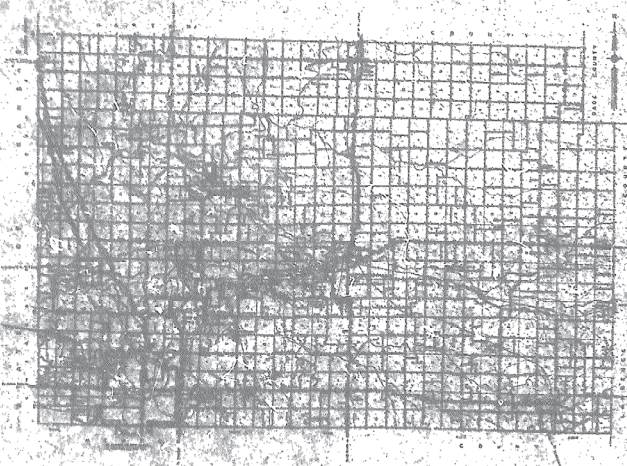
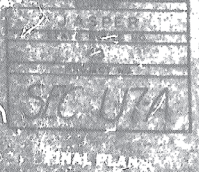


LOCATION MAP



MISSOURI  
STATE HIGHWAY COMMISSION  
PLAN AND PROFILE  
OF PROPOSED  
STATE ROAD

FEDERAL AID PROJECT  
JASPER COUNTY



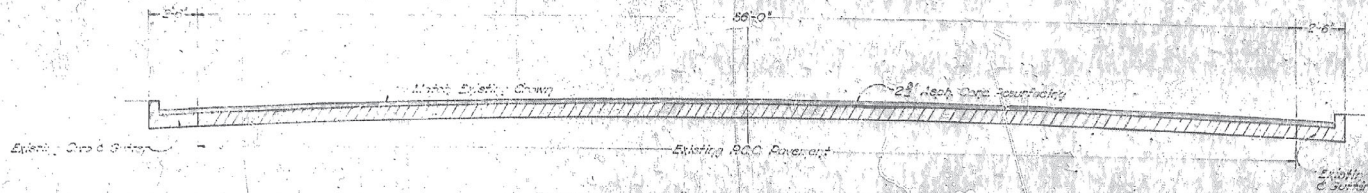
100-47424-2000-101

310

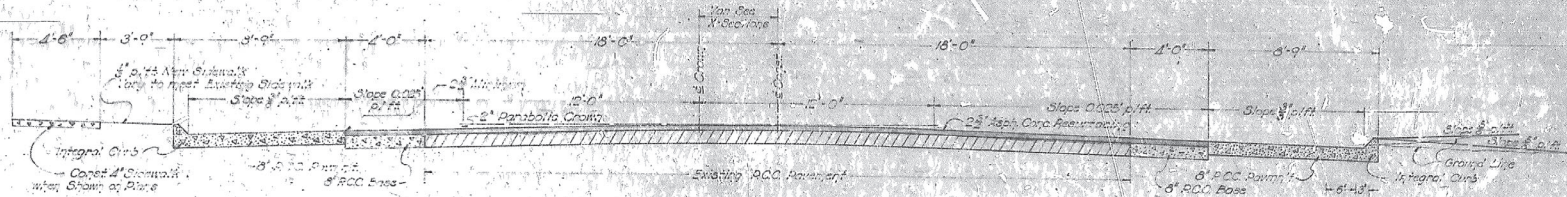
**CONVENTIONAL SIGNS**

STATE AND NATIONAL LINE	LEWIS
COUNTY LINE	CULVERTS
CITY, VILLAGE OR BOROUGH	DROP SHEET
TOWNSHIP LINE	TROLLEY POLE
SECTION LINE	POWER POLE
GRAND LINE	TELEPHONE OR TELEGRAPH POLE
TRAIL LINE	MASH
GRAND BAY	LEWIS
UNFERCED PROPERTY	GROUND ELEVATION
RIGHT OF WAY LINE	GRADE ELEVATION
TRAVELED WAY	SURFACE LINE
RAILROAD	GRADE LINE
RETAINING WALL	
BASE OR SURVEY LINE	

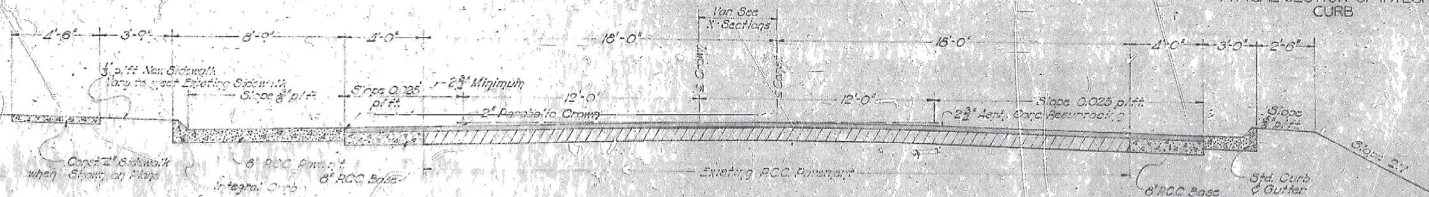




TYPICAL SECTION ASPHALTIC CONCRETE RESURFACING  
STA 0+00 TO STA 8+41



TYPICAL SECTION 8\"/>



TYPICAL SECTION 8\"/>

TYPICAL SECTION OF INTEGRAL CURB



DETAIL OF ASPHALTIC CONCRETE RESURFACING

TYPICAL SECTION IS  
RTE 43 JASPER COUNTY  
SECTION U7-A

311

NO.	DATE	REVISION

NO.	DATE	REVISION



LOCATION North Main Street to Maple Lane East Street North to F Street  
 Graded Earth Within Existing PCC Pavement Culverts & Asphaltic  
 TYPE Concrete Pavement Width From 41' to 60' to Width

# MISSOURI STATE HIGHWAY COMMISSION

## SUMMARY OF QUANTITIES

Sheet No 1 of 2

FINAL PLANS

FED. ROAD DIST. NO.	STATE	PROJECT	FISCAL YEAR	HEET NO.	TOTAL SHEETS
5	MO.				
CIV. NO.	COUNTY	SHEET NO.	SHEET NO.		
7	JAS. PER	13	13		

### GENERAL SUMMARY

EXCAVATION		36" CURB & GUTTER		LENGTH OF PROJECT		ITEM NO.	DESCRIPTION	UNIT	TOTAL UNITS	NO. LISTS
Station	Excavation	Station	Excavation	Station	End of Project					
10100	0.00	10100	0.00	10100	End of Project					
10105	15.96	10105	15.96	10105	End of Project					
10110	31.92	10110	31.92	10110	End of Project					
10115	47.88	10115	47.88	10115	End of Project					
10120	63.84	10120	63.84	10120	End of Project					
10125	79.80	10125	79.80	10125	End of Project					
10130	95.76	10130	95.76	10130	End of Project					
10135	111.72	10135	111.72	10135	End of Project					
10140	127.68	10140	127.68	10140	End of Project					
10145	143.64	10145	143.64	10145	End of Project					
10150	159.60	10150	159.60	10150	End of Project					
10155	175.56	10155	175.56	10155	End of Project					
10160	191.52	10160	191.52	10160	End of Project					
10165	207.48	10165	207.48	10165	End of Project					
10170	223.44	10170	223.44	10170	End of Project					
10175	239.40	10175	239.40	10175	End of Project					
10180	255.36	10180	255.36	10180	End of Project					
10185	271.32	10185	271.32	10185	End of Project					
10190	287.28	10190	287.28	10190	End of Project					
10195	303.24	10195	303.24	10195	End of Project					
10200	319.20	10200	319.20	10200	End of Project					
10205	335.16	10205	335.16	10205	End of Project					
10210	351.12	10210	351.12	10210	End of Project					
10215	367.08	10215	367.08	10215	End of Project					
10220	383.04	10220	383.04	10220	End of Project					
10225	399.00	10225	399.00	10225	End of Project					
10230	414.96	10230	414.96	10230	End of Project					
10235	430.92	10235	430.92	10235	End of Project					
10240	446.88	10240	446.88	10240	End of Project					
10245	462.84	10245	462.84	10245	End of Project					
10250	478.80	10250	478.80	10250	End of Project					
10255	494.76	10255	494.76	10255	End of Project					
10260	510.72	10260	510.72	10260	End of Project					
10265	526.68	10265	526.68	10265	End of Project					
10270	542.64	10270	542.64	10270	End of Project					
10275	558.60	10275	558.60	10275	End of Project					
10280	574.56	10280	574.56	10280	End of Project					
10285	590.52	10285	590.52	10285	End of Project					
10290	606.48	10290	606.48	10290	End of Project					
10295	622.44	10295	622.44	10295	End of Project					
10300	638.40	10300	638.40	10300	End of Project					
10305	654.36	10305	654.36	10305	End of Project					
10310	670.32	10310	670.32	10310	End of Project					
10315	686.28	10315	686.28	10315	End of Project					
10320	702.24	10320	702.24	10320	End of Project					
10325	718.20	10325	718.20	10325	End of Project					
10330	734.16	10330	734.16	10330	End of Project					
10335	750.12	10335	750.12	10335	End of Project					
10340	766.08	10340	766.08	10340	End of Project					
10345	782.04	10345	782.04	10345	End of Project					
10350	798.00	10350	798.00	10350	End of Project					
10355	813.96	10355	813.96	10355	End of Project					
10360	829.92	10360	829.92	10360	End of Project					
10365	845.88	10365	845.88	10365	End of Project					
10370	861.84	10370	861.84	10370	End of Project					
10375	877.80	10375	877.80	10375	End of Project					
10380	893.76	10380	893.76	10380	End of Project					
10385	909.72	10385	909.72	10385	End of Project					
10390	925.68	10390	925.68	10390	End of Project					
10395	941.64	10395	941.64	10395	End of Project					
10400	957.60	10400	957.60	10400	End of Project					
10405	973.56	10405	973.56	10405	End of Project					
10410	989.52	10410	989.52	10410	End of Project					
10415	1005.48	10415	1005.48	10415	End of Project					
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10425	1037.40	10425	1037.40	10425	End of Project					
10430	1053.36	10430	1053.36	10430	End of Project					
10435	1069.32	10435	1069.32	10435	End of Project					
10440	1085.28	10440	1085.28	10440	End of Project					
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10455	1133.16	10455	1133.16	10455	End of Project					
10460	1149.12	10460	1149.12	10460	End of Project					
10465	1165.08	10465	1165.08	10465	End of Project					
10470	1181.04	10470	1181.04	10470	End of Project					
10475	1197.00	10475	1197.00	10475	End of Project					
10480	1212.96	10480	1212.96	10480	End of Project					
10485	1228.92	10485	1228.92	10485	End of Project					
10490	1244.88	10490	1244.88	10490	End of Project					
10495	1260.84	10495	1260.84	10495	End of Project					
10500	1276.80	10500	1276.80	10500	End of Project					
10505	1292.76	10505	1292.76	10505	End of Project					
10510	1308.72	10510	1308.72	10510	End of Project					
10515	1324.68	10515	1324.68	10515	End of Project					
10520	1340.64	10520	1340.64	10520	End of Project					
10525	1356.60	10525	1356.60	10525	End of Project					
10530	1372.56	10530	1372.56	10530	End of Project					
10535	1388.52	10535	1388.52	10535	End of Project					
10540	1404.48	10540	1404.48	10540	End of Project					
10545	1420.44	10545	1420.44	10545	End of Project					
10550	1436.40	10550	1436.40	10550	End of Project					
10555	1452.36	10555	1452.36	10555	End of Project					
10560	1468.32	10560	1468.32	10560	End of Project					
10565	1484.28	10565	1484.28	10565	End of Project					
10570	1500.24	10570	1500.24	10570	End of Project					
10575	1516.20	10575	1516.20	10575	End of Project					
10580	1532.16	10580	1532.16	10580	End of Project					
10585	1548.12	10585	1548.12	10585	End of Project					
10590	1564.08	10590	1564.08	10590	End of Project					
10595	1580.04	10595	1580.04	10595	End of Project					
10600	1596.00	10600	1596.00	10600	End of Project					
10605	1611.96	10605	1611.96	10605	End of Project					
10610	1627.92	10610	1627.92	10610	End of Project					
10615	1643.88	10615	1643.88	10615	End of Project					
10620	1659.84	10620	1659.84	10620	End of Project					
10625	1675.80	10625	1675.80	10625	End of Project					
10630	1691.76	10630	1691.76	10630	End of Project					
10635	1707.72	10635	1707.72	10635	End of Project					
10640	1723.68	10640	1723.68	10640	End of Project					
10645	1739.64	10645	1739.64	10645	End of Project					
10650	1755.60	10650	1755.60	10650	End of Project					
10655	1771.56	10655	1771.56	10655	End of Project					
10660	1787.52	10660	1787.52	10660	End of Project					
10665	1803.48	10665	1803.48	10665	End of Project					
10670	1819.44	10670	1819.44	10670	End of Project					
10675	1835.40	10675	1835.40	10675	End of Project					
10680	1851.36	10680	1851.36	10680	End of Project					
10685	1867.32	10685	1867.32	10685	End of Project					



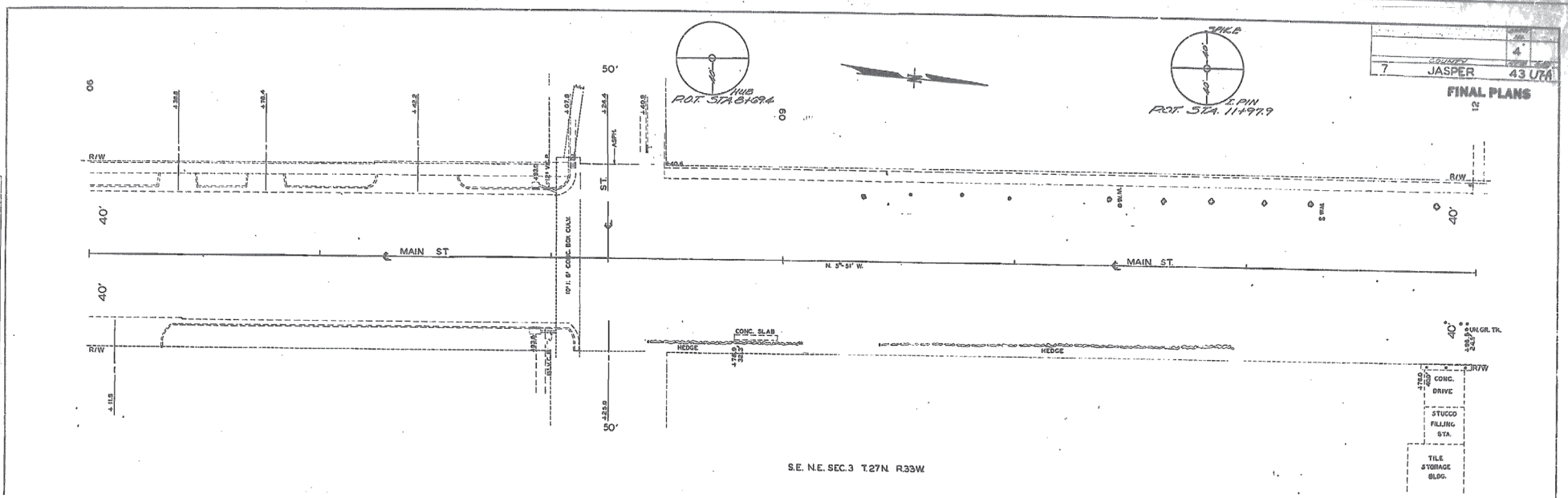








7	JASPER	43 UTA
FINAL PLANS		



S.E. N.E. SEC. 3 T.27N R.33W

- CONC. DRIVE
- STUCCO FILLING STA.
- TILE STORAGE SLUG.

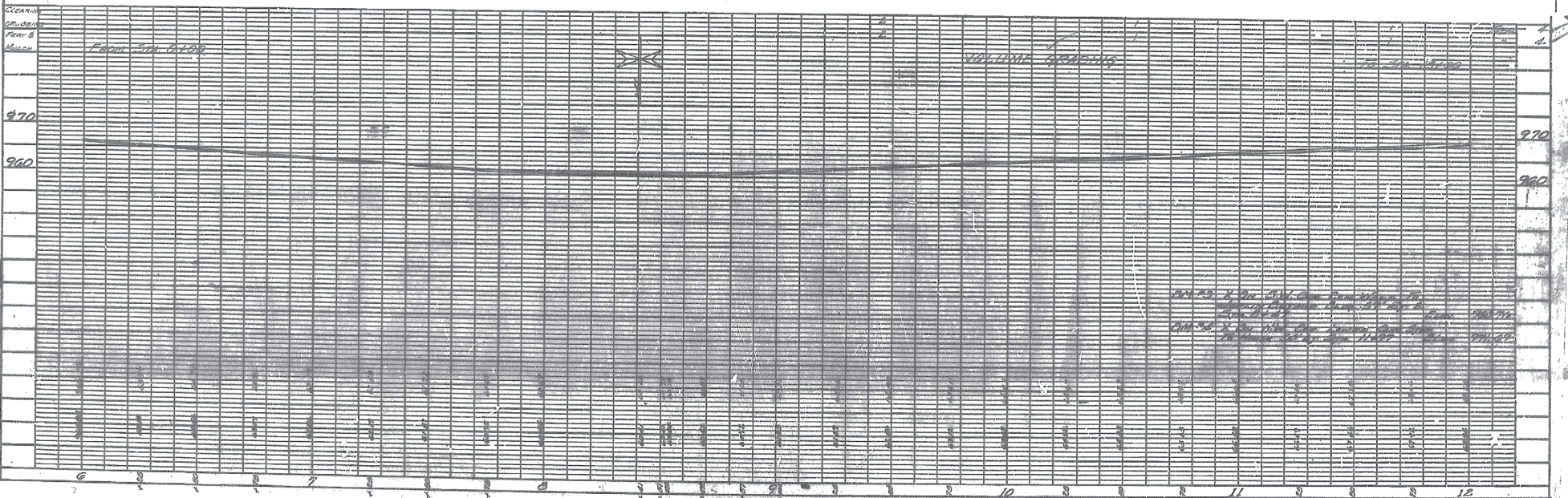
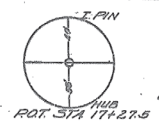


PLATE 1 - PLAN - PROPOSED D.P. & A.C. STAIRS  
 REUFEL & ESER CO., NEW YORK

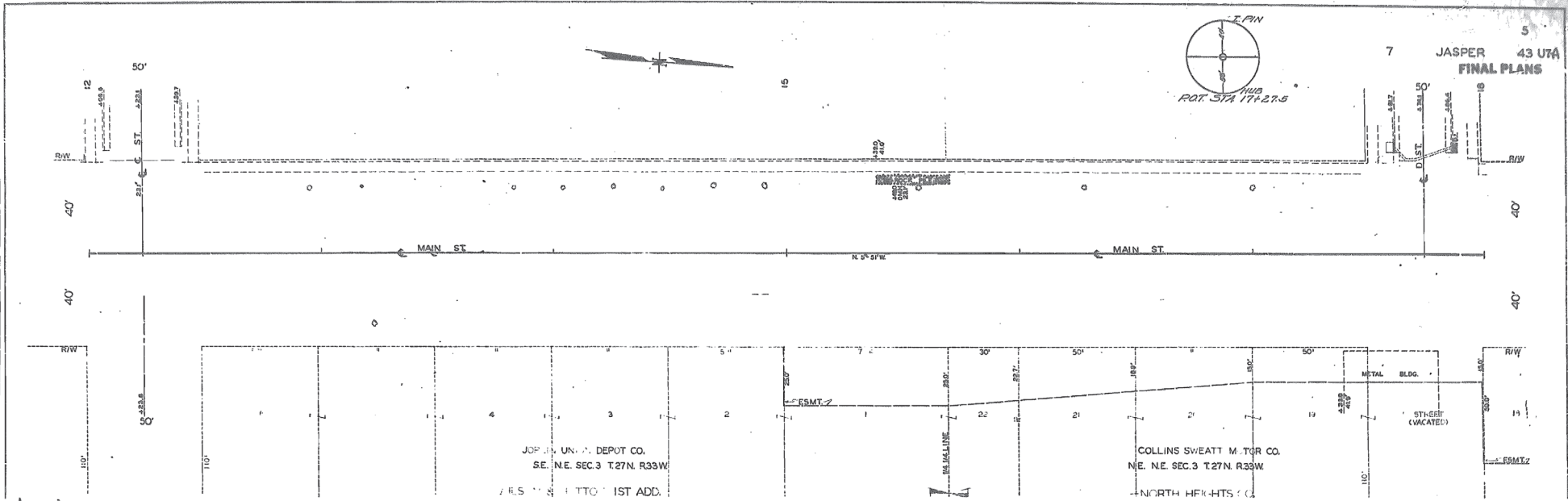
H. & S. CO. N.Y.



PLAN  
 SHEET NO. 12  
 PROJECT NO. 43 UTA  
 DRAWN BY: J. H. ...  
 CHECKED BY: ...  
 DATE: ...



5  
 JASPER 43 UTA  
 FINAL PLANS



JORDAN UNIV. DEPOT CO.  
 SE. NE. SEC. 3 T. 27N. R. 33W  
 1/2 S. 1/4 E. 1/4 T. 27N. R. 33W

COLLINS SWEATT M. TOR CO.  
 NE. NE. SEC. 3 T. 27N. R. 33W  
 NORTH HEIGHTS CO.

316

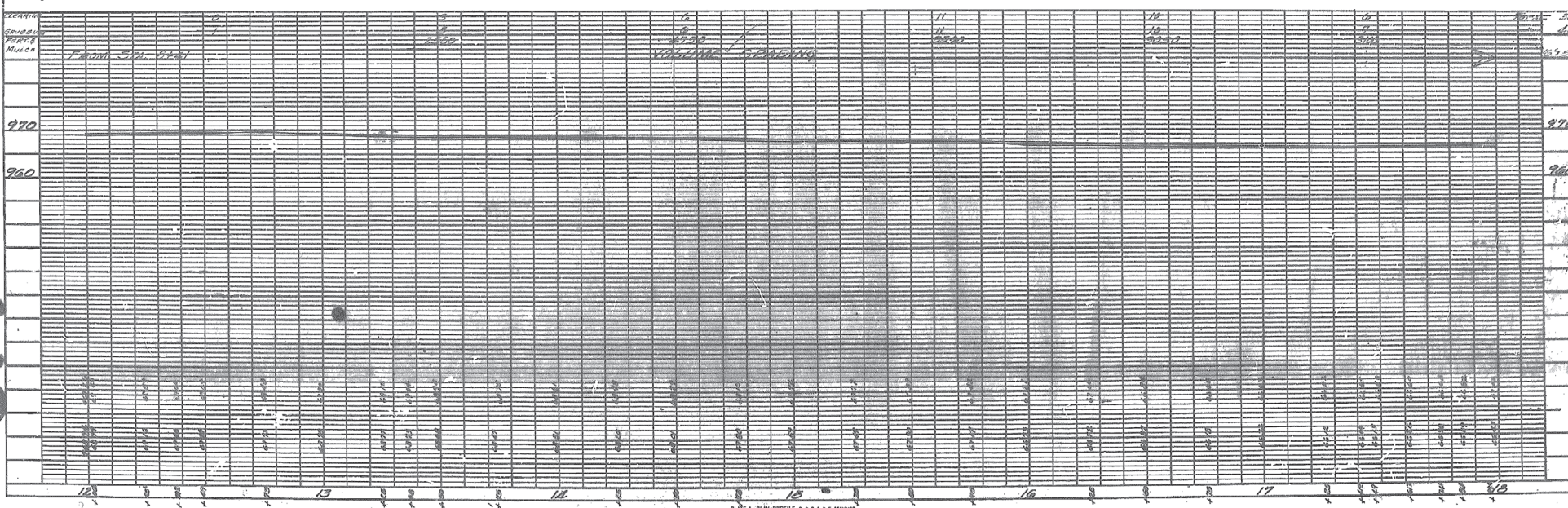


PLATE 1 - PLAN-PROFILE OF P. H. & C. STANBRO  
 HUNTER & CO. CO. NEW YORK



PLAN  
 2" = 40'  
 1/4" = 10'  
 1/8" = 5'  
 1/16" = 2.5'  
 1/32" = 1.25'

6  
 7 JASPER 43 UTA  
 FINAL PLANS  
 8

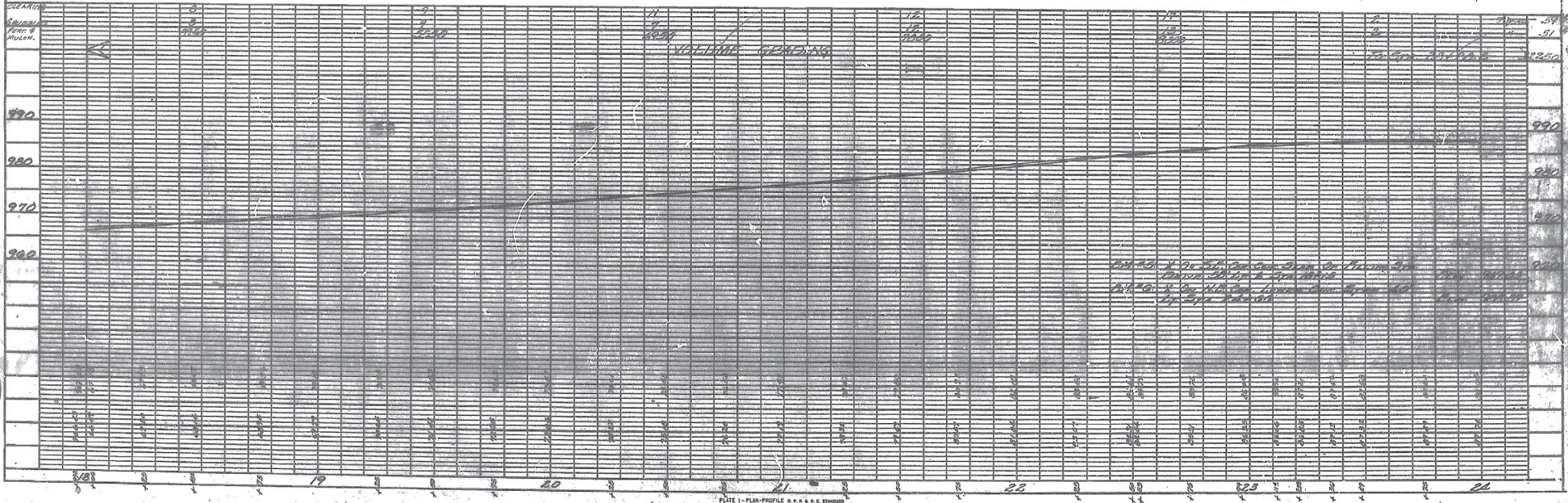
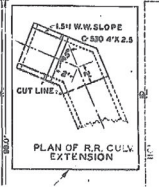
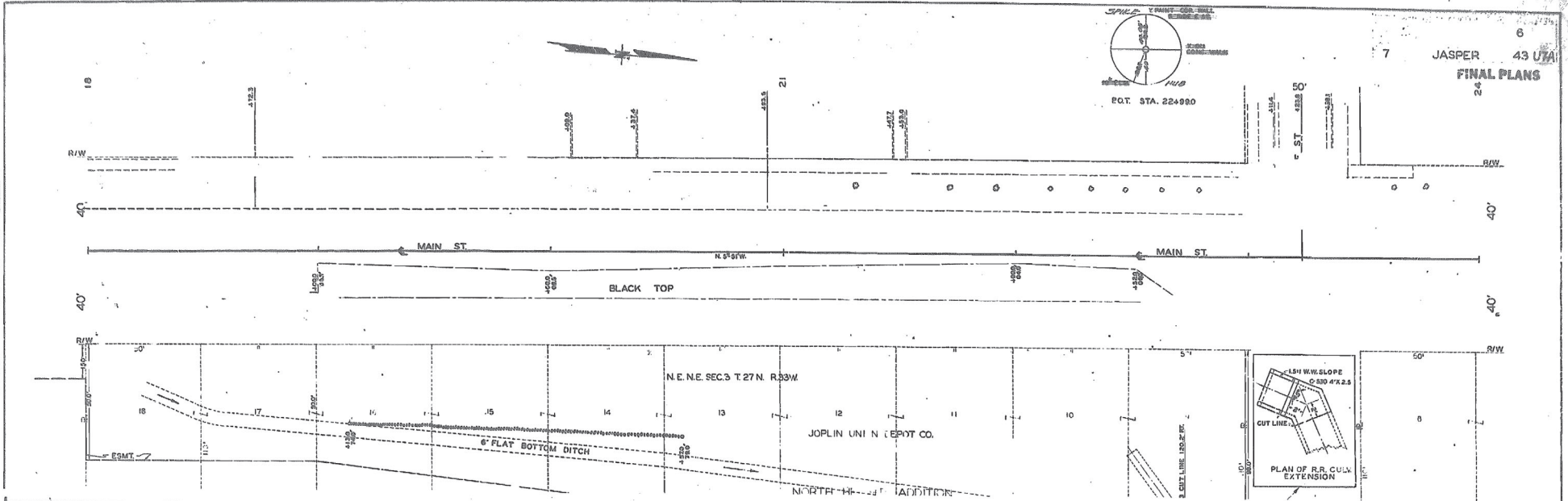
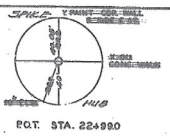


PLATE 1 - PLAN PROFILE OF R.R. & S.E. STRONG  
 SURVEY & ENGINEERING CO. NEW YORK

K.E.C. OR. 10-10-10





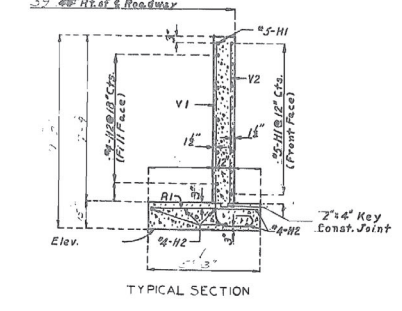
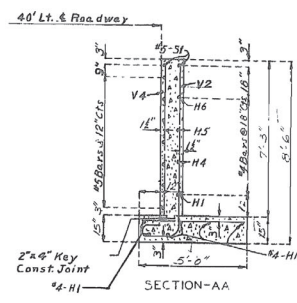
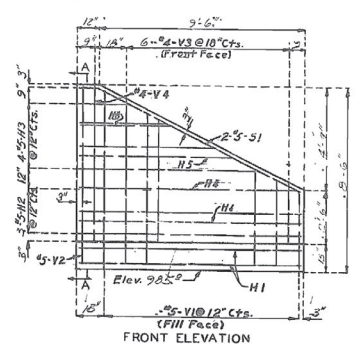
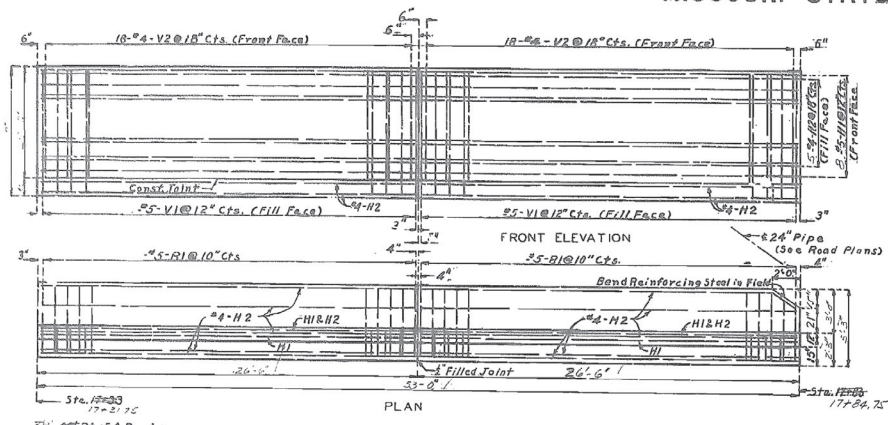


# MISSOURI STATE HIGHWAY DEPARTMENT

FED. ROAD DIST. NO.	STATE NO.	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
8			19		

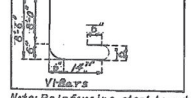
FINAL PLANS  
FINAL PLANS

# 8



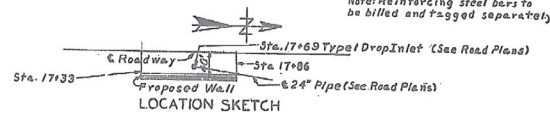
**DETAIL OF FILLED JOINT**  
Note: Use bevel as shown for exposed faces of all joints consisting of joint filler.

No.	Size	Length	Mark
54	#5	11.5'	V1
56	#2	7.0'	V2
78	#5	26.3'	H1
82	#2	26.3'	H2
84	#5	5.0'	R1

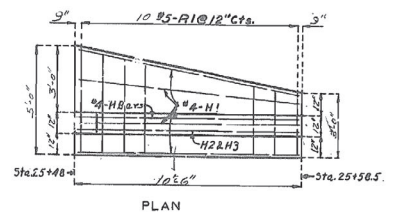


Note: Reinforcing steel bars to be billed and tagged separately.

FINAL QUANTITIES	
Concrete Class "B" Cu.Yds.	28.4
Reinforcing Steel Lbs.	2030.7
Class 3 Excavation Cu.Yds.	25.3

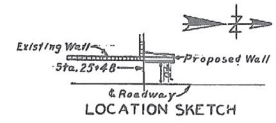


RETAINING WALL FROM STA 17+33 TO STA 17+86  
DRAWING NO L-651



Note: Excavation below and outside the limits of Roadway Excavation shall be paid for as class 3 Excavation for structures.

FINAL QUANTITIES	
Concrete Class "B" Cu.Yds.	3.9
Reinforcing Steel Lbs.	300
Class 3 Excavation Cu.Yds.	16.5



RETAINING WALL FROM STA. 25+48 TO STA. 25+58.5  
DRAWING NO. L-652

REINFORCING STEEL				Cutting and Bending Diagrams			
No.	Size	Length	Mark	10'-6"	6'-4"	9'-0"	7'-5"
1	#5	10'-6"	V1	8'-0"	4'-0"	9'-0"	7'-5"
3	#4	9'-0"	V3	8'-0"	4'-0"	9'-0"	7'-5"
7	#4	7'-0"	V4	8'-0"	4'-0"	9'-0"	7'-5"
7	#4	10'-3"	H1	8'-0"	4'-0"	9'-0"	7'-5"
3	#5	10'-3"	H2	8'-0"	4'-0"	9'-0"	7'-5"
2	#5	10'-6"	H3	8'-0"	4'-0"	9'-0"	7'-5"
1	#4	10'-0"	H4	8'-0"	4'-0"	9'-0"	7'-5"
1	#4	7'-0"	H5	8'-0"	4'-0"	9'-0"	7'-5"
1	#4	4'-0"	H6	8'-0"	4'-0"	9'-0"	7'-5"
5	#5	7'-3"	R1	8'-0"	4'-0"	9'-0"	7'-5"
2	#5	11'-9"	S1	8'-0"	4'-0"	9'-0"	7'-5"

Note: Reinforcing steel bars to be billed and tagged separately.

B.M. No. 5 Elev. 969.03 on S.E. corner slab of Filling Station Drive 3' 8" L1 @ Main St. Sta. 17+15.0.  
B.M. No. 6 Elev. 988.57 on N.E. corner lower concrete step 40' Lt. @ Main St. Sta. 24+66.0

**RETAINING WALLS**  
STATE ROAD-NORTH MAIN STREET IN JOPLIN  
ABOUT 10 MILE SOUTH OF JOPLIN  
PROJECT NO. RT-43-SEC. U7-A STA. 25+48 TO 25+58.5  
JASPER COUNTY

DESIGNED BY: J. A. Willingham DATE: 4/19/1954  
APPROVED BY: R. M. Littleton DATE: 4/19/1954

L-651  
L-652

FINAL PLANS

Drawn March 1954 by T.O.S.  
Checked Apr. 1954 by J.E.L.

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 1



320

MISSOURI PROJECT NO. 7 JASPER 15' 4" FINAL PLANS  
 CL 3 Exc. On 15" R.C.P. 16.722  
 1/2 (57 + 20) X 10.5 + 10 X 1.5 = 16.722  
 Use 3.5 Cu Yd

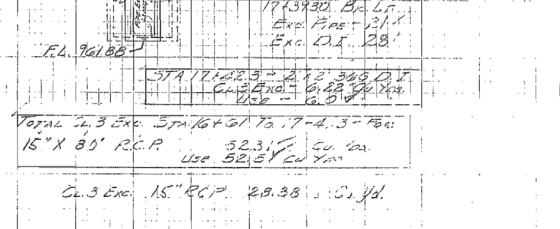
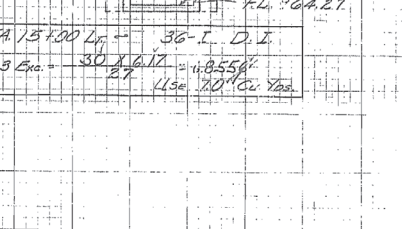
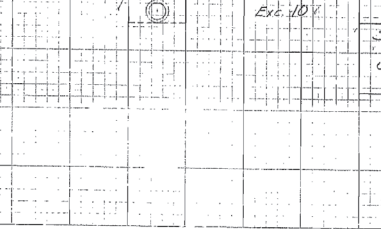
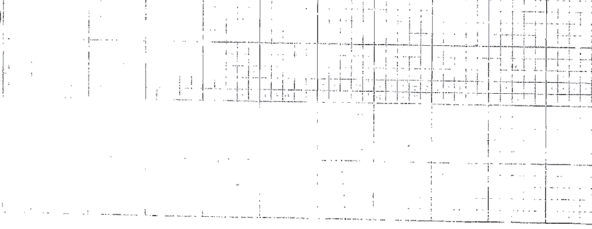
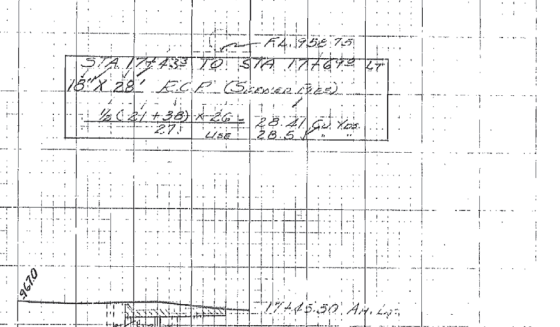
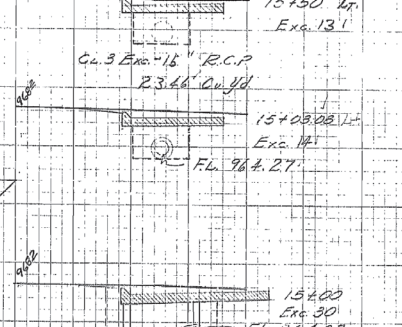
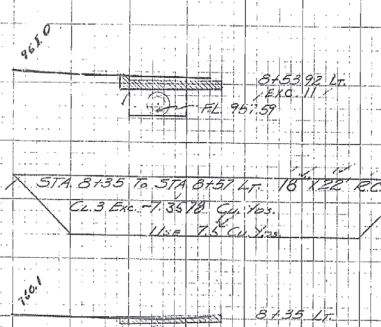
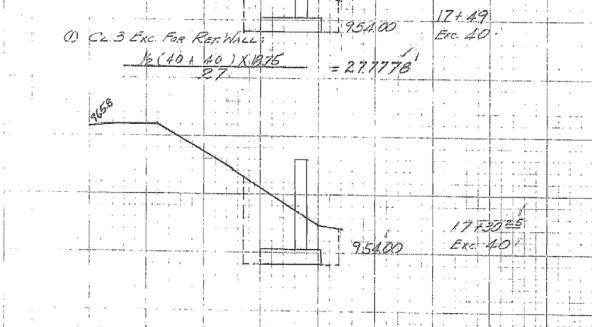
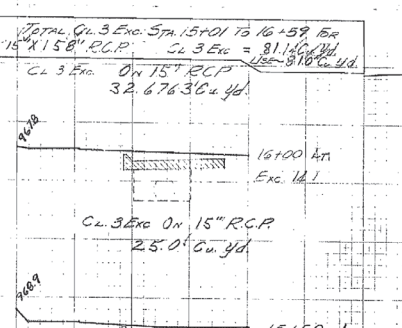
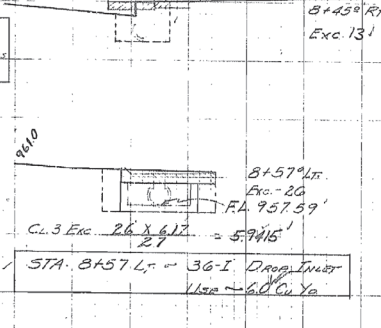
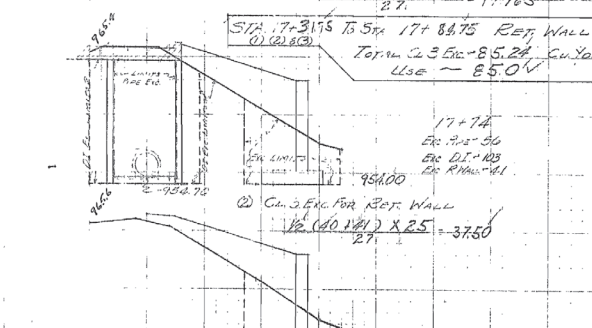
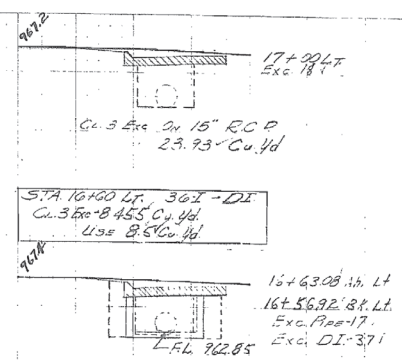
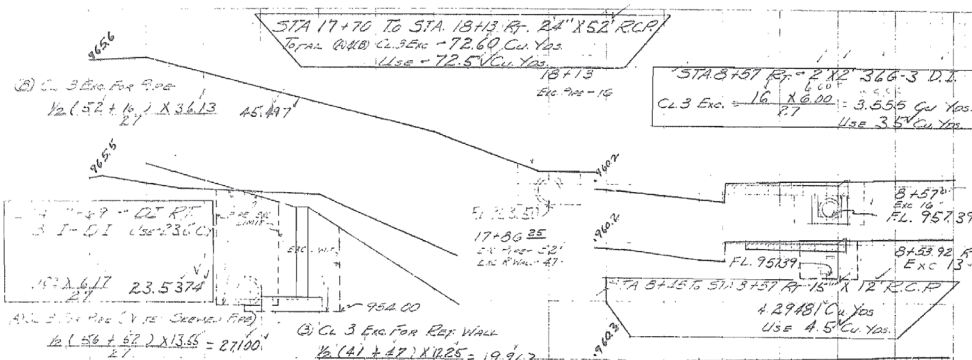
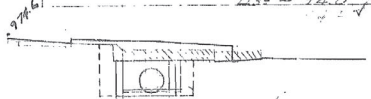


PLATE 3 CROSS SECTION H 11 11 11 11  
 DATE: 10/10/11  
 CHECKED: [Signature]  
 DRAWN: [Signature]



10

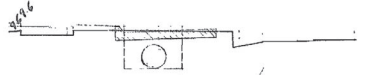
STA. 18+06.8 TO 19+99.3 L  
 15" X 19" R.C.P.  
 (1) 10' TOTAL C.C. 3 Exc. - 94.14 Cu Yds  
 Use - 94.0 "



20+00.3  
 Exc. DI - 27  
 Exc. Pipe - 13

C.C. 3 Pipe - 1/2 (13.13) X 97.8 = 46.85 CY

C.C. 3 Exc. DI - 27 X 200 = 6.00  
 Syn. 20+00.3 - 358.2 C.I.  
 Total C.C. 3 Exc. = 12.00 CY  
 Use 16.0 "

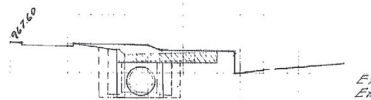


17+100  
 Exc. 13

C.C. 3 Pipe - 1/2 (15.13) X 91.20 = 47.59 Cu Yds



16+08.80  
 Exc. 15

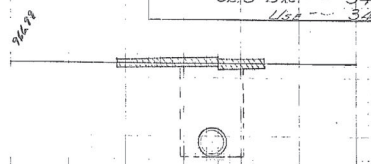


18+05.8  
 Exc. DI - 24  
 Exc. Pipe - 16

STA 18+05.8 36 B-1" DI  
 2' X 6.00 = 5.33  
 Use 5.5 Cu Yds C.C. 3

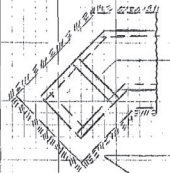
C.C. 3 Pipe - 1/2 (16.0 + 16) X 33 = 34.22 Cu Yds

STA 17+11.76 18" X 35" R.C.P.  
 C.C. 3 Exc. - 34.2 Cu Yds  
 Use - 34.0 "



17+69  
 Exc. 40

STA. 22+13.0 4' X 2.5' - 4' C.C. 3 Extension  
 5' total C-5.30 1.5:1 Wing Wall Slopes  
 4.5' skew



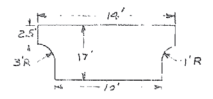
10.72  
 10.72 X 72 = 772.17  
 772.17 X 5.08 X 1.38 = 5400.00  
 5400.00 / 27 = 200.00  
 Use 3.5 "

321



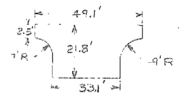
FINAL PLANS

11



STA 12+10.1 To 10+27.9 LT  
P.C.C. PAV. ENT  
 $\frac{14 \times 2.5 + 10 \times 1.5 + 17 \times 1.5}{9} = 20.2984$  Sq Yds  
USE 20 Sq Yds

STA 9+50.3 To 12+93.9 RT  
CURB & GUTTER 943.6 LN. FT.



STA 12+23.1 LT P.C.C. PAV.  
ST. APPR.  
 $\frac{(6 \times 20) + (2.2 \times 29.2) + (11.2 \times 11.2) + (10.7 \times 11.2)}{9} = 88.1020$  USE 88.0 Sq Yds

STA 12+06.5 RT CONC. CURB  
Misc 1-Z

Sta. 8+57.1 Lt. DI. & 15'x10' VC.P.  
Misc 1-Z

Sta 8+41.4 To 8+84.4 LT  
P.C.C. Sidewalk  
IX 18.1 = 14.78 Sq Yd.  
Use 14 Sq Yd

Sta 8+40.9 To 10+10.1 LT  
GUTTER & CURB 192.5 LN. FT.

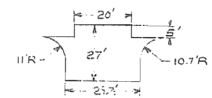
Sta. 8+23.5 - 8 ASD Lt. P.C.C. Remnant  
 $\frac{6.5 \times 8}{9} = 5.777$  Sq Yds  
Use 6 Sq Yds

Sta 8+40 To 9+00 RT CURB  
& GUTTER 75.4 LN. FT.

Sta 3+11 Lt. P.C.C. Remnant  
 $\frac{120 \times 21 + (7.5 \times 1.5) + (26.5 \times 6.3)}{9} = 35.43$  Sq Yds  
Use 35.5 Sq Yds

Sta. 12+39.7 To 17+61.7 LT  
CURB & GUTTER 562.5 LN. FT.

Sta. 12+33.5 To 12+41.5 Lt. P.C.C. Rem.  
 $\frac{5 \times 8}{9} = 4.444$  Sq Yds  
Use 4.5 Sq Yds  
Sta 12+30 RT. CONC. SLAB  
 $\frac{15 \times 9}{9} = 15.2$  Sq. Yds  
Use 15 Sq. Yds.



Sta. 17+69 Rt. BRICK M.H.  
& 18" R.C.P.  
Misc. 1-Z

Sta 12+06.5 RT CONC. CURB  
Misc 1-Z

Sta 11+98.6 To 12+06.3 LT  
P.C.C. SIDEWALK  
 $\frac{(4.5 \times 7.5) + (4.5 \times 13.7)}{9} = 10.40$   
Use 11 Sq Yds

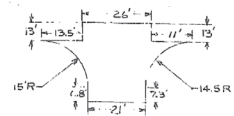
Sta. 11+77.5 To 12+12.5 Lt. P.C.C. Rem.  
 $\frac{5 \times 1.5}{9} = 0.833$  Sq Yds  
Use 0.833 Sq Yds

Sta 11+87 RT. CONC. SIGN  
FDN.  
Misc 1-Z

Sta 11+54.4 RT. CONC. CURB  
Misc 1-Z

Sta 10+95 RT. CONC. WALL  
Misc. 1-Z

Sta 10+27 To 12+06.3 LT  
CURB & GUTTER 200.5 LN. FT.



Sta. 17+74.1 LT. P.C.C. PAV.  
ST. APPR.  
 $\frac{(13 \times 2.6) + (21 \times 21.5) + (14.5 \times 14.5) + (15 \times 11.5)}{9} = 98.8005$   
USE 99 Sq. Yds.

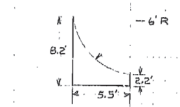
Sta. 17+69 Rt. BRICK M.H.  
& 18" R.C.P.  
Misc. 1-Z

Sta. 17+69 To 18+13 RT.  
24" R.C.P.  
Misc 1-Z

Sta. 17+50 To 17+61.2 LT.  
P.C.C. SIDEWALK  
 $\frac{(6.5 \times 4.5) + (9.8 \times 4.5)}{9} = 8.15$   
Use 8 Sq. Yds.

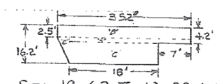
Sta 17+42.3 Lt. D.I. & 15'x8' VC.P.  
Misc 1-Z

Sta 17+39.6 Rt. D.I. & 2'x2' CONC. BOX  
Misc. 1-Z



Sta. 12+40.6 RT. P.C.C. PAV.  
RETURN  
 $\frac{2.3 \times 5.5 \times 4.6}{9} = 2.2028$   
USE 2 Sq Yds.

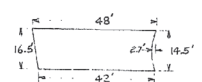
Sta. 12+40.2 To 12+47.6 LT  
P.C.C. SIDEWALK  
 $\frac{74 \times 10.8}{9} = 15.4578$  Sq Yds.  
USE 15 Sq Yds.



Sta. 19+63 To 19+98.6 LT.  
P.C.C. PAV. ENT.  
 $\frac{17 \times 2.6 + 2.5 \times 5.5 + 12 \times 11.5 + 12.2 \times 11.5}{9} = 46.263$   
USE 46.5 Sq. Yds.

Sta 19+00 RT. CONC. POST  
Misc. 1-Z

Sta 18+96 To 19+63 LT. CURB  
& GUTTER 67 LN. FT.



Sta. 18+49.3 To 18.95.3 LT.  
P.C.C. PAV. ENT.  
 $\frac{6 \times 16.5 + 15.5 \times 4.2 + 27 \times 14.5 + 20.6}{9} = 74.84$   
USE 75 Sq Yds

Sta. 18+40.5 To 20+43.5 LT  
P.C.C. SIDEWALK  
 $\frac{107.8 \times 4.5 + 50.1 \times 6 + 4.5 \times 1 \times 5.5}{9} = 114.86$   
USE 115 Sq. Yds.

Sta. 18+06.3 RT. D.I.  
Misc. 1-Z

Sta. 18+05.8 Lt. D.I. & 15'x10' VC.P.  
Misc. 1-Z

Sta. 17+86.9 To 17+77.8 LT  
P.C.C. SIDEWALK  
 $\frac{26.8 \times 4.5}{9} = 13.4$  Sq Yds.  
USE 13 Sq. Yds.

Sta 17+86.4 To 18+49.3 LT.  
CURB & GUTTER 80 LN. FT.

Sta 21+61 RT. CONC. POST  
Misc. 1-Z

Sta 21+53 Lt. 4" V.C.P. DRAIN  
Misc. 1-Z

Sta 21+47.7 Lt. 4" V.C.P. DRAIN  
Misc 1-Z

Sta 21+31 RT. CONC. POST  
Misc 1-Z

Sta 20+69 Lt. 3' P.C.C. SIDEWALK  
 $\frac{3 \times 4.75}{9} = 1.583$  USE 2 Sq. Yds.

Sta 20+37.4 Lt. 4" V.C.P. DRAIN  
Misc. 1-Z

Sta. 20+09 Lt. 4" V.C.P. DRAIN  
Misc. 1-Z

Sta. 20+42 RT. CONC. POST.  
Misc. 1-Z

Sta. 20+15.1 Lt. 2' P.C.C. SIDEWALK  
 $\frac{2 \times 13.7}{9} = 3.044$  Sq. Yds.  
USE 3 Sq. Yds.

Sta. 20+11 RT. CONC. POST  
Misc. 1-Z

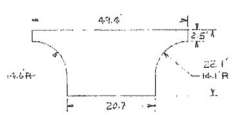
Sta. 19+98.4 To 23+11.4 LT. CURB  
& GUTTER 334.0 LN. FT.



12

STA. 23+36.6 To 28+20.5 Lt  
PCC SIDEWALK  
 $(6.2 \times 5.0 + 5.0 \times 13.0) + (23+48.7 - 23+36.6) \times 4.0 = (27 \times 5.0 + 27.0 - 23 + 62.7) \times 4.0$   
= 244.4 Sq Yds.  
Use 244 Sq. Yds.

STA. 23+36.1 To 26+54.9 Lt.  
CURB & GUTTER 340.0 LN. FT.



STA. 23+23.6 Lt PCC, PAV.  
ST. APPR.  
 $49.9 \times 2.5 + 20.7 \times 1.0 + 14.4 \times \frac{2.5^2 + 1.0^2}{2} = 170.0$   
= 68.6254 Sq. Yds  
Use 69 Sq. Yds.

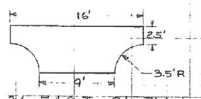
STA. 22+98.9 To 23+10.9 Lt.  
PCC SIDEWALK  
 $5.2 \times 5.0 + 9.0 \times 1.0 + 1.0 \times 13.0 = 13.46$   
Use 13 Sq. Yds.

STA. 27+10 Rt. CONC. POST  
Misc 1-Z

STA. 22+53.7 To 27+63.5 Rt.  
CURB & GUTTER 509.8 LN. FT.

STA. 26+70.7 To 28+20.5 Lt.  
CURB & GUTTER 153.1 LN. FT.

STA 22+51 Rt. CONC. POST  
Misc 1-Z



STA. 22+50 Rt. 4'x26' CONC.  
R.R. CULV.  
Misc. 1-Z

STA. 26+54.9 To 26+71.1 Lt. PCC, PAV. ENT.  
 $16 \times 2.5 + 9 \times 3.5 + \frac{(6.5^2 - 3.5^2)}{2} = 8.5286$   
Use 8.5 Sq. Yds.

STA. 22+21.6 Lt. PCC SIDEWALK  
 $4.5 \times 4.5 = 2.25$  Sq. Yds.  
Use 2 Sq. Yds.

STA 26+16 Rt. CONC. POST  
Misc 1-Z

STA. 21+91 Rt. CONC. POST  
Misc. 1-Z

STA 24+94 Rt. CONC. POST  
Misc. 1-Z

303

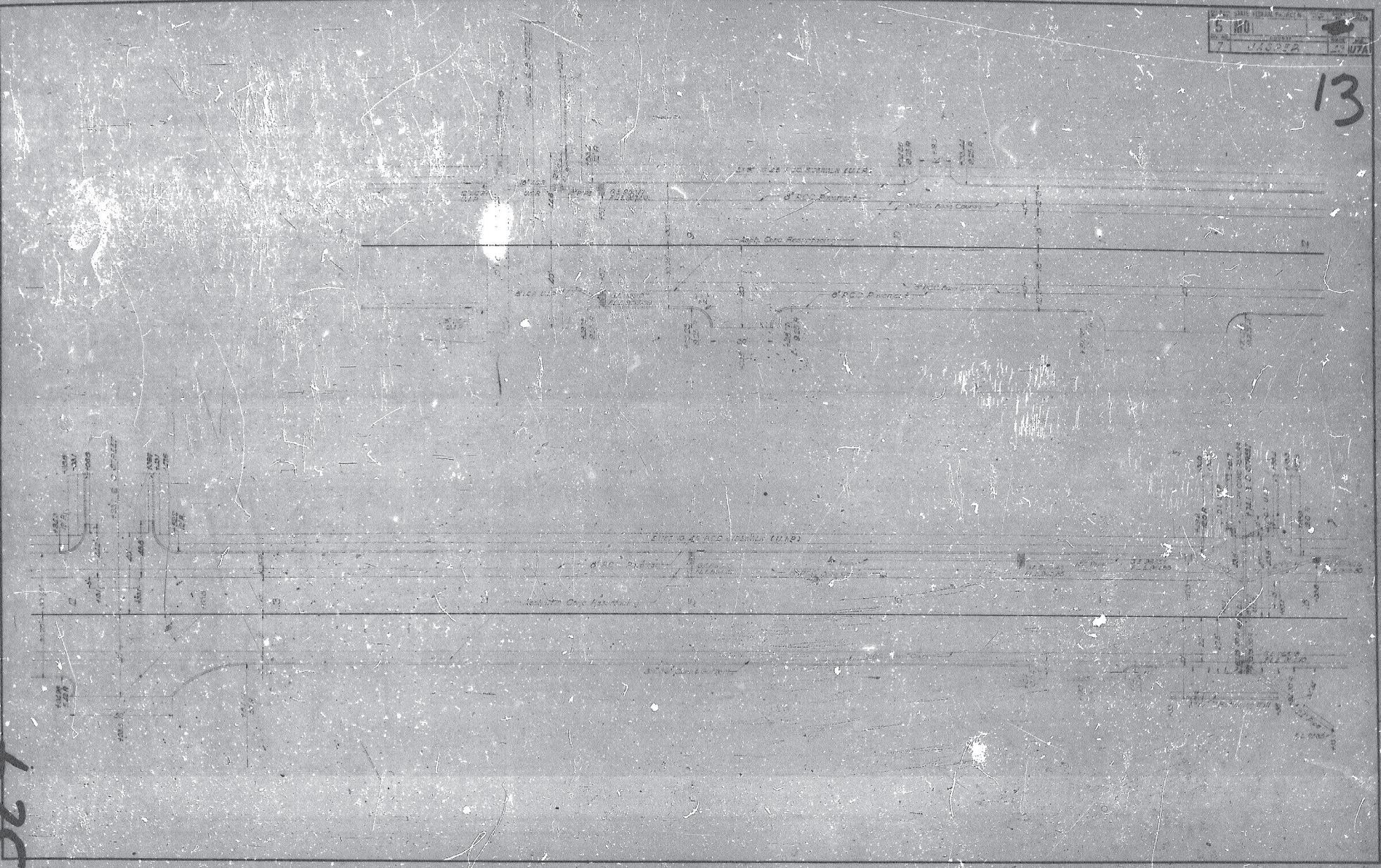


13

NO.	DATE	REVISION

NO.	DATE	REVISION

324



NO.	DATE	REVISION

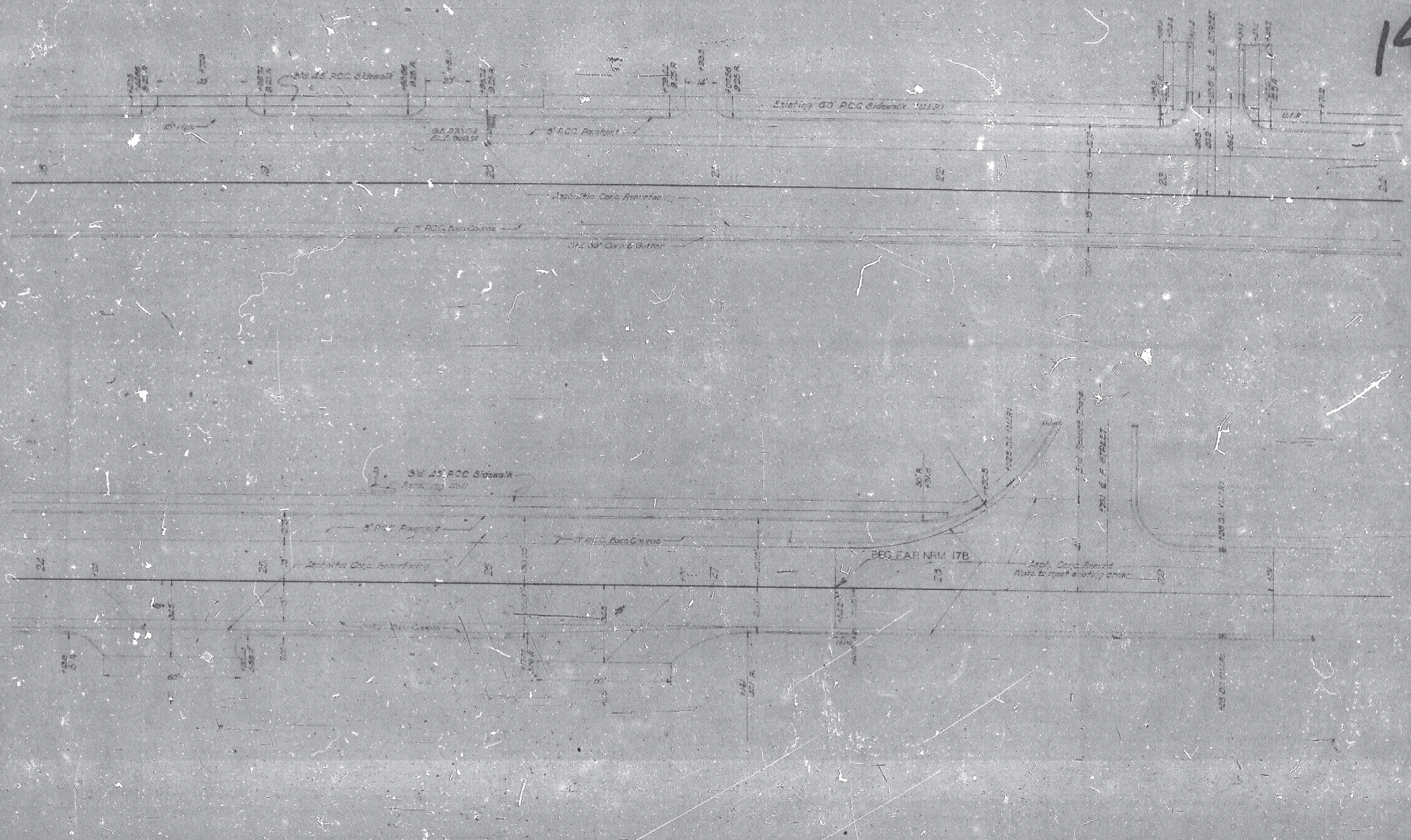


14

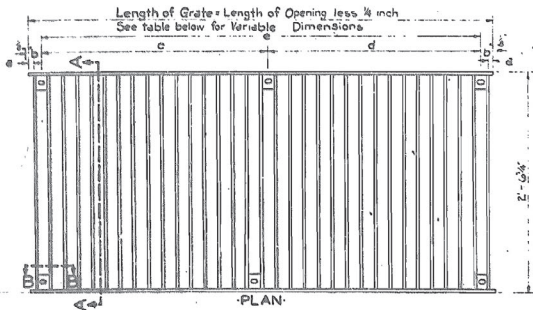
DATE	NO.	BY

DATE	NO.	BY

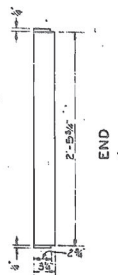
325



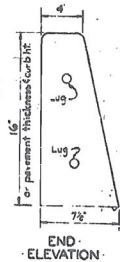
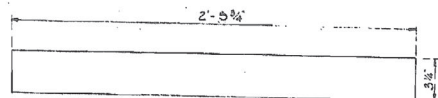




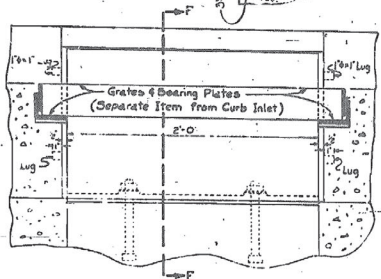
GRATE



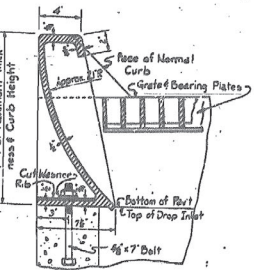
BEARING BAR



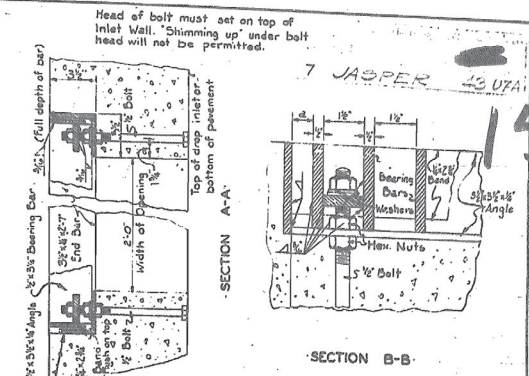
END ELEVATION



FRONT ELEVATION

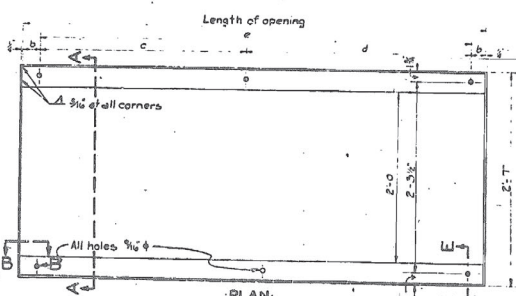


SECTION F-F

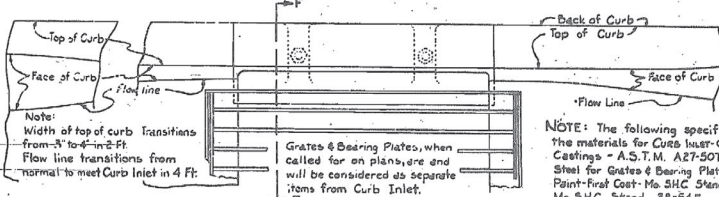


SECTION A-A

SECTION B-B



BEARING PLATE



PLAN CURB INLET

Size	Weight	Type	Bars		Variable Dimensions						
			No.	Width of End Bars	a	b	c	d	e		
2'-0" x 2'-0"	205	Fabricated	11	1/2"	10	1 1/4"	1 3/4"	3"	---	---	1'-0"
2'-0" x 3'-0"	305	Fabricated	17	1/2"	16	1 3/4"	1 3/4"	3"	---	---	2'-6"
2'-0" x 4'-0"	405	Fabricated	23	1/2"	22	1 3/4"	1 3/4"	3"	1'-0"	1'-0"	---
2'-0" x 5'-0"	505	Fabricated	29	1/2"	28	1 3/4"	1 3/4"	3"	2'-2"	2'-4"	---
2'-0" x 5'-6"	555	Fabricated	32	1/2"	31	1 3/4"	1 3/4"	3"	2'-6"	2'-6"	---

\* Weight shown on plans are the estimated weights of Fabricated type Grates and Bearing Plates. Payment will be made on the actual weight of Grates & Bearing Plates used on the Project.

NOTE: The following specifications shall govern the materials for CURB INLET, GRATES & BEARING PLATES Castings - A.S.T.M. A27-50T, G.P. 65-35, Structural Steel for Grates & Bearing Plates A.S.T.M. A7-50T Paint-Finish Coat: Mo. S.I.C. Stand 36-34C, Second Coat Mo. S.I.C. Stand 36-34F.

All Bearing Plates and all fabricated Grates shall have 2 coats of paint applied in the shop or field by any method that will insure a complete cover for each coat. All bruises or damage to paint shall be retouched before item is installed. When bolts are cut in the field, threads must be cleaned to permit the final nut to run freely on the bolt. This Drawing is not to scale Follow Dimensions.

MISSOURI STATE HIGHWAY COMMISSION

CURB INLET, GRATES, AND BEARING PLATES

SUBMITTED BY *R.P. ...* APPROVED BY *R.M. ...*  
 ENGR. OF SURVEYS & PLANS CHIEF ENGINEER

DRAWN 12-52 REVISION 10-55

36A-SPECIAL

925

15

7 JASPER 10 UTA



TYP SEC & Earthwork

Standards

RTE 43  
Section 17 U7A  
County Jasper

Sheet # 16

Surface - curb & Gutter  
Approaches  
25D-13  
25BB-7

DRAINAGE

36B4

36G-3

~~36I-10~~

C530

C110R

CON-REINF APPURTS.

FINISH ETC

15C-1

C110R3

Bridges

MISC.

26A-10