

MECHANICALLY STABILIZED EARTH (MSE) RETAINING WALL SYSTEM

General Notes:

Design Specifications:

2002 AASHTO LFD (17th Ed.) Standard Specifications (Section 5, ASD Design) Seismic Performance Category B Acceleration Coefficient = 0.12

Design Loading:

 ϕ_b = 25° and Unit weight, χ_b = 130 pcf for retained backfill material to be retained by the mechanically stabilized earth wall system.

 $\phi_{ extstyle au}$ = 25° for unimproved foundation ground where wall is to bear. $\Phi_{\mathcal{F}} = 34^{\circ}$ for improved foundation ground where wall is to bear.

The allowable bearing pressure for unimproved foundation ground 3.4 ksf. The allowable bearing pressure for improved foundation ground 4.3 ksf.

The maximum applied bearing pressure for the controlling design case at the foundation level shall be shown on the shop drawings and shall be less than the allowable bearing pressure for foundation ground and provided herein. For seismic design the maximum applied bearing pressure shall be less than two times the allowable bearing pressure.

Allowable bearing pressure and limits of improved foundation ground shall not be adjusted from that as shown on the plans.

Contractor shall include ϕ (actual $\phi \geq 34^{\circ}$) and the total unit weight χ , for the select granular backfill (reinforced backfill and wedge area backfill) for structural systems on shop drawings. Contractor shall identify source of select granular backfill material, submit proctor in accordance with AASHTO T 99 (ASTM D698) and gradation with the shop drawings. When backfill material is too coarse to develop a proctor curve the contractor shall determine the maximum dry density (relative density) in accordance with ASTM D4253 and ASTM D4254 and assume percent passing the 200 sieve for optimum water content.

Total unit weight, $\mathcal{Y}_{-}=$ (95% compaction) x (maximum dry density) x (1 + optimum water content)

Design $\Phi_r = 34^\circ$ for the select granular backfill (reinforced backfill) only for structural systems.

Factor of safety shall be 2.0 for overturning and 1.5 for sliding.

For seismic design the factor of safety shall be 1.5 for overturning and 1.1 for sliding.

Use default values for the pullout friction factor, F*, in accordance with AASHTO figure 5.8.5.2A. unless MoDOT has pre-approved a higher value for a specific product. For approved steel strips not shown in AASHTO figure, 5.8.5.2A, use F* \leq 2.0 at zero depth and F* \leq Tan ϕ_r at 20 feet depth and ϕ_r design = 34°. F* values shall be shown on the shop drawings.

Design Unit Stresses:

All concrete for leveling pad and coping shall be Class B or B-1 with f'c = 4000 psi.

The minimum compressive strength of concrete for precast panel shall be 4,000 psi in accordance with Sec 1052.

Miscellaneous:

The MSE wall system shall be built vertical.

The MSE wall system shall be built in accordance with Sec 720.

The MSE wall system shall be a large block wall system.

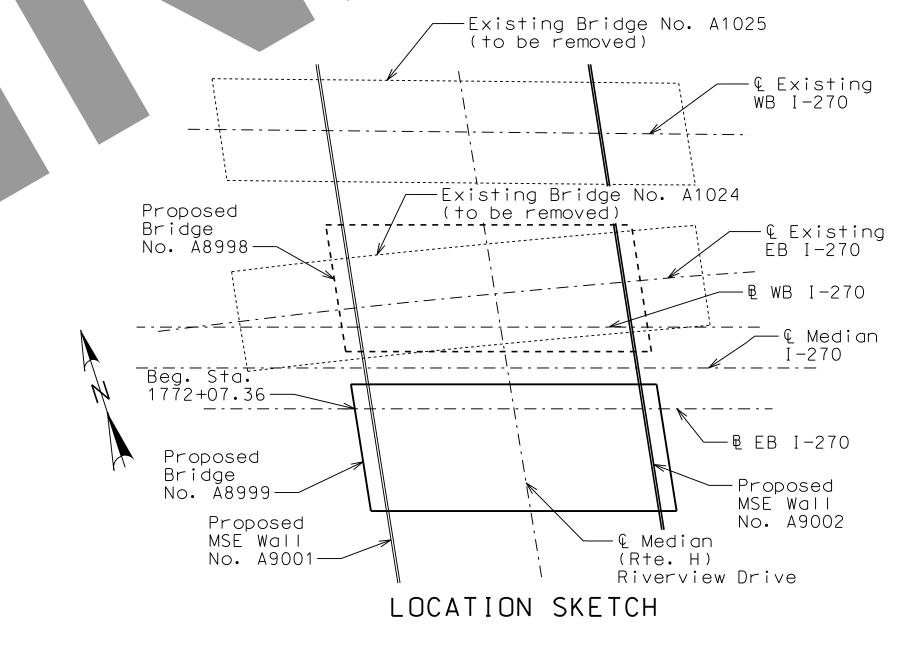
Panel and coping (or capstone) reinforcement shall be epoxy coated.

A filter cloth meeting the requirements for a Separation Geotextile material shall be placed between the select granular backfill for structural systems and the backfill being retained by the mechanically stabilized earth wall system.

Coping shall be required on this structure. When CIP coping sections extend beyond the limits of a single panel, bond breaker (roofing felt or other approved alternate) between wall panel and coping is required. Coping joints shall use 3/4-inch chamfers and shall be sealed with 3/4-inch joint filler. Coping reinforcement shall terminate 1 1/2-inch minimum from face of coping joint.

The top and bottom elevations are given for a vertical wall. If a battered small block wall system is used, the height of the wall shall be adjusted as necessary to fit the ground slope and the concrete leveling pad shall be adjusted as necessary to account for the wall batter. If a fence is built on an extended gutter, then the height of the wall shall be adjusted further.

The contractor shall be solely responsible to coordinate construction of the wall with bridge and roadway construction and ensure that the bridge and roadway construction, resulting or existing obstructions, shall not impact the construction or performance of the wall. Soil reinforcement shall be designed and placed to avoid damage by pile driving, guardrail post installation, utility and sign foundations. (See Roadway and Bridge plans.)



Estimated Quantitie	es	
I tem		Total
Aesthetic Concrete Stain	LS	1
Mechanically Stabilized Earth Wall Systems	sq. foot	4001
Temporary MSE Wall	LS	1

Designed:JEK Detailed:JEK Checked: TPL

MSE Wall Systems Data Table										
Proprietary Wall S	ystems	Combi	nation W	Nall Systems						
Manufacturer	System	Facing Unit Manufacturer	Facing Unit	Geogrid Manufacturer	Geogrid					

MSE Wall Systems Data Table is to be completed by MoDOT construction personnel to record the manufacturer of the proprietary wall system or the manufacturers of the combination wall system that was used for constructing the MSE wall.

ROUTE EB I-270 FROM LILAC AVENUE TO MISSOURI STATE LINE ABOUT 1.0 MILES EAST OF LILAC AVENUE STATION 12+06.66 ALONG & SB ROUTE H

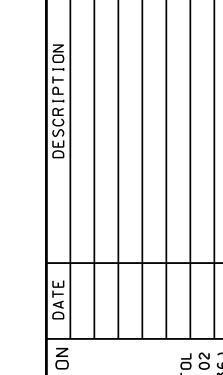
RETAINING WALL UNDER END BENT NO. 3

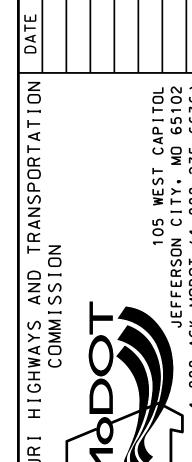
AT BRIDGE A8998 AND BRIDGE A8999

SAURABH SHYAM MITTAL NUMBER PE-2019010896 ONAL W

DATE P	REPARED							
3/16/2022								
ROUTE	STATE							
I-270	MO							
DISTRICT	SHEET NO.							
BR	2							
COU	NTY							

		PRO	DJE(СТ	NO.						
	BRIDGE NO. A9002										
Z											







O AND RVIEW ΛĒ O/R WAL ET **6** $\sim >$ | |----

General Notes Cont.:

Designed: JEK

Detailed: JEK Checked: TPL

Minimum 18" wide Geotextile strips shall be centered at vertical and horizontal joints of panel. Geotextile material shall be adhered to back face of panel using an adhesive compound supplied by the manufacturer. All edges of each fabric strip shall provide a positive seal. A minimum 18 overlap shall be provided between spliced filter fabric.

Aluminized soil reinforcement shall have edges coated with coating material per manufacturer.

Upper two layers of soil reinforcement shall be extended 3 feet beyond

Soil reinforcement shall be spaced to avoid roadway drop inlet behind

the lower layers The splay angle should be less than 15° and tensile capacity of splayed

reinforcement shall be reduced by the cosine of the splay angle. Soil reinforcement shall clear the obstruction by at least 3 inches.

No reinforcement shall be left unconnected to the wall face or arbitrarily cut/bent in the field to avoid the obstruction.

Where interference between the vertical obstruction and the soil reinforcement is unavoidable, the design of the wall near the obstruction may be modified using one of the alternatives in FHWA-NHI-10-24, Section 5.4.2. Show detail layout on the drawings. For wall designs with horizontal obstructions in reinforced soil mass, see FHWA-NHI-10-024, Section 5.4.3.

Excavation quantities and pay items are given on the roadway plans. Excavation quantities are based on a soil reinforcement length of 0.75 x Height of the Wall in ft. The soil reinforcement length may vary based upon the wall design selected by the contractor. Plan excavation quantities will be paid regardless of any actual quantities removed based on the soil reinforcement length and design selected.

TYPICAL SECTION THRU

LARGE BLOCK WALL SHOWING FILTER CLOTH

- (1) Inverted U-shape reinforced capstone may be used in lieu of coping. Panel dowels for level-up concrete shall be required, and provided by manufacturer. The dowels shall be field trimmed to clear the capstone by a minimum of 1 1/2 inches and a maximum of 2 1/2 inches.
- (2) Topmost layer of reinforcement shall be fully covered with select granular backfill for structural systems, as approved by the wall manufacturer, before placement of the Separation Geotextile.

Mat	erial Prop	erties Used in Design							
Reinf. Fi Granular	II/Select Backfill	Active Compute	Force stions	Foundation					
φ°	γ (pcf)	φ°	γ (pcf)	φ°	brack				

Note: MSE Wall designer shall include table on shop drawings and provide values used in the design computations. Effects of cohesion shall be ignored unless approved by the engineer.

DETAILS FOR MSE WALL Note: This drawing is not to scale. Follow dimensions.

Sheet No. 3 of 10

Manufacturer shall show drain details on design plans to be submitted as shown on MoDOT MSE wall plans and/or roadway plans.

(3) Minimum 6" diameter perforated PVC or PE pipe.

Contractor shall modify the drain details as shown if it will improve flow as may be the case for stepped leveling pad, and for an uneven ground line (approval of the engineer required).

(4) Select granular backfill shall extend a minimum of 12" beyond the end of \leq all soil reinforcement. Where the angle, 0, between the retained backfill excavation/fill line and the horizontal is less than 90°, the wedge area backfill between 0 and 90° shall be filled with select granular backfill for structural systems meeting the requirements of Section 1010.

- For (45° + $\phi_{\rm b}$ /3) < θ ≤ 90°, properties for retained backfill shall be used for active force computations.

- For $\theta \leq (45^{\circ} + \phi_{6}/3)$, contractor shall have the option to use select granular backfill, ϕ_{r} , or better aggregate material, ϕ_{w} for active force computations in the wedge area backfill. For active force computations, the angle of internal friction for wedge area backfill material, ϕ_r or ϕ_w , shall be limited to 34° unless determined otherwise in accordance with Section 1010. If ϕ_{μ} or $\phi_{\mu\nu} > 34^{\circ}$ is desired for wedge area backfill then test report shall be submitted with shop drawings. ϕ_r or ϕ_w shall not be greater than 40° for computations. Final configuration of this option shall be sent to Geotechnical Section for a new overall global stability analysis. Design ϕ_{w} shall be shown on the shop drawings if used.

The slope excavation line shall be benched and separation geotextile shall be placed between the retained backfill and either select granular backfill or better aggregate material, and between the select granular backfill and better aggregate material.

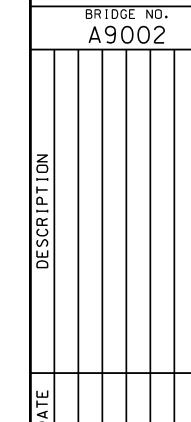
Show range of acceptable theta (0) angle on shop drawings which must be consistent with design computations and proposed construction of wall. Show active force computation properties on shop drawings and in design computations. Coordination between wall designer (manufacturer) and contractor is required before shop drawing submittal.

SAURABH SHYAM MITTAL NUMBER PE-2019010896 OSSIONAL E

DATE PREPARED 3/16/2022 ROUTE STATE I - 270DISTRICT COUNTY

ST. LOUIS CITY JOB NO. J6I3020C CONTRACT ID.

PROJECT NO.

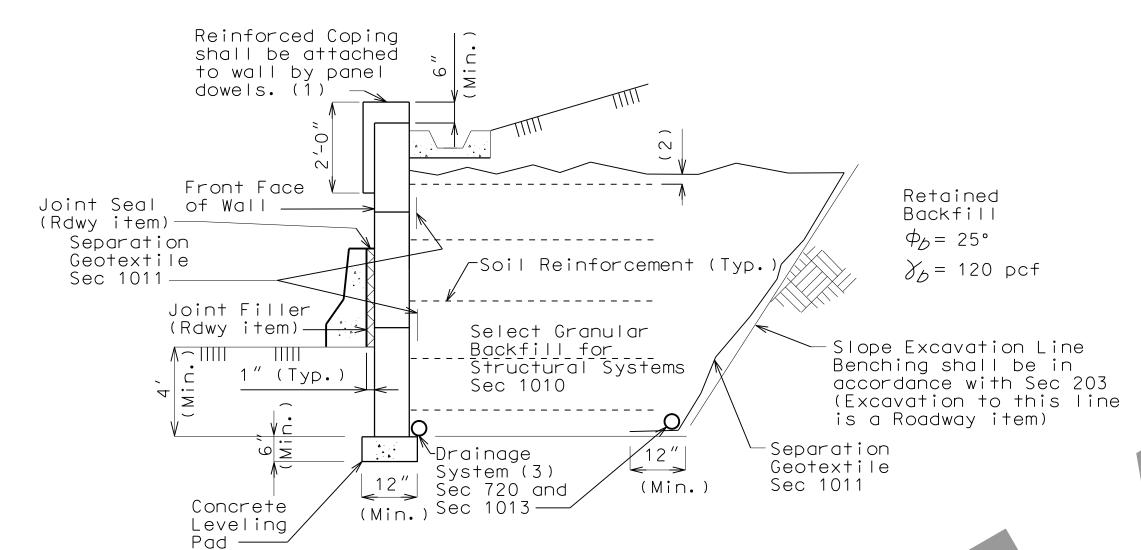


HODNEL SHEDIN

02 02 \triangle Z \Box \triangleleft ΛE >OK **F** <| ---- \square

7-----

 $4' - 6\frac{3}{4}''$



TYPICAL SECTION THRU LARGE BLOCK WALL

Note: For additional information, see "TYPICAL SECTION" THRU LARGE BLOCK WALL SHOWING FILTER CLOTH".

Underpass Luminaire Mounted ! • Pile to Pier or Abutment Wall 11 11 -----H----Reinforced Coping shall be attached to wall by panel dowels. (1) — -Separation Geotextile 4" (Min.) at all panel joints Sec 1011 Front Face ----of MSE Wall ---Backfi └─Soil Reinforcement (Typ) $\gamma_p = 130 \text{ pcf}$ -Separation Geotextile at all panel joints Slope Excavation Line Sec 1011 Select Granular Benching shall be in - - - - - - - - - - - - - - +I- - H - - - - . accordance with Sec 203 Backfill for Structural Systems (Excavation to this line Sec 1010 ~>> is a Roadway item) Separation Geotextil -----Seotextile at all panel joints Sec 1011 Drainage System *** Sec 720 and Sec 1013 Pipe Pile Spacers (Included in work for Bridge Nos. A8998 & A8999)

TYPICAL SECTION THRU LARGE BLOCK WALL UNDER BRIDGE

Note: This drawing is not to scale. Follow dimensions.

Note: For additional information, see "TYPICAL SECTION THRU LARGE BLOCK WALL SHOWING FILTER CLOTH".

Designed: Detailed: Checked:

DETAILS FOR MSE WALL

Sheet No. 4 of 10

Note:

Holes shall be 5/8" round and extended 4" into the third layer of blocks, recessed 2" deep by 1 1/2" round.

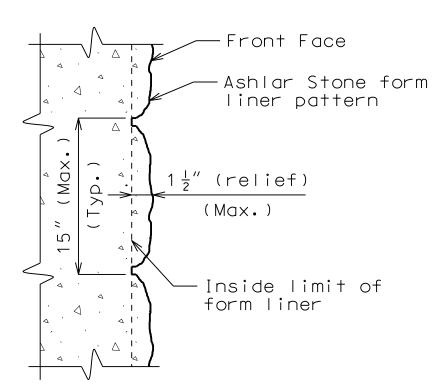
Rods or reinforcing bars shall be secured by an approved resin anchor system in accordance with Sec 1039.

Recess hole shall be backfilled with non-shrink cement grout.

- (1) Inverted U-shape reinforced capstone may be used in lieu of coping. Panel dowels for level-up concrete shall be required, and provided by manufacturer. The dowels shall be field trimmed to clear the capstone by a minimum of 1 1/2 inches and a maximum of 2 1/2 inches.
- (2) Topmost layer of reinforcement shall be fully covered with select granular backfill for structural systems, as approved by the wall manufacturer, before placement of the Separation Geotextile.
- (3) Minimum 6" diameter perforated PVC or PE pipe.

Manufacturer shall show drain details on design plans to be submitted as shown on MoDOT MSE wall plans and/or roadway plans.

Contractor shall modify the drain details as shown if it will improve flow as may be the case for stepped leveling pad, and for an uneven ground line (approval of the engineer required).



FORM LINER DETAIL (Large Block Wall)

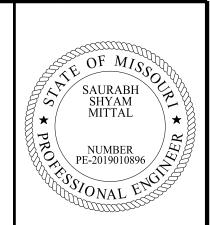
Notes:

The cost of form liners for MSE wall systems, complete in place, will be considered completely covered by the contract unit price for Mechanically Stabilized Earth Wall System.

Form liner shall be constructed in accordance with Special Provisions.

The following is a list of form liner manufacturers and types which may be used. Depth of relief for all form liner pattern's shall vary up to 1 1/2". The height of any single 'stone' shall be 15" maximum.

- Scott System, Inc.: Form liner pattern #167 "Ashlar Stone"
- Fitzgerald Formliners: Form liner pattern #16986 "Ashlar Stone"
- Greenstreak: Form liner pattern #330 "Ashlar Stone"
- Spec Formliners: Form liner pattern #1515 "Ashlar Stone"
- Customrock: Form liner pattern #12020 "Tollway Ashlar"
- An approved equal



DATE PREPARED 3/16/2022

I - 270DISTRICT

COUNTY ST. LOUIS CITY JOB NO. J6I3020C

PROJECT NO.

CONTRACT ID.

BRIDGE NO. A9002

AND TE

ERVIEW A9002 OF 10 $\overset{\wedge}{\square}$ \triangleleft VE > OO/R WAL **/** <| ⊢ -

CLIENT Homer & Shiftin, Inc.	M	ILLENNI	Millennia Professional Services				В	SOR	ING	i NU	JME		NE 1 0	
DATE STARTED 3/13/20 COMPLETED 3/13/20 GROUND LELVATION 439.3 ft HOLE SIZE Inches	PAC	PESSIONAL SERVI		PROJEC	T NAME	Inters	state 270 a	t River	view [Orive I	mprov	ement	S	
DRILLING METHOD_Hollow Stem Auper, No Rock Core DRILLING METHOD_Hollow Stem Auper, No Rock Core LOGGED BY F. Khan	PRO	OJECT N	IUMBER MG20007	PROJEC	T LOCAT	TION _	St. Louis, M	Missou	ri					
DRILLING METHOD Hollow Sigm Auger, NO Rock Core LOGGED BY E.Khan CHECKED BY Schaelfler AFEN OF DRILLING — not measured LATEN OF DRILLING — not measured — not measured LATEN OF DRILLING — not measured LATEN OF DRILLING — not measured LATEN OF DRILLING — not measured — not measured — not measured LATEN OF DRILLING — not measured — no	DA	TE STAF	RTED 3/13/20 COMPLETED 3/13/20	GROUNE	ELEVA	TION _	439.3 ft		HOLE	SIZE	inch	es		
DRILLING METHOD Hollow Sigm Auger, NO Rock Core LOGGED BY E.Khan CHECKED BY Schaelfler AFEN OF DRILLING — not measured LATEN OF DRILLING — not measured — not measured LATEN OF DRILLING — not measured LATEN OF DRILLING — not measured LATEN OF DRILLING — not measured — not measured — not measured LATEN OF DRILLING — not measured — no														
Concrete (10.07)								50 ft / F	-lev 4'	20 80 1	ft			
### AFTER DRILLING														
### A ST 100 18-18 100	2		T. Mian Cilconed Di 3. Schaener					ot mea	<u>isui eu</u>					
SS Concrete (10.0") A38.5 Fill. Black to grey. lean CLAY, with cinders, sand, and gravel SS G7 S-B-7 (15) 3.0 20	PI NO			АГ	IER DRI	LLING				1				
Concrete (10.0")	= □ ≥		MATERIAL DESCRIPTION		SAMPLE TYPE NUMBER	ECOVERY (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (tsf)	DRY UNIT WT. (pcf)	MOISTURE CONTENT (%)			ASTICITY NO INDEX	INES CONTENT (%)
FILL: Black to grey, lean CLAY, with cinders, sand, and gravel - undrained shear strength at 6.0 ft = 1.03 TSF - undrained shear strength at 6.0 ft = 1.03 TSF Brown, lean CLAY (CL), trace sand - grey, with sand below 13.5 ft - grey, with sand below 13.5 ft - undrained shear strength at 16.0 ft = 0.24 TSF - undrained shear strength at 16.0 ft = 1.03 TSF - undrained shear strength at 16.0 ft = 1.03 TSF - undrained shear strength at 16.0 ft = 1.03 TSF - undrained shear strength at 16.0 ft = 1.03 TSF - undrained shear strength at 16.0 ft = 1.03 TSF - undrained shear strength at 16.0 ft = 1.03 TSF - undrained shear strength at 16.0 ft = 1.03 TSF - undrained shear strength at 16.0 ft = 1.03 TSF - undrained shear strength at 16.0 ft = 1.03 TSF - undrained shear strength at 16.0 ft = 1.03 TSF - undrained shear strength at 16.	5 0	PA	Concrete (10.0")	400.5										
- undrained shear strength at 6.0 ft = 1.03 TSF - undrained shear strength at 6.0 ft = 1.03 TSF - undrained shear strength at 1.00 ft = 1.03 TSF - grey, with sand below 13.5 ft - grey, with sand below 13.5 ft - undrained shear strength at 16.0 ft = 0.24 TSF - undrained shear strength at 16														
- undrained shear strength at 6.0 ft = 1.03 TSF - undrained shear strength at 6.0 ft = 1.03 TSF - undrained shear strength at 16.0 ft = 0.24 TSF - grey, with sand below 13.5 ft - undrained shear strength at 16.0 ft = 0.24 TSF - undrained she	2		TILL. Diack to grey, lear of AT, with chiacis, sand, and g	iavoi										
- undrained shear strength at 6.0 ft = 1.03 TSF - undrained shear strength at 6.0 ft = 1.03 TSF - undrained shear strength at 16.0 ft = 0.24 TSF - grey, with sand below 13.5 ft - undrained shear strength at 16.0 ft = 0.24 TSF - undrained she														
- undrained shear strength at 6.0 ft = 1.03 TSF - undrained shear strength at 6.0 ft = 1.03 TSF - undrained shear strength at 16.0 ft = 0.24 TSF - grey, with sand below 13.5 ft - undrained shear strength at 16.0 ft = 0.24 TSF - undrained she	2 -	-												
- undrained shear strength at 6.0 ft = 1.03 TSF Brown, lean CLAY (CL), trace sand - grey, with sand below 13.5 ft - grey, with sand below 13.5 ft - undrained shear strength at 16.0 ft = 0.24 TSF - undrained shear strength at 16.0 ft = 0.24 TSF - undrained shear strength at 16.0 ft = 0.24 TSF ST	<u>{</u>	-			√ ss	67	8-8-7	2.0		20				
Brown, lean CLAY (CL), trace sand 10	5				1	67	(15)	3.0		20				
Brown, lean CLAY (CL), trace sand 10														
Brown, lean CLAY (CL), trace sand 10			- undrained shear strength at 6.0 ft = 1.03 TSF											
Brown, lean CLAY (CL), trace sand 431.05 - grey, with sand below 13.5 ft - grey, with sand below 13.5 ft - undrained shear strength at 16.0 ft = 0.24 TSF - undrained shear strength at 16.0 ft = 0.24 TSF - undrained shear strength at 16.0 ft = 0.24 TSF ST	<u> </u>	-	and an			83			99	18				
Brown, lean CLAY (CL), trace sand SS 100 3-2-4 1.75 18				<i>4</i> 31 05	2									
10 SS 100 3-2-4 1.75 18 - grey, with sand below 13.5 ft - undrained shear strength at 16.0 ft = 0.24 TSF - undrained shear strength at 16.0 ft = 0.24 TSF - undrained shear strength at 16.0 ft = 0.24 TSF ST 100 1.0 98 26 ✓ SS 100 2-4-6 0.5 - orange-brown, with sand, trace gravel below 19.8 ft SHALE: Grey, highly weathered - near vertical fracture at 26.0 ft SHALE: Grey, worderately hard, thin-bedded, slightly weathered - near vertical fracture at 26.0 ft SHALE: Grey, worderately hard, thin-bedded, slightly weathered - near vertical fracture at 26.0 ft SHALE: Grey, very soft to soft, moderately to highly weathered - near vertical fracture at 26.0 ft SHALE: Grey, very soft to soft, moderately to highly weathered - near vertical fracture at 26.0 ft Grey, very soft to soft, moderately to highly weathered - near vertical fracture at 26.0 ft Grey, very soft to soft, moderately to highly weathered - near vertical fracture at 26.0 ft Grey, very soft to soft, moderately to highly weathered - near vertical fracture at 26.0 ft Grey, very soft to soft, moderately to highly weathered - near vertical fracture at 26.0 ft - near vertical fracture at 26.0	_ □		Brown, lean CLAY (CL), trace sand	431.03_	1									
- grey, with sand below 13.5 ft - undrained shear strength at 16.0 ft = 0.24 TSF - undrained shear strength at 16.0 ft = 0.24 TSF - undrained shear strength at 16.0 ft = 0.24 TSF ST 100 1.0 98 26 SS 100 2.4-6 (10) 0.5 24 - orange-brown, with sand, trace gravel below 19.8 ft SHALE: Grey, highly weathered - the strength at 16.0 ft = 0.24 TSF ST 100 1.0 98 26 SS 100 18-18-7 100 18	<u> </u>	-////	Brown, lear GE/TT (GE), trade saria		\bigvee ss	100		1.75		18				
- undrained shear strength at 16.0 ft = 0.24 TSF - undrained shear strength at 16.0 ft = 0.24 TSF - undrained shear strength at 16.0 ft = 0.24 TSF ST	10				/\ 3		(6)	•	,					
- undrained shear strength at 16.0 ft = 0.24 TSF - undrained shear strength at 16.0 ft = 0.24 TSF - undrained shear strength at 16.0 ft = 0.24 TSF ST												· `		
- undrained shear strength at 16.0 ft = 0.24 TSF - undrained shear strength at 16.0 ft = 0.24 TSF - undrained shear strength at 16.0 ft = 0.24 TSF ST														
- undrained shear strength at 16.0 ft = 0.24 TSF - undrained shear strength at 16.0 ft = 0.24 TSF - undrained shear strength at 16.0 ft = 0.24 TSF ST	? !	-////												
- undrained shear strength at 16.0 ft = 0.24 TSF - undrained shear strength at 16.0 ft = 0.24 TSF - undrained shear strength at 16.0 ft = 0.24 TSF ST	<u>-</u> 2 -	-////												
- undrained shear strength at 16.0 ft = 0.24 TSF - undrained shear strength at 16.0 ft = 0.24 TSF - undrained shear strength at 16.0 ft = 0.24 TSF ST	920		- grev with sand below 13.5 ft		1 00		0.45							
- undrained shear strength at 16.0 ft = 0.24 TSF - undrained shear strength at 16.0 ft = 0.24 TSF - undrained shear strength at 16.0 ft = 0.24 TSF ST 100 1.0 98 26 SS 100 2-4-6 (10) 0.5 24 - orange-brown, with sand, trace gravel below 19.8 ft SHALE: Grey, highly weathered - forey, moderately hard, thin-bedded, slightly weathered - near vertical fracture at 26.0 ft SHALE: Grey, very soft to soft, moderately to highly weathered - harder below 28.0 ft - fractured from 28.0-29.0 ft		. *////	groy, with band bolow 10.0 it		\bigvee_{4}	100		1.0		25				
20 - orange-brown, with sand, trace gravel below 19.8 ft - orange-brown, with sand, trace gravel below 19.8 ft SHALE: Grey, highly weathered - near vertical fracture at 26.0 ft SHALE: Grey, very soft to soft, moderately to highly weathered - harder below 28.0 ft - fractured from 28.0-29.0 ft	15						(-)				-			
20 - orange-brown, with sand, trace gravel below 19.8 ft - orange-brown, with sand, trace gravel below 19.8 ft SHALE: Grey, highly weathered - near vertical fracture at 26.0 ft SHALE: Grey, very soft to soft, moderately to highly weathered - harder below 28.0 ft - fractured from 28.0-29.0 ft		-////												
- orange-brown, with sand, trace gravel below 19.8 ft SHALE: Grey, highly weathered LIMESTONE: Grey, moderately hard, thin-bedded, slightly weathered - near vertical fracture at 26.0 ft SHALE: Grey, very soft to soft, moderately to highly weathered - harder below 28.0 ft - fractured from 28.0-29.0 ft			- undrained shear strength at 16.0 ft = 0.24 TSF		ST	400			00	00				
- orange-brown, with sand, trace gravel below 19.8 ft SHALE: Grey, highly weathered LIMESTONE: Grey, moderately hard, thin-bedded, slightly weathered - near vertical fracture at 26.0 ft SHALE: Grey, very soft to soft, moderately to highly weathered - harder below 28.0 ft - fractured from 28.0-29.0 ft						100		1.0	98	26				
- orange-brown, with sand, trace gravel below 19.8 ft SHALE: Grey, highly weathered LIMESTONE: Grey, moderately hard, thin-bedded, slightly weathered - near vertical fracture at 26.0 ft SHALE: Grey, very soft to soft, moderately to highly weathered - harder below 28.0 ft - fractured from 28.0-29.0 ft	<u>-</u>	<i>\\\\\\</i>	abla											
- orange-brown, with sand, trace gravel below 19.8 ft SHALE: Grey, highly weathered LIMESTONE: Grey, moderately hard, thin-bedded, slightly weathered - near vertical fracture at 26.0 ft SHALE: Grey, very soft to soft, moderately to highly weathered - harder below 28.0 ft - fractured from 28.0-29.0 ft	;	-////	<u>+</u>		√ ss	100	2-4-6	0.5		24				
SHALE: Grey, highly weathered LIMESTONE: Grey, moderately hard, thin-bedded, slightly weathered - near vertical fracture at 26.0 ft SHALE: Grey, very soft to soft, moderately to highly weathered - harder below 28.0 ft - fractured from 28.0-29.0 ft	20				6	100	(10)	0.5		24				
SHALE: Grey, highly weathered LIMESTONE: Grey, moderately hard, thin-bedded, slightly weathered - near vertical fracture at 26.0 ft SHALE: Grey, very soft to soft, moderately to highly weathered - harder below 28.0 ft - fractured from 28.0-29.0 ft	7/1		- orange-brown, with sand, trace gravel below 19.8 ft		•									
SHALE: Grey, highly weathered LIMESTONE: Grey, moderately hard, thin-bedded, slightly weathered - near vertical fracture at 26.0 ft SHALE: Grey, very soft to soft, moderately to highly weathered - harder below 28.0 ft - fractured from 28.0-29.0 ft	<u>7</u> -	-////												
Grey, highly weathered SS 100 18-18- 50/5" LIMESTONE: Grey, moderately hard, thin-bedded, slightly weathered - near vertical fracture at 26.0 ft SHALE: Grey, very soft to soft, moderately to highly weathered - harder below 28.0 ft - fractured from 28.0-29.0 ft	- -			417.3										
LIMESTONE: Grey, moderately hard, thin-bedded, slightly weathered - near vertical fracture at 26.0 ft SHALE: Grey, very soft to soft, moderately to highly weathered - harder below 28.0 ft - fractured from 28.0-29.0 ft	Ŕ													
LIMESTONE: Grey, moderately hard, thin-bedded, slightly weathered - near vertical fracture at 26.0 ft SHALE: Grey, very soft to soft, moderately to highly weathered - harder below 28.0 ft - fractured from 28.0-29.0 ft	3				1			-						
LIMESTONE: Grey, moderately hard, thin-bedded, slightly weathered - near vertical fracture at 26.0 ft SHALE: Grey, very soft to soft, moderately to highly weathered - harder below 28.0 ft - fractured from 28.0-29.0 ft	<u>}</u>					100				48				
LIMESTONE: Grey, moderately hard, thin-bedded, slightly weathered - near vertical fracture at 26.0 ft SHALE: Grey, very soft to soft, moderately to highly weathered - harder below 28.0 ft - fractured from 28.0-29.0 ft	25	5		413.8	/ \		30/3	-			-			
Grey, moderately hard, thin-bedded, slightly weathered - near vertical fracture at 26.0 ft SHALE: Grey, very soft to soft, moderately to highly weathered - harder below 28.0 ft - fractured from 28.0-29.0 ft	2 2		LIMESTONE:											
SHALE: Grey, very soft to soft, moderately to highly weathered - harder below 28.0 ft - fractured from 28.0-29.0 ft			Grey, moderately hard, thin-bedded, slightly weathered	412.8										
Grey, very soft to soft, moderately to highly weathered - harder below 28.0 ft - fractured from 28.0-29.0 ft	<u> </u>				RC									
- harder below 28.0 ft - fractured from 28.0-29.0 ft	<u>-</u>		Grey, very soft to soft, moderately to highly weathered		1									
			- harder below 28.0 ft											
	30		- tractured from 28.0-29.0 ft - soft, highly weathered from 29.5-30.0 ft											

CLIENT Homer & Shiffin, Inc. PROJECT NUMBER MG20007 PROJECT LOCATION St. Louis, Missouri PROJECT LOCATION St. Louis, Missouri PROJECT NUMBER MG20007 MATERIAL DESCRIPTION SHALE: Grey, very soft to soft, moderately to highly weathered (continued) - rock core qu at 30.3 ft = 3.590 psi - calcareous from 30.0-33.0 ft LIMESTONE: Arailaceous, greenish grey, soft to moderately hard, banded to medium-bedded, slightly weathered Refusal at 25.0 feet. Bottom of borehole at 40.0 feet.	R NB-7 SE 2 OF 2
PROJECT NUMBER MG20007 PROJECT LOCATION St. Louis, Missouri MATERIAL DESCRIPTION MATERIAL DESCRIPTION St. Louis, Missouri ATTERIAL DESCRIPTION MATERIAL DESCRIPTION St. Louis, Missouri ATTERIAL DESCRIPTION MATERIAL DESCRIPTION St. Louis, Missouri ATTERIAL DESCRIPTION St. Louis, Missouri ATTERIAL DESCRIPTION St. Louis, Missouri ATTERIAL DESCRIPTION RC (30) AND MO	nts
MATERIAL DESCRIPTION SHALE: Grey, very soft to soft, moderately to highly weathered (continued) - rock core qu at 30.3 ft = 3,890 psi - calcareous from 30.0-33.0 ft LIMESTONE: Argillaceous, greenish grey, soft to moderately hard, banded to medium-bedded, slightly weathered RC 100 3 10	
Grey, very soft to soft, moderately to highly weathered (continued) - rock core qu at 30.3 ft = 3,890 psi - calcareous from 30.0-33.0 ft LIMESTONE: Argillaceous, greenish grey, soft to moderately hard, banded to medium-bedded, slightly weathered RC 100 2 (100) RC 100 3 (57)	
3 (57) 399.3 Refusal at 25.0 feet.	

HIGHWAYS AND TRA COMMISSION

I-270/RIVERVIEW MSE WALL A9002 SHEET 5 OF 10

I-270 AND RIVERVIEW

SAURABH SHYAM MITTAL

NUMBER PE-2019010896

DATE PREPARED 3/16/2022

DISTRICT SHEET NO.

BR 5

COUNTY ST. LOUIS CITY JOB NO. J6I3O2OC

CONTRACT ID.

PROJECT NO.

BRIDGE NO.

Note: This drawing is not to scale. Follow dimensions.

(Continued Next Page)

MIL	LLENNIZ ESENCIVAL SEPTIME	Millennia Professional Services				В	OR	ING	NU	JME		NB 1 0	
CLIE	NT Ho	orner & Shifrin, Inc.	PROJEC [*]	T NAME	Inters	state 270 a	t River	view D	Orive Ir	nprov	ement	S	
PRO	JECT N	UMBER MG20007	PROJEC	T LOCAT	ION _	St. Louis, N	/lissou	ri					
DAT	E STAR	TED 3/12/20 COMPLETED 3/12/20	GROUND	ELEVA	TION _	438.9 ft		HOLE	SIZE	inch	es		
DRIL	LING C	ONTRACTOR Bulldog, CME550X, Efficiency: 95%	GROUND	WATER	LEVE	LS:							
DRIL	LING N	IETHOD Hollow Stem Auger	$oxed{oxtime}$ at	TIME OF	DRIL	L ING 18.5	60 ft / E	Elev 42	20.40 1	ft			
LOG	GED B	F. Khan CHECKED BY J. Schaeffer				.ING no							
ੁ ਲ NOT	ES 4.0	ft west of design due to location of marked utilities	AF	TER DRII	LLING								
				111			_				ERBE		╘
RIVE INTERCHANGE DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION		SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN (tsf)	DRY UNIT WT (pcf)	MOISTURE CONTENT (%)	LIQUID	PLASTIC WI	PLASTICITY NO INDEX	FINES CONTENT (%)
	0 4 7	Concrete (10.0")	438.1										
A\\270 AT RIVERVIE	-	FILL: Brown to grey, lean CLAY (CL), with sand, trace slag		\ <i>I</i>									
DAT			434.15	SS 1	67	5-7-11 (18)	3.0		15				
	-///	Black to brown, hard, lean to fat CLAY (CL-CH)		/ \ '		(10)							
Stylen St		- undrained shear strength at 8.0 ft = 0.93 TSF	427.9	ST 2	58		2.25	104	20				
FILES\2020\MG20007 I-270 RIV	- - - -	Grey to brown, lean CLAY (CL), trace sand and pockets o - trace organics below 13.5 ft	f fat clay	SS 3	100	3-3-3 (6)	1.75		25				
T - 12/17/20 11:22 - G:\PROJECT	- - - - -	 	416.9	SS 4	89	4-5-6 (11)			21				
GEOTECH BH COLUMNS - GINT STD US LAB.GDT - 12/17/20 11:22 - G		Brown to orange, clayey SAND (SC), trace gravel	410.9	SS 5	72	3-3-5 (8)			34				
30 30		SHALE: Light grey, highly weathered	409.9	SS 6	100	20-23-18 (41)			18				

	MILI	LENNIA	Millennia Professional Services				E	BOR	ING	NU		BER PAGE		
	CLIEN	NT Ho	orner & Shifrin, Inc.	PROJECT	NAME _.	Inters	tate 270 a	at River	view D	rive In	nprove	ements	i	
	PROJ	ECT N	UMBER MG20007	PROJECT	LOCAT	ION _	St. Louis,	Missou	ri					
vT.GPJ	S DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION		SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (tsf)	DRY UNIT WT. (pcf)	MOISTURE CONTENT (%)	LIQUID	PLASTIC FIMIT LIMIT	PLASTICITY B	FINES CONTENT (%)
EW DRIVE INTERCHANGE GINT.GPJ			SHALE: Light grey, highly weathered (continued) Refusal at 33.5 feet. Bottom of borehole at 33.8 feet.	405.15	SS 7	100	50/2"			10				
ELD DATA\\270 AT RIVERVIEW														
- G:\PROJECT FILES\2020\MG20007 I-270 RIVERVIEW DRIVE INTERCHANGE\FIELD DATA\1270 AT														
MMG20007 I-270 RIVERVIEV														
IS LAB.GDT - 12/17/20 11:22														
ECH BH COLUMNS - GINT STD US LAB.GDT														
ECH														



DATE PREPARED									
3/16/2022									
ROUTE	STATE								
I -270	MO								
DISTRICT	SHEET NO.								
BR	6								
COU									

ST. LOUIS CITY
JOB NO.
J6I3020C CONTRACT ID.

PROJECT NO. BRIDGE NO.

HIGHWAYS AND TRAI COMMISSION

I-270/RIVERVIEW MSE WALL A9002 SHEET 6 OF 10 I-270 AND Riverview

Sheet No. 6 of 10

Designed: JAA Detailed: JAA Checked: JEK

(Continued Next Page)

MILL	ENNI.	Millennia Professional Services				В	OR	ING	NU	JME		NE 1 0	
CLIEN	IT Ho	orner & Shifrin, Inc.	PROJEC [*]	T NAME	Inters	state 270 a	t Rive	rview [Orive I	mprov	ement	S	
		IUMBER MG20007				St. Louis, N				·			
DATE	STAR	RTED 3/23/20 COMPLETED 3/23/20	_ GROUND	ELEVA	TION _	439 ft		HOLE	SIZE	_inch	es		
DRILL	ING C	CONTRACTOR Bulldog, CME75LC, Efficiency: 94%	_ GROUND	WATER	LEVE	LS:							
DRILL	ING N	METHOD Hollow Stem Auger	_ ∑ A T	TIME OF	DRIL	LING _19.0	00 ft / I	Elev 4	20.00	ft			
LOGG	ED B	Y B.Fisher CHECKED BY J. Schaeffer	_ AT	END OF	DRILL	.ING no	ot mea	sured					
NOTE	s		_ A F	TER DRI	LLING	_15.50 ft /	Elev	423.50) ft				
				E E	%		j	<u> </u> -	(6)		ERBE		N
DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION		SAMPLE TYP NUMBER	RECOVERY (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (tsf)	DRY UNIT WT (pcf)	MOISTURE CONTENT (%)	LIQUID	PLASTIC LIMIT	PLASTICITY INDEX	FINES CONTENT (%)
0	P 15 A	Concrete (10.0")	139.2									Г.	ш.
 		Brown to grey, lean CLAY (CL), with silt	438.2										
<u> </u>													
_		- undrained shear strength at 3.0 ft = 0.67 TSF		ST	92		>4.5	98	25	42	20	22	
5				1									
		- undrained shear strength at 8.0 ft = 1.02 TSF - dark grey below 9.0 ft	407.5	ST 2	92		3.5	102	19				
_		Brown to grey, clayey SILT (ML), trace sand	427.5										
 		- undrained shear strength at 13.0 ft = 0.67 TSF		ST 3	83		2.0	102	22				
		$ar{ar{m{\Lambda}}}$											
- - 		Orange, fat CLAY (CH), with silt and sand	422.5										
20		Orange clayey SAND (SC), with silt and gravel	419.5	SS 4	100	5-4-6 (10)			25				
		Orange to grey, shaly, fat CLAY (CH) SHALE: Dark grey, weathered	417										
				1 00		44.05.01	-			-			
25				SS 5	33	14-25-31 (56)	_		13				
<u> </u>		- dark grey to light grey below 28.0 ft		1.00			-						
			409.6	X ss $_{6}$	91	13-50/5"			11				
		Refusal at 29.4 feet. Bottom of borehole at 29.4 feet.			-								

DATE PREPARED

3/16/2022

ROUTE STATE

I -270 MO

DISTRICT SHEET NO

BR 7 ST. LOUIS CITY

JOB NO.

J6 I 3020C

CONTRACT ID. PROJECT NO. BRIDGE NO.

I-270/RIVERVIEW MSE WALL A9002 SHEET 7 OF 10 I-270 AND RIVERVIEW

Sheet No. 7 of 10

MIL	LENNI	Millennia Professional Services				В	SOR	ING) N	JME		SE 1 0			
PROFES	SIONAL SERVIC		ROJEC	T NAME	Inter	state 270 a	t River	view C	Orive I	mprov	ement	's			
									<u> </u>	Πρισν	CITICITO	.5			
	DATE STARTED 3/17/20 COMPLETED 3/17/20			PROJECT LOCATION St. Louis, Missouri GROUND ELEVATION 438.7 ft HOLE SIZE inches											
	DRILLING CONTRACTOR Bulldog, CME75LC, Efficiency: 94%														
		IETHOD Hollow Stem Auger, NQ Rock Core				LING <u>19.0</u>)() ft / F	=lov 4 ⁻	19 7 0 -	f t					
		B.Fisher CHECKED BY J. Schaeffer													
NOTI		of the structure of the		TER DRI			ot moe	<u>isarca</u>							
1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1				ATTERBERG -											
HIVERVIEW DRIVE INTERCHANGE GINT.GPJ DEPTH O (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION		SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (tsf)	DRY UNIT WT. (pcf)	MOISTURE CONTENT (%)		PLASTIC WIT	S ≻	FINES CONTENT (%)		
M M M	P & 4	Concrete (10.0")	437.9												
		FILL: Brown to grey, lean CLAY, with silt, trace organics and gravel		SS 1	83	4-7-8 (15)	4.5		13						
0/2		Blue-grey, fat CLAY (CH), with silt	435.7												
			422.2	SS 2	100	7-6-9 (15)	3.5		23	52	19	33			
∐		 - dark brownish grey silt layers observed at 5 ft and 6 ft (Possorganic silt) 	sfbły <u>∠</u> /												
		Brown and grey, lean CLAY (CL)	_ ′	SS 3	89	3-4-8 (12)	3.75		16						
				SS 4	89	3-2-3 (5)	2.5		19						
SPROJECT FILESZOZOWIGZOUOV 1-270 RIVERVIEW DRIVE IN TERCHANGENFIELD DATANIZYO AT THE STATE OF THE	-	- with sand below 13.5 ft													
15		- undrained shear strength at 14.0 ft = 0.44 TSF		ST 5	100		2.5	100	22						
		- brown-orange, with sand lenses below 18.0 ft													
20				SS 6	89	3-4-9 (13)	2.75		15						
LAB.GDI - 12/2		Brown, clayey SAND (SC)	416.7												
25				SS 7	100	8-8-12 (20)	-		37						
36 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 -		Dark grey to blue-grey, shaly, fat CLAY (CH)	<u>411.7</u>												
30				SS 8	83	16-25-24 (49)	4.5		12						

(Continued Next Page)

BORING NUMBER SB-8 PAGE 2 OF 2 Millennia Professional Services PROJECT NAME Interstate 270 at Riverview Drive Improvements CLIENT Horner & Shifrin, Inc. PROJECT LOCATION St. Louis, Missouri PROJECT NUMBER MG20007 **ATTER**BERG MATERIAL DESCRIPTION Dark grey to blue-grey, shaly, fat CLAY (CH) (continued) SS 100 50/3" 9 100 RC (81) 8 LIMESTONE: Greenish grey, argillaceous, soft to moderately hard, thin to medium-bedded, slightly weathered, with shale inclusions - rock core qu at 36.5 ft = 5,840 psi RC | 100 2 (100) RC 100 SHALE: 3 (71) Grey, soft to moderately hard - rock core qu at 42.7 ft = 5,790 psi RC 96 4 (93) 394.4 LIMESTONE: White to grey, moderately hard to hard, thin to medium-bedded, slightly weathered - interbedded with shale from 44.3-44.9 ft - shale partings at 45.0 and 46.2 ft RC 100 5 (75) 1.5" chert seam at 47.0 ft - stylolites at 47.6, 47.7, 48.0, and 48.4 ft 389.2 Refusal at 33.8 feet. Bottom of borehole at 49.5 feet.

SAURABH SHYAM MITTAL NUMBL. PE-2019010896

> DATE PREPARED 3/16/2022 ROUTE I -270 DISTRICT COUNTY ST. LOUIS CITY

JOB NO. J6I3020C CONTRACT ID.

PROJECT NO. BRIDGE NO. A9002

WAYS AND TRAI COMMISSION

I-270/RIVERVIEW MSE WALL A9002 SHEET 8 OF 10 I-270 AND RIVERVIEW

MIL	LENNIL BODIAL SERVICE	Millennia Professional Services				В	BOR	ING	S NU	JME		2 SB 1 0				
PAOFESE	SIONAL SERVIC	orner & Shifrin, Inc.	PROJEC	PROJECT NAME _Interstate 270 at Riverview Drive Improvements												
PROJ	ECT N	UMBER MG20007	PROJECT LOCATION St. Louis, Missouri													
DATE	DATE STARTED _3/13/20 COMPLETED _3/13/20															
DRILL	ING C	ONTRACTOR Bulldog, CME550X, Efficiency: 95%														
DRILL	ING N	IETHOD Hollow Stem Auger	AT TIME OF DRILLING _28.50 ft / Elev 410.10 ft AT END OF DRILLING _27.00 ft / Elev 411.60 ft													
LOGG	SED B	F. Khan CHECKED BY J. Schaeffer	▼ AT	END OF	DRILL	ING <u>27.0</u>	0 ft / E	lev 41	l1.60 f	<u>t</u>						
NOTE	S		AF	TER DRI	LLING					•						
O DEPTH	GRAPHIC LOG	MATERIAL DESCRIPTION		SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (tsf)	DRY UNIT WT. (pcf)	MOISTURE CONTENT (%)		PLASTIC HIMIT LIMIT		FINES CONTENT (%)			
	P 4 4	Concrete (10.0")	437.8													
 		POSSIBLE FILL: Brown and grey, lean CLAY (CL)														
5		POSSIBLE FILL: Dark grey, lean CLAY (CL), trace pock	433.1_	SS 1	72	6-6-6 (12)	2.5		22							
		clay, trace organics - undrained shear strength at 6.0 ft = 1.48 TSF		ST 2	42		1.5	103	22							
10				SS 3	89	3-3-3 (6)	1.5		11							
		Brown and grey, lean CLAY (CL)	426.6_													
 15				SS 4	100	2-3-3 (6)	2.0		29							
		Brown to orange, lean CLAY (CL), with sand, trace gravel	422.1													
20			417.1	SS 5	89	2-7-6 (13)			15							
		SHALE: Tan to yellow-brown, weathered, clayey		90		0 0 0	_									
25				SS 6	72	8-8-9 (17)	_		12							
30		- grey below 28.5 ft		SS 7	100	11-20-26 (46)	-		14							

(Continued Next Page)

BORING NUMBER SB-9 PAGE 2 OF 2 Millennia Professional Services CLIENT Horner & Shifrin, Inc. PROJECT NAME Interstate 270 at Riverview Drive Improvements PROJECT LOCATION St. Louis, Missouri PROJECT NUMBER MG20007 MATERIAL DESCRIPTION SHALE: Tan to yellow-brown, weathered, clayey (continued) SS 8 100 10-21-36 (57) 399.8 SS 100 50/3" Refusal at 38.5 feet. Bottom of borehole at 38.8 feet.

OF MISS
SAURABH
SHYAM
MITTAL

DATE PREPARED 3/16/2022 I -270 DISTRICT

ST. LOUIS CITY JOB NO. J6I3020C CONTRACT ID.

PROJECT NO. BRIDGE NO. A9002

HIGHWAYS AND TRA COMMISSION

I-270/RIVERVIEW MSE WALL A9002 SHEET 9 OF 10

I-270 AND RIVERVIEW

MIL	LENNIL GEFFU	Millennia Professional Services				В	DRII	NG	NUI	MBI		SB- E 1 C					
CLIEN	IT H	orner & Shifrin, Inc.	PROJEC	T NAME	Inter	state 270 a	t River	view [Orive II	mprov	ement	s					
	PROJECT NUMBER MG20007				PROJECT LOCATION St. Louis, Missouri												
	DATE STARTED 3/17/20 COMPLETED 3/17/20																
	DRILLING CONTRACTOR Bulldog, CME550X, Efficiency: 95%																
		IETHOD Hollow Stem Auger, NQ Rock Core	$ar{igstyle}$ AT	TIME OF	DRIL	LING <u>18.5</u>	50 ft / I	Elev 4	20.00	ft							
		Y F. Khan CHECKED BY J. Schaeffer				 _ING ne											
?) ft west of design due to location of marked utilities		TER DRI													
												TERBERG					
DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION		SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (tsf)	DRY UNIT WT (pcf)	MOISTURE CONTENT (%)	LIQUID	PLASTIC WIT	PLASTICITY INDEX	FINES CONTENT (%)				
i	P	Concrete (10.0")	437.7														
		Grey, lean CLAY (CL), with pockets of fat clay	107.7														
5				SS 1	67	5-4-5 (9)	2.75	-	21								
				SS 2	61	2-2-3 (5)	2.0		22								
10		- trace sand below 9.0 ft		ST 3	100		2.25	102	17								
15		- grey to brown below 13.5 ft		SS 4	100	2-3-4 (7)	1.25	_	25								
		☑ Brown to grey, lean CLAY (CL), pockets of fat clay	420.5														
20		-with sand below 19.0 feet.		SS 5	67	1-3-2 (5)	1.5		27								
			414.8														
25		SHALE: Grey, weathered		SS 6	94	7-14-15 (29)	_		12								
		Brownish-grey, fat CLAY (CH)	411_	1 00		0.0.4		 									
30				SS 7	100	2-3-4 (7)	1.25		19								

(Continued Next Page)

BORING NUMBER SB-10 PAGE 2 OF 2 Millennia Professional Services PROJECT NAME Interstate 270 at Riverview Drive Improvements CLIENT Horner & Shifrin, Inc. PROJECT LOCATION St. Louis, Missouri PROJECT NUMBER MG20007 MATERIAL DESCRIPTION Brownish-grey, fat CLAY (CH) (continued) 404.5 SS 100 23-50/5" 10 SHALE: Grey, soft 80 (72) - rock core qu at 37.0 ft = 1,550 psi - calcareous from 37.5-39.0 ft RC RC 88 2 (47) - very soft below 42.0 ft - blue-grey below 44.0 ft 392.5 RC 95 LIMESTONE: 3 (90) Greyish-white, moderately hard to hard, thin to medium bedded, with shale seams - healed vertical fracture from 48.5-49.0 ft 389.5 Refusal at 34.0 feet. Bottom of borehole at 49.0 feet.

BORING DATA
Note: For locations of borings, see Sheet No. 1.

DATE PREPARED 3/16/2022 I -270 DISTRICT

COUNTY ST. LOUIS CITY JOB NO. J6I3020C CONTRACT ID.

BRIDGE NO.

PROJECT NO.

A9002

WAYS AND TRAI COMMISSION

I-270/RIVERVIEW MSE WALL A9002 SHEET 10 OF 10 I-270 AND RIVERVIEW