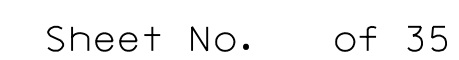


The Commission does not represent or warrant that any such boring data accurately depicts the conditions to be encountered in constructing this project. A contractor assumes all risks it may encounter in basing its bid prices, time or schedule of performance on the boring data depicted here or those available from the district, or on any other documentation not expressly warranted, which the contractor may obtain from the Commission.

STA. 90+00.00 RETAINING WALL A9001 =
STA. 12+06.66 RIVERVIEW DR. NORTHBOUND
33.04' RT
BEGIN WALL



...\\001_B_A9002_01_J6I3020C_I50_I270.dgn 3:50:08 PM 3/16/2022

253' MECHANICALLY STABILIZED EARTH (MSE) RETAINING WALL SYSTEM

General Notes:

Design Specifications:

2002 AASHTO LFD (17th Ed.) Standard Specifications (Section 5, ASD Design)
Seismic Performance Category B
Acceleration Coefficient = 0.12

Design Loading:

$\phi_D = 25^\circ$ and Unit weight, $\gamma_D = 130$ pcf for retained backfill material to be retained by the mechanically stabilized earth wall system.

$\phi_F = 25^\circ$ for unimproved foundation ground where wall is to bear.

$\phi_F = 34^\circ$ for improved foundation ground where wall is to bear.

The allowable bearing pressure for unimproved foundation ground 3.4 ksf.
The allowable bearing pressure for improved foundation ground 4.3 ksf.

The maximum applied bearing pressure for the controlling design case at the foundation level shall be shown on the shop drawings and shall be less than the allowable bearing pressure for foundation ground and provided herein. For seismic design the maximum applied bearing pressure shall be less than two times the allowable bearing pressure.

Allowable bearing pressure and limits of improved foundation ground shall not be adjusted from that as shown on the plans.

Contractor shall include ϕ_r (actual $\phi \geq 34^\circ$) and the total unit weight γ_r , for the select granular backfill (reinforced backfill and wedge area backfill) for structural systems on shop drawings. Contractor shall identify source of select granular backfill material, submit proctor in accordance with AASHTO T 99 (ASTM D698) and gradation with the shop drawings. When backfill material is too coarse to develop a proctor curve the contractor shall determine the maximum dry density (relative density) in accordance with ASTM D4253 and ASTM D4254 and assume percent passing the 200 sieve for optimum water content.

Total unit weight, $\gamma_r = (95\% \text{ compaction}) \times (\text{maximum dry density}) \times (1 + \text{optimum water content})$

Design $\phi_r = 34^\circ$ for the select granular backfill (reinforced backfill) only for structural systems.

Factor of safety shall be 2.0 for overturning and 1.5 for sliding.

For seismic design the factor of safety shall be 1.5 for overturning and 1.1 for sliding.

Use default values for the pullout friction factor, F^* , in accordance with AASHTO figure 5.8.5.2A, unless MoDOT has pre-approved a higher value for a specific product. For approved steel strips not shown in AASHTO figure 5.8.5.2A, use $F^* \leq 2.0$ at zero depth and $F^* \leq \tan \phi_r$ at 20 feet depth and ϕ_r design = 34° . F^* values shall be shown on the shop drawings.

Design Unit Stresses:

All concrete for leveling pad and coping shall be Class B or B-1 with $f'c = 4000$ psi.

The minimum compressive strength of concrete for precast panel shall be 4,000 psi in accordance with Sec 1052.

Miscellaneous:

The MSE wall system shall be built vertical.

The MSE wall system shall be built in accordance with Sec 720.

The MSE wall system shall be a large block wall system.

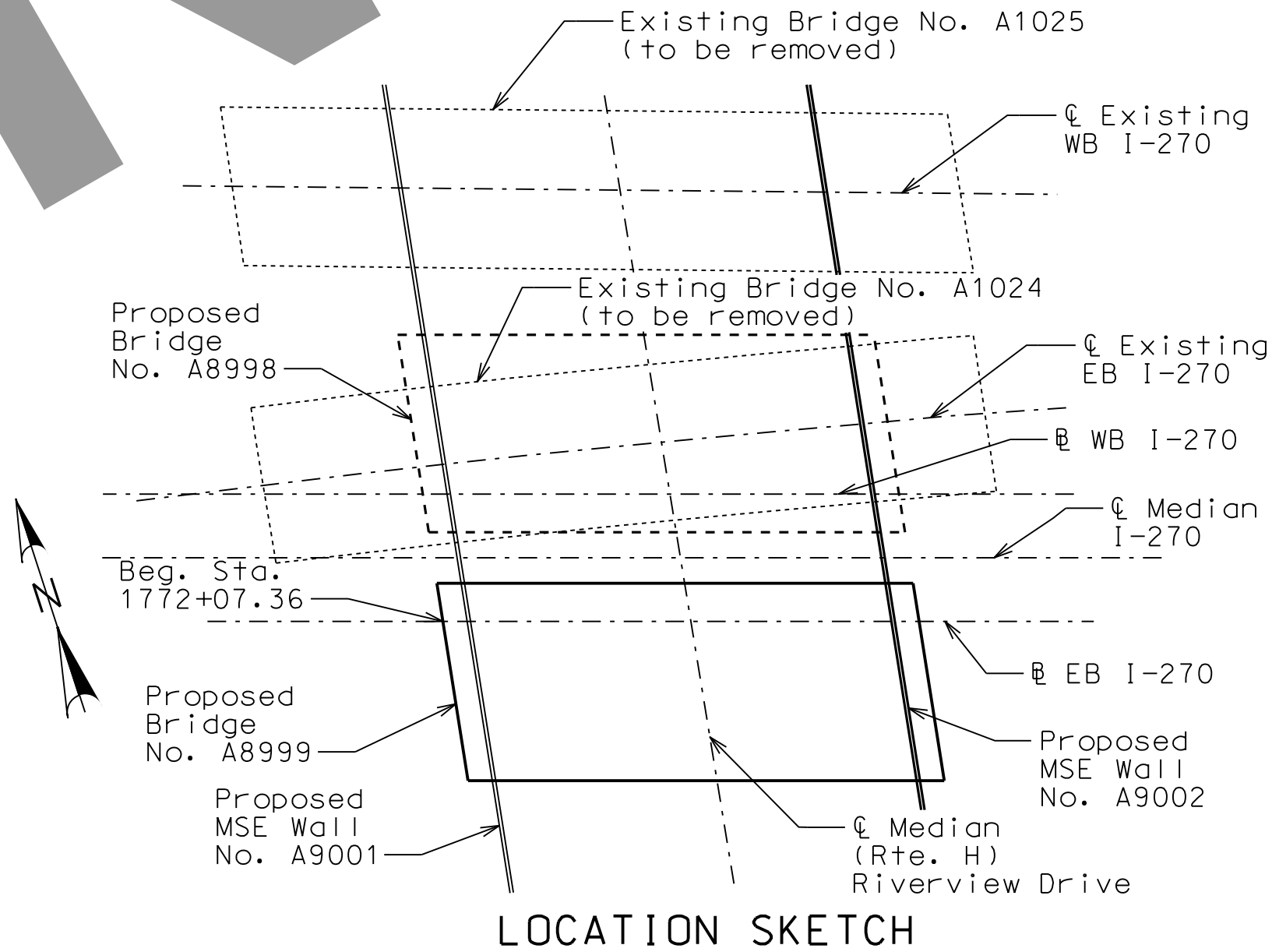
Panel and coping (or capstone) reinforcement shall be epoxy coated.

A filter cloth meeting the requirements for a Separation Geotextile material shall be placed between the select granular backfill for structural systems and the backfill being retained by the mechanically stabilized earth wall system.

Coping shall be required on this structure. When CIP coping sections extend beyond the limits of a single panel, bond breaker (roofing felt or other approved alternate) between wall panel and coping is required. Coping joints shall use 3/4-inch chamfers and shall be sealed with 3/4-inch joint filler. Coping reinforcement shall terminate 1 1/2-inch minimum from face of coping joint.

The top and bottom elevations are given for a vertical wall. If a battered small block wall system is used, the height of the wall shall be adjusted as necessary to fit the ground slope and the concrete leveling pad shall be adjusted as necessary to account for the wall batter. If a fence is built on an extended gutter, then the height of the wall shall be adjusted further.

The contractor shall be solely responsible to coordinate construction of the wall with bridge and roadway construction and ensure that the bridge and roadway construction, resulting or existing obstructions, shall not impact the construction or performance of the wall. Soil reinforcement shall be designed and placed to avoid damage by pile driving, guardrail post installation, utility and sign foundations. (See Roadway and Bridge plans.)



RETAINING WALL UNDER END BENT NO. 3
AT BRIDGE A8998 AND BRIDGE A8999

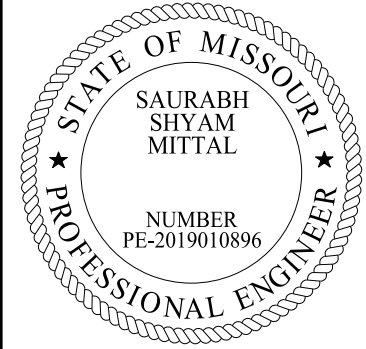
ROUTE EB I-270 FROM LILAC AVENUE TO MISSOURI STATE LINE
ABOUT 1.0 MILES EAST OF LILAC AVENUE
STATION 12+06.66 ALONG SB ROUTE H

Estimated Quantities		
Item		Total
Aesthetic Concrete Stain	LS	1
Mechanically Stabilized Earth Wall Systems	sq. foot	4001
Temporary MSE Wall	LS	1

Designed:JEK
Detailed:JEK
Checked: TPL

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 2 of 10



DATE PREPARED 3/16/2022	
ROUTE I-270	STATE MO
DISTRICT BR	SHEET NO. 2
COUNTY ST. LOUIS CITY	
JOB NO. J613020C	
CONTRACT ID.	

PROJECT NO.
BRIDGE NO. A9002

DESCRIPTION	DATE

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

MoDOT

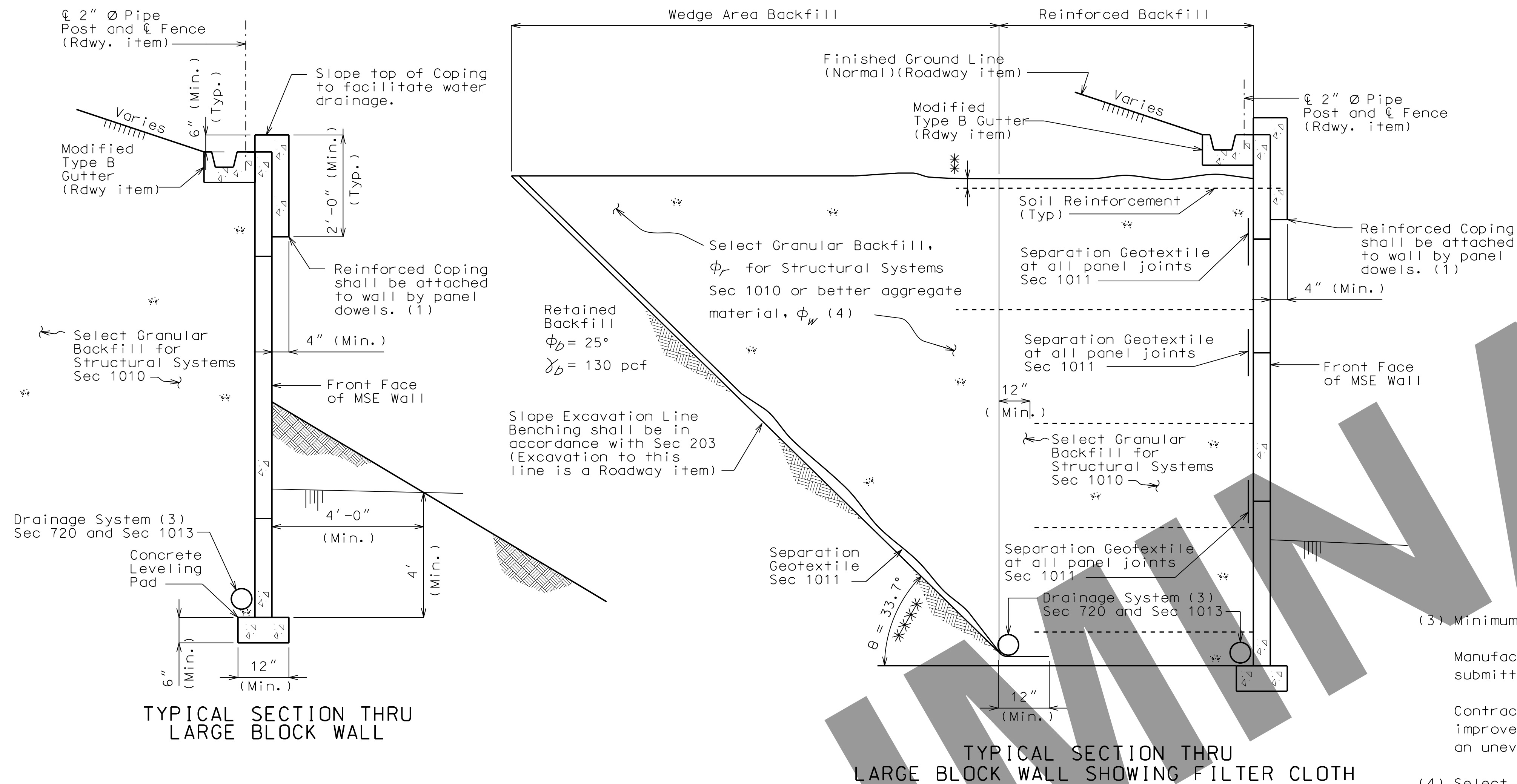
105 WEST CAPITOL
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I-270 AND RIVERVIEW

I-270/RIVERVIEW
MSE WALL A9002
SHEET 2 OF 10



General Notes Cont.:

Minimum 18" wide Geotextile strips shall be centered at vertical and horizontal joints of panel. Geotextile material shall be adhered to back face of panel using an adhesive compound supplied by the manufacturer. All edges of each fabric strip shall provide a positive seal. A minimum 18" overlap shall be provided between spliced filter fabric.

Aluminized soil reinforcement shall have edges coated with coating material per manufacturer.

Soil reinforcement shall be spaced to avoid roadway drop inlet behind wall.

Upper two layers of soil reinforcement shall be extended 3 feet beyond the lower layers.

The splay angle should be less than 15° and tensile capacity of splayed reinforcement shall be reduced by the cosine of the splay angle. Soil reinforcement shall clear the obstruction by at least 3 inches.

No reinforcement shall be left unconnected to the wall face or arbitrarily cut/bent in the field to avoid the obstruction.

Where interference between the vertical obstruction and the soil reinforcement is unavoidable, the design of the wall near the obstruction may be modified using one of the alternatives in FHWA-NHI-10-24, Section 5.4.2. Show detail layout on the drawings. For wall designs with horizontal obstructions in reinforced soil mass, see FHWA-NHI-10-024, Section 5.4.3.

Excavation quantities and pay items are given on the roadway plans. Excavation quantities are based on a soil reinforcement length of 0.75 x Height of the Wall in ft. The soil reinforcement length may vary based upon the wall design selected by the contractor. Plan excavation quantities will be paid regardless of any actual quantities removed based on the soil reinforcement length and design selected.

- (1) Inverted U-shape reinforced capstone may be used in lieu of coping. Panel dowels for level-up concrete shall be required, and provided by manufacturer. The dowels shall be field trimmed to clear the capstone by a minimum of 1 1/2 inches and a maximum of 2 1/2 inches.
- (2) Topmost layer of reinforcement shall be fully covered with select granular backfill for structural systems, as approved by the wall manufacturer, before placement of the Separation Geotextile.

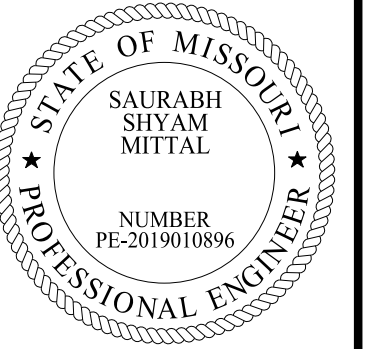
Material Properties Used in Design				
Reinf. Fill/Select Granular Backfill		Active Force Computations		Foundation
ϕ°	γ (pcf)	ϕ°	γ (pcf)	ϕ°

Note: MSE Wall designer shall include table on shop drawings and provide values used in the design computations. Effects of cohesion shall be ignored unless approved by the engineer.

DETAILS FOR MSE WALL

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 3 of 10



DATE PREPARED

5/16/2022

DATE	STATE
070	MO

270	MU
-----	----

SHEET NO
7

R	5
COUNTY	

COUNTY
LOUIS CLT


LOUIS CII
LOB NO

6130200

0150200
CONTRACT ID.

PROJECT NO.

BRIDGE NO.

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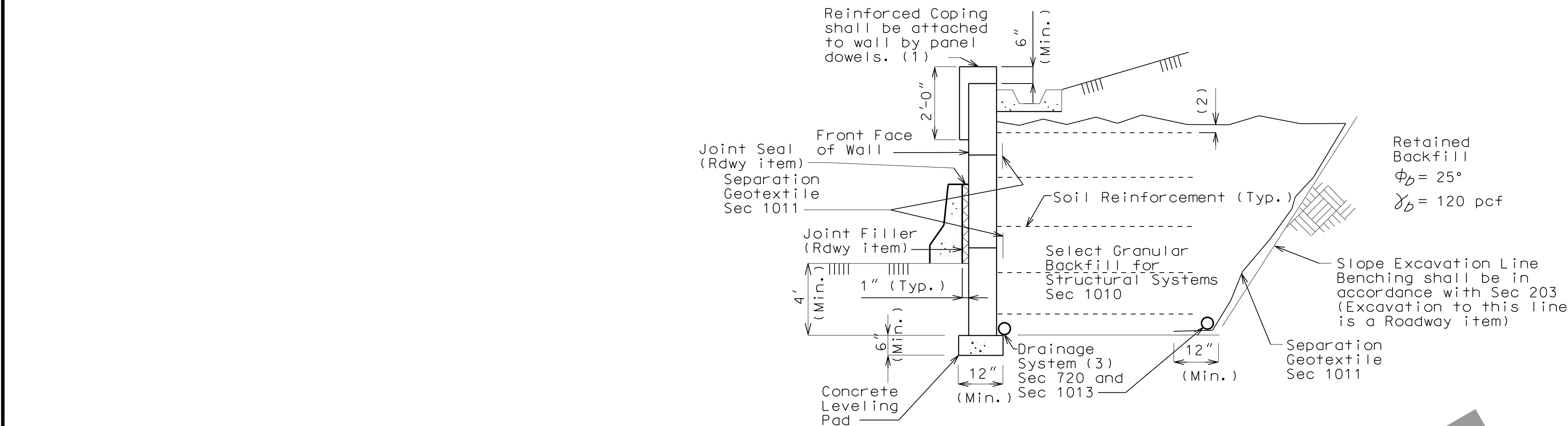
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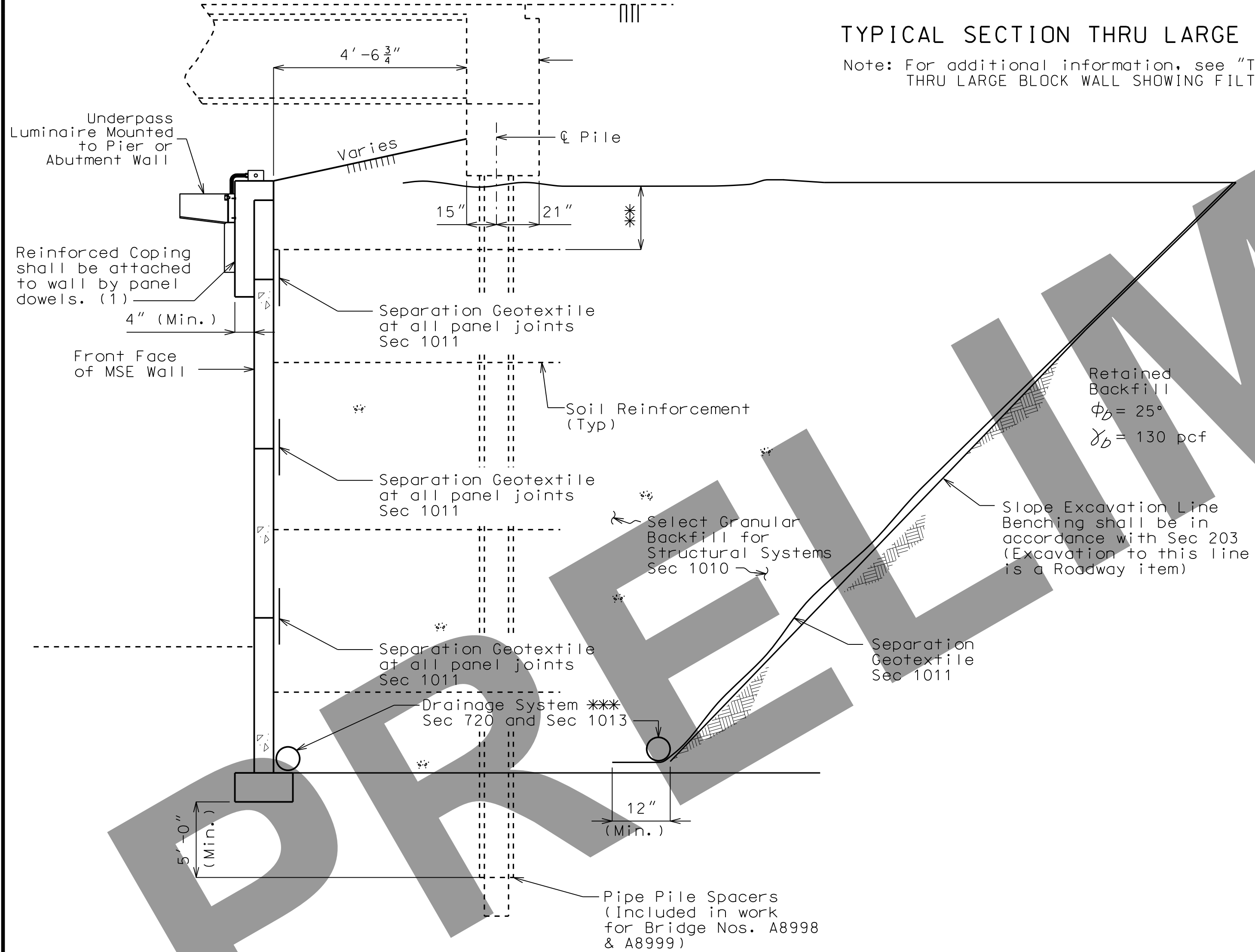
DISCIPLINE: PROFESSIONAL ENGINEERING
CERTIFICATE OF AUTHORITY: 000159
EXPIRATION DATE: DECEMBER 31, 2022

RIVERVIEW
I-270/RIVERVIEW
MSE WALL A9002
SHEET 3 OF 10



TYPICAL SECTION THRU LARGE BLOCK WALL

Note: For additional information, see "TYPICAL SECTION THRU LARGE BLOCK WALL SHOWING FILTER CLOTH".



TYPICAL SECTION THRU LARGE BLOCK WALL UNDER BRIDGE

Note: For additional information, see "TYPICAL SECTION THRU LARGE BLOCK WALL SHOWING FILTER CLOTH".

Note:

Holes shall be 5/8" round and extended 4" into the third layer of blocks, recessed 2" deep by 1 1/2" round.

Rods or reinforcing bars shall be secured by an approved resin anchor system in accordance with Sec 1039.

Recess hole shall be backfilled with non-shrink cement grout.

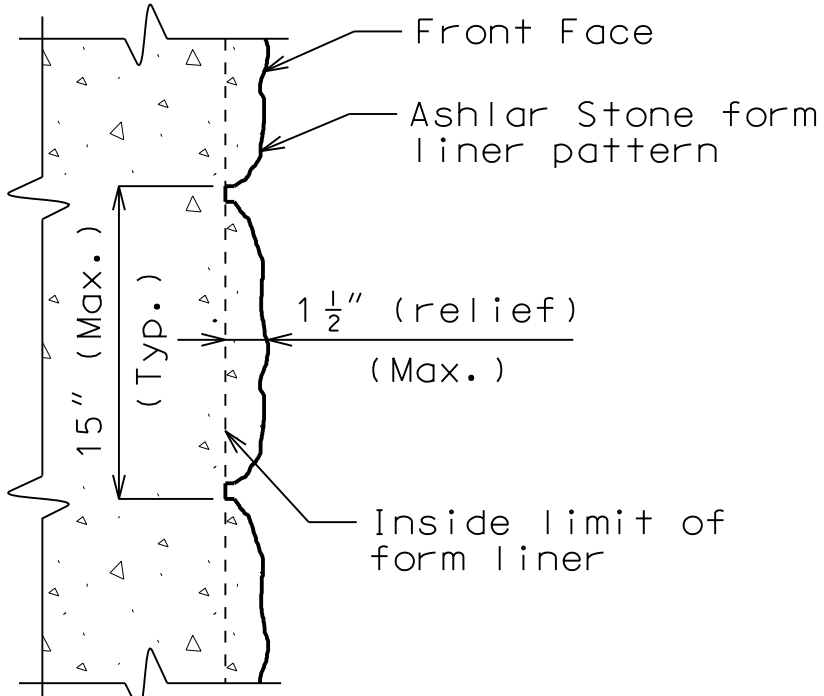
(1) Inverted U-shape reinforced capstone may be used in lieu of coping. Panel dowels for level-up concrete shall be required, and provided by manufacturer. The dowels shall be field trimmed to clear the capstone by a minimum of 1 1/2 inches and a maximum of 2 1/2 inches.

(2) Topmost layer of reinforcement shall be fully covered with select granular backfill for structural systems, as approved by the wall manufacturer, before placement of the Separation Geotextile.

(3) Minimum 6" diameter perforated PVC or PE pipe.

Manufacturer shall show drain details on design plans to be submitted as shown on MoDOT MSE wall plans and/or roadway plans.

Contractor shall modify the drain details as shown if it will improve flow as may be the case for stepped leveling pad, and for an uneven ground line (approval of the engineer required).



FORM LINER DETAIL (Large Block Wall)

Notes:

The cost of form liners for MSE wall systems, complete in place, will be considered completely covered by the contract unit price for Mechanically Stabilized Earth Wall System.

Form liner shall be constructed in accordance with Special Provisions.

The following is a list of form liner manufacturers and types which may be used. Depth of relief for all form liner patterns shall vary up to 1 1/2". The height of any single 'stone' shall be 15" maximum.

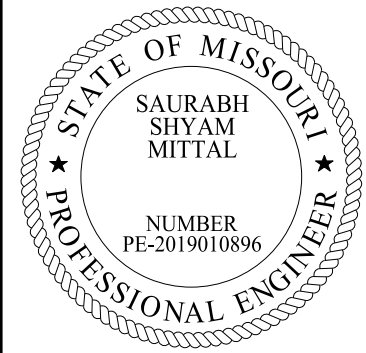
- Scott System, Inc.: Form liner pattern #167 "Ashlar Stone"
- Fitzgerald Formliners: Form liner pattern #16986 "Ashlar Stone"
- Greenstreak: Form liner pattern #330 "Ashlar Stone"
- Spec Formliners: Form liner pattern #1515 "Ashlar Stone"
- Customrock: Form liner pattern #12020 "Tollway Ashlar"
- An approved equal

Designed:
Detailed:
Checked:

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 4 of 10

DETAILS FOR MSE WALL



DATE PREPARED
3/16/2022

ROUTE I-270 STATE MO
DISTRICT BR SHEET NO. 4

COUNTY
ST. LOUIS CITY

JOB NO.
J613020C

CONTRACT ID.

PROJECT NO.

BRIDGE NO.
A9002

DESCRIPTION	DATE

MISSOURI HIGHWAYS AND TRANSPORTATION COMMISSION

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JEFFERSON CITY, MO 65102
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I-270 AND RIVERVIEW
I-270/RIVERVIEW MSE WALL A9002
SHEET 4 OF 10



PAGE 1 OF 2

CLIENT <u>Horner & Shiffrin, Inc.</u> PROJECT NUMBER <u>MG20007</u> DATE STARTED <u>3/13/20</u> COMPLETED <u>3/13/20</u> DRILLING CONTRACTOR <u>Bulldog, CME550X, Efficiency: 95%</u> DRILLING METHOD <u>Hollow Stem Auger, NQ Rock Core</u> LOGGED BY <u>F. Khan</u> CHECKED BY <u>J. Schaeffer</u> NOTES _____	PROJECT NAME <u>Interstate 270 at Riverview Drive Improvements</u> PROJECT LOCATION <u>St. Louis, Missouri</u> GROUND ELEVATION <u>439.3 ft</u> HOLE SIZE <u>inches</u> GROUND WATER LEVELS: <u>▽</u> AT TIME OF DRILLING <u>18.50 ft / Elev 420.80 ft</u> AT END OF DRILLING <u>--- not measured</u> AFTER DRILLING <u>---</u>
---	---

[illegible]

(Continued Next Page)



PAGE 2 OF 2

CLIENT <u>Horne & Shifrin, Inc.</u>	PROJECT NAME <u>Interstate 270 at Riverview Drive Improvements</u>
PROJECT NUMBER <u>MG20007</u>	PROJECT LOCATION <u>St. Louis, Missouri</u>

[illegible]

Refusal at 25.0 feet.
Bottom of borehole at 40.0 feet.

DATE PREPARED	
3/16/2022	
ROUTE	STATE
I-270	MO
DISTRICT	SHEET NO.
BR	5


COUNTY
ST. LOUIS CITY

JOB NO.
J6 I3020C

CONTRACT ID.

PROJECT NO.

BRIDGE NO.
A9002

[illegible]

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EXPIRATION DATE: DECEMBER 31, 2022

I-270 AND
RIVERVIEW
I-270/RIVERVIEW
MSE WALL A9002
SHEET 5 OF 10

Note: For locations of borings, see Sheet No. 1.

Designed: JAA
Detailed: JAA
Checked: JEK

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 5 of 10



BORING NUMBER NB-8

PAGE 1 OF 2

CLIENT	Horner & Shiffin, Inc.	PROJECT NAME	Interstate 270 at Riverview Drive Improvements
PROJECT NUMBER	MG20007	PROJECT LOCATION	St. Louis, Missouri
DATE STARTED	3/12/20	COMPLETED	3/12/20
DRILLING CONTRACTOR	Bulldog, CME550X, Efficiency: 95%	GROUND ELEVATION	438.9 ft
DRILLING METHOD	Hollow Stem Auger	HOLE SIZE	inches
LOGGED BY	F. Khan	CHECKED BY	J. Schaeffer
NOTES	4.0 ft west of design due to location of marked utilities		
		GROUND WATER LEVELS:	
		▽ AT TIME OF DRILLING	18.50 ft / Elev 420.40 ft
		AT END OF DRILLING	--- not measured
		AFTER DRILLING	---

DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (tsf)	DRY UNIT WT. (pcf)	MOISTURE CONTENT (%)	ATTERBERG LIMITS				FINES CONTENT (%)
									LIQUID LIMIT	PLASTIC LIMIT	PLASTICITY INDEX		
0		Concrete (10.0")	438.1										
		FILL: Brown to grey, lean CLAY (CL), with sand, trace slag											
5		Black to brown, hard, lean to fat CLAY (CL-CH)	SS 1	67	5-7-11 (18)	3.0		15					
		- undrained shear strength at 8.0 ft = 0.93 TSF	ST 2	58		2.25	104	20					
10		Grey to brown, lean CLAY (CL), trace sand and pockets of fat clay	427.9										
		- trace organics below 13.5 ft	SS 3	100	3-3-3 (6)	1.75		25					
15		- grey to black below 18.0 ft - brown, with sand below 18.5 ft	SS 4	89	4-5-6 (11)			21					
20		Brown to orange, clayey SAND (SC), trace gravel	416.9										
			SS 5	72	3-3-5 (8)			34					
25													
30		SHALE: Light grey, highly weathered	409.9					18					
			SS 6	100	20-23-18 (41)								

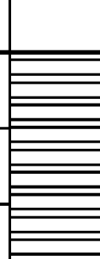
(Continued Next Page)



BORING NUMBER NB-8

PAGE 2 OF 2

CLIENT <u>Horne & Shifrin, Inc.</u>	PROJECT NAME <u>Interstate 270 at Riverview Drive Improvements</u>
PROJECT NUMBER <u>MG20007</u>	PROJECT LOCATION <u>St. Louis, Missouri</u>

DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (tsf)	DRY UNIT WT. (pcf)	MOISTURE CONTENT (%)	ATTERBERG LIMITS			FINES CONTENT (%)
									LIQUID LIMIT	PLASTIC LIMIT	PLASTICITY INDEX	
30		SHALE: Light grey, highly weathered (<i>continued</i>)										
		Refusal at 33.5 feet	405.15	SS	100	50/2"		10				

Refusal at 33.5 feet.
Bottom of borehole at 33.8 feet.

DATE PREPARED	
3/16/2022	
ROUTE	STATE
I-270	MO
DISTRICT	SHEET NO.
BR	6

COUNTY
ST. LOUIS CITY

J6I3020C

PROJECT NO.

BRIDGE NO.

A9002

[illegible]

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I-270 AND
RIVERVIEW
I-270/RIVERVIEW
MSE WALL A9002
SHEET 6 OF 10

BORING DATA

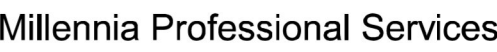
Note: For locations of borings, see Sheet No. 1.

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 6 of 10

Designed: JAA
Detailed: JAA
Checked: JEK

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BORING NUMBER NB-9

PAGE 1 OF 1

CLIENT <u>Horner & Shifrin, Inc.</u>	PROJECT NAME <u>Interstate 270 at Riverview Drive Improvements</u>
PROJECT NUMBER <u>MG20007</u>	PROJECT LOCATION <u>St. Louis, Missouri</u>
DATE STARTED <u>3/23/20</u> COMPLETED <u>3/23/20</u>	GROUND ELEVATION <u>439 ft</u> HOLE SIZE <u>inches</u>
DRILLING CONTRACTOR <u>Bulldog, CME75LC, Efficiency: 94%</u>	GROUND WATER LEVELS:
DRILLING METHOD <u>Hollow Stem Auger</u>	▽ AT TIME OF DRILLING <u>19.00 ft / Elev 420.00 ft</u>
LOGGED BY <u>B.Fisher</u> CHECKED BY <u>J. Schaeffer</u>	AT END OF DRILLING <u>--- not measured</u>
NOTES _____	▽ AFTER DRILLING <u>15.50 ft / Elev 423.50 ft</u>

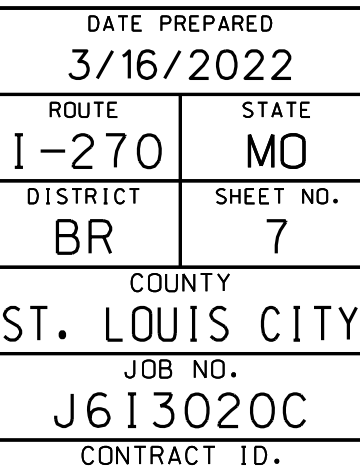
[illegible]

BORING DATA

Note: For locations of borings, see Sheet No. 1.

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 7 of 10



PROJECT NO.

BRIDGE NO.
A9002

[illegible]

**MISSOURI HIGHWAYS AND TRANSPORTATION
COMMISSION**

MoDOT

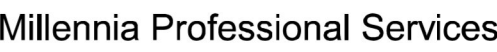
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I-270 AND
RIVERVIEW
I-270/RIVERVIEW
MSE WALL A9002
SHEET 7 OF 10



BORING NUMBER SB-8

PAGE 1 OF 2

CLIENT	Horner & Shiffrin, Inc.	PROJECT NAME	Interstate 270 at Riverview Drive Improvements
PROJECT NUMBER	MG20007	PROJECT LOCATION	St. Louis, Missouri
DATE STARTED	3/17/20	COMPLETED	3/17/20
DRILLING CONTRACTOR	Bulldog, CME75LC, Efficiency: 94%	GROUND ELEVATION	438.7 ft
DRILLING METHOD	Hollow Stem Auger, NQ Rock Core	HOLE SIZE	inches
LOGGED BY	B.Fisher	CHECKED BY	J. Schaeffer
NOTES	5.0 ft west of design due to location of marked utilities		
		GROUND WATER LEVELS:	
		▽ AT TIME OF DRILLING	19.00 ft / Elev 419.70 ft
		AT END OF DRILLING	--- not measured
		AFTER DRILLING	---

DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (tsf)	DRY UNIT WT. (pcf)	MOISTURE CONTENT (%)	ATTERBERG LIMITS				FINES CONTENT (%)
									LIQUID LIMIT	PLASTIC LIMIT	PLASTICITY INDEX		
0		Concrete (10.0")	437.9										
		FILL: Brown to grey, lean CLAY, with silt, trace organics and gravel	X SS 1	83	4-7-8 (15)	4.5		13					
		Blue-grey, fat CLAY (CH), with silt											
5		- dark brownish grey silt layers observed at 5 ft and 6 ft (Possibly organic silt) Brown and grey, lean CLAY (CL)	X SS 2	100	7-6-9 (15)	3.5		23	52	19	33		
			X SS 3	89	3-4-8 (12)	3.75		16					
10			X SS 4	89	3-2-3 (5)	2.5		19					
15		- with sand below 13.5 ft - undrained shear strength at 14.0 ft = 0.44 TSF	ST 5	100		2.5	100	22					
20		- brown-orange, with sand lenses below 18.0 ft - weathered rock fragments with coal pieces at 20.0 ft	X SS 6	89	3-4-9 (13)	2.75		15					
		Brown, clayey SAND (SC)											
25			X SS 7	100	8-8-12 (20)			37					
		Dark grey to blue-grey, shaly, fat CLAY (CH)											
30			X SS 8	83	16-25-24 (49)	4.5		12					

(Continued Next Page)

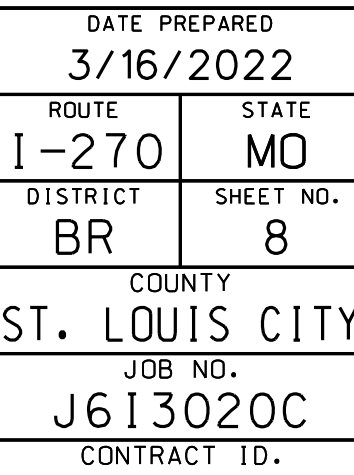


BORING NUMBER SB-8

PAGE 2 OF 2


CLIENT <u>Horner & Shifrin, Inc.</u>	PROJECT NAME <u>Interstate 270 at Riverview Drive Improvements</u>
PROJECT NUMBER <u>MG20007</u>	PROJECT LOCATION <u>St. Louis, Missouri</u>

DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (tsf)	DRY UNIT WT. (pcf)	MOISTURE CONTENT (%)	ATTERBERG LIMITS			FINES CONTENT (%)
									LIQUID LIMIT	PLASTIC LIMIT	PLASTICITY INDEX	
30		Dark grey to blue-grey, shaly, fat CLAY (CH) <i>(continued)</i>										
		405.2	SS 9	100	50/3"			8				
35		LIMESTONE: Greenish grey, argillaceous, soft to moderately hard, thin to medium-bedded, slightly weathered, with shale inclusions	RC 1	100 (81)								
		- rock core qu at 36.5 ft = 5,840 psi										
			RC 2	100 (100)								
40		397.9	RC 3	100 (71)								
		SHALE: Grey, soft to moderately hard										
		- rock core qu at 42.7 ft = 5,790 psi										
		394.4	RC 4	96 (93)								
45		LIMESTONE: White to grey, moderately hard to hard, thin to medium-bedded, slightly weathered										
		- interbedded with shale from 44.3-44.9 ft										
		- shale partings at 45.0 and 46.2 ft										
		- 1.5" chert seam at 47.0 ft										
		- stylolites at 47.6, 47.7, 48.0, and 48.4 ft	RC 5	100 (75)								
		389.2										
Refusal at 33.8 feet. Bottom of borehole at 49.5 feet.												



PROJECT NO.

BRIDGE NO.
A9002

[illegible]

MISSOURI HIGHWAYS AND TRANSPORTATION
COMMISSION

105 WEST CAPITOL
JEFFERSON CITY, MO 65102
1-888-ASK-MODOT (1-888-275-6636)



I-270 AND
RIVERVIEW
I-270/RIVERVIEW
MSE WALL A9002
SHEET 8 OF 10

BORING DATA

Note: For locations of borings, see Sheet No. 1.

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 8 of 10

Designed: JAA
Detailed: JAA
Checked: JEK



PAGE 1 OF 2

CLIENT <u>Horne & Shiffrin, Inc.</u>	PROJECT NAME <u>Interstate 270 at Riverview Drive Improvements</u>
PROJECT NUMBER <u>MG20007</u>	PROJECT LOCATION <u>St. Louis, Missouri</u>
DATE STARTED <u>3/17/20</u> COMPLETED <u>3/17/20</u>	GROUND ELEVATION <u>438.5 ft</u> HOLE SIZE <u> inches</u>
DRILLING CONTRACTOR <u>Bulldog, CME550X, Efficiency: 95%</u>	GROUND WATER LEVELS:
DRILLING METHOD <u>Hollow Stem Auger, NQ Rock Core</u>	▽ AT TIME OF DRILLING <u>18.50 ft / Elev 420.00 ft</u>
LOGGED BY <u>F. Khan</u> CHECKED BY <u>J. Schaeffer</u>	AT END OF DRILLING <u>--- not measured</u>
NOTES <u>5.0 ft west of design due to location of marked utilities</u>	AFTER DRILLING <u>---</u>

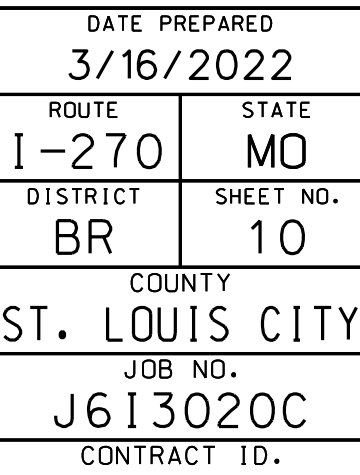

(Continued Next Page)



PAGE 2 OF 2

CLIENT Horne & Shifrin, Inc. **PROJECT NAME** Interstate 270 at Riverview Drive Improvements
PROJECT NUMBER MG20007 **PROJECT LOCATION** St. Louis, Missouri

DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (tsf)	DRY UNIT WT. (pcf)	MOISTURE CONTENT (%)	ATTERBERG LIMITS			FINES CONTENT (%)
									LIQUID LIMIT	PLASTIC LIMIT	PLASTICITY INDEX	
30		Brownish-grey, fat CLAY (CH) <i>(continued)</i>										
35		SHALE: Grey, soft	SS 8	100	23-50/5"			10				
40		- rock core qu at 37.0 ft = 1,550 psi - calcareous from 37.5-39.0 ft	RC 1	80 (72)								
45		- very soft below 42.0 ft - blue-grey below 44.0 ft	RC 2	88 (47)								
		LIMESTONE: Greyish-white, moderately hard to hard, thin to medium bedded, with shale seams	RC 3	95 (90)								
		- healed vertical fracture from 48.5-49.0 ft										
<p>Refusal at 34.0 feet. Bottom of borehole at 49.0 feet.</p>												

BRIDGE NO.
A9002[illegible]

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I-270 AND
RIVERVIEW
I-270/RIVERVIEW
MSE WALL A9002
SHEET 10 OF 10

Note: For locations of borings, see Sheet No. 1.

Designed: JAA
Detailed: JAA
Checked: JEK

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 10 of 10