

# General Notes:

#### Design Specifications:

2002 AASHTO LFD (17th Ed.) Standard Specifications (Section 5, ASD Design) Seismic Performance Category B Acceleration Coefficient = 0.12

Design Loading:

 $\phi_b$  = 25° and Unit weight,  $\chi_b$  = 130 pcf for retained backfill material to be retained by the mechanically stabilized earth wall system.

 $\phi_{ extstyle au}$  = 25° for unimproved foundation ground where wall is to bear.  $\Phi_{\mathcal{F}} = 34^{\circ}$  for improved foundation ground where wall is to bear.

The allowable bearing pressure for unimproved foundation ground 3.0 ksf. The allowable bearing pressure for improved foundation ground 4.3 ksf.

The maximum applied bearing pressure for the controlling design case at the foundation level shall be shown on the shop drawings and shall be less than the allowable bearing pressure for foundation ground and provided herein. For seismic design the maximum applied bearing pressure shall be less than two times the allowable bearing pressure.

Allowable bearing pressure and limits of improved foundation ground shall not be adjusted from that as shown on the plans.

Contractor shall include  $\phi$  (actual  $\phi \geq 34^{\circ}$ ) and the total unit weight  $\chi$ , for the select granular backfill (reinforced backfill and wedge area backfill) for structural systems on shop drawings. Contractor shall identify source of select granular backfill material, submit proctor in accordance with AASHTO T 99 (ASTM D698) and gradation with the shop drawings. When backfill material is too coarse to develop a proctor curve the contractor shall determine the maximum dry density (relative density) in accordance with ASTM D4253 and ASTM D4254 and assume percent passing the 200 sieve for optimum water content.

Total unit weight,  $\mathcal{Y}_{-}=$  (95% compaction) x (maximum dry density) x (1 + optimum water content)

Design  $\Phi_r = 34^\circ$  for the select granular backfill (reinforced backfill) only for structural systems.

Factor of safety shall be 2.0 for overturning and 1.5 for sliding.

For seismic design the factor of safety shall be 1.5 for overturning and 1.1 for sliding.

Use default values for the pullout friction factor, F\*, in accordance with AASHTO figure 5.8.5.2A. unless MoDOT has pre-approved a higher value for a specific product. For approved steel strips not shown in AASHTO figure, 5.8.5.2A, use F\*  $\leq$  2.0 at zero depth and F\*  $\leq$  Tan  $\phi_r$  at 20 feet depth and  $\phi_r$  design = 34°. F\* values shall be shown on the shop drawings.

### Design Unit Stresses:

All concrete for leveling pad and coping shall be Class B or B-1 with f'c = 4000 psi.

The minimum compressive strength of concrete for precast panel shall be 4,000 psi in accordance with Sec 1052.

#### Miscellaneous:

The MSE wall system shall be built vertical.

The MSE wall system shall be built in accordance with Sec 720.

The MSE wall system shall be a large block wall system.

Panel and coping (or capstone) reinforcement shall be epoxy coated.

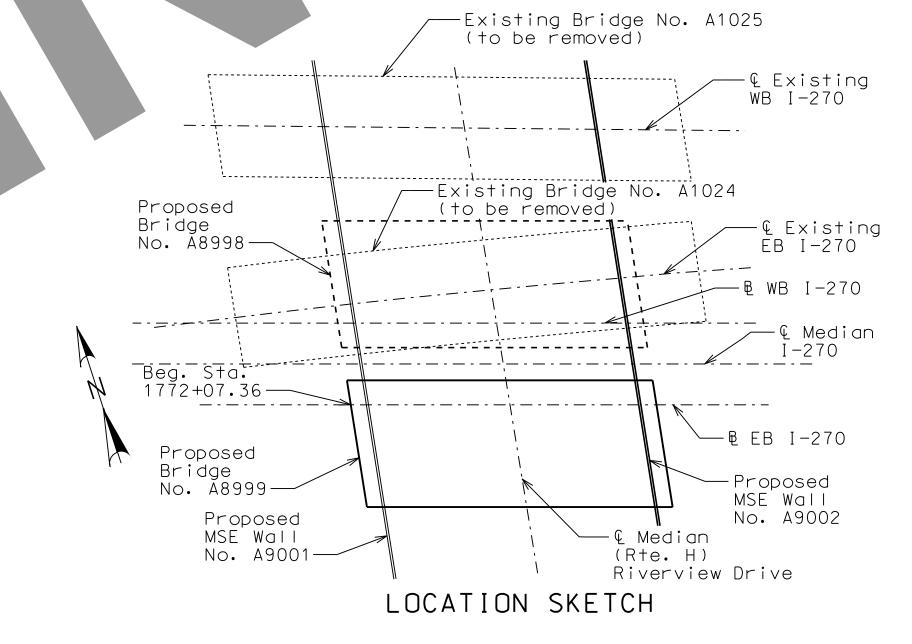
281' MECHANICALLY STABILIZED EARTH (MSE) RETAINING WALL SYSTEM

A filter cloth meeting the requirements for a Separation Geotextile material shall be placed between the select granular backfill for structural systems and the backfill being retained by the mechanically stabilized earth wall system.

Coping shall be required on this structure. When CIP coping sections extend beyond the limits of a single panel, bond breaker (roofing felt or other approved alternate) between wall panel and coping is required. Coping joints shall use 3/4-inch chamfers and shall be sealed with 3/4-inch joint filler. Coping reinforcement shall terminate 1 1/2-inch minimum from face of coping joint.

The top and bottom elevations are given for a vertical wall. If a battered small block wall system is used, the height of the wall shall be adjusted as necessary to fit the ground slope and the concrete leveling pad shall be adjusted as necessary to account for the wall batter. If a fence is built on an extended gutter, then the height of the wall shall be adjusted further.

The contractor shall be solely responsible to coordinate construction of the wall with bridge and roadway construction and ensure that the bridge and roadway construction, resulting or existing obstructions, shall not impact the construction or performance of the wall. Soil reinforcement shall be designed and placed to avoid damage by pile driving, guardrail post installation, utility and sign foundations. (See Roadway and Bridge plans.)



Estimated Quantitie	es .	
I tem		Total
Aesthetic Concrete Stain	LS	1
Mechanically Stabilized Earth Wall Systems	sq. foot	4819
Temporary MSE Wall	LS	1

Designed:JEK Detailed:JEK Checked: TPL

MSE Wall Systems Data Table										
Proprietary Wall Sy	ystems	Combi	nation	Wall Systems						
Manufacturer	System	Facing Unit Manufacturer								

MSE Wall Systems Data Table is to be completed by MoDOT construction personnel to record the manufacturer of the proprietary wall system or the manufacturers of the combination wall system that was used for constructing the MSE wall.

RETAINING WALL UNDER END BENT NO. 1 AT BRIDGE A8998 AND BRIDGE A8999

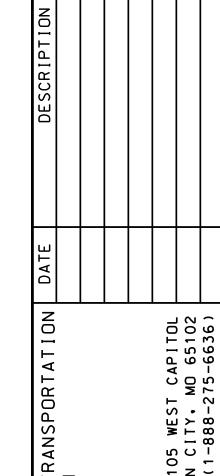
ROUTE EB I-270 FROM LILAC AVENUE TO MISSOURI STATE LINE ABOUT 1.0 MILES EAST OF LILAC AVENUE STATION 12+24.28 ALONG & SB ROUTE H

SAURABH SHYAM MITTAL NUMBER PE-2019010896 ONAL W

DATE P	REPARED								
3/16/2022									
ROUTE	STATE								
I-270	MO								
DISTRICT	SHEET NO.								
BR 2									
COU	NTY								

ST. LOUIS CIT	Υ
JOB NO.	_
J6I3020C	
CONTRACT ID.	

		PRO	)JE	СТ	NO.		_						
BRIDGE NO. A9001													





### General Notes Cont.:

Minimum 18" wide Geotextile strips shall be centered at vertical and horizontal joints of panel. Geotextile material shall be adhered to back face of panel using an adhesive compound supplied by the manufacturer. All edges of each fabric strip shall provide a positive seal. A minimum 18 overlap shall be provided between spliced filter fabric.

Aluminized soil reinforcement shall have edges coated with coating material per manufacturer.

Soil reinforcement shall be spaced to avoid roadway drop inlet behind

Upper two layers of soil reinforcement shall be extended 3 feet beyond the lower layers

The splay angle should be less than 15° and tensile capacity of splayed reinforcement shall be reduced by the cosine of the splay angle. Soil reinforcement shall clear the obstruction by at least 3 inches.

No reinforcement shall be left unconnected to the wall face or arbitrarily cut/bent in the field to avoid the obstruction.

Where interference between the vertical obstruction and the soil reinforcement is unavoidable, the design of the wall near the obstruction may be modified using one of the alternatives in FHWA-NHI-10-24, Section 5.4.2. Show detail layout on the drawings. For wall designs with horizontal obstructions in reinforced soil mass, see FHWA-NHI-10-024, Section 5.4.3.

Excavation quantities and pay items are given on the roadway plans. Excavation quantities are based on a soil reinforcement length of 0.75 x Height of the Wall in ft. The soil reinforcement length may vary based upon the wall design selected by the contractor. Plan excavation quantities will be paid regardless of any actual quantities removed based on the soil reinforcement length and design selected.

TYPICAL SECTION THRU

LARGE BLOCK WALL SHOWING FILTER CLOTH

Contractor to overexcavate 5 feet below bottom of wall where wall exceeds 10 feet height.

- 1) Inverted U-shape reinforced capstone may be used in lieu of coping. Panel dowels for level-up concrete shall be required, and provided by manufacturer. The dowels shall be field trimmed to clear the capstone by a minimum of 1 1/2 inches and a maximum of 2 1/2 inches.
- (2) Topmost layer of reinforcement shall be fully covered with select granular backfill for structural systems, as approved by the wall manufacturer, before placement of the Separation Geotextile.

Material Properties Used in Design									
	II/Select Backfill			Foundation					
φ°	γ(pcf)	φ°	γ (pcf)	φ°					

Note: MSE Wall designer shall include table on shop drawings and provide values used in the design computations. Effects of cohesion shall be ignored unless approved by the engineer.

DETAILS FOR MSE WALL

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 3 of 10

DATE PREPARED 3/16/2022 ROUTE STATE I - 270DISTRICT

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SAURABH SHYAM MITTAL

NUMBER PE-2019010896

COUNTY ST. LOUIS CITY JOB NO. J6I3020C CONTRACT ID.

PROJECT NO.

BRIDGE NO. A9001

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(3) Minimum 6" diameter perforated PVC or PE pipe.

Manufacturer shall show drain details on design plans to be submitted as shown on MoDOT MSE wall plans and/or roadway plans.

Contractor shall modify the drain details as shown if it will improve flow as may be the case for stepped leveling pad, and for an uneven ground line (approval of the engineer required).

- (4) Select granular backfill shall extend a minimum of 12" beyond the end of  $\leq$ all soil reinforcement. Where the angle, 0, between the retained backfill excavation/fill line and the horizontal is less than 90°, the wedge area backfill between 0 and 90° shall be filled with select granular backfill for structural systems meeting the requirements of Section 1010.
  - For  $(45^{\circ} + \phi_{6}/3)$  <  $\theta \leq 90^{\circ}$ , properties for retained backfill shall be used for active force computations.
  - For  $\theta \leq (45^{\circ} + \phi_{0}/3)$ , contractor shall have the option to use select granular backfill,  $\phi_{r}$  , or better aggregate material,  $\phi_{w}$  for active force computations in the wedge area backfill. For active force computations, the angle of internal friction for wedge area backfill material,  $\phi_r$  or  $\phi_w$  , shall be limited to 34° unless determined otherwise in accordance with Section 1010. If  $\phi_{\mu}$  or  $\phi_{\mu\nu} > 34^{\circ}$  is desired for wedge area backfill then test report shall be submitted with shop drawings.  $\phi_r$  or  $\phi_w$  shall not be greater than 40° for computations. Final configuration of this option shall be sent to Geotechnical Section for a new overall global stability analysis. Design  $\phi_{w}$  shall be shown on the shop drawings if used.

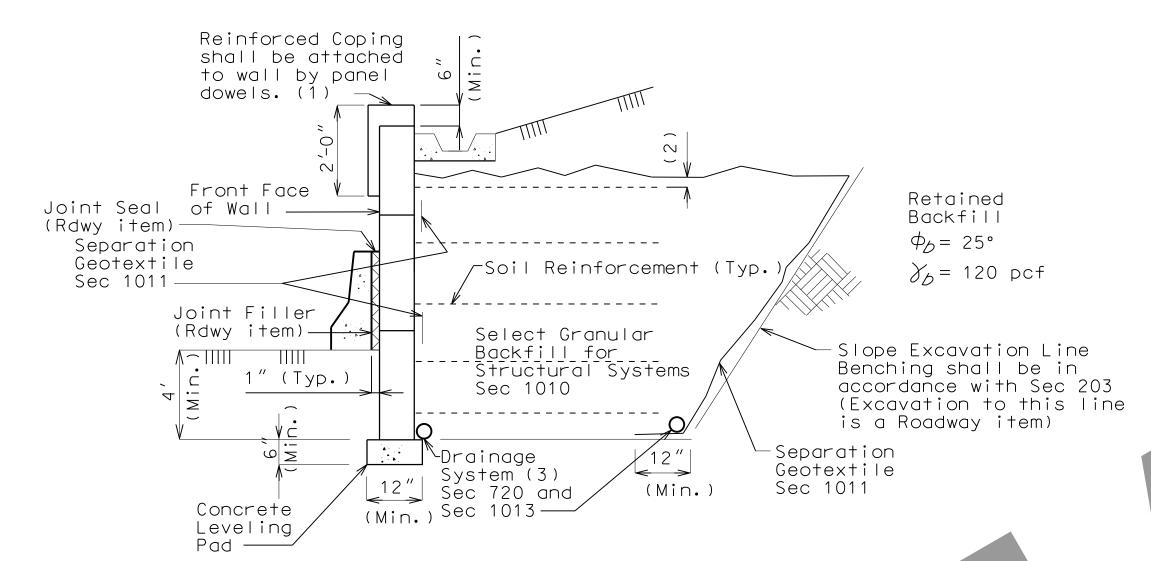
The slope excavation line shall be benched and separation geotextile shall be placed between the retained backfill and either select granular backfill or better aggregate material, and between the select granular backfill and better aggregate material.

Show range of acceptable theta (0) angle on shop drawings which must be consistent with design computations and proposed construction of wall. Show active force computation properties on shop drawings and in design computations. Coordination between wall designer (manufacturer) and contractor is required before shop drawing submittal.

Designed: JEK Detailed: JEK Checked: TPL

**7**-----

 $4' - 6\frac{3}{4}''$ 



# TYPICAL SECTION THRU LARGE BLOCK WALL

Note: For additional information, see "TYPICAL SECTION THRU LARGE BLOCK WALL SHOWING FILTER CLOTH".

#### Underpass Luminaire Mounted to Pier or Abutment Wall 11 11 -----H----Reinforced Coping shall be attached to wall by panel dowels. (1) — -Separation Geotextile 4" (Min.) at all panel joints Sec 1011 Front Face ----of MSE Wall ---Backfi └─Soil Reinforcement (Typ) $\gamma_p = 130 \text{ pcf}$ -Separation Geotextile at all panel joints Slope Excavation Line Sec 1011 Select Granular Benching shall be in - - - - - - - - - - - - - - +I- - H - - - - . accordance with Sec 203 Backfill for Structural Systems (Excavation to this line Sec 1010 ~>> is a Roadway item) Separation Geotextil -----Seotextile at all panel joints Sec 1011 Drainage System <del>\*\*\*</del> Sec 720 and Sec 1013 Pipe Pile Spacers (Included in work for Bridge Nos. A8998 & A8999)

TYPICAL SECTION THRU LARGE BLOCK WALL UNDER BRIDGE

Note: This drawing is not to scale. Follow dimensions.

Note: For additional information, see "TYPICAL SECTION THRU LARGE BLOCK WALL SHOWING FILTER CLOTH".

Designed:JEK Detailed:JEK Checked: TPL DETAILS FOR MSE WALL

Sheet No. 4 of 10

#### Note:

Holes shall be 5/8" round and extended 4" into the third layer of blocks, recessed 2" deep by 1 1/2" round.

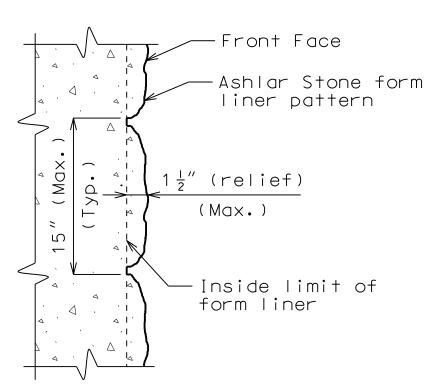
Rods or reinforcing bars shall be secured by an approved resin anchor system in accordance with Sec 1039.

Recess hole shall be backfilled with non-shrink cement grout.

- (1) Inverted U-shape reinforced capstone may be used in lieu of coping. Panel dowels for level-up concrete shall be required, and provided by manufacturer. The dowels shall be field trimmed to clear the capstone by a minimum of 1 1/2 inches and a maximum of 2 1/2 inches.
- (2) Topmost layer of reinforcement shall be fully covered with select granular backfill for structural systems, as approved by the wall manufacturer, before placement of the Separation Geotextile.
- (3) Minimum 6" diameter perforated PVC or PE pipe.

Manufacturer shall show drain details on design plans to be submitted as shown on MoDOT MSE wall plans and/or roadway plans.

Contractor shall modify the drain details as shown if it will improve flow as may be the case for stepped leveling pad, and for an uneven ground line (approval of the engineer required).



FORM LINER DETAIL (Large Block Wall)

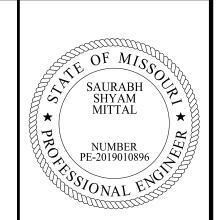
## Notes:

The cost of form liners for MSE wall systems, complete in place, will be considered completely covered by the contract unit price for Mechanically Stabilized Earth Wall System.

Form liner shall be constructed in accordance with Special Provisions.

The following is a list of form liner manufacturers and types which may be used. Depth of relief for all form liner pattern's shall vary up to 1 1/2". The height of any single 'stone' shall be 15" maximum.

- Scott System, Inc.: Form liner pattern #167 "Ashlar Stone"
- Fitzgerald Formliners: Form liner pattern #16986 "Ashlar Stone"
- Greenstreak: Form liner pattern #330 "Ashlar Stone"
- Spec Formliners: Form liner pattern #1515 "Ashlar Stone"
- Customrock: Form liner pattern #12020 "Tollway Ashlar"
- An approved equal



DATE PREPARED 3/16/2022

ROUTE STATE

I - 270 MO

DISTRICT SHEET NO.

BR 4

ST. LOUIS CITY
JOB NO.
J6I3020C

PROJECT NO.

CONTRACT ID.

BRIDGE NO.
A9001

COMMISSION

105 WEST CAPITOL

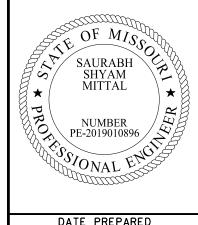
COMMISSIO

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CIPLINE: PROFESSIONAL ENGINEERING

I-270 AND
RIVERVIEW
-270/RIVERVIEW
MSE WALL A9001
SHEET 4 OF 10

PAOFESSIO	T Ho	rner & Shifrin, Inc.	_ PROJEC	T NAME	Inters	state 270 a	t Rive	rview [	Orive I	mprov	ement	s	
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		TED 3/23/20 COMPLETED 3/23/20				438.8 ft		HOLE	SIZE	<u>inch</u>	nes		
		ONTRACTOR Bulldog, CME75LC, Efficiency: 94%											
		ETHOD Hollow Stem Auger				_ING _ 8.00							
		B.Fisher CHECKED BY J. Schaeffer				<b>ING</b> n	ot mea	<u>asured</u>					
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3	$\bowtie$	·											
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$\dashv$	$\bowtie$	- 3.0" of wood at 3.5 ft		1 SS	50	8-6-6	2.5		11				
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	$\bowtie$												
	$\bowtie$												
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_}	$\bowtie$	- 3.0" sand seam at 8.5 ft		2		4-6-8	0.5	1	04				
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		- undrained shear strength at 13.0 ft = 1.71 TSF		ST	83		3.5	106	21				
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-													
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		Brown to grey, riighly weathered								-			
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		Bottom of borehole at 21.5 leet.											



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COUNTY
ST. LOUIS CITY
JOB NO.
J6 I 3020C
CONTRACT ID.

PROJECT NO.

BRIDGE NO.

HIGHWAYS AND TRAN COMMISSION

I-270 AND RIVERVIEW I-270/RIVERVIEW MSE WALL A9001 SHEET 5 OF 10

CLIENT Homer & Shiffin, Inc.  PROJECT NUMBER MG20007  PROJECT NUMBER MG20007  DRILLING CONTRACTOR Buildoo, CMESSIX Efficiency 59%  DRILLING CONTRACTOR Buildoo, CMESSIX Efficiency 59%  DRILLING CONTRACTOR Buildoo, CMESSIX Efficiency 59%  CHECKED BY J. Schaelfor AT RIVE OF DRILLING — not encountered	MIL	LENNIZ BOONAL SEPANIC	Millennia Professional Services				В	SOR	ING	S NU	JME		NE 1 0	
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NOTES   3.0 ft west of design due to location of marked utilities   AFTER DRILLING   ATTERBERG   LIMITS   LIM		GED BY	Y B.Fisher CHECKED BY J. Schaeffer	АТ	END OF	DRILL	. <b>ING</b> n	ot mea	sured					
August   A	E NOTE	<b>S</b> 3.0	) ft west of design due to location of marked utilities	AF	TER DRI	LLING								
Concrete (10.0")  438.3  Crushed limestone (2.0")  Brown, lean CLAY (CL), trace sand  SS 72 4-5-6 2.0 22  - undrained shear strength at 8.0 ft = 1.40 TSF  ST 67 3.75 106 24  Bitue-grey, lean to fat CLAY (CL-CH)  SS 89 9 3-3-4 2.5 24  SHALE:  SHAL	DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION		MPLE TYPE NUMBER	COVERY % (RQD)	BLOW COUNTS N VALUE)	OCKET PEN. (tsf)	(Y UNIT WT. (pcf)	AOISTURE ONTENT (%)	l	_IMITS	S    ≻	ES CONTENT (%)
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- undrained shear strength at 8.0 ft = 1.40 TSF  ST 2 67 3.75 106 21  Blue-grey, lean to fat CLAY (CL-CH)  SS 89 3-3-4 2.5 24  SHALE: Blue-grey to brown, clayey, trace limestone fragments  20  LIMESTONE: Grey to blue-grey, slightly to moderately weathered, moderately hard to hard, banded to thin bedded, finely-crystalline  LIMESTONE: Grey to blue-grey, slightly to moderately weathered, moderately hard to hard, banded to thin bedded, finely-crystalline  SHALE:  ST 00 50/2"  SC 100 2 (83)	ANG -													
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- undrained shear strength at 8.0 ft = 1.40 TSF    10	≝ <b>≝</b>													
Blue-grey, lean to fat CLAY (CL-CH)  SS 89 3-3-4 2.5 24  SHALE: Blue-grey to brown, clayey, trace limestone fragments  LIMESTONE: Grey to blue-grey, slightly to moderately weathered, moderately hard to hard, banded to thin bedded, finely-crystalline  LIMESTONE: Grey to blue-grey, slightly to moderately weathered, moderately hard to hard, banded to thin bedded, finely-crystalline  SHALE:  SHALE:  Blue-grey, lean to fat CLAY (CL-CH)  SS 89 3-3-4 2.5 24  A16.1  SS 100 50/2"  RC 100 2 (83)  SHALE:  SH	χ Π		- undrained shear strength at 8.0 ft = 1.40 TSF		ST									
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Blue-grey, lean to fat CLAY (CL-CH)  SS 89 3-3-4 2.5 24  SHALE: Blue-grey to brown, clayey, trace limestone fragments  A16.1  LIMESTONE: Grey to blue-grey, slightly to moderately weathered, moderately hard to hard, banded to thin bedded, finely-crystalline  SHALE: SHALE:  SHALE	77-1													
Blue-grey, lean to fat CLAY (CL-CH)  SS 89 3-3-4 2.5 24  SHALE: Blue-grey to brown, clayey, trace limestone fragments  A16.1  LIMESTONE: Grey to blue-grey, slightly to moderately weathered, moderately hard to hard, banded to thin bedded, finely-crystalline  SHALE:  SHALE:  SHALE:  SHALE:  SHALE:  SHALE:  SHALE:  Blue-grey, lean to fat CLAY (CL-CH)  SS 89 3-3-3-4 2.5  24  A16.1  SS 83 2-4-10 2.0  If RC 100 2 (83)	2000			426.1										
SHALE: Blue-grey to brown, clayey, trace limestone fragments  LIMESTONE: Grey to blue-grey, slightly to moderately weathered moderately hard to hard, banded to thin bedded, finely-crystalline  SHALE:  SHALE:  SHALE:  SHALE:  SHALE:  SC 100 2 (83)  RC 100 2 (83)	- IMGZ		Blue-grey, lean to fat CLAY (CL-CH)											
SHALE: Blue-grey to brown, clayey, trace limestone fragments  SHALE: Blue-grey to brown, clayey, trace limestone fragments  SS 83 24-10 2.0 16  416.1  LIMESTONE: Grey to blue-grey, slightly to moderately weathered, moderately hard to hard, banded to thin bedded, finely-crystalline  SS 100 50/2*  SRC 100 2 (83)  SHALE: SHALE:  SHALE:	)  -  -				SS	89		2.5		24				
SHALE: Blue-grey to brown, clayey, trace limestone fragments  SS 4 83 2410 2.0 16  LIMESTONE: Grey to blue-grey, slightly to moderately weathered, moderately hard to hard, banded to thin bedded, finely-crystalline  SS 100 50/2"  RC 100 2 (83)  SHALE:  SHALE:  SHALE:  SHALE:  RC 100 2 (83)	15				3		(1)		-					
SHALE: Blue-grey to brown, clayey, trace limestone fragments  SS 83 24-10 2.0 16  LIMESTONE: Grey to blue-grey, slightly to moderately weathered, moderately hard to hard, banded to thin bedded, finely-crystalline  SS 100 50/2"  RC 100 2 (83)  SHALE:  SHALE:  RC 0 2														
SHALE: Blue-grey to brown, clayey, trace limestone fragments  SS 83 2-4-10 2.0 16  LIMESTONE: Grey to blue-grey, slightly to moderately weathered, moderately hard to hard, banded to thin bedded, finely-crystalline  SS 100 50/2"  SRC 100 2 (83)  SHALE:  SHALE:  SHALE:  RC 0 0  CHART TO BE 100 2 (83)	Ž Ž													
SHALE: Blue-grey to brown, clayey, trace limestone fragments  20  416.1  LIMESTONE: Grey to blue-grey, slightly to moderately weathered, moderately hard to hard, banded to thin bedded, finely-crystalline  SHALE:  SHALE:  SHALE:  RC 100 2 (83)  411.6  RC 0 3 (0)				421 1										
Blue-grey to brown, clayey, trace limestone fragments  A16.1  LIMESTONE: Grey to blue-grey, slightly to moderately weathered, moderately hard to hard, banded to thin bedded, finely-crystalline  SS 100 50/2"  SRC 100 2 (83)  RC 100 2 (83)  SHALE: SHALE: SHALE:  SRC 0 0 (0)														
20  416.1  LIMESTONE: Grey to blue-grey, slightly to moderately weathered, moderately hard to hard, banded to thin bedded, finely-crystalline  SHALE:  SHALE:  RC 0 2 (83)			Blue-grey to brown, clayey, trace limestone fragments		√ ss	83		2.0		16				
LIMESTONE: Grey to blue-grey, slightly to moderately weathered, moderately hard to hard, banded to thin bedded, finely-crystalline  SHALE:  SHALE:  SHALE:  RC 0	20				/\ 4		(14)		_					
LIMESTONE: Grey to blue-grey, slightly to moderately weathered, moderately hard to hard, banded to thin bedded, finely-crystalline  SHALE:  SHALE:  RC 0 2 (83)	<u>.</u>													
LIMESTONE: Grey to blue-grey, slightly to moderately weathered, moderately hard to hard, banded to thin bedded, finely-crystalline  SHALE:  SHALE:  Converted to the moderate ly and a learner with the second secon	AB.C													
LIMESTONE: Grey to blue-grey, slightly to moderately weathered, moderately hard to hard, banded to thin bedded, finely-crystalline  SS 100 50/2"  3 P1 RC (27) 1 RC 100 2 (83)  SHALE: SHALE: SHALE: SHALE:				440.4										
Grey to blue-grey, slightly to moderately weathered, moderately hard to hard, banded to thin bedded, finely-crystalline  SS 100 50/2"  5 91  RC (27)  1  RC 100  2 (83)  SHALE:  SHALE:  RC 0  3 (0)			LIMESTONE:	416.1										
25   RC   100   2   (83)   SHALE:   RC   0   2   (9)   (10			Grey to blue-grey, slightly to moderately weathered, moderately	erately	SS 5		50/2"	7		3				
1 \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	25		nard to hard, banded to thin bedded, linely-crystalline		RC									
SHALE:  SHALE:  RC   100   2   (83)	OLUN OLUN				$H^{\frac{1}{2}}$	, , ,								
SHALE:    Create fit to mand anotable hand, colored as   1.6	표 진 -													
SHALE:  RC 0 2 (0)	<u>-</u> -			411.6	2	(83)								
I			SHALE: Grey, soft to moderately hard, calcareous		RC 3	0 (0)								

(Continued Next Page)

	Nation	ENNIA	Millennia Profes	ssional Services					E	BOR	ING	NU		ER PAGE		
	CLIEN		er & Shifrin, Inc.  MBER MG20007						tate 270 a			Orive In	nprove	ements	S	
GPJ	DEPTH (ft)	GRAPHIC LOG		MATERIAL DES	SCRIPTION		SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (tsf)	DRY UNIT WT. (pcf)	MOISTURE CONTENT (%)		PLASTIC LIMIT	PLASTICITY B INDEX	FINES CONTENT (%)
EOTECH BH COLUMNS - GINT STD US LAB.GDT - 12/17/20 11:22 - G:\PROJECT FILES\2020\MG20007 I-270 RIVERVIEW DRIVE INTERCHANGE\FIELD DATA\1270 AT RIVERVIEW DRIVE INTERCHANGE GINT.	35			Refusal at Bottom of boreh	si ft 23.5 feet.	400.6	RC 4	100 (75)		1.213						



DATE PREPARED								
3/16/2022								
ROUTE	STATE							
I - 270	MO							
DISTRICT	SHEET NO.							
BR	6							
COU	NTY							

ST. LOUIS CITY

JOB NO.

J6 I 3 O 2 O C

CONTRACT ID.

PROJECT NO. BRIDGE NO.

HIGHWAYS AND TRAN COMMISSION

I-270/RIVERVIEW MSE WALL A9001 SHEET 6 OF 10 I-270 AND Riverview

Sheet No. 6 of 10

MILL	LENNI	Millennia Professional Services				В	OR	ING	NU	JME		NE 1 0	
PROFESE	IIONAL SERVIC	orner & Shifrin, Inc.	PROJEC	T NAME	Inters	state 270 a	t River	view C	Orive Ir	mprov	ement	s	
1		IUMBER MG20007				St. Louis, N			<u> </u>		<u> </u>		
1		TED 3/2/20 COMPLETED 3/2/20				438.4 ft			SIZE	inch	ies		
		CONTRACTOR Bulldog, CME550X, Efficiency: 95%											
1		IETHOD Hollow Stem Auger, NQ Rock Core				LING 8.00	) ft / El	lev 430	0.40 ft				
		B.Fisher CHECKED BY J. Schaeffer				 _ING no							
NOTE	<b>S</b> 5.0	) ft west of design due to location of marked utilities		TER DRI									
	O		(%)		ERG	ËNT							
O DEPTH	GRAPHIC LOG	MATERIAL DESCRIPTION		SAMPLE TYPE NUMBER	RECOVERY (RQD)	BLOW COUNTS (N VALUE	POCKET PEN. (tsf)	DRY UNIT WT (pcf)	MOISTURE CONTENT (%)	LIQUID	PLASTIC LIMIT	PLASTICITY INDEX	FINES CONTENT (%)
	P	Concrete (10.0")	437.57										
   		FILL: Brown lean CLAY and GRAVEL	101.01										
		POSSIBLE FILL: Tan, clayey SAND (SC)	434.57	√ ss	67	3-7-8	_		7				
5				1	07	(15)	_		'				
  -  -  -  -		$ar{\Sigma}$											
10		Light brown, sandy SILT (ML), trace clay	429.4	SS 2	67	4-4-4 (8)	1.5	,	19				
;  ; 		Grey, clayey SILT (ML)	427.4_										
  		- 3.0" rocky seam at 13.8 ft		SS 3	78	7-7-8 (15)	>4.5		21				
- - - -		Brown to grey, lean CLAY (CL)	421.4										
<del> </del>						\							
20				SS 4	100	2-3-5 (8)	2.0		23				
			414.7										
25		SHALE: Brown, grey, purple, and blue-grey, weathered, clayey		SS 5	89	11-12-15 (27)	_		15				
			410.4										

	MIL	LENNI	Millennia Professional Services				В	SOR	ING	NU	JME	BER PAGE	<b>NB</b>	
	CLIEN	NT H	orner & Shifrin, Inc.  IUMBER MG20007				state 270 a St. Louis, I			Orive I	mprov	ement	S	
I.GP.I	DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION		SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (tsf)	DRY UNIT WT. (pcf)	MOISTURE CONTENT (%)		PLASTIC PLASTIC LIMIT	EX EX	FINES CONTENT (%)
TECH BH COLUMNS - GINT STD US LAB GDT - 12/17/20 11:22 - G:\PRO:IECT FILES\200201 - 270 RIVERVIEW DRIVE INTERCHANGE\EIFLD DATA\270 AT RIVERVIEW DRIVE INTERCHANGE	35		SHALE: Bluegrey, soft, calcareous - vertical fracture from 28.6-28.8 ft - 6.0" limestone layer at 28.8 ft - 3.0" near vertical fracture at 30.5 ft  - rock core qu at 32.3 ft = 2,540 psi - core loss from 32.7-32.9 ft  - 5.5" weathered clayey shale seam at 34.5 ft  - very soft from 38.0-38.3 ft  Refusal at 28.0 feet. Bottom of borehole at 43.0 feet.	395.4	RC 1  RC 3	100 (97 (89) 100 (92)	50/1"			8				

SAURABH SHYAM MITTAL NUMBER PE-2019010896

DATE PREPARED 3/16/2022 DISTRICT SHEET NO. 7

ST. LOUIS CITY
JOB NO.
J6I3020C CONTRACT ID.

PROJECT NO. BRIDGE NO.

HIGHWAYS AND TRAI COMMISSION

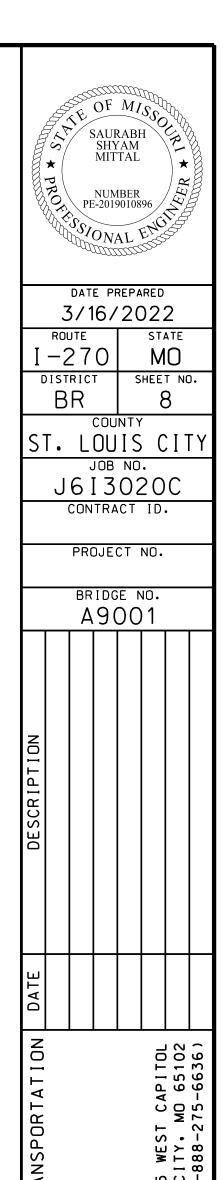
I-270 AND RIVERVIEW I-270/RIVERVIEW MSE WALL A9001 SHEET 7 OF 10

Sheet No. 7 of 10

Designed: JAA Detailed: JAA Checked: JEK

(Continued Next Page)

	<b>ENINI</b>	Millennia Professional Services				В	BOR	ING	NU	JME		<b>SB</b> 1 0		
PAOFESSI	T H	orner & Shifrin, Inc.	PROJEC	T NAME	Inters	state 270 a	t River	view D	Orive Ir	mprove	<u>ement</u>	s		
		IUMBER MG20007	PROJEC	ROJECT NAME Interstate 270 at Riverview Drive Improvements  ROJECT LOCATION St. Louis, Missouri										
DATE	STAF	RTED 3/2/20 COMPLETED 3/2/20	GROUND ELEVATION 438.3 ft HOLE SIZE inches											
DRILL	ING (	CONTRACTOR Bulldog, CME550X, Efficiency: 95%	GROUND WATER LEVELS:											
DRILL	ING N	METHOD Hollow Stem Auger	<b>AT TIME OF DRILLING</b> _7.00 ft / Elev 431.30 ft											
LOGG	ED B	Y B.Fisher CHECKED BY J. Schaeffer												
NOTE	<b>S</b> <u>5.</u> 0	Oft west of design due to location of marked utilities	AFTER DRILLING											
DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION		SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (tsf)	DRY UNIT WT. (pcf)	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTIC WE LIMIT		FINES CONTENT (%)	
0	> \ \ \ \	Concrete (10.0")	437.47											
		POSSIBLE FILL: Grey to brown, lean CLAY (CL), with s gravel		√ ss		3-2-5								
5		POSSIBLE FILL: Tan, clayey SAND (SC)		<u> 1</u>	61	(7)	0.25		10					
		- approx. 12.0" layer of lean clay at 6.0 ft ∑		ST 2	100			111	17					
10		Top to groy loop CLAV (CL) with cond	427.3_	SS 3	100	4-6-7 (13)	-	,	22					
		Tan to grey, lean CLAY (CL), with sand		√ ss		1-3-3								
15				4	100	(6)	2.0		19					
; - -														
		- blue-grey, more plastic below 18.0 ft		SS 5	100	2-3-5	1.5		25					
20		SHALE: Blue-grey to brown, weathered	416.3	7\ 3		(8)								
25			411.3	SS 6	100	5-14-17 (31)	>4.5		15					
		Refusal at 27.0 feet. Bottom of borehole at 27.0 feet.		SS 7		50/0"								



HIGHWAYS AND TRAN COMMISSION

I-270 AND RIVERVIEW I-270/RIVERVIEW MSE WALL A9001 SHEET 8 OF 10

Designed: JAA Detailed: JAA Checked: JEK

	MILL	ENNI.	Millennia Professional Services				В	OR	ING	S NU			<b>SB</b> 1 01		
	CLIEN	<b>IT</b> Ho	orner & Shifrin, Inc.	PROJEC <sup>-</sup>	Г NAME	Inters	state 270 a	t River	view D	Orive Ir	nprove	ement	S		
1				PROJECT NAME Interstate 270 at Riverview Drive Improvements PROJECT LOCATION St. Louis, Missouri											
[	DATE	STAR	TED 3/3/20 COMPLETED 3/3/20	GROUND ELEVATION 438.3 ft HOLE SIZE inches											
[	DRILL	ING C	CONTRACTOR Bulldog, CME550X, Efficiency: 95%	GROUND WATER LEVELS:											
[	DRILL	ING N	IETHOD Hollow Stem Auger, NQ Rock Core	<b>AT TIME OF DRILLING</b> _7.00 ft / Elev 431.30 ft											
<u> </u>   L	OGG	ED B	Y B.Fisher CHECKED BY J. Schaeffer												
	NOTE	<b>S</b> <u>5.0</u>	) ft west of design due to location of marked utilities	AFTER DRILLING											
NVE INTERCHANGE GIL		GRAPHIC LOG	MATERIAL DESCRIPTION		SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (tsf)	DRY UNIT WT. (pcf)	MOISTURE CONTENT (%)	L	PLASTIC WE LIMIT	PLASTICITY BY BY INDEX	FINES CONTENT (%)	
ا ا	0	D	Concrete (10.0")	437.5											
	-		Brown-orange to grey, lean CLAY (CL), trace sand	10710											
	- - 5				SS 1	100	3-5-5 (10)	2.5		17					
	-		- possible sand layer below 8.0 ft		ST 2	0									
	10		- no recovery at ST-2, classifications from cuttings												
	- -		Grey, fat CLAY (CH), trace sand and gravel	427.3											
777	45				$\left  \begin{array}{c} SS \\ 3 \end{array} \right $	89	3-2-3 (5)	1.75		22				ľ	
	<u>15</u> - -		- undrained shear strength at 15.0 ft = 0.60 TSF		ST 4	100		0.75	102	24					
0.03 EAB.GDI = 12/17/20 1			- with sand below 18.5 ft		SS 5	83	2-2-4 (6)	1.0		18					
	- 25				SS 6	56	2-2-5 (7)	1.0		10					
֝֝֝֟֝֡֟֝֡֟֡֟֝֟֡֟֝֟֟֟֝ <del>֡</del>	_														

409.8

(Continued Next Page)

	MIL	LENNI	Millennia Professional Services				E	3OF	RING	S NU	JME		2 SB	
	CLIE	NT H	orner & Shifrin, Inc.	PROJEC	T NAME	Inter	state 270 a	t Rive	rview [	Orive I	mprov	ement	s	
	PROJ	IECT N	IUMBER MG20007	PROJEC	T LOCA	LION _	St. Louis, I	Missou	uri					
IT.GPJ	DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION		SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (tsf)	DRY UNIT WT. (pcf)	MOISTURE CONTENT (%)		PLASTIC HIMIT LIMIT		FINES CONTENT (%)
22 - G:\PROJECT FILES\2020\MG20007 I-270 RIVERVIEW DRIVE INTERCHANGE\FIELD DATA\1270 AT RIVERVIEW DRIVE INTERCHANGE GINT.GPJ	40		LIMESTONE: Grey, moderately hard, thin bedded, coarsely-crystalline, moderately weathered SHALE: Grey, soft to moderately hard, calcareous SHALY LIMESTONE: Grey, moderately hard, finely-crystalline, thin to medium to moderately weathered SHALE: Grey, soft to moderately hard  - soft below 35.0 ft - rock core qu at 35.5 ft = 5,090 psi  LIMESTONE: Grey, argillaceous, moderately hard, thin to thick bedded, finely-crystalline, slightly weathered - with shale inclusions from 38.0-38.5 ft  Refusal at 28.5 feet. Bottom of borehole at 43.5 feet.	400.3	RC 2	100 (87) 100 (77) 100 (100) 93 (82)	50/1"			10				
:CH BH COLUMNS - GINT STD US LAB.GDT - 12/17/20 11:22 - G:\														

SAURABH SHYAM MITTAL NUMBER PE-2019010896 DATE PREPARED 3/16/2022

DISTRICT SHEET NO.

BR 9

COUNTY ST. LOUIS CITY JOB NO. J6I3O2OC CONTRACT ID.

PROJECT NO. BRIDGE NO.

HIGHWAYS AND TRAN COMMISSION

HORNER

I-270/RIVERVIEW MSE WALL A9001 SHEET 9 OF 10 I-270 AND Riverview

	X	Millennia Professional Services			E	BOR	RING	)N ŧ	JME		<b>SB</b> 1 0		
PROFESE	LENNI. BIONAL BERVIO		CT NAME	Inter	state 270 a	t Rive	rview C	Orive I	mprov	ement	'S		
1								<u> </u>	mprov	CITICITE	<u> </u>		
PROJECT NUMBER     MG20007     PROJECT LOCATION     St. Louis, Missouri       DATE STARTED     3/3/20     COMPLETED     3/3/20     GROUND ELEVATION     438.1 ft     HOLE SIZE _ inches													
DRILLING CONTRACTOR Bulldog, CME550X, Efficiency: 95% GROUND WATER LEVELS:													
	DRILLING METHOD Hollow Stem Auger  ZAT TIME OF DRILLING 9.00 ft / Elev 429.10 ft												
	LOGGED BY B.Fisher CHECKED BY J. Schaeffer AT END OF DRILLING not measured												
		of the north and 5.0 ft west of design due to location of marked utilities											
ATTEDRED											ERG	<u> </u>	
DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (tsf)	DRY UNIT WT. (pcf)	MOISTURE CONTENT (%)	LIQUID			FINES CONTENT (%)	
0	P 5 4	Concrete (10.0") 437.3						1	1		<del>                                     </del>		
 		Orange-brown to grey, lean to fat CLAY (CL-CH), with sand, trace gravel	<del></del>										
5		432.	ss 1	94	3-3-6 (9)	2.5		14	49	18	31		
		Grey, lean CLAY (CL), trace sand lenses											
-		<u>¥</u> 428.6	$S \setminus S$	89	2-2-4 (6)	1.0		17					
10		Tan SAND (SC) with silt and clay	/ \	1	(0)		-						
		Grey, lean CLAY (CL), trace sand and gravel	1										
15		<ul><li>- undrained shear strength at 13.0 ft = 0.76 TSF</li><li>- with gravel below 14.0 ft</li></ul>	ST 3	83		1.5	110	20					
20			SS 4	100	1-2-2 (4)	1.25		23	-				
25		- 2.0" fragmented rock layer at 23.8 ft	SS 5	78	4-3-3 (6)	2.0	_	25	_				
		410.6	S	<b>1</b> 100	50/0"								
		Refusal at 27.5 feet. Bottom of borehole at 27.5 feet.	6	100	<u> </u>	J							



ST. LOUIS CITY
JOB NO.
J6 I 3020C
CONTRACT ID.

PROJECT NO.

BRIDGE NO.

HIGHWAYS AND TRAN COMMISSION

I-270 AND RIVERVIEW I-270/RIVERVIEW MSE WALL A9001 SHEET 10 OF 10

Designed: JAA Detailed: JAA Checked: JEK