

281' MECHANICALLY STABILIZED EARTH (MSE) RETAINING WALL SYSTEM

General Notes:

Design Specifications:

2002 AASHTO LFD (17th Ed.) Standard Specifications (Section 5, ASD Design)
Seismic Performance Category B
Acceleration Coefficient = 0.12

Design Loading:

$\phi_D = 25^\circ$ and Unit weight, $\gamma_D = 130$ pcf for retained backfill material to be retained by the mechanically stabilized earth wall system.

$\phi_F = 25^\circ$ for unimproved foundation ground where wall is to bear.

$\phi_F = 34^\circ$ for improved foundation ground where wall is to bear.

The allowable bearing pressure for unimproved foundation ground 3.0 ksf.
The allowable bearing pressure for improved foundation ground 4.3 ksf.

The maximum applied bearing pressure for the controlling design case at the foundation level shall be shown on the shop drawings and shall be less than the allowable bearing pressure for foundation ground and provided herein. For seismic design the maximum applied bearing pressure shall be less than two times the allowable bearing pressure.

Allowable bearing pressure and limits of improved foundation ground shall not be adjusted from that as shown on the plans.

Contractor shall include ϕ_r (actual $\phi \geq 34^\circ$) and the total unit weight γ_r , for the select granular backfill (reinforced backfill and wedge area backfill) for structural systems on shop drawings. Contractor shall identify source of select granular backfill material, submit proctor in accordance with AASHTO T 99 (ASTM D698) and gradation with the shop drawings. When backfill material is too coarse to develop a proctor curve the contractor shall determine the maximum dry density (relative density) in accordance with ASTM D4253 and ASTM D4254 and assume percent passing the 200 sieve for optimum water content.

Total unit weight, $\gamma_r = (95\% \text{ compaction}) \times (\text{maximum dry density}) \times (1 + \text{optimum water content})$

Design $\phi_r = 34^\circ$ for the select granular backfill (reinforced backfill) only for structural systems.

Factor of safety shall be 2.0 for overturning and 1.5 for sliding.

For seismic design the factor of safety shall be 1.5 for overturning and 1.1 for sliding.

Use default values for the pullout friction factor, F^* , in accordance with AASHTO figure 5.8.5.2A, unless MoDOT has pre-approved a higher value for a specific product. For approved steel strips not shown in AASHTO figure 5.8.5.2A, use $F^* \leq 2.0$ at zero depth and $F^* \leq \tan \phi_r$ at 20 feet depth and ϕ_r design = 34° . F^* values shall be shown on the shop drawings.

Design Unit Stresses:

All concrete for leveling pad and coping shall be Class B or B-1 with $f'c = 4000$ psi.

The minimum compressive strength of concrete for precast panel shall be 4,000 psi in accordance with Sec 1052.

Miscellaneous:

The MSE wall system shall be built vertical.

The MSE wall system shall be built in accordance with Sec 720.

The MSE wall system shall be a large block wall system.

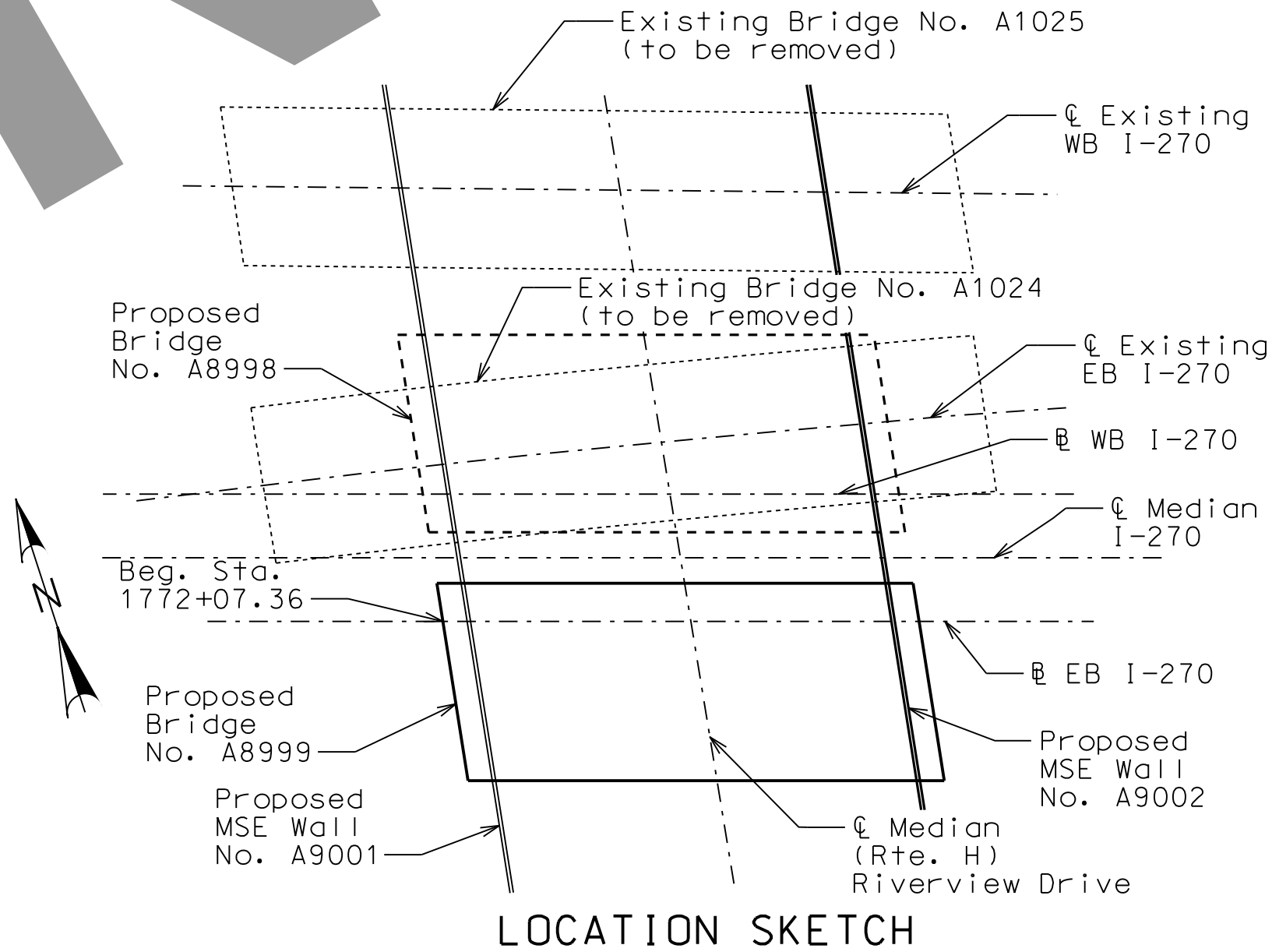
Panel and coping (or capstone) reinforcement shall be epoxy coated.

A filter cloth meeting the requirements for a Separation Geotextile material shall be placed between the select granular backfill for structural systems and the backfill being retained by the mechanically stabilized earth wall system.

Coping shall be required on this structure. When CIP coping sections extend beyond the limits of a single panel, bond breaker (roofing felt or other approved alternate) between wall panel and coping is required. Coping joints shall use 3/4-inch chamfers and shall be sealed with 3/4-inch joint filler. Coping reinforcement shall terminate 1 1/2-inch minimum from face of coping joint.

The top and bottom elevations are given for a vertical wall. If a battered small block wall system is used, the height of the wall shall be adjusted as necessary to fit the ground slope and the concrete leveling pad shall be adjusted as necessary to account for the wall batter. If a fence is built on an extended gutter, then the height of the wall shall be adjusted further.

The contractor shall be solely responsible to coordinate construction of the wall with bridge and roadway construction and ensure that the bridge and roadway construction, resulting or existing obstructions, shall not impact the construction or performance of the wall. Soil reinforcement shall be designed and placed to avoid damage by pile driving, guardrail post installation, utility and sign foundations. (See Roadway and Bridge plans.)



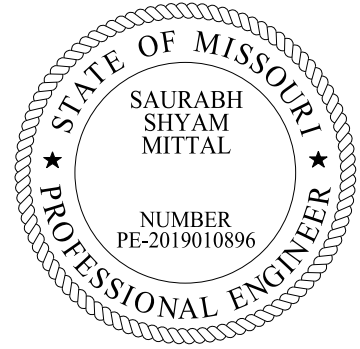
Estimated Quantities		
Item		Total
Aesthetic Concrete Stain	LS	1
Mechanically Stabilized Earth Wall Systems	sq. foot	4819
Temporary MSE Wall	LS	1

MSE Wall Systems Data Table					
Proprietary Wall Systems		Combination Wall Systems			
Manufacturer	System	Facing Unit Manufacturer	Facing Unit	Geogrid Manufacturer	Geogrid

MSE Wall Systems Data Table is to be completed by MoDOT construction personnel to record the manufacturer of the proprietary wall system or the manufacturers of the combination wall system that was used for constructing the MSE wall.

RETAINING WALL UNDER END BENT NO. 1
AT BRIDGE A8998 AND BRIDGE A8999

ROUTE EB I-270 FROM LILAC AVENUE TO MISSOURI STATE LINE
ABOUT 1.0 MILES EAST OF LILAC AVENUE
STATION 12+24.28 ALONG SB ROUTE H



DATE PREPARED 3/16/2022	
ROUTE I-270	STATE MO
DISTRICT BR	SHEET NO. 2
COUNTY ST. LOUIS CITY	
JOB NO. J613020C	
CONTRACT ID.	

PROJECT NO.
BRIDGE NO. A9001

DESCRIPTION	DATE

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

MoDOT

105 WEST CAPITOL
JEFFERSON CITY, MO 65102
1-888-ASK-MODOT (1-888-275-6636)

HORNER SHIFFRIN

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CERTIFICATE OF AUTHORITY: 000159
EXPIRATION DATE: DECEMBER 31, 2022

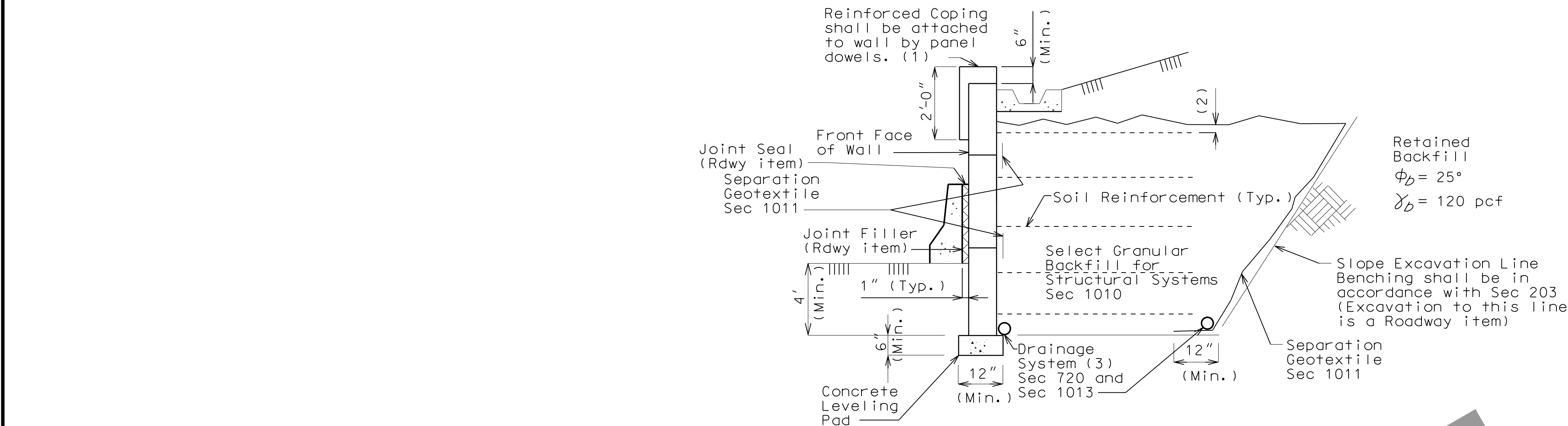
I-270 AND RIVERVIEW

I-270/RIVERVIEW
MSE WALL A9001
SHEET 2 OF 10

Designed:JEK
Detailed:JEK
Checked: TPL

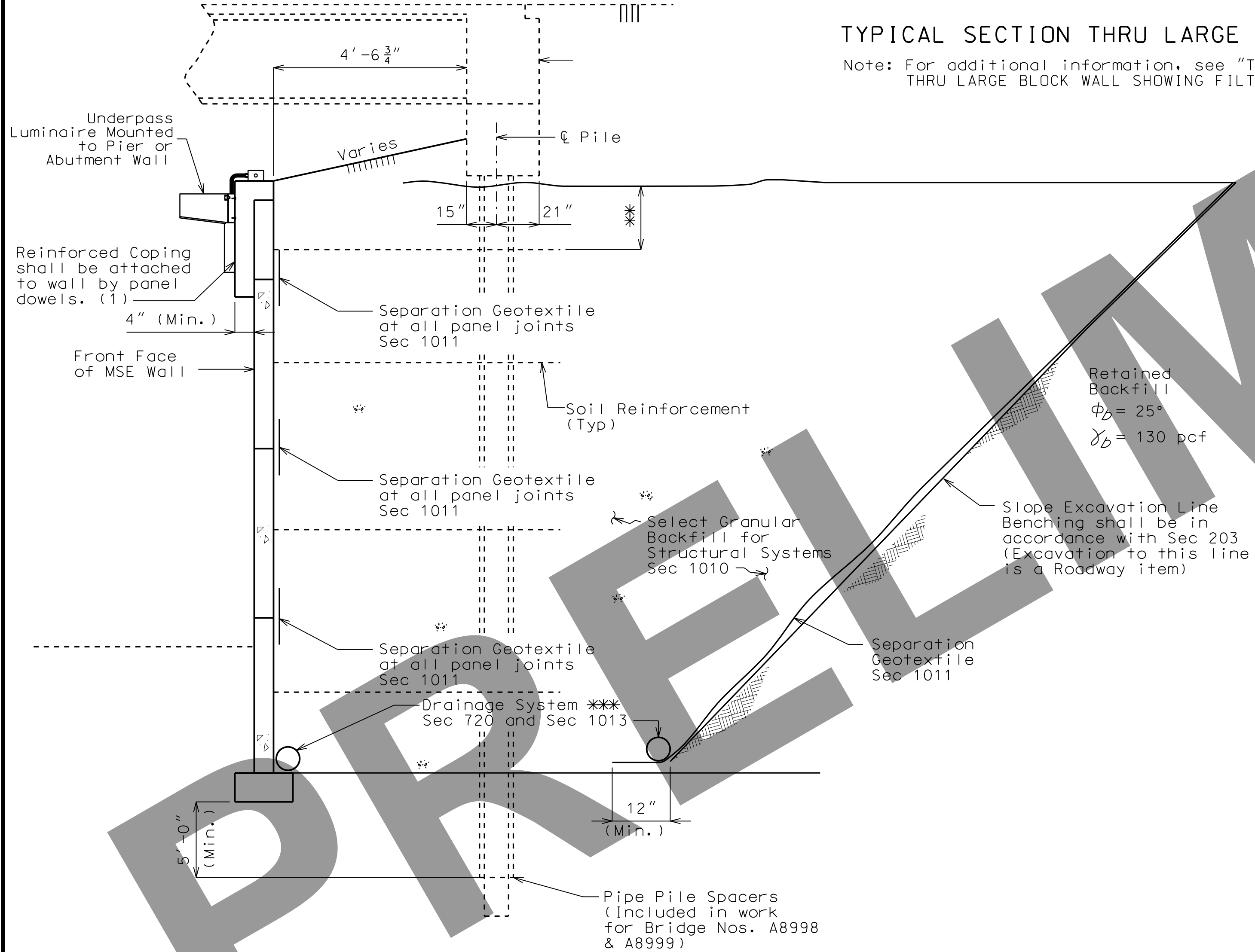
Note: This drawing is not to scale. Follow dimensions.

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TYPICAL SECTION THRU LARGE BLOCK WALL

Note: For additional information, see "TYPICAL SECTION THRU LARGE BLOCK WALL SHOWING FILTER CLOTH".



TYPICAL SECTION THRU LARGE BLOCK WALL UNDER BRIDGE

Note: For additional information, see "TYPICAL SECTION THRU LARGE BLOCK WALL SHOWING FILTER CLOTH".

Note:

Holes shall be 5/8" round and extended 4" into the third layer of blocks, recessed 2" deep by 1 1/2" round.

Rods or reinforcing bars shall be secured by an approved resin anchor system in accordance with Sec 1039.

Recess hole shall be backfilled with non-shrink cement grout.

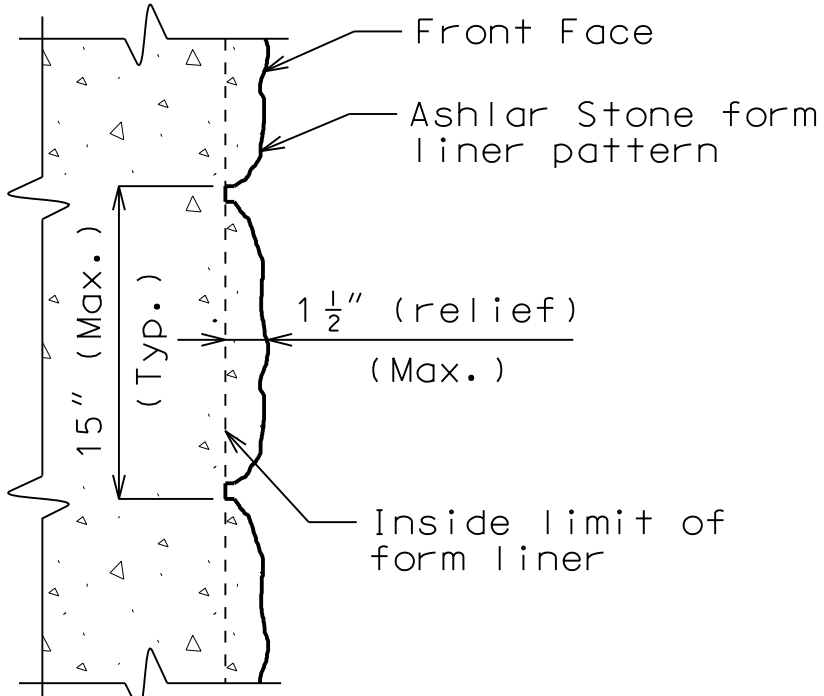
(1) Inverted U-shape reinforced capstone may be used in lieu of coping. Panel dowels for level-up concrete shall be required, and provided by manufacturer. The dowels shall be field trimmed to clear the capstone by a minimum of 1 1/2 inches and a maximum of 2 1/2 inches.

(2) Topmost layer of reinforcement shall be fully covered with select granular backfill for structural systems, as approved by the wall manufacturer, before placement of the Separation Geotextile.

(3) Minimum 6" diameter perforated PVC or PE pipe.

Manufacturer shall show drain details on design plans to be submitted as shown on MoDOT MSE wall plans and/or roadway plans.

Contractor shall modify the drain details as shown if it will improve flow as may be the case for stepped leveling pad, and for an uneven ground line (approval of the engineer required).



FORM LINER DETAIL (Large Block Wall)

Notes:

The cost of form liners for MSE wall systems, complete in place, will be considered completely covered by the contract unit price for Mechanically Stabilized Earth Wall System.

Form liner shall be constructed in accordance with Special Provisions.

The following is a list of form liner manufacturers and types which may be used. Depth of relief for all form liner patterns shall vary up to 1 1/2". The height of any single 'stone' shall be 15" maximum.

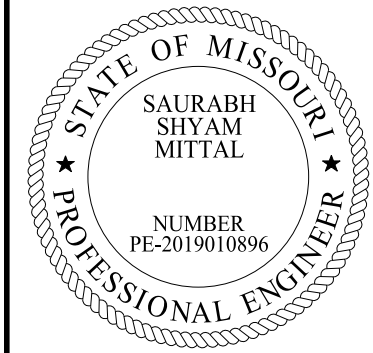
- Scott System, Inc.: Form liner pattern #167 "Ashlar Stone"
- Fitzgerald Formliners: Form liner pattern #16986 "Ashlar Stone"
- Greenstreak: Form liner pattern #330 "Ashlar Stone"
- Spec Formliners: Form liner pattern #1515 "Ashlar Stone"
- Customrock: Form liner pattern #12020 "Tollway Ashlar"
- An approved equal

Designed:JEK
Detailed:JEK
Checked: TPL

Note: This drawing is not to scale. Follow dimensions.

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DETAILS FOR MSE WALL



DATE PREPARED
3/16/2022

ROUTE I-270 STATE MO
DISTRICT BR SHEET NO. 4

COUNTY
ST. LOUIS CITY

JOB NO.
J613020C

CONTRACT ID.

PROJECT NO.

BRIDGE NO.
A9001

DESCRIPTION	DATE

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

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I-270 AND RIVERVIEW
I-270/RIVERVIEW MSE WALL A9001
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CLIENT <u>Horne & Shiffrin, Inc.</u>	PROJECT NAME <u>Interstate 270 at Riverview Drive Improvements</u>
PROJECT NUMBER <u>MG20007</u>	PROJECT LOCATION <u>St. Louis, Missouri</u>
DATE STARTED <u>2/28/20</u> COMPLETED <u>2/28/20</u>	GROUND ELEVATION <u>439.1 ft</u> HOLE SIZE <u>inches</u>
DRILLING CONTRACTOR <u>Bulldog, CME550X, Efficiency: 95%</u>	GROUND WATER LEVELS:
DRILLING METHOD <u>Hollow Stem Auger, NQ Rock Core</u>	AT TIME OF DRILLING <u>--- not encountered</u>
LOGGED BY <u>B.Fisher</u> CHECKED BY <u>J. Schaeffer</u>	AT END OF DRILLING <u>--- not measured</u>
NOTES <u>3.0 ft west of design due to location of marked utilities</u>	AFTER DRILLING <u>---</u>

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CLIENT Horne & Shifrin, Inc. **PROJECT NAME** Interstate 270 at Riverview Drive Improvements
PROJECT NUMBER MG20007 **PROJECT LOCATION** St. Louis, Missouri

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Refusal at 23.5 feet.
Bottom of borehole at 38.5 feet.

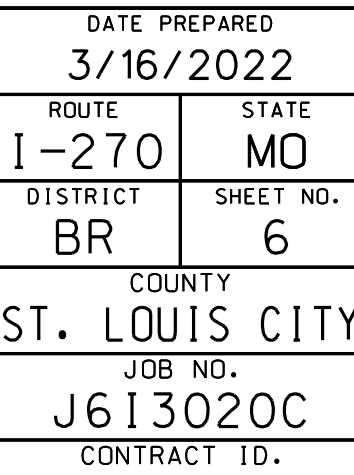
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Note: For locations of borings, see Sheet No. 1.

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 6 of 10

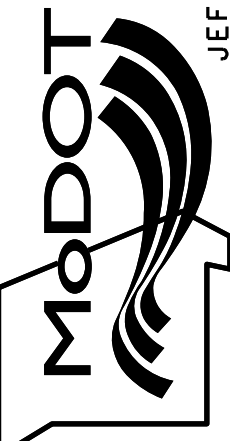
Designed: JAA
Detailed: JAA
Checked: JEK



PROJECT NO.

BRIDGE NO.
A9001

DATE	DESCRIPTION

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I-270 AND RIVERVIEW

I-270/RIVERVIEW
MSE WALL A9001
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PAGE 1 OF 2

CLIENT <u>Horne & Shifrin, Inc.</u>	PROJECT NAME <u>Interstate 270 at Riverview Drive Improvements</u>
PROJECT NUMBER <u>MG20007</u>	PROJECT LOCATION <u>St. Louis, Missouri</u>
DATE STARTED <u>3/2/20</u> COMPLETED <u>3/2/20</u>	GROUND ELEVATION <u>438.4 ft</u> HOLE SIZE <u> inches</u>
DRILLING CONTRACTOR <u>Bulldog, CME550X, Efficiency: 95%</u>	GROUND WATER LEVELS:
DRILLING METHOD <u>Hollow Stem Auger, NQ Rock Core</u>	▽ AT TIME OF DRILLING <u>8.00 ft / Elev 430.40 ft</u>
LOGGED BY <u>B.Fisher</u> CHECKED BY <u>J. Schaeffer</u>	AT END OF DRILLING <u>--- not measured</u>
NOTES <u>5.0 ft west of design due to location of marked utilities</u>	AFTER DRILLING <u>---</u>

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(Continued Next Page)



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CLIENT Horne & Shifrin, Inc. **PROJECT NAME** Interstate 270 at Riverview Drive Improvements
PROJECT NUMBER MG20007 **PROJECT LOCATION** St. Louis, Missouri

DEPTH DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY % (ROD)	BLOW COUNTS (N VALUE)	POCKET PEN. (tsf)	DRY UNIT WT. (pcf)	MOISTURE CONTENT (%)	ATTERBERG LIMITS			FINES CONTENT (%)
									LIQUID LIMIT	PLASTIC LIMIT	PLASTICITY INDEX	
30		SHALE: Bluegrey, soft, calcareous - vertical fracture from 28.6-28.8 ft - 6.0" limestone layer at 28.8 ft	SS 6	100	50/1"			8				
		- 3.0" near vertical fracture at 30.5 ft	RC 1	97 (89)								
		- rock core qu at 32.3 ft = 2,540 psi - core loss from 32.7-32.9 ft										
35		- 5.5" weathered clayey shale seam at 34.5 ft	RC 2	100 (92)								
		- very soft from 38.0-38.3 ft										
40		- 4.0" weathered clay seam at 42.5 ft	RC 3	100 (98)								

Refusal at 28.0 feet.
Bottom of borehole at 43.0 feet.

395.4

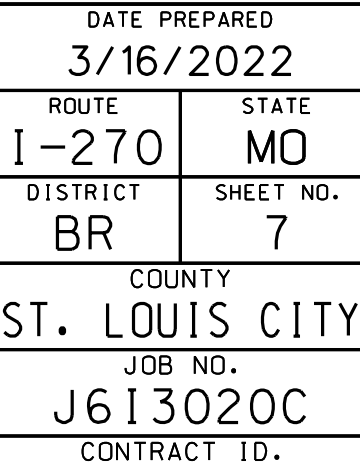
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Note: For locations of borings, see Sheet No. 1.

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 7 of 10

Designed: JAA
Detailed: JAA
Checked: JEK



PROJECT NO.

BRIDGE NO.
A9001

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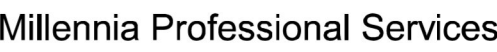
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I-270 AND
RIVERVIEW
I-270/RIVERVIEW
MSE WALL A9001
SHEET 7 OF 10



BORING NUMBER SB-2

PAGE 1 OF 1

CLIENT	Horner & Shiffrin, Inc.	PROJECT NAME	Interstate 270 at Riverview Drive Improvements
PROJECT NUMBER	MG20007	PROJECT LOCATION	St. Louis, Missouri
DATE STARTED	3/2/20	COMPLETED	3/2/20
DRILLING CONTRACTOR	Bulldog, CME550X, Efficiency: 95%	GROUND ELEVATION	438.3 ft
DRILLING METHOD	Hollow Stem Auger	HOLE SIZE	inches
LOGGED BY	B.Fisher	CHECKED BY	J. Schaeffer
NOTES	5.0 ft west of design due to location of marked utilities		
		GROUND WATER LEVELS:	
		▽ AT TIME OF DRILLING	7.00 ft / Elev 431.30 ft
		AT END OF DRILLING	--- not measured
		AFTER DRILLING	---

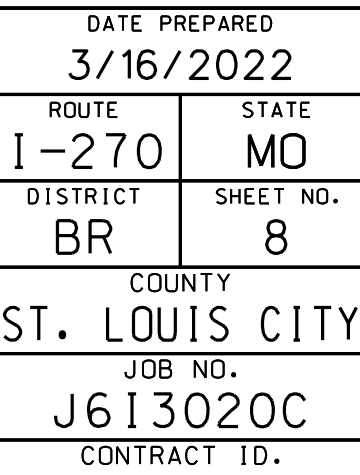
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BORING DATA

Note: For locations of borings, see Sheet No. 1.

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 8 of 10



PROJECT NO.

BRIDGE NO.
A9001

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I-270/RIVERVIEW
MSE WALL A9001
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