APPROVAL OF THE ENGINEER.

TRAFFIC CONTROL LEGEND

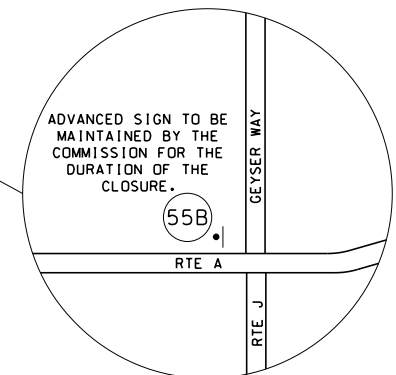
- SIGN (SINGLE SIDED)
- CHANNELIZER
- E TYPE III MOVABLE BARRICADE
- CONCRETE BARRIER



SCOTLAND COUNTY

ROAD CLOSED
1 MILES AHEAD
LOCAL TRAFFIC ONLY

R11-3a (55B) ADVANCED ROAD CLOSURE SIGN



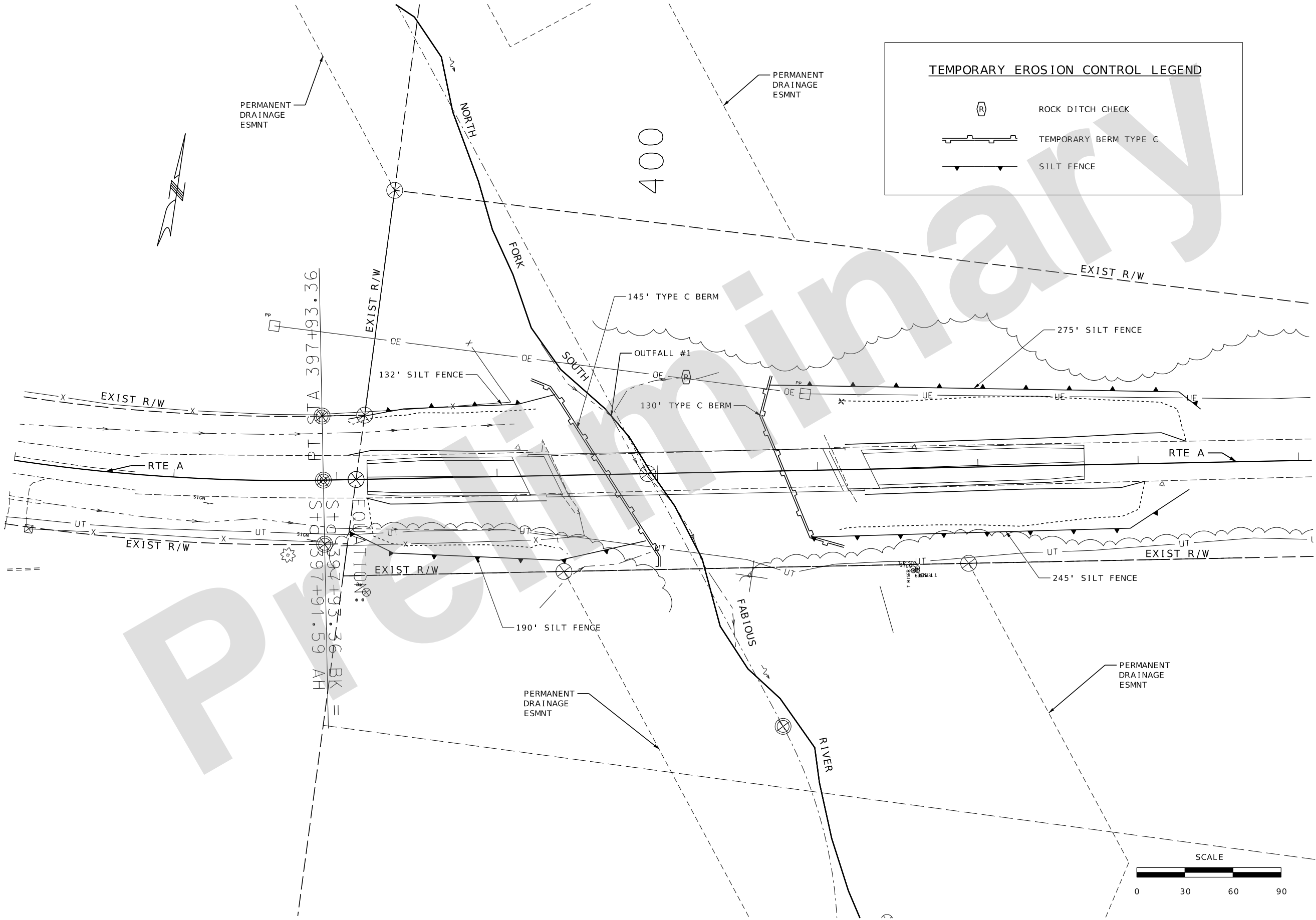
DATE PREPARED 6/4/2024	
ROUTE A	STATE MO
DISTRICT NE	SHEET NO. 7
COUNTY ADAIR	
JOB NO. JNE0140	
CONTRACT ID.	
PROJECT NO.	
BRIDGE NO.	
DESCRIPTION	DATE
MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION	
105 WEST CAPITOL JEFFERSON CITY, MO 65102 1-888-ASK-MODOT (1-888-273-6636)	
TRAFFIC CONTROL SHEET 1 OF 1	

TEMPORARY EROSION CONTROL LEGEND

(R) ROCK DITCH CHECK

TEMPORARY BERM TYPE C

SILT FENCE



DATE PREPARED 6/4/2024	
ROUTE A	STATE MO
DISTRICT NE	SHEET NO. 8
COUNTY ADAIR	
JOB NO. JNE0140	
CONTRACT ID.	

PROJECT NO.
BRIDGE NO.

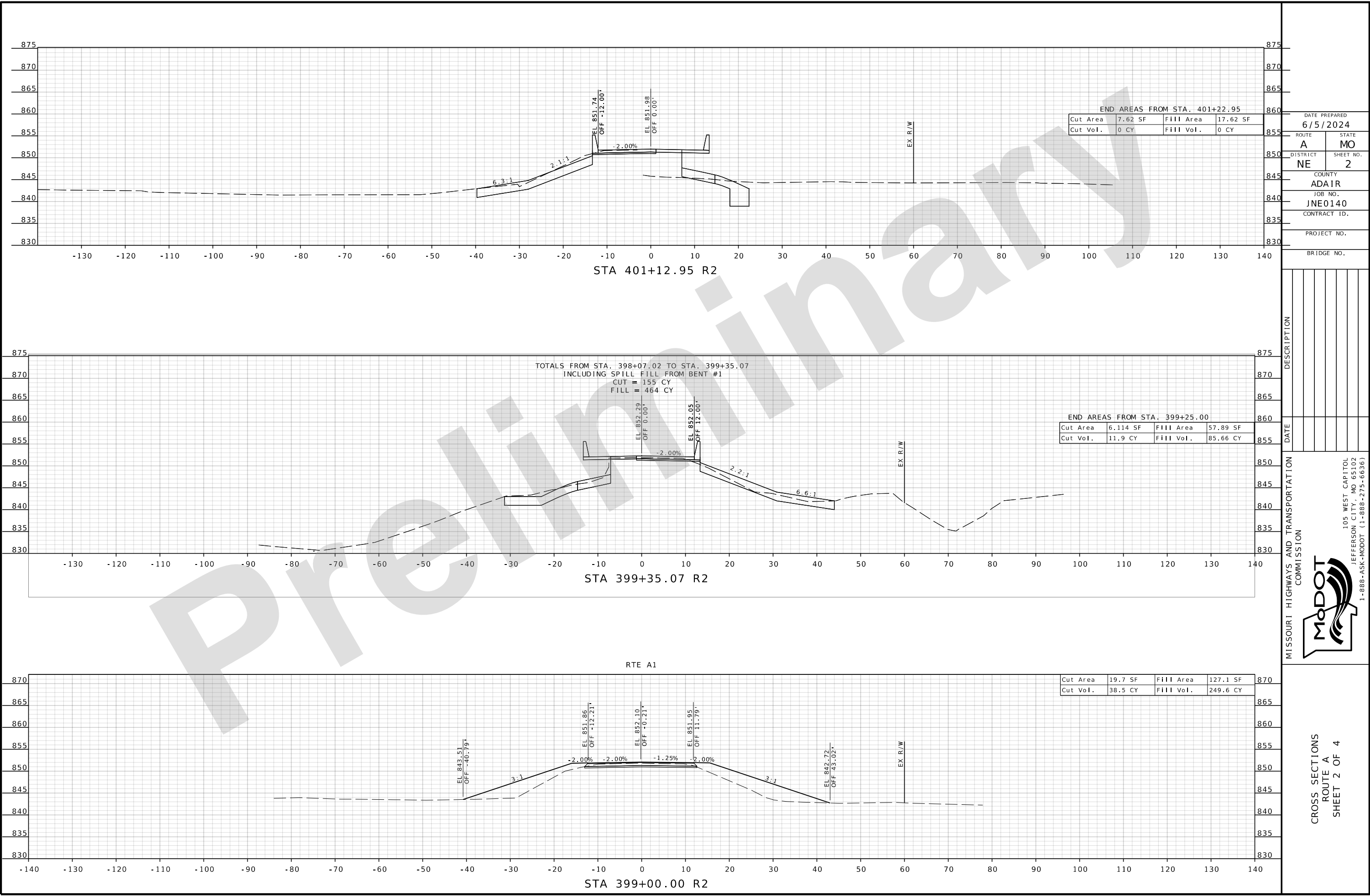
DESCRIPTION	DATE

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

MoDOT

105 WEST CAPITOL
JEFFERSON CITY, MO 65102
1-888-ASK-MODOT (1-888-273-6636)

EROSION CONTROL SHEET
SHEET 1 OF 1



DATE PREPARED
6/5/2024

ROUTE
A

DISTRICT
NE

COUNTY
ADAIR

JOB NO.
JNE0140

CONTRACT ID.

PROJECT NO.

BRIDGE NO.

STATE
MO

SHEET NO.
2

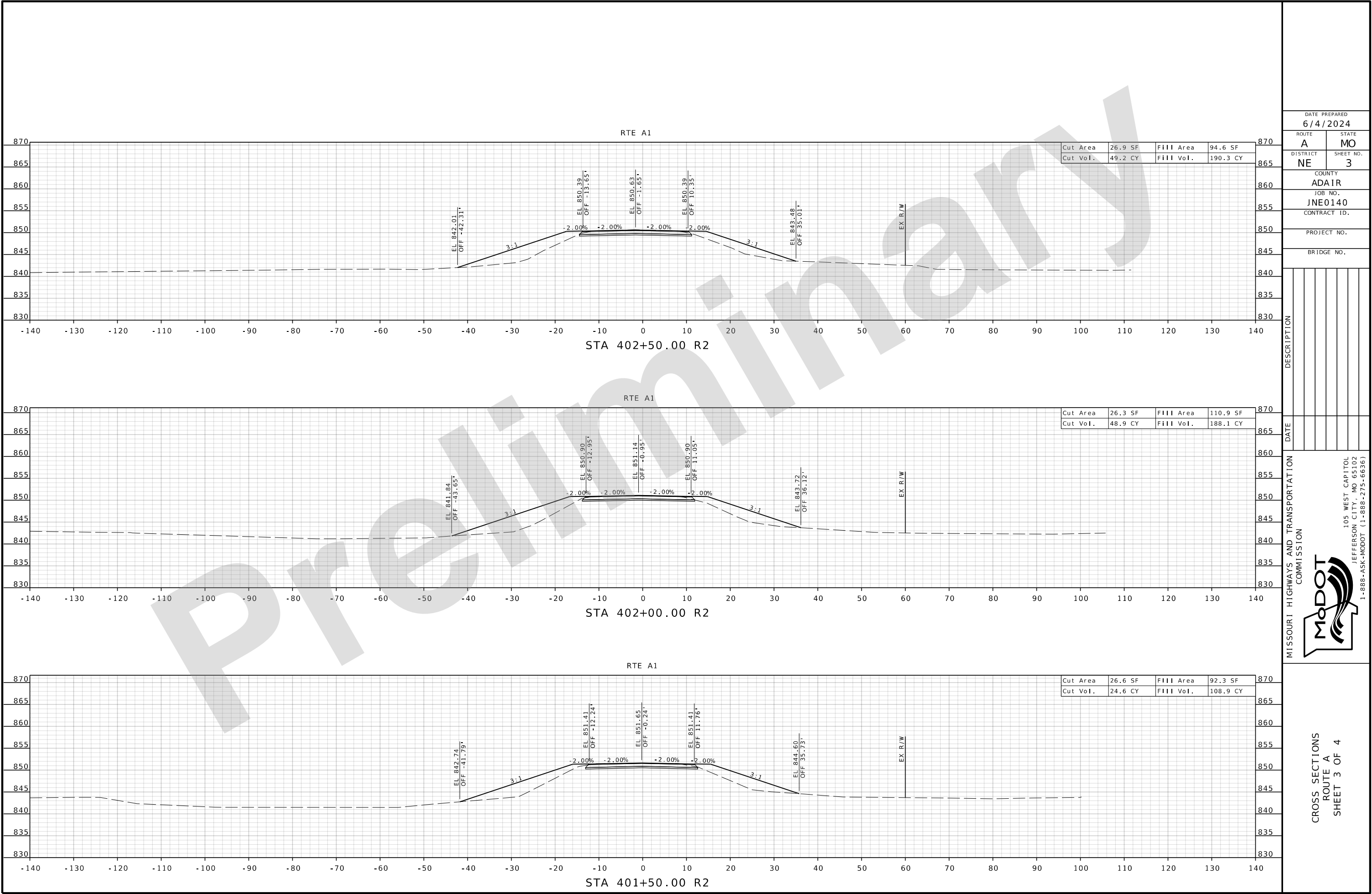
DESCRIPTION

DATE

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

105 WEST CAPITOL
JEFFERSON CITY, MO 65102
1-888-ASK-MODOT (1-888-273-6636)

CROSS SECTIONS
ROUTE A
SHEET 2 OF 4



DATE PREPARED
6/4/2024

ROUTE
A

STATE
MO

DISTRICT
NE

SHEET NO.
3

COUNTY
ADAIR

JOB NO.
JNE0140


CONTRACT ID.

PROJECT NO.

BRIDGE NO.

DESCRIPTION	DATE

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION



105 WEST CAPITOL
JEFFERSON CITY, MO 65102
1-888-ASK-MODOT (1-888-273-6636)

CROSS SECTIONS
ROUTE A
SHEET 3 OF 4

General Notes:

Design Specifications:
2020 AASHTO LRFD Bridge Design Specifications (9th Ed.)
2011 AASHTO Guide Specifications for LRFD Seismic Bridge Design (2nd Ed.) and 2014 Interim Revisions (Seismic Details)
Seismic Design Category = x
Design earthquake response spectral acceleration coefficient at x.x second period, SD1 = xxxg
Acceleration Coefficient (effective peak ground acceleration coefficient), As = xxxg

Design Loading:
Vehicular = HL-93
Future Wearing Surface = 35 Lb./Sq. Ft.
Earth = 120 Lb./Cu. Ft.
Equivalent Fluid Pressure = 45 Lb./Cu. Ft.
Superstructure: Simply-Supported, Non-Composite for dead load.
Continuous Composite for live load.
Design Unit Stresses:
Class B Concrete (Substructure) f'c = 3,000 psi
Class B-1 Concrete (Barrier) f'c = 4,000 psi
Class B-2 Concrete (Superstructure, except Barrier and Prestressed Girders) f'c = 4,000 psi
Reinforcing Steel (Grade 60) fy = 60,000 psi
Welded or Seamless Steel Shell (pipe) for CIP Pile (ASTM Grade 3) fy = 45,000 psi
For Prestressed Girder Stresses, see Sheets No. 14 thru 15.

Neoprene Pads:
Plain and Laminated Neoprene Bearing Pads shall be 60 durometer and shall be in accordance with Sec. 716.

Joint Filler:
All joint filler shall be in accordance with Sec. 1057 for preformed sponge rubber expansion and partition joint filler, except as noted.

Reinforcing Steel:
Minimum clearance to reinforcing steel shall be 1 1/2", unless otherwise shown.

Minimum clearance between galvanized piles and uncoated (plain) reinforcing steel including bar supports shall be 1 1/2". Nylon, PVC, or polyethylene spacers shall be used to maintain clearance. Nylon cable ties shall be used to bind the spacers to the reinforcement.

Traffic Handling:
Structure to be closed during construction.
Traffic to be maintained on other routes during construction.
See roadway plans for traffic control.

Estimated Quantities for Slab on Concrete NU-Girder		
Item		Total
Class B-2 Concrete	cu. yard	xxx
Reinforcing Steel (Epoxy Coated)	pound	xxxx

Notes:
The table of Estimated Quantities for Slab on Concrete NU Girder represents the quantities used by the State in preparing the cost estimate for concrete slabs. The area of the concrete slab will be measured to the nearest square yard longitudinally from end of slab to end of slab and transversely from out to out of bridge slab (or with the horizontal dimensions as shown on the plan of slab). Payment for stay-in-place forms or conventional forms, all concrete, and epoxy coated reinforcing steel will be considered completely covered by the contract unit price for the slab. Variations may be encountered in the estimated quantities but the variations cannot be used for an adjustment in the contract unit price.

Method of forming the slab shall be as shown on the plans and in accordance with Sec 703. All hardware for forming the slab to be left in place as a permanent part of the structure shall be coated in accordance with ASTM A123 or ASTM B633 with a thickness class SC 4 and a finish type I, II or III.

Slab shall be cast-in-place with conventional forms or stay-in-place corrugated steel forms. Precast panels will not be permitted.

Estimated Quantities				
Item		Substr.	Superstr.	Total
Class 1 Excavation	cu. yard	xx		xx
Bridge Approach Slab (Minor)	sq. yard		xxx	xxx
Galvanized Cast-in-Place Concrete Pile (14 in.)	linear foot	xxx		xxx
Galvanized Cast-in-Place Concrete Pile (16 in.)	linear foot	xxx		xxx
Dynamic Pile Testing	each	xx		xx
Dynamic Pile Restrike Testing	each	xx		xx
Pile Point Reinforcement	each	xx		xx
Class B Concrete (Substructure)	cu. yard	xxx		xxx
Type D Barrier	linear foot		xxx	xxx
Slab on Concrete NU-Girder	sq. yard		xxx	xxx
NU 43, Prestressed Concrete NU-Girder	linear foot		xxx	xxx
Reinforcing Steel (Bridges)	pound	xxxx		xxxx
Slab Drain	each		xx	xx
Vertical Drain at End Bents	each			x
Plain Neoprene Bearing Pad	each		xx	xx
Laminated Neoprene Bearing Pad	each		xx	xx

Notes:
All concrete above the construction joint in the end bents is included in the Estimated Quantities for Slab on Concrete NU-Girder.

All reinforcement in the end bents is included in the Estimated Quantities for Slab on Concrete NU-Girder.

All reinforcement in the intermediate bent concrete diaphragms except reinforcement embedded in the beam cap is included in the Estimated Quantities for Slab on Concrete NU-Girder.

All concrete above the intermediate beam cap is included in the Estimated Quantities for Slab on Concrete NU-Girder.

All reinforcement in cast-in-place pile at intermediate bent is included in the substructure quantities.

Foundation Data				
Type	Design Data	Bent Number		
		1	2	3
Load Bearing Pile	Pile Type and Size	CECIP **	CECIP **	CECIP **
	Number	*	*	*
	Approximate Length Per Each	ft	*	*
	Pile Point Reinforcement	ea	*	*
	Min. Galvanized Penetration (Elev.)	ft	*	*
	Est. Max. Scour Depth xxx (Elev.)	ft	*	*
	Minimum Tip Penetration (Elev.)	ft	*	*
	Criteria for Min. Tip Penetration	*	*	*
	Pile Driving Verification Method	(1)	(1)	(1)
	Minimum Nominal Axial Compressive Resistance (MNACR)	kip	*	*
	Portion of MNACR Required at End of Initial Drive	kip	*	*
	Resistance Factor	*	*	*

Minimum Nominal Axial Compressive Resistance = $\frac{\text{Maximum Factored Loads}}{\text{Resistance Factor}}$

DT = Dynamic Testing

Dynamic Testing shall be performed on the first pile installed at each bent.

The test piles at End Bents No. 1 and 3 shall be driven to an end-of-initial drive resistance of approximately xxx kips, which is estimated to occur at a pile tip elevation of approximately xxx. The test piles at intermediate Bents No. 2 shall be driven to an end-of-initial drive resistance of approximately xxx kips, which is estimated to occur at a pile tip elevation of approximately xxx. Subsequently, pile setups and the minimum nominal axial compressive resistance shall be confirmed by a restrrike test performed not less than 24 hours after end of initial drive.

Pile point reinforcement need not be galvanized. Shop drawings will not be required for pile point reinforcement.

Estimated Maximum Scour Depth (Elevation) shown is for verifying Minimum Nominal Axial Compressive Resistance using dynamic testing only where pile resistance contribution above this Elevation shall not be considered.

All piling shall be galvanized down to the minimum galvanized penetration (elevation).

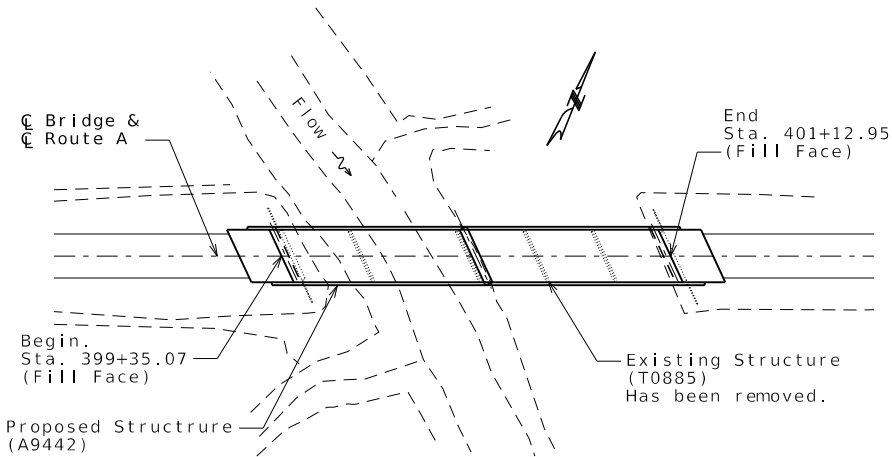
The contractor shall make every effort to achieve the minimum galvanized penetration (elevation) shown on the plans for all piles. Deviations in penetration less than 5 feet of the minimum will be considered acceptable provided the contractor makes the necessary corrections to ensure the minimum penetration is achieved on subsequent piles.

* Piles are located within the Heavy Revetment on spill slopes.

List of Drawings

SHEET NO.	DESCRIPTION
1.	GENERAL PLAN & ELEVATION
2.	GENERAL NOTES & QUANTITIES
3.	GALVANIZED CLOSED END CAST-IN-PLACE (CECIP) CONCRETE PILES
4.	DETAILS OF END BENT NO. 1
5.	DETAILS OF END BENT NO. 1
6.	DETAILS OF END BENT NO. 1
7.	DETAILS OF END BENT NO. 1
8.	VERTICAL DRAIN AT END BENTS
9.	DETAILS OF INTERMEDIATE BENT NO. 2
10.	DETAILS OF END BENT NO. 3
11.	DETAILS OF END BENT NO. 3
12.	DETAILS OF END BENT NO. 3
13.	DETAILS OF END BENT NO. 3
14.	NU-GIRDERS-SPANS (1-2) AND (2-3)
15.	NU-GIRDERS (ALTERNATE REINFORCEMENT)-SPANS (1-2) AND (2-3)
16.	DETAILS OF DIAPHRAGM AT INTERMEDIATE BENT NO. 2
17.	SLAB DRAINS
18.	BEAM CAMBER DIAGRAM & MISC. SLAB DETAILS
19.	SLAB PLAN & SECTION
20.	TYPE D BARRIER
21.	TYPE D BARRIER AT END BENTS
22.	BRIDGE APPROACH SLAB (MINOR)
23.	BILL OF REINFORCING STEEL
24.	BILL OF REINFORCING STEEL
25.	BILL OF REINFORCING STEEL
26.	"AS BUILT PILE" DATA
27.	BORING DATA
28.	BORING DATA

Hydrologic Data
Drainage Area = 23.0 mi²
Design Flood Frequency = 25 years
Design Flood Discharge = 4300 cfs
Design Flood (D.F.) Elevation = 845.3
Base Flood (100-year)
Base Flood Elevation = 846.3
Base Flood Discharge = 6000 cfs
Estimated Backwater = 1.0 ft
Average Velocity thru Opening = 7.3 ft/s
Freeboard (50-year)
Freeboard = 2.1 ft
Roadway Overtopping
Overtopping Flood Discharge = N/A
Overtopping Flood Frequency = > 500 year
500-year Flood Elevation = 847.3



LOCATION SKETCH

DATE PREPARED
06/07/2024

ROUTE
A

STATE
MO

DISTRICT
BR

SHEET NO.
2

COUNTY
ADAIR

JOB NO.
JNE0140

CONTRACT ID.

PROJECT NO.

BRIDGE NO.
A9442

DESCRIPTION

DATE

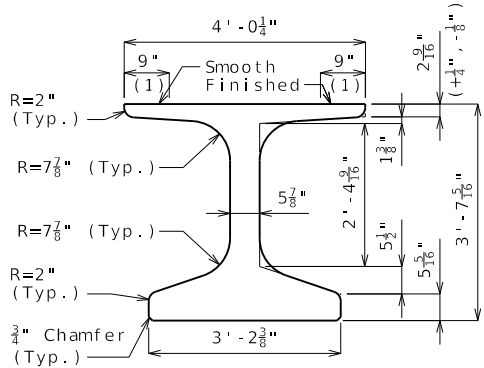
MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

105 WEST CAPITOL
JEFFERSON CITY, MO 65102
1-888-ASK-MODOT (1-888-275-6636)

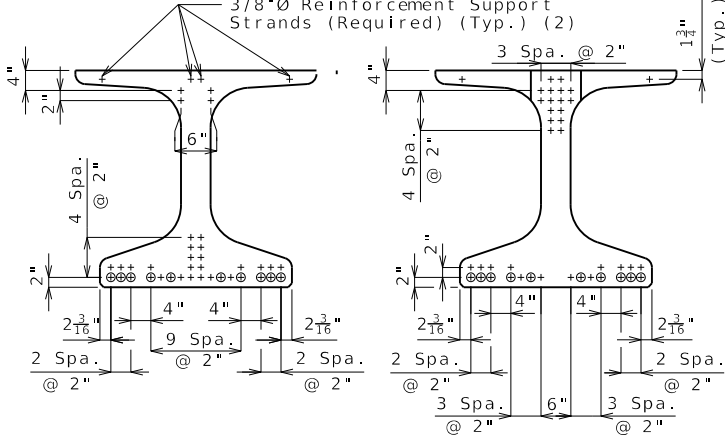
VEENSTRA & KIMM INC

Liberty, Missouri 64069
P.O. Box 236
816-781-6182
816-781-0643 (FAX)
Certificate of Authority No. F00362673

- (1) Fabricator shall apply a bond breaker to this region.
- (2) Outer strands tensioned to 2.02 kips/strand and inner strands to 8 kips/strand. Placed symmetrical about \bar{C} Girder. May be moved laterally in pairs.

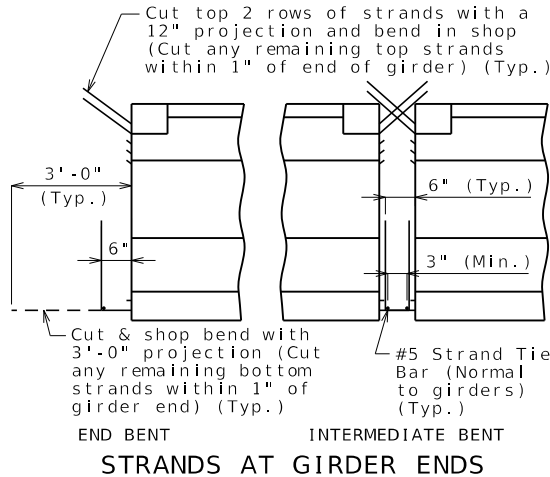


DIMENSIONS



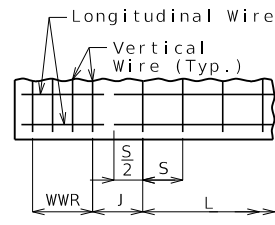
\bar{C} GIRDER
STRAND ARRANGEMENT

+ Indicates prestressing strand. o Indicates cut & shop bend with 3'-0" projection.



END BENT INTERMEDIATE BENT
STRANDS AT GIRDER ENDS

- (3) Bent 1: 1'-8 3/8" Bent 2: 1'-7 1/8" (Span 1-2) Bent 2: 1'-6 7/8" (Span 2-3) Bent 3: 1'-8 3/8" Bent 2: 0'-7 1/2" (Span 1-2) Bent 2: 0'-7 1/4" (Span 2-3)



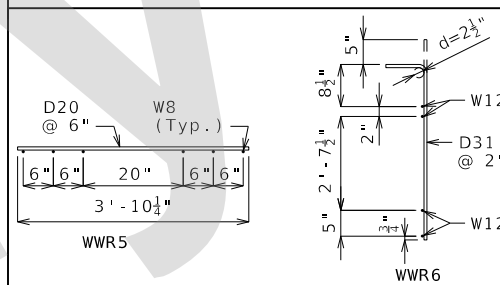
WELDED WIRE PLACEMENT

S = Vertical wire spacing
L = Length of WWR mats
J = Distance between WWR mats

Bill of Reinforcing Steel - Each Girder			
No.	Size/Mark	Length	Shape
186	5 B1	5'-0"	11S
206	4 D1	4'-0"	9S
2	4 G3	4'-3"	20
2	4 G4	2'-3"	20
2	4 G5	2'-11"	20
6	4 G6	Varies	20

Bending Diagrams

Welded Wire Reinforcement - Each Girder



All dimensions are out to out.

Hooks and bends shall be in accordance with the CRSI Manual of Standard Practice for Detailing Reinforced Concrete Structures, Stirrup and Tie Dimensions.

Actual bar lengths are measured along centerline of bar to the nearest inch.

Minimum clearance to reinforcing shall be 1", unless otherwise shown.

All bar reinforcement shall be Grade 60.

WWR shall not be epoxy coated.

G4 and G5 not required for interior girders. Half no. of G3, G4, G5 and G6 not required for ext. girders of end spans.

General Notes:

Concrete for prestressed beams shall be Class A-1 with $f'c = 8000$ psi and $f'ci = 6500$ psi.

Use 36 strands, 0.6"Ø Grade 270, with an initial prestress force of 1582 kips.

Pretensioned members shall be in accordance with Sec 1029.

Fabricator shall be responsible for location and design of lifting devices.

Exterior and interior girders are the same except: coil ties, top flange blockout, coil inserts for slab drains.

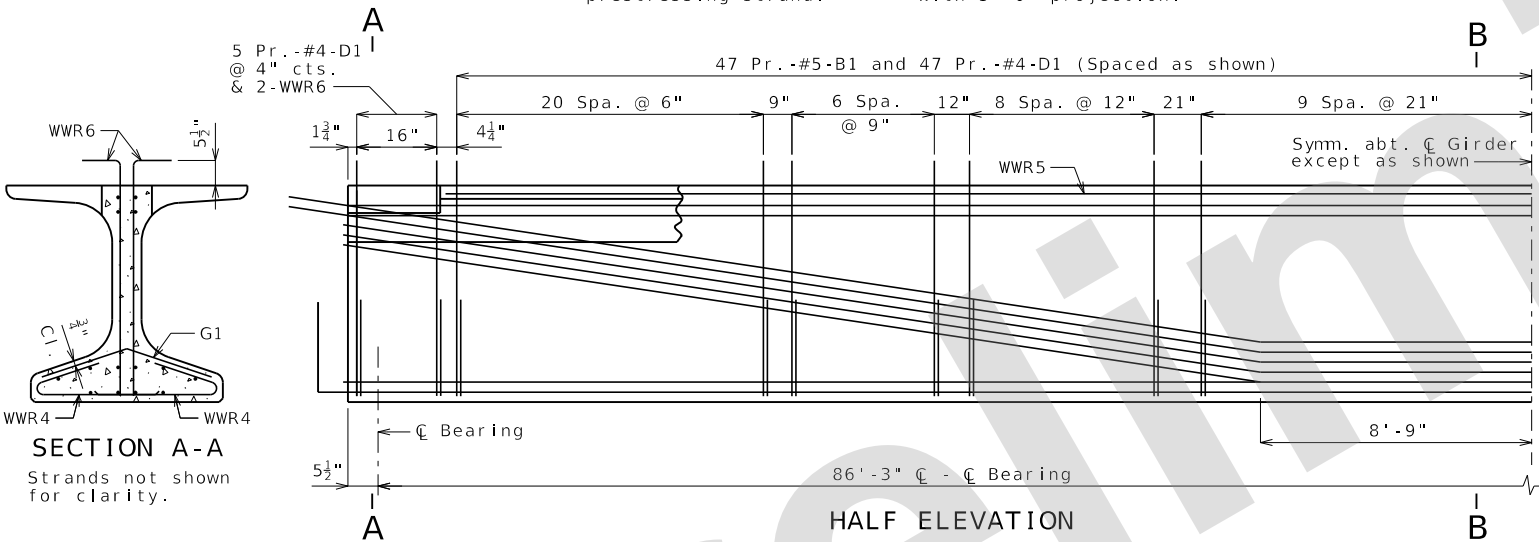
The contractor shall provide bracing necessary for lateral and torsional stability of the girders during construction of the concrete slab and remove the bracing after the slab has attained 75% design strength. Contractor shall not drill holes in the girders.

For Girder Camber Diagram, see Sheet No. 18.

For location of coil inserts at slab drains, see Sheet No. 19.

For location of coil ties at concrete diaphragms and integral bents, see Sheets No. 4, 10 and 16.

Alternate bar reinforcing steel details are provided and may be used. The same type of reinforcing steel shall be used for all girders in all spans.

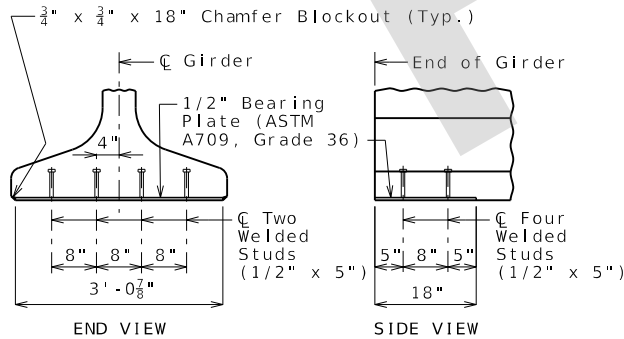


HALF ELEVATION

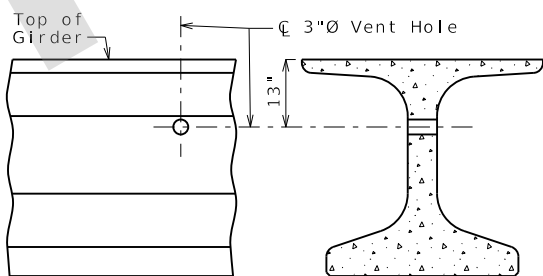
Reinforcement support strands not shown for clarity.

SECTION A-A
Strands not shown for clarity.

SECTION B-B
Strands not shown for clarity.



BEARING PLATE

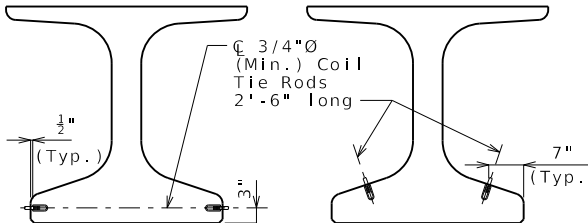


PART ELEVATION

PART SECTION

VENT HOLE

Place vent holes at or near upgrade 1/3 point of girders and clear reinforcing steel or strands by 1 1/2" minimum.

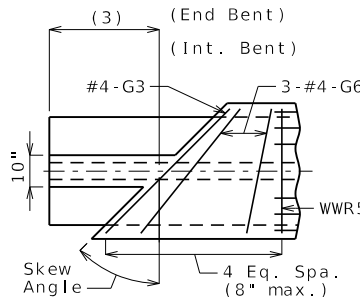


CLOSED DIAPHRAGMS AND INTEGRAL BENTS

OPEN DIAPHRAGMS

COIL TIES

Exclude coil tie at exterior face of exterior girders except at integral end bents.



INTERIOR GIRDER AT ALL BENTS & EXTERIOR GIRDER AT END BENT
TOP FLANGE BLOCKOUT
Mirror for right advanced.

NU-GIRDERS (ALTERNATE REINFORCEMENT) - SPANS (1-2) AND (2-3)

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 15 of 28

Detailed Jun. 2024
Checked Jun. 2024

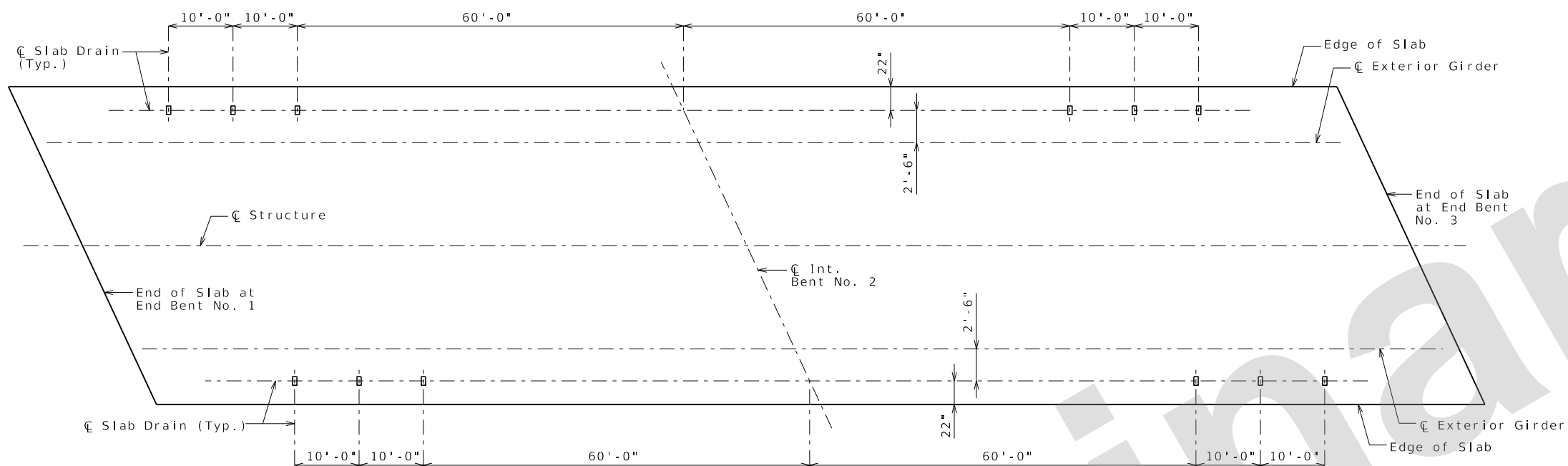
DESCRIPTION	
DATE	

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION	
DATE	

VEENSTRA & KIMM INC	
DATE	

Liberty, Missouri 64069
P.O. Box 236
816-781-6182
816-781-0643 (FAX)
Certificate of Authority No. F00362673

105 WEST CAPITOL
JEFFERSON CITY, MO 65102
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General Notes:

Contractor shall have the option to construct either steel or FRP slab drains. All drains shall be of same type.

Slab drain bracket assembly shall be ASTM A709 Grade 36 steel.

Locate drains in slab by dimensions shown in Part Section Near Drain.

Reinforcing steel shall be shifted to clear drains.

The coil inserts and bracket assembly shall be galvanized in accordance with ASTM A123.

All bolts, hardened washers, lock washers and nuts shall be galvanized in accordance with AASHTO M 232 (ASTM A153), Class C.

All 1/2"Ø bolts shall be ASTM A307.

Shop drawings will not be required for the slab drains and the bracket assembly.

The coil insert required for the bracket assembly attachment shall be located on the prestressed girder shop drawings.

Coil inserts shall have a concrete pull-out strength (ultimate load) of at least 2,500 pounds in 5,000 psi concrete.

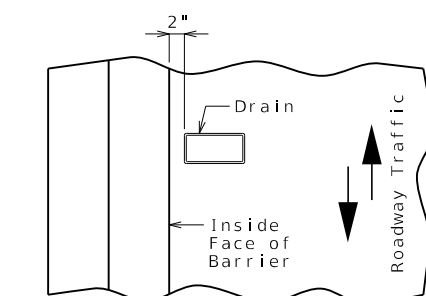
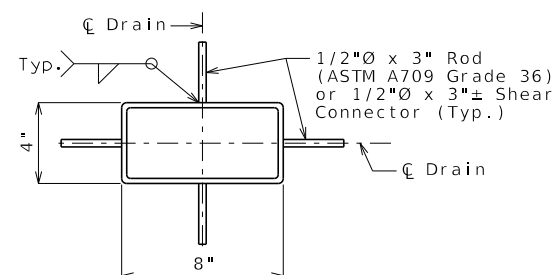
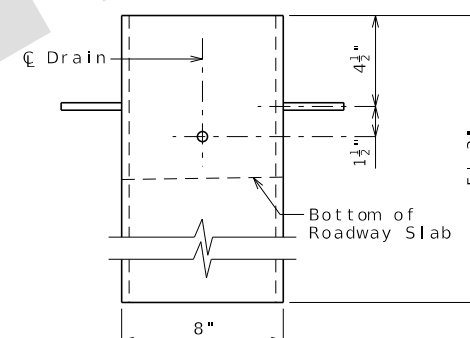
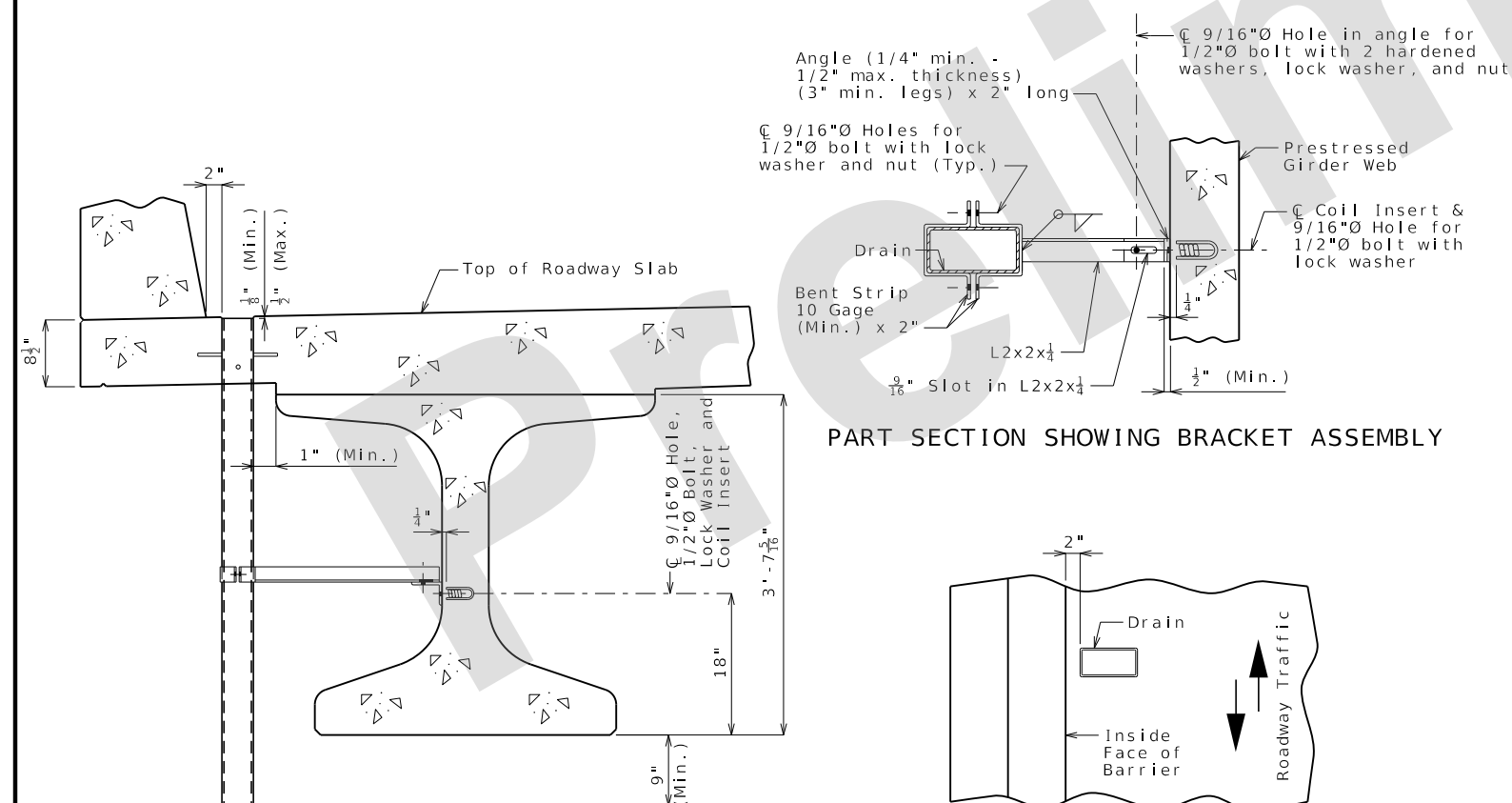
The bolt required to attach the slab drain bracket assembly to the prestressed girder web shall be supplied by the prestressed girder fabricator.

Notes for Steel Drain:

Slab drains may be fabricated of either 1/4" welded sheets of ASTM A709 Grade 36 steel or from 1/4" structural steel tubing ASTM A500 or A501.

Outside dimensions of drains are 8" x 4".

The drains shall be galvanized in accordance with ASTM A123.



SLAB DRAINS

DATE PREPARED	
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A	MO
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BR	17
COUNTY	
ADAIR	
JOB NO.	
JNE0140	
CONTRACT ID.	

PROJECT NO.
BRIDGE NO. A9442

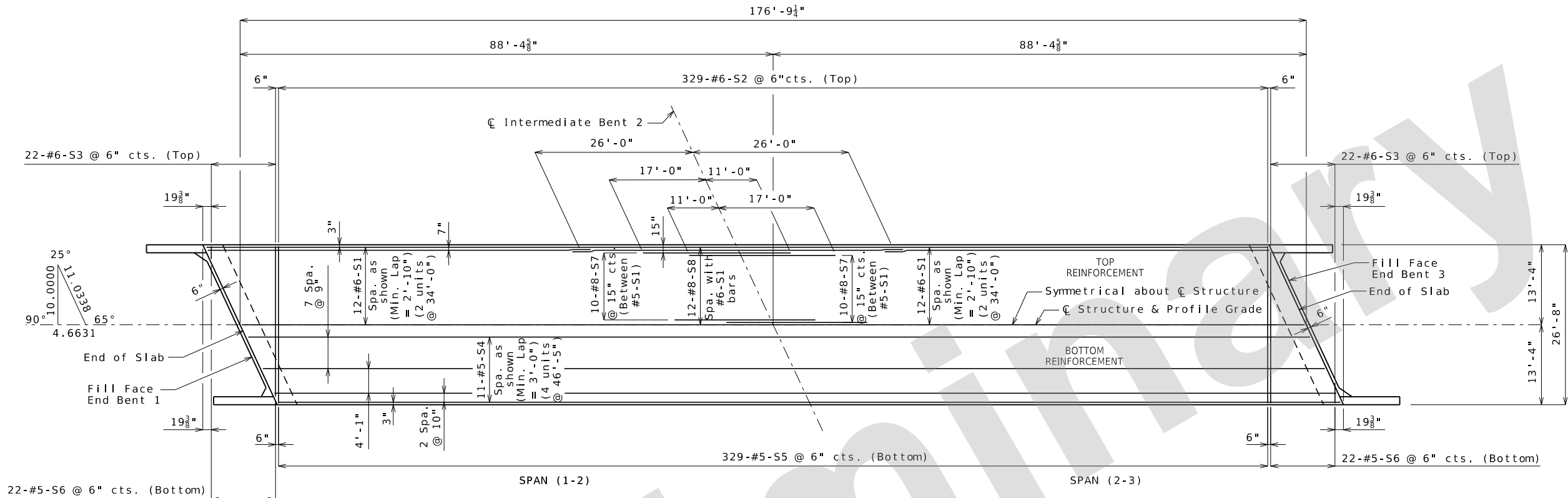
[illegible]MISSOURI HIGHWAYS AND TRANSPORTATION
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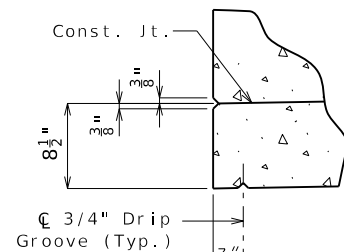


**VEENSTRA
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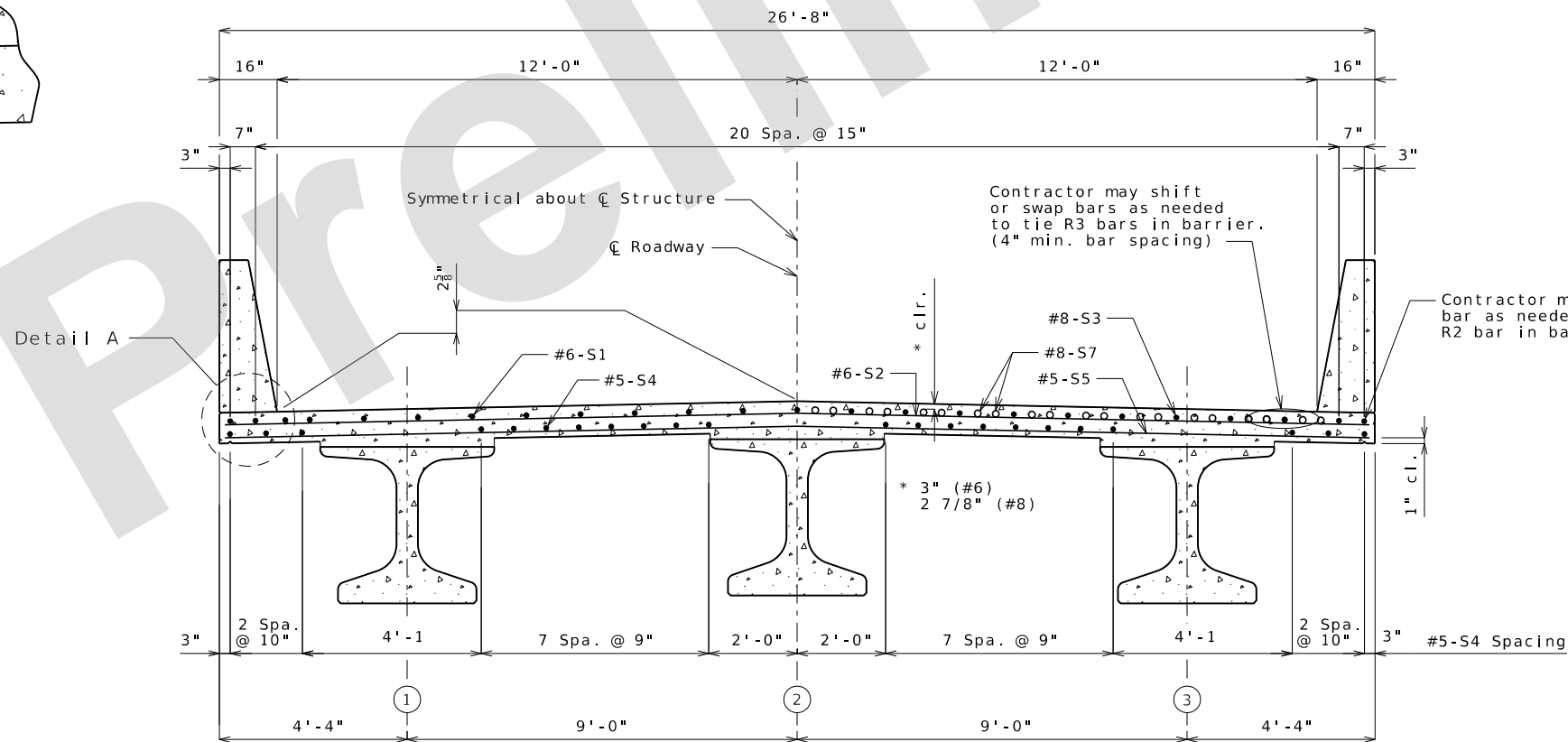
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816-781-6182 816-781-0643 (FAX)
Certificate of Authority No. F00362673



PLAN OF SLAB SHOWING REINFORCEMENT

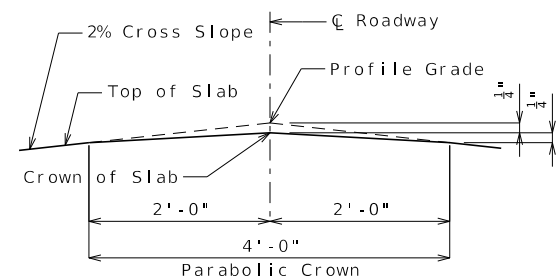


DETAIL A



TYPICAL HALF SECTION NEAR MID SPAN

TYPICAL HALF SECTION NEAR INTERMEDIATE BENT



PARABOLIC CROWN

Notes:
 For details of barrier, see Sheets No. 20 & 21.
 For details and locations of slab drains, see Sheet No. 17.
 For theoretical haunch, see Sheet No. 18.
 For bottom of slab elevations along beam, see Sheet No. 18.
 For slab pouring sequence, see Sheet No. 18.
 Longitudinal slab dimensions are measured horizontally.

Detailed May 2024
 Checked Jun. 2024

Sheet No. 19 of 28

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DATE PREPARED 06/07/2024	
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DISTRICT BR	SHEET NO. 19

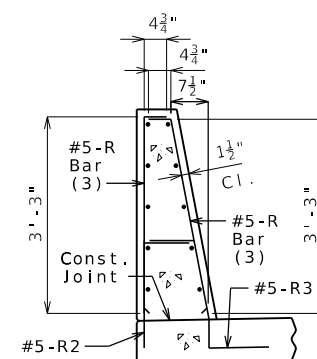
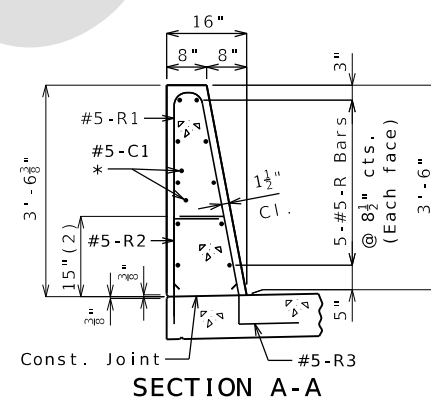
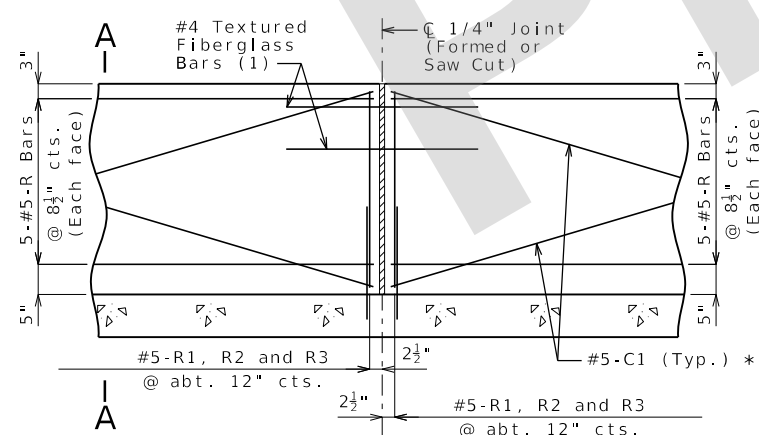
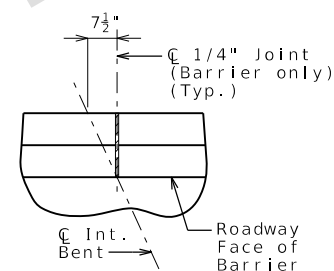
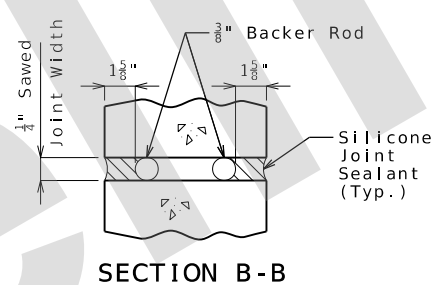
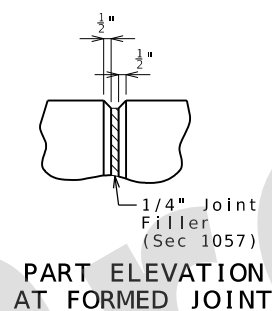
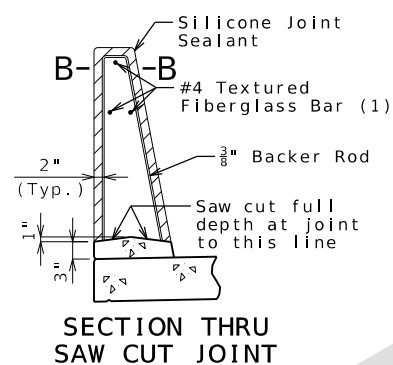
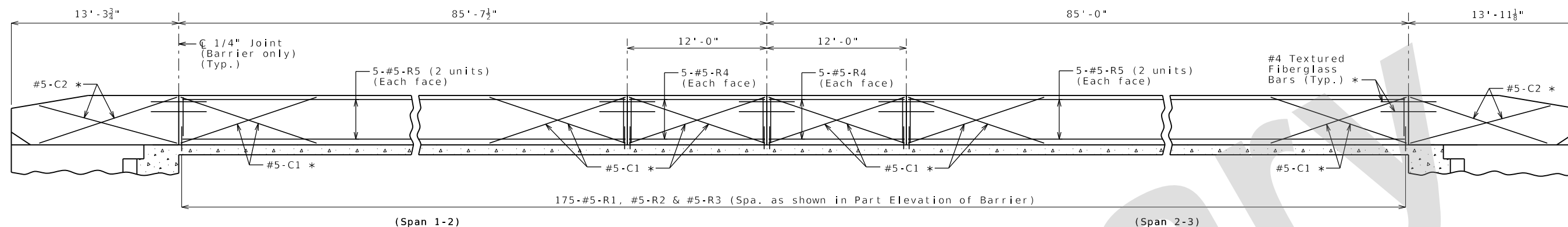
COUNTY ADAIR
JOB NO. JNE0140
CONTRACT ID.

PROJECT NO.
BRIDGE NO. A9442

DESCRIPTION	DATE

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION	105 WEST CAPITOL JEFFERSON CITY, MO 65102 1-888-ASK-MODOT (1-888-273-6636)
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TYPE D BARRIER

Sheet No. 20 of 28

General Notes:

* Slip-formed option only.

Conventional forming or slip forming may be used. Saw cut joints may be used with conventional forming.

Top of barrier shall be built parallel to grade and barrier joints (except at end bents) normal to grade.

All exposed edges of barrier shall have either a 1/2-inch radius or a 3/8-inch bevel, unless otherwise noted.

Payment for all concrete and reinforcement, complete in place, will be considered completely covered by the contract unit price for Type D Barrier per linear foot.

Concrete in barrier shall be Class B-1.

Measurement of barrier is to the nearest linear foot for each structure, measured along the outside top of slab from end of wing to end of wing.

Concrete traffic barrier delineators shall be placed on top of the barrier as shown on Missouri Standard Plan 617.10 and in accordance with Sec 617. Delineators on bridges with two-lane, two-way traffic shall have retroreflective sheeting on both sides. Concrete traffic barrier delineators will be considered completely covered by the contract unit price for Type D Barrier.

Joint sealant and backer rods shall be in accordance with Sec 717 for silicone joint sealant for saw cut and formed joints.

For slip-formed option, both sides of barrier shall have a vertically broomed finish and the top shall have a transversely broomed finish.

DATE PREPARED	
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ROUTE	STATE
A	MO
DISTRICT	SHEET NO.
BR	20
COUNTY	
ADAIR	
JOB NO.	
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CONTRACT ID.	

PROJECT NO.
BRIDGE NO. A9442

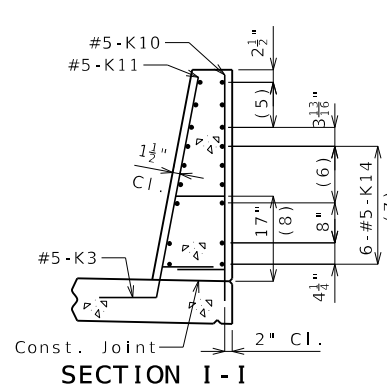
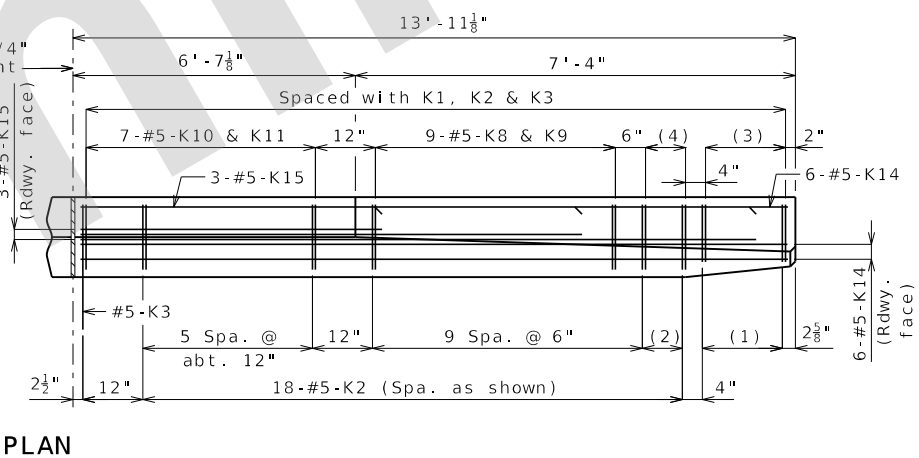
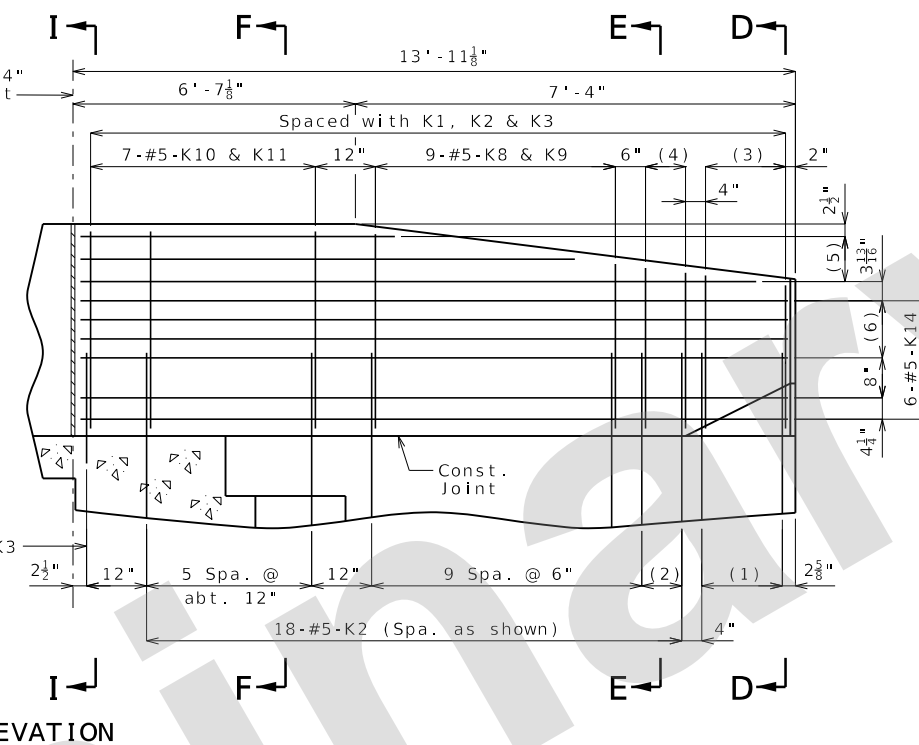
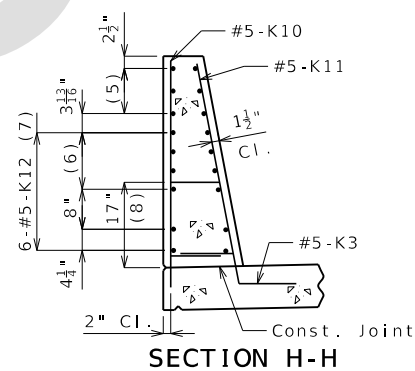
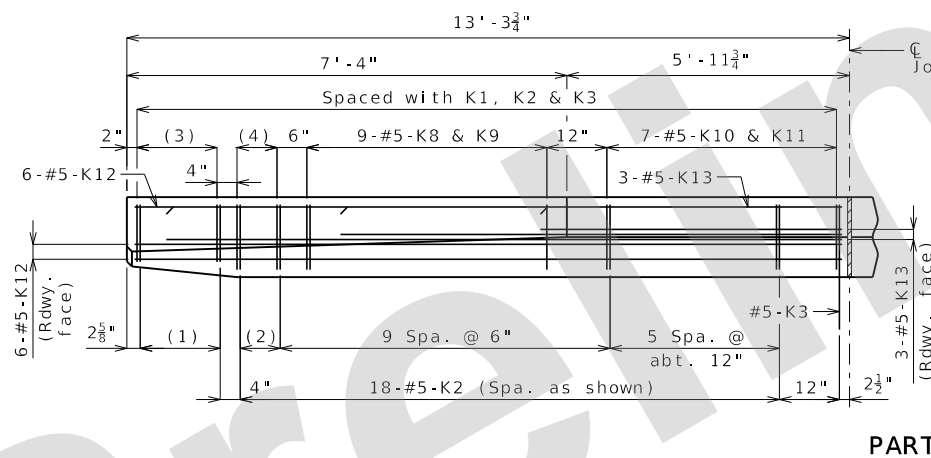
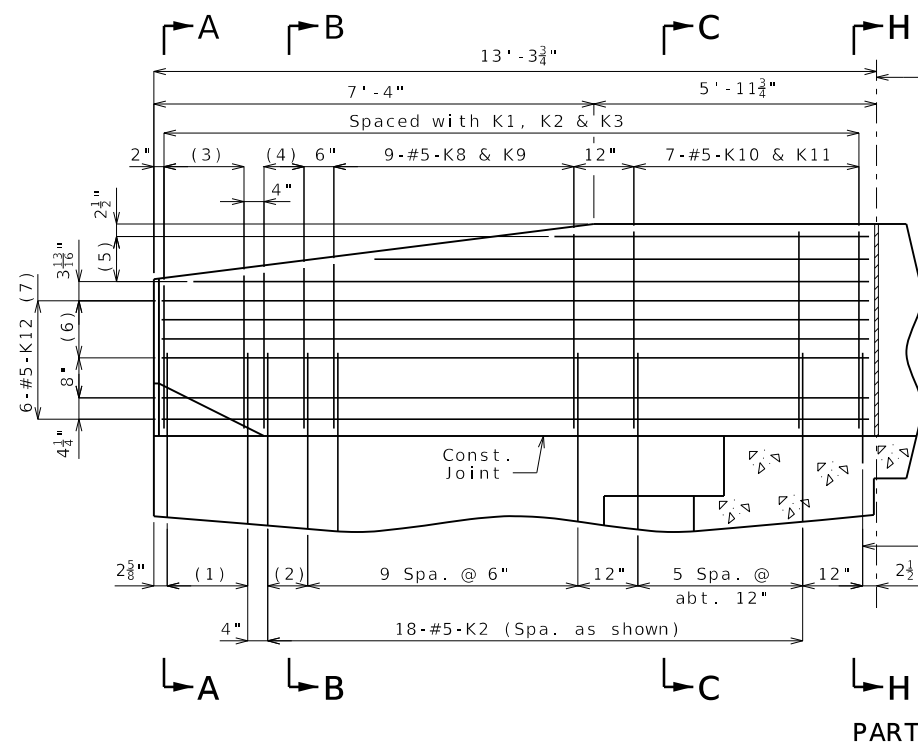
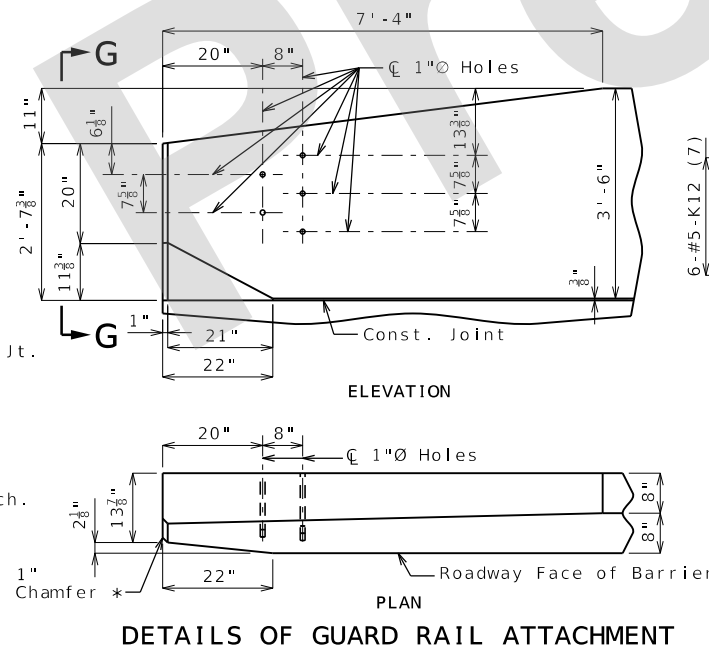
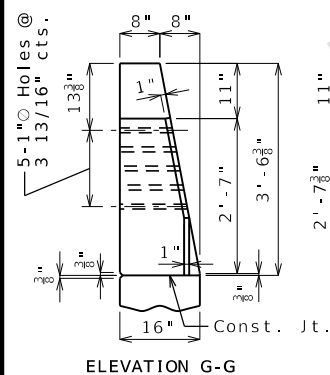
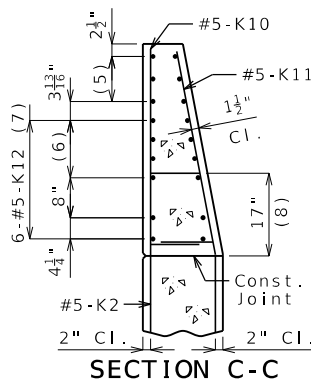
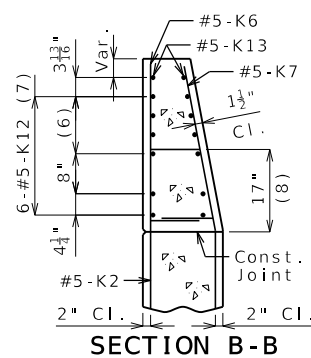
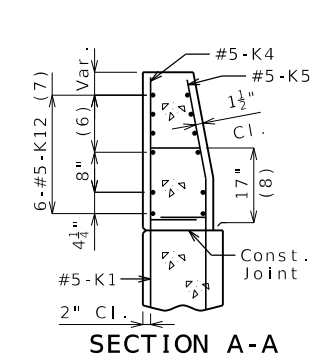
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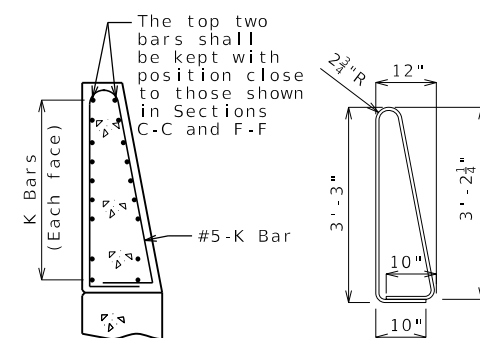


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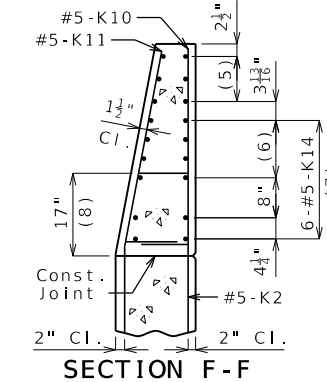
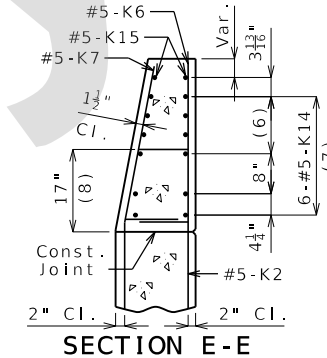
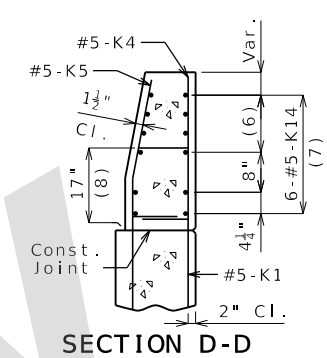
- (1) 5-#5-K1 @ 4" cts.
- (2) 2 spaces @ 4"
- (3) 5-#5-K4 & K5
- (4) 3-#5-K6 & K7
- (5) 3-#5-K13 or K15 @ 4 $\frac{1}{2}$ " cts.,
each face
- (6) 3 spaces @ 3 $\frac{1}{16}$ "
- (7) Spaced as shown, each face
- (8) To top of bar



**K10-K11 BAR PERMISSIBLE
ALTERNATE SHAPE**
(Other K bars not shown for clarity)

The K10-K11 bar combination may be furnished as one bar as shown, at the contractor's option.

All dimensions are out to out.



General Notes:

Concrete traffic barrier delineators shall be placed on top of the barrier as shown on Missouri Standard Plan 617.10 and in accordance with Sec 617. Delineators on bridges with two-lane, two-way traffic shall have retroreflective sheeting on both sides. Concrete traffic barrier delineators will be considered completely covered by the contract unit price for Type D Barrier.

Reinforcing Steel:

Minimum clearance to reinforcing steel shall be 1 1/2" except as shown for bars embedded into end bent.

TYPE D BARRIER AT END BENTS

(Left barrier shown, right barrier similar)

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 21 of 28

DATE PREPARED	
06/07/2024	
ROUTE	STATE
A	MO
DISTRICT	SHEET NO.
BR	21

COUNTY
ADAIR
JOB NO.
JNE0140
CONTRACT ID.

PROJECT NO.

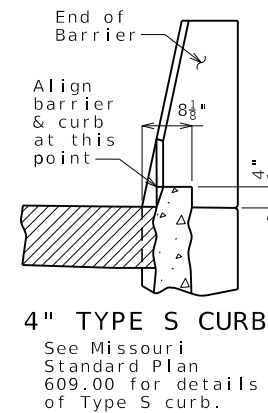
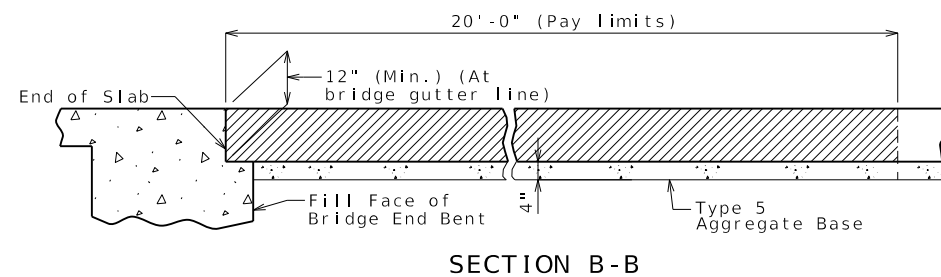
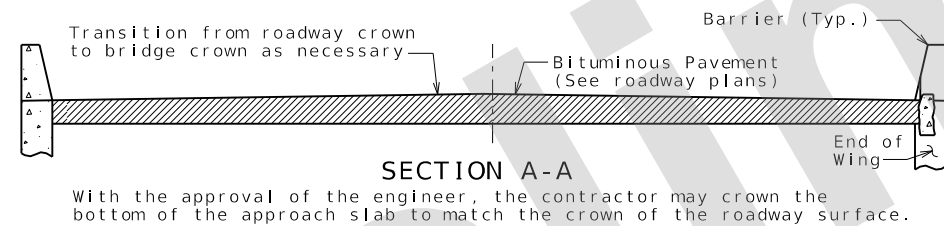
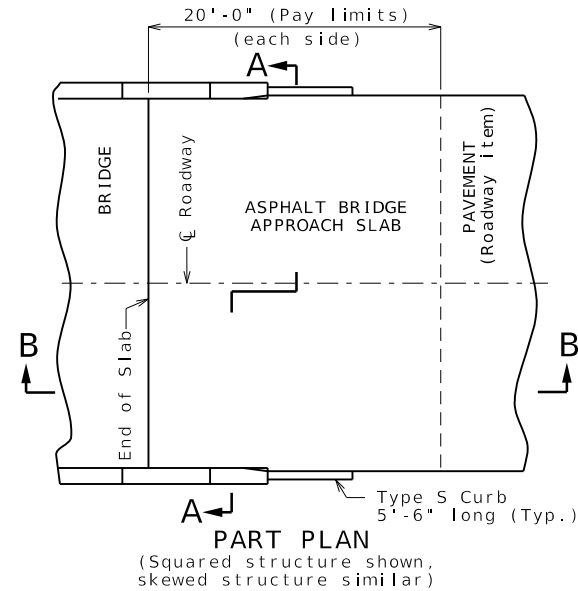
BRIDGE NO.
A9442

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General Notes:

Contractor shall construct the asphalt slab. The concrete slab is not allowed.

The contractor shall pour and satisfactorily finish the bridge slab before placing the bridge approach slab.

Notes For Asphalt Slab Only:

Payment for furnishing all materials, labor and excavation necessary to construct the asphalt bridge approach slab, including tack, curb, and Type 5 aggregate base within the pay limits shown, complete in place, will be considered completely covered by the contract unit price for Bridge Approach Slab (Minor) per square yard.

Application of tack is required between lifts per Sec 403.

DATE PREPARED 06/07/2024	
ROUTE A	STATE MO
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COUNTY ADAIR	
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BRIDGE NO. A9442	
DESCRIPTION	
DATE	

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ASPHALT SLAB

BRIDGE APPROACH SLAB (MINOR)

Integral end bents shown, non-integral end bent similar.

BORING LOG NO. B-21										Page 1 of 2				
PROJECT: MoDOT FARM Bridges Project					CLIENT: Missouri Department of Transportation Hannibal, Missouri									
SITE: Project J2S3318 - Bridge T0885 Adair County, MO														
MODEL LAYER	GRAPHIC LOG	LOCATION See Exploration Plan			DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	SAMPLE NUMBER	RECOVERY (In.)	FIELD TEST RESULTS	N ₆₀	POCKET PENETROMETER (tsf)	WATER CONTENT (%)	ATTERBERG LIMITS
		Northing: 1635837.041 Easting: 1660785.155 Approximate Surface Elev.: 843.5 (Ft.) +/-												
		DEPTH ELEVATION (Ft.)												
		0.5 <u>ROOT ZONE</u> , (Approximately 6 inches) 843 +/-												
2		<u>LEAN CLAY (CL)</u> , with sand, trace organics, brown and grayish brown, medium stiff to stiff				×		1	14	3-3-4 N=7	11	0.5	21.0	
		5.0 838.5 +/-			5	×		2	15	4-4-4 N=8	12	0.75	21.8	
		<u>SANDY LEAN CLAY (CL)</u> , brown and grayish brown, very soft to soft				×		3	18	0-0-0 N=0	0	0.0	24.9	
5		with sand seams, gray			10	×		4	11	0-1-2 N=3	5	0.0	24.7	
		17.0 826.5 +/-			15	×		5	11	0-0-1 N=1	2	0.0	24.0	
		<u>POORLY GRADED SAND (SP)</u> , trace clay, fine to medium grained, gray, very loose to medium dense			20	×		6	18	2-1-1 N=2	3		23.7	
					25	×		7	11	0-0-1 N=1	2		21.3	
4					30	×		8	10	7-7-8 N=15	23		15.4	
					35	×		9	14	5-7-10 N=17	26		15.1	
		37.0 806.5 +/-			40	×		10	15	4-6-6 N=12	19	1.75	26.4	
3		<u>SANDY FAT CLAY (CH)</u> , trace gravel, gray, stiff												
		42.0 801.5 +/-			45	×		11	18	7-5-11 N=16	25	1.75	22.2	
5		<u>SANDY LEAN CLAY (CL)</u> , trace gravel, gray, very stiff				×		12	18	6-7-10 N=17	26	2.0	21.8	
		52.0 791.5 +/-			50									
Stratification lines are approximate. In-situ, the transition may be gradual.														
Hammer Type: Automatic														
Advancement Method: 0-20 feet: Hollow-stem augers 20-100 feet: Mud rotary					See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any). See Supporting Information for explanation of symbols and abbreviations. Elevations provided by others					Notes: 92.5% Hammer efficiency				
Abandonment Method: Boring backfilled with bentonite grout upon completion														
WATER LEVEL OBSERVATIONS					<div style="font-size: 48pt; font-weight: bold; margin: 0;">Terracon</div> <div style="font-size: 10pt; margin: 5px 0 0 0;">11600 Lilburn Park Rd Saint Louis, MO</div>					Boring Started: 07-15-2020		Boring Completed: 07-15-2020		
9 feet while drilling										Drill Rig: DR840		Driller: DH		
										Project No.: 1520P078				

BORING LOG NO. B-21										Page 2 of 2		
PROJECT: MoDOT FARM Bridges Project					CLIENT: Missouri Department of Transportation Hannibal, Missouri							
SITE: Project J2S3318 - Bridge T0885 Adair County, MO												
MODEL LAYER	GRAPHIC LOG	LOCATION See Exploration Plan	DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	SAMPLE NUMBER	RECOVERY (in.)	FIELD TEST RESULTS	N ₆₀	POCKET PENETROMETER (tsf)	WATER CONTENT (%)	ATTERBERG LIMITS
		Northing: 1635837.041 Easting: 1660785.155 Approximate Surface Elev.: 843.5 (Ft.) +/-										LL-PL-P
		DEPTH ELEVATION (Ft.)										
		SANDY LEAN CLAY (CL), trace gravel, gray, stiff to very stiff	55	X		13	18	6-7-9 N=16	25	2.0	23.3	
		with sand seams	60	X		14	18	8-8-11 N=19	29	2.0	21.5	
			65	X		15	18	5-7-8 N=15	23	1.5	23.4	
			70	X		16	18	7-8-9 N=17	26	1.75	18.8	
			75	X		17	18	9-9-11 N=20	31	2.0	19.5	
			80	X		18	18	7-10-9 N=19	29	1.75	18.9	
			85	X		19	18	7-9-11 N=20	31	1.75	18.1	
			90	X		20	18	6-10-13 N=23	35	2.0	17.6	
			95	X		21	18	6-11-12 N=23	35	2.0	16.6	
		100.0 743.5+/-	100	X		22	18	8-13-13 N=26	40	2.25	18.6	
Boring Terminated at 100 Feet												
Stratification lines are approximate. In-situ, the transition may be gradual. Hammer Type: Automatic												
Advancement Method: 0-20 feet: Hollow-stem augers 20-100 feet: Mud rotary				See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (if any). See Supporting Information for explanation of symbols and abbreviations. Elevations provided by others				Notes: 92.5% Hammer efficiency				
Abandonment Method: Boring backfilled with bentonite grout upon completion												
WATER LEVEL OBSERVATIONS												
9 feet while drilling												
				Terracon				Boring Started: 07-15-2020 Boring Completed: 07-15-2020				
				11600 Lilburn Park Rd Saint Louis, MO				Drill Rig: DR840 Driller: DH				
								Project No.: 1520P078				

[illegible]

BORING LOG NO. B-22							Page 1 of 2											
PROJECT: MoDOT FARM Bridges Project					CLIENT: Missouri Department of Transportation Hannibal, Missouri													
SITE: Project J2S3318 - Bridge T0885 Adair County, MO																		
MODEL LAYER	GRAPHIC LOG	LOCATION See Exploration Plan			DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	SAMPLE NUMBER	RECOVERY (in.)	FIELD TEST RESULTS	N ₆₀	POCKET PENETROMETER (tsf)	WATER CONTENT (%)	ATTERBERG LIMITS				
		Northing: 1635810.351 Easting: 1661011.86 Approximate Surface Elev.: 845.0187 (Ft.) +/-												LL-PL-P				
		DEPTH ELEVATION (Ft.)																
1		0.5' ROOT ZONE, (Approximately 6 inches) 844.5+/-					X	1	16	4-3-4 N=7	11	1.0	21.0					
2		LEAN CLAY (CL), with sand, trace organics, grayish brown, medium stiff					X	2	15	4-3-3 N=6	9	1.0	23.9					
		6.0 839+/-			5		X	3	18	2-4-4 N=8	12	0.75	22.6					
5		SANDY LEAN CLAY (CL), grayish brown and gray, stiff					X	4	18	4-4-5 N=9	14	1.0	21.8					
		12.0 833+/-			10													
3		FAT CLAY (CH), with sand, grayish brown and gray, very soft					X	5	12	0-0-0 N=0	0	0.0	38.0	53-21-32				
		17.0 828+/-			15													
4		POORLY GRADED SAND (SP), with clay seams, fine to coarse grained, gray, very loose					X	6	18	0-1-2 N=3	5	0.25	19.4					
		27.0 818+/-			20		X	7	9	2-2-1 N=3	5	0.25	19.1					
2		SANDY LEAN CLAY (CL), gray, very soft to medium stiff					X	8	28	2-2-3 N=5	8	0.5	31.8					
		42.0 803+/-			30		X	9	18	0-0-1 N=1	2	0.25	25.0					
3		FAT CLAY (CH), with sand, very soft					X	10	15	0-0-2 N=2	3	0.25	21.4					
		47.0 798+/-			40		X	11	18	0-0-0 N=0	0	0.5	12.2	56-24-32				
5		SANDY LEAN CLAY (CL), trace gravel, gray, very stiff					X	12	18	6-9-7 N=16	25	2.0	21.5					
		52.0 793+/-			50													
Stratification lines are approximate. In-situ, the transition may be gradual.					Hammer Type: Automatic													
Advancement Method: 0-20 feet: Hollow-stem augers 20-100 feet: Mud rotary					See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).					Notes: 92.5% Hammer efficiency								
Abandonment Method: Boring backfilled with bentonite grout upon completion					See Supporting Information for explanation of symbols and abbreviations.													
					Elevations provided by others													
WATER LEVEL OBSERVATIONS																		
▽ 13 feet while drilling																		

BORING LOG NO. B-22										Page 2 of 2		
PROJECT: MoDOT FARM Bridges Project					CLIENT: Missouri Department of Transportation Hannibal, Missouri							
SITE: Project J2S3318 - Bridge T0885 Adair County, MO												
MODEL LAYER	GRAPHIC LOG	LOCATION See Exploration Plan	DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	SAMPLE NUMBER	RECOVERY (in.)	FIELD TEST RESULTS	N ₆₀	POCKET PENETROMETER (tsf)	WATER CONTENT (%)	ATTERBERG LIMITS
		Northing: 1635810.351 Easting: 1661011.86 Approximate Surface Elev.: 845.0187 (Ft.) +/-	DEPTH ELEVATION (Ft.)									LL-PL-P
		SANDY LEAN CLAY (CL), trace gravel, gray, very stiff				13	18	5-7-9 N=16	25	2.0	23.8	
			55			14	18	9-11-11 N=22	34	2.0	17.2	
			60			15	18	6-8-9 N=17	26	2.0	22.3	
			65			16	18	5-7-19 N=26	40	2.0	21.7	
			70			17	18	7-7-9 N=16	25	2.0	20.2	
			75			18	5	8-9-11 N=20	31	2.0	19.2	
		with sand seams, hard	80			19	18	12-28-37 N=65	100	0.25	12.9	
			85			20	18	6-9-10 N=19	29	2.0	18.8	
			90			21	18	8-10-14 N=24	37	2.0	17.4	
			95			22	18	8-11-15 N=26	40	2.0	19.0	
		100.0 745+/-	100									
Boring Terminated at 100 Feet												
Stratification lines are approximate. In-situ, the transition may be gradual.												
Hammer Type: Automatic												
Advancement Method: 0-20 feet: Hollow-stem augers 20-100 feet: Mud rotary				See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (if any).				Notes: 92.5% Hammer efficiency				
Abandonment Method: Boring backfilled with bentonite grout upon completion				See Supporting Information for explanation of symbols and abbreviations.								
				Elevations provided by others								
WATER LEVEL OBSERVATIONS				Terracon 11600 Lilburn Park Rd Saint Louis, MO				Boring Started: 07-14-2020				
13 feet while drilling								Boring Completed: 07-14-2020				
								Drill Rig: DR840				
								Driller: DH				
								Project No.: 1520P078				

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