

May 22, 2020

To: Plan Holders for Improvements to the Monett Regional Airport Pierce City, Missouri Project No. 20-098B-1

Transmitted herewith is Addendum No. 1 to the Issued for Bid Contract Documents, Specifications and Plans dated May 18, 2020 for Improvements to the Monett Regional Airport.

Schedule I – Earthwork and Grading for Relocation of Runway 18/36

Sincerely,

Jviation, Inc.

Mark J. Lovato, P.E. Project Manager



Main 573.636.3200 Fax 573.636.3201 931 Wildwood Drive, Suite 101 | Jefferson City, MO 65109 JVIATION.COM



Addendum No. 1 May 22, 2020 To: Contract Documents, Specifications, and Plans Project No. 20-098B-1 Dated: May 18, 2020

ADDENDUM NO. 1 TO CONTRACT DOCUMENTS, SPECIFICATIONS AND PLANS FOR IMPROVEMENTS TO THE MONETT REGIONAL AIRPORT PIERCE CITY, MISSOURI PROJECT NO. 20-098B-1

To All Bidders: You are requested to make all changes and/or additions contained in this addendum to the Bidding Documents. Failure to acknowledge this Addendum in Proposal shall result in rejection of bid. Bidders are informed that the above referenced Contract Documents, Specifications and Plans are modified as follows as of May 22, 2020:

1. <u>CONTRACT DOCUMENTS/SPECIFICATIONS</u>

Contract Documents: Appendix APP-C

Section: APP-C

Line: All

Revision: This section is revised to include the final Geotechnical Report

Justification: This change was to revise the draft Geotechnical Report with the Final Geotechnical Report.

2. **QUESTIONS**

- 1. Will a Winter shutdown be anticipated?
 - a. Yes, the funding for this project is anticipated for later in the summer and therefore the contractor shall assume a Winter Shutdown period to be requested when work is no longer practical.
- 2. I was wondering if the cad file is available for use in our earthwork takeoff's for the grading of the monett airport?
 - a. Sorry, we do not give out models or CADD files for the bid. A model can be requested by the Contractor that wins the Award, with a signed disclaimer that the model is for reference only and that the plans dictate.
- Will the Final Geotech Report be included with the documents as opposed to the Draft Report.?
 a. Yes, see attached Final Geotechnical Report.
- 4. Also, who would I talk with about digging test holes next week?
 - a. This question is being evaluated and will be answered in the next Addendum.
- 5. Is there an engineer's estimate or range available for this project?
 - a. No, we do not provide an engineer's estimate or range before the bid.

Questions will be accepted via written email format only to Jviation, Inc. (<u>Mark.Lovato@jviation.com</u>) until Thursday June 4, 2020 at 5:00 p.m. (CST).

** END OF ADDENDUM NO. 1 **

REPORT OF SUBSURFACE EXPLORATION AND GEOTECHNICAL ENGINEERING EVALUATION

MONETT REGIONAL AIRPORT RUNWAY 18/36 RELOCATION AND RECONSTRUCTION MONETT, MISSOURI TSI PROJECT NUMBER 20202013

JVIATION 131 Wildwood Drive, Street, Suite 101 Jefferson City, Missouri 65109



8248 NW 101st Terrace, #5 Kansas City, Missouri 64153

May 19, 2020



May 19, 2020

Mr. Jeffery Hogan, PE **JVIATION** 931 Wildwood Drive, Suite 101 Jefferson City, Missouri 65109

Re: Report of Subsurface Exploration and **Geotechnical Engineering Evaluation Monett Regional Airport Runway 18/36 Relocation and Reconstruction** Monett, Missouri **TSi Project No. 20202013**

Dear Mr. Hogan:

TSi Geotechnical, Inc. (TSi) has completed the authorized subsurface exploration and geotechnical engineering evaluation for the referenced project and is pleased to submit this report of our findings to Jviation. The purpose of our work was to determine subsurface conditions at specific exploration locations and to gather data on which to prepare geotechnical recommendations for the design and construction of the proposed additions to Monett Regional Airport in Monett, Missouri. This report describes the exploration procedures used, documents the data obtained, and presents our evaluations and recommendations relative to the geotechnical engineering aspects of the project.

We appreciate the opportunity to assist you with this project. If you have any questions, or if we may be of further service to you, please call us.

Respectfully submitted, **TSI GEOTECHNICAL, INC.**

Fred H. Held III Project Manager

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Morris M Dirnberger, PE Senior Geotechnical Engineer

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Denise B. Hervey, PE Principal

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SUBSURFACE EXPLORATION AND GEOTECHNICAL ENGINEERING EVALUATION MONETT REGIONAL RUNWAY 18/36 RELOCATION AND RECONSTRUCTION MONETT, MISSOURI

1.0 SCOPE OF SERVICES

This report summarizes the results of a geotechnical study performed for the proposed relocation and reconstruction of runway 18/36 at the Monett Regional Airport in Monett, Missouri. The study was performed in general accordance with TSi's proposal to Jviation dated March 11, 2020, which identified the following items for inclusion in this study report:

- Subsurface conditions at the boring locations;
- Laboratory test results;
- Influence of groundwater on the project;
- Pavement recommendations;
- Anticipated Settlement;
- Slope stability analysis;
- Subgrade improvement recommendations;
- Subgrade reaction modulus;
- General construction considerations; and
- Recommendations for fill and backfill materials, placement, and compaction.

2.0 SITE AND PROJECT DESCRIPTIONS

The project is planned to include the relocation and reconstruction of Runway 18/36 along with a parallel taxiway and associated connector taxiways at the Monett Regional Airport in Monett, Missouri. The project will also include the construction of fills ranging in depth to approximately 35 feet and 500 feet in width to establish the new Runway Safety Area and a 6,000 foot long by 100 foot wide Portland Cement Concrete runway. The existing runway 18/36 will be analyzed for repurposing into a parallel taxiway. Borrow source material is planned to be obtained near the site and will be analyzed for use as fill material.

The general location of the project site is shown in Figure 1 in Appendix A . The Site and Boring Location Plan, Figures 2A and 2B in Appendix A, provides a more detailed plan of the project area.

3.0 FIELD EXPLORATION AND LABORATORY TESTING

3.1 FIELD EXPLORATION

TSi conducted an exploration program between April 1 and April 10, 2020. The exploration consisted of 55 borings designated as borings B-1 to B-55, and 10 pavement cores designated as C-1 to C-10. Forty five (45) borings were drilled to planned depths of 10 feet; 3 borings to planned depths of 20 feet; 3 borings to planned depths of 30 feet; and 4 borings to planned depths of 60 feet or auger refusal. Bulk samples were obtained from selected surficial boring locations to depths of approximately 5 feet. The boring logs from this exploration are included in Appendix B. The boring locations were selected and staked in the field by Jviation.

All borings were drilled using a CME-550 ATV-mounted drill rig to advance continuous flight auger drilling tools to the requested depth. A geotechnical specialist from TSi directed the exploration procedures in the field, maintained a field log of the conditions encountered in the borings, and collected and classified the samples recovered. Split-spoon samples were recovered from the borings using a 2-inch outside-diameter, split-barrel sampler, driven by an automatic hammer in accordance with ASTM D 1586. Three-inch Shelby tube samples were obtained in accordance with ASTM D 1587 from limited borings. The Shelby tube samples were preserved by sealing the entire sample in the tube. Borings were backfilled with auger cuttings, compacted with the drill auger and the pavement replaced with a cold asphalt patch.

The results of the field tests and measurements were recorded on field logs and appropriate data sheets by TSi's geotechnical specialist. Those data sheets and logs contain information concerning the exploration methods, samples attempted and recovered, indications of the presence of various subsurface materials, and the observation of groundwater. The field logs and data sheets contain the engineer's interpretations of the conditions between samples, based on the performance of the exploration equipment and the cuttings brought to the surface. The final logs included in this report were based on the field logs, modified as appropriate based on the results of laboratory testing of soil samples.

3.2 LABORATORY TESTING

A laboratory testing program was conducted by TSi to determine selected engineering properties of the obtained soil samples. The following laboratory tests were performed on the samples recovered from the borings:

- Visual description by color and texture of each sample (ASTM 2488);
- Natural moisture content of each sample (ASTM D 2216);
- Unconfined compressive strength of selected cohesive samples (ASTM D 2166);
- Unit weight of selected cohesive samples (ASTM D 7263);
- Atterberg limits on selected cohesive samples (ASTM D 4318);
- Grain size of selected samples (ASTM D 422);
- Consolidation tests on selected samples (ASTM D2435)
- Free Swell Test
- Modified Proctor compaction of selected samples (ASTM D 1157); and,
- California Bearing Ratio (CBR) (ASTM D 1883).

The results of most of the laboratory tests are summarized on the boring logs. Results of the standard Proctors, CBRs, unconfined tests, and consolidation tests are included in Appendix C. The analyses and conclusions contained in this report are based on field and laboratory test results and on the interpretations of the subsurface conditions as reported on the logs. Only data pertinent to the objectives of this report have been included on the logs; therefore, these logs should not be used for other purposes.

4.0 SUBSURFACE CONDITIONS

Details of the subsurface conditions encountered at the boring locations are shown on the logs in Appendix B. The general subsurface conditions encountered and their pertinent engineering characteristics are described in the following paragraphs. Conditions represented by the borings should be considered applicable only at these locations on the dates shown; the reported conditions may be different at other locations or at other times.

4.1 GENERALIZED SUBSURFACE PROFILE

Sixty Five (65) borings were drilled. Ten (10) borings were drilled in the existing Runway 18/36 and in the footprint of the proposed Taxiway A relocation at the airport site, designated as C-1 to C-10. The pavements consisted of 8.5 to 6.5 inches of Portland Cement Concrete (PCC) generally underlain by 3.5 to 5.5 inches of crushed limestone gravel base. In all borings, the pavement section was underlain by lean and fat clay fill at depths ranging from approximately 3.0 to 10.0 feet, where these borings were terminated. Blow counts ranged from 7 blows per foot (bpf) to 50/2.5" bpf. Moisture contents in this fill ranged from 12% to 46%. Liquid Limits (LL) ranged from 40 to 82, and Plastic Indexes (PI) ranged from 22 to 48.

The surficial material at most borings (B-1 to B-55) consisted of approximately 6 inches of topsoil. Borings B-1, B-2, and Borings B-8 through B-11 encountered approximately 3.0 feet to 8.0 feet of fill underlying the surficial topsoil. Standard Penetration Test (N-values) in the fill ranged from 8 to 48 blows per foot (bpf). Moisture contents ranged from 15% - 28%; liquid limits (LL) from 42 to 66; and plasticity indexes (PI) of 19 to 38.

The surficial pavement and fill materials were underlain by native topsoil at the boring locations. The native soils primarily consisted of lean clays, fat clays (CL and CH, respectively, in accordance with Unified Soil Classification System (USCS), silts and elastic silts (MLand MH, respectively, in accordance to the USCS), and clayey gravels clays (GC according to the USCS). Standard penetration tests (N-values) in the native soils varied significantly across the site ranging from 1 blow per foot (bpf) to 50/0" bpf. Higher blow counts may be attributed to the sampler being driven on chert or rock fragments. Moisture contents ranged from 8% to 54%. Atterberg Limit tests performed within the native soils resulted in Liquid Limits (LL) of 28 to 86 and Plasticity Indexes (PI) of 9 to 57. Dry unit weight values in the lean and fat clays ranged from 56 to 110 pounds per cubic foot (pcf) and twelve (12) undrained shear strength values ranged from 0.18 to 1.58 tons per square foot. The native soils continued to the termination depth of 10.0 ft. in thel borings except in Borings B-22, -24, -32, and -35 which were terminated at 30 feet; Borings B-49 and 51, terminated at 20 feet; Borings B-26, 27, and 28, terminated at 60 feet; and Boring B-29 terminated at 42.7 feet.

4.2 GROUNDWATER

Groundwater was observed in Borings B-27, B-28, and C-06 during drilling at a depths of 1.0 and 33.5 feet, respectively, below ground surface. The presence or absence of groundwater at a particular location does not necessarily mean that groundwater will be present or absent at that

location at other times. Seasonal variations and other unknown considerations will cause fluctuations in water levels and the presence of water in the soils.

5.0 Engineering Assessments and Recommendations

5.1 GLOBAL STABILITY ANALYSES

Global stability analyses were performed for the proposed side slopes of the 500 foot runway and safety area. Analyses were performed for both the long term (drained) and after construction (undrained) conditions. The slope analyses were based on borings located near the deepest fill areas. The global stability analyses figures are located in Appendix D of this report. Based on preliminary design figures, we understand the side slopes will be on the order of 5.5 Horizontal to 1.0 Vertical (5.5 H:1.0V) and transition to approximately 33.5 Horizontal to 1.0 Vertical (33.5H:1.0V). The analyses were completed at locations of the deepest fill areas of 20 feet and 35 feet. The results produced minimum factors of safety of 1.3 or greater at each side slope. The analyses were performed with SLOPE/W where a factor of safety (FS) greater than 1.5 is generally accepted for the short term condition, and 1.3 for the long term.

5.2 FOUNDATION SETTLEMENT

The amount of settlement caused by consolidation of native soils below the newly placed embankment fill along the runway and safe area at the deep fills of 20 and 35 feet are estimated to be between 1 and 2 feet. Additionally, the settlement of the new embankment fill under its own weight will be on the order of 4 to 12 inches. To help mitigate compression of the new embankment, the embankment fill should be compacted to 90% of modified Proctor maximum dry density in areas of fill less than 15 feet; 92% of modified Proctor in fill areas more than 15 feet. The elastic component of embankment settlement should occur as the fill is placed, but consolidation settlement of foundation soils could continue for weeks or months. This selfweight settlement is material and time-dependent and would be additive to the foundation consolidation settlements discussed previously; unless construction can be staged so that the pavements that will be supported by the new fill are built after the fill-induced settlements are essentially complete. Due to the variability of the soils encountered on this site, including sand and gravel content, plasticity, and depth to bedrock (not ecountered), consolidation settlement estimates could be higher and take longer to occur. Based on some of the fat clays encountered in the borings with a smaller amounts of sand and gravel, settlements of foundation soils could exceed 2 feet and take over two years to occur for fills as high as 35 feet. Lower fill heights would be less than these maximum estimates.

The embankment areas should be monitored for settlement and construction delayed until the settlement is mostly. Consolidation testing provides only general guidance for calculating the magnitude and rate of settlements. These settlements provide only general guidance for calculating the estimated quantities and potential delays after the fill has been placed prior to pavement construction. Therefore, TSi recommends that ground surface settlement monuments be installed at locations approaching and within deeper fill areas after fill placement is complete, and that settlements be monitored periodically by surveying methods. We suggest using monuments such as screwed elements bearing below a depth of 2.5 feet, such as screw anchors, so that seasonal changes in moisture and frost effects do not skew the surveyed elevations. Based on the monitoring survey data, TSi would assess the magnitude and rate of settlements. If the data reveals unanticipated magnitudes or rates, TSi will confer with Jviation to decide upon an

appropriate course of action. If requested, TSi can provide guidance for a settlement monitoring program.

Monitoring could also consist of below grade plates to track settlement of the native clay soils, and stakes to track the settlement of both the proposed embankment fill and native soils as outlined in the Missouri Department of Transportation (MoDOT) standard specifications. The settlement plates should be measured weekly after fill placement is complete. Settlement of the embankment should be monitored until weekly settlement is less than 1/8 inch between consecutive readings at all locations

This settlement could be accelerated by the installation of wick drains with a sand blanket. Wick drains accelerate the settlement process by providing a shorter drainage path for water to escape. The degree of acceleration is based on the wick drain spacing and the preload. Recommendations regarding the installation of wick drains and preload would be a design/build contractor's responsibility.

Wick drains are mandrel-pushed or vibrated into the soil, having a central plastic core surrounded by a filter jacket, and pushed to the bottom of the consolidating layer. As the mandrel is withdrawn, the undamaged wick drain is left in place within the soill mass.

These methods are typically designed and installed by a specialty contractor, who should be capable of verifying the improvement in settlement characteristics, along with providing preliminary construction costs, early in the design phase. Because of the specialty nature of these technologies, the design/builder should accept responsibility for the performance of their methods and products. TSi can provide contact information for several contractors upon request. Based on our experience with similar projects in the same geologic setting, this approach is likely to be feasible for this project.

5.3 SWELLING CLAY CONSIDERATIONS

Fat clays (CH) were encountered at several of the boring locations at shallow depths of just below the pavement at the proposed taxiway, to approximately 10 feet in the area of the proposed new runway and safe area, and could be encountered in the subgrade. Fat clays tend to swell when water is absorbed and shrink as the material dries. Potential detrimental effects include heaving, settlement, and differential movements. This concern applies to this material whether it is in its natural condition or used as fill material.

If encountered, we recommend that fat clay soils be overexcavated to a depth of at least 2 feet below pavement areas for shrink and swell considerations. The overexcavated fat clay soils should be replaced with a volume-stable material, such as a low-plasticity silty or lean clay or a well-graded crushed limestone. The new material should be placed and compacted in accordance with the recommendations provided later in this memorandum. TSi suggests that unit rates for removal and replacement of fat clay in this manner be included in the bid documents so that a comparison of such rates can be part of the selection process.

In addition to the recommendations discussed above, some relatively basic design and construction considerations are recommended that will aid in maintaining the moisture content of the fat clays and reducing the potential for swell. Avoiding conditions that could result in excessive wetting or drying of the subgrade will reduce the potential for volume instability and associated distress. The following design and construction precautions are recommended:

- 1. Positive surface drainage should be provided during and after construction to prevent ponding of water around the structures and on or adjacent to pavements.
- 2. Stormwater runoff from the building roofs should be collected by a gutter system, or other means, and carried away from the structures to avoid saturating the subgrade under and adjacent to the structures.
- 3. Deep-rooted trees or shrubs planted for landscaping purposes should be kept a distance of one mature height away from the buildings to prevent their roots from withdrawing excessive moisture from the underlying clay soils.
- 4. Excessive watering of grass or shrubs adjacent to the buildings or pavements should be avoided.
- 5. Foundation drains should be used for below grade areas and either daylighted or drained to a sump pit and removed by a pump.

5.4 PAVEMENT DESIGN

TSi recommends a well-graded aggregate base, such as MoDOT Type 5 or equivalent, directly underlying the pavements with a minimum thickness of 6 inches. Fill material below the pavements should consist of a minimum of 18 inches of low volume change (LVC) material consistent with requirements outlined in Section 6.3 of this report.

Four California Bearing Ratio (CBR) tests were conducted from bulk samples in selected borings on the native soils with composite samples of subgrade soil from 1 to 5 feet. Modified Proctor tests yielded maximum dry densities ranging from 103.3 to 106.1 pounds per cubic foot (pcf) and optimum moisture contents ranging from 16.2 to 20.8%. The CBR's ranged from 1.4 to 7.1 (see Table 1 below). The CBR test results are included in Appendix C of this report. Based on the laboratory results, TSi recommends a CBR of 4 for use in designing the flexible pavement sections for this site. In accordance with the Federal Aviation Administration (FAA) Advisor Circular 150/5320-6F, a CBR value of 4 equates to a modulus-of-subgrade reaction, k_S , for the soil of approximately 69 pounds per cubic inch (pci). A subgrade modulus value can be estimated by using the CBR value with the following equation, as taken from the FAA design guide:

Subgrade modulus (psi) = 1500 x (CBR)

Rigid pavement design can be based on a modulus-of-subgrade reaction (k) of 69 pounds per cubic inch (pci) for the soil subgrade. These values for rigid and flexible pavement design are based on the requirement that the pavement subgrade is prepared in accordance with the recommendations provided in this report.

Asphaltic concrete pavements should have a minimum thickness of 6.0 inches with a 6.0-inch thick crushed aggregate base. If the crushed aggregate base is increased to 9.0 inches, the asphalt thickness could be reduced to 4.0 inches. The asphaltic concrete pavement section should include a minimum surface course thickness of 2.0 inches. The crushed aggregate base for asphaltic pavements should consist of MoDOT Type 5, or equivalent. Fill material below the pavements should consist of a minimum of 12 inches of low volume change (LVC) material consistent with requirements outlined in Section 6.3 of this report.

For areas where heavy wheel loads will be concentrated pavements should be constructed with a minimum thickness of 8.0 inches of Portland cement concrete. We recommend that a 6.0-inch base course of well-graded crushed limestone, such as MoDOT Type 5, be placed below all concrete pavements.

Stabilization of the subgrade will provide a stiffer, more durable subgrade, which will improve the durability of the pavements. It will also provide a subgrade that is less prone to disturbance under construction traffic, especially during rainy weather. If the design team desires to increase the CBR value of the subgrade, subgrade stabilization with fly ash or lime is recommended. Our experience indicates that higher CBR values can be achieved with fly ash-treated subgrade. The CBR value for lime treated soils will be greater than the CBR values measured in the clays. The stabilized soils should be compacted as recommended in Section 6.4 of this report. The CBR value used in design for treated soils should be verified during construction using dynamic cone penetrometer (DCP) tests.

A lime- or fly ash-treated Proctor and CBR testing was not included in the scope of this exploration. The exact amount of lime or fly ash application should be determined in the laboratory before construction begins. If requested, TSi can perform additional services to determine the optimal percentage of fly ash or lime application in the laboratory using additional soil samples from the site to perform standard Proctor and CBR tests at varying fly ash or lime percentages. The recommended pavement sections are minimum design thicknesses; periodic maintenance should be anticipated. The recommended pavement sections are not intended to support construction traffic.

Sample	Sample Depth (Feet)	USCS	Optimum Moisture Content (%)	Maximum Dry Density (pcf)	Dry Density at 90% Compaction (pcf)	CBR Value
Bulk B-49	1-5	CH LL=56 PI=36	16.2	106.1	95.5	7.1
Bulk B-53	1-5	CH LL=87 PI=63	20.8	104.0	93.6	4.3
Bulk C-3	1-5	CH LL=66 PI=45	17.3	103.3	93.0	5.3
Bulk C-7	1-5	CH LL=96 PI=70	17.2	105.8	95.2	1.4

TABLE 1. SUMMARY OF PROCTOR AND CBR TESTS

USCS = Unified Soil Classification System Maximum dry density as determined by ASTM D 1157 pcf = pounds per cubic foot

6.0 SITE PREPARATION AND EXCAVATION CONSIDERATIONS

6.1 SUBGRADE PREPARATION

Construction areas should be stripped of existing pavement, organic soil, and any deleterious materials prior to site excavation and grading. Care should be taken during stripping to prevent excessive disturbance of the underlying soil. After the removal of these materials, and where further excavation is not required, the exposed subgrade should be proofrolled. Proofrolling is accomplished by passing over the subgrade with proper equipment, such as a loaded tandem-axle dump truck or scraper, and observing the subgrade for pockets of excessively soft, wet, disturbed, or otherwise unsuitable soils. Any unacceptable materials thus found should be excavated and either recompacted or replaced with new structural fill.

Prior to placing fill in any area, the subgrade should be scarified to a depth of about 6 inches, the moisture content adjusted to near its optimum moisture content, and the subgrade recompacted in accordance with recommendations made in subsequent sections of this report. The recommended proofrolling and/or scarification and recompaction may be waived if, in the opinion of a geotechnical engineer, this procedure would be detrimental or unnecessary. Following satisfactory preparation of the subgrade, controlled fill material may be placed.

6.2 SUBGRADE PROTECTION

Construction areas should be properly drained in order to reduce or prevent surface runoff from collecting on the exposed subgrade. Any ponded water on the exposed subgrade should be removed immediately. Temporary stormwater swales and collection areas may be required to control surface water flow into low areas of the site.

To prevent unnecessary disturbance of the subgrade soils, heavy construction vehicles should be restricted from traveling through the finished subgrade. If areas of disturbed subgrade develop, they should be properly repaired in accordance with the recommendations in this report.

Immediately prior to construction of the pavement, it is recommended that the exposed subgrade be evaluated to determine whether moisture contents are within the recommended range and to identify areas disturbed by construction operations. Moisture conditioning of wet or dry areas is recommended prior to construction of the pavement section. Areas disturbed by construction traffic should be reworked.

6.3 FILL AND BACKFILL MATERIALS

In general, fills should consist of clay or well-graded granular materials with a maximum particle size of 2 inches. The soils encountered in the borings are suitable for use as fill. Low volume change (LVC) fill should consist of approved, well-graded granular materials or low to moderate plasticity cohesive soil. Low to moderate plasticity cohesive materials used as LVC fill should consist of inorganic clay with a liquid limit less than 45 and a plasticity index of less than 25. Fill materials from off-site sources should be approved prior to their use. Soil with decayable material such as wood, trash, metal, or vegetation is typically not acceptable.

Some of the fill material may require the addition of moisture prior to compaction. This should be performed in a controlled manner using a tank truck with a spray bar, and the moistened soil should be thoroughly blended with a disk or pulverizer to produce a uniform moisture content. Repeated passages of the equipment may be required to achieve a uniform moisture content. If fill is placed during the winter season, fill materials should be carefully observed to see that no ice or frozen soils are placed as fill or remain in the base materials upon which fill is placed.

Some of the fill material may require moisture reduction prior to compaction. During warm weather, moisture reduction can generally be accomplished by disking, or otherwise aerating the soil. When air-drying is not possible, a moisture-reducing chemical additive, such as lime or Class C fly ash, may be used as a drying agent.

6.4 FILL AND BACKFILL PLACEMENT

Cohesive fill should be compacted to a dry density of at least 90% of the modified Proctor maximum dry density (ASTM D 1157) of the soil. Fill depths of 15 feet or more should be compacted to at least 92% of this criteria. Granular material, such as crushed limestone, placed for structure or pavement support, should be compacted to at least 95% of the modified Proctor maximum dry density. The moisture content of lean clay or granular fill at the time of compaction should be within $\pm 3\%$ of the optimum moisture content of the material as determined by the modified Proctor compaction test. Fill should be placed in loose lifts not in excess of 8 inches thick, and compacted to the aforementioned criterion. However, it may be necessary to place fill in thinner lifts to achieve the recommended compaction when using small hand-operated equipment.

7.0 CONSTRUCTION OBSERVATION AND TESTING

It is recommended that TSi be retained during construction to perform testing and observation services for the following items:

- Observation and documentation of the exposed soil after stripping topsoil and/or pavement, during scarification, compaction, and proofrolling;
- Testing of asphalt and/or concrete materials used for paving; and
- Placement and compaction of fill materials.

These Quality Assurance services should help verify the design assumptions and maintain construction procedures in accordance with the project plans, specifications, and good engineering practice.

8.0 REPORT LIMITATIONS

This geotechnical report has been prepared for the exclusive use of **JVIATION** for the specific application to the subject project. The information and recommendations contained in this report have been made in accordance with generally accepted geotechnical and foundation engineering practices; no other warranties are implied or expressed.

The assessments and recommendations submitted in this report are based in part upon the data obtained from the borings. The nature and extent of variations between the borings may not be evident at this time. If variations appear evident at a later date, it may be necessary to re-evaluate the recommendations of this report.

We emphasize that this report was prepared for design purposes only and may not be sufficient to prepare an accurate construction bid. Contractors reviewing this report should acknowledge that the information and recommendations contained herein are for design purposes.

If conditions at the site have changed due to natural causes or other operations, this report should be reviewed by TSi to determine the applicability of the analyses and recommendations considering the changed conditions. The report should also be reviewed by TSi if changes occur in the project location, size, and type, in the planned loads, elevations, grading and site development plans or the project concepts.

TSi requests the opportunity to review the final plans and specifications for the project prior to construction to verify that the recommendations in this report are properly interpreted and incorporated in the design and construction documents. If TSi is not accorded the opportunity to make this recommended review, we can assume no responsibility for the misinterpretation of our recommendations.

APPENDIX A







APPENDIX B

				RING NO. B-01						0240		hnical I 01 Terra		(A)	M	
Pro	oject	Desc	riptio	n: Monett Regional Monett, Missouri		location of I	Runv	vay 1	18-36	Kans	sas City	, Missou	uri 64	153 🖌		Sī
				· ·								7965 (8	16) 28	33-393	8 FAX	
Depth, feet	Samples	Sample #	Graphic Log	Surface EI.: Location: See Site Location	n Plan	9	Recovery %	RQD	Penetration Blows Per 6 inches	Hand Penetrometer, Qu TSF	Undrained Shear Strength, TSF	Unit Dry Weight, Ib/cu ft.	Water Content, %	Liquid Limit	Plastic Limit	Plasticity Index
			<u>, 1 1,</u>	Brown, lean CLAY	, with orgar	nics	_									
	X	SS-1		Brown and gray, le with gravel and sa	ean CLAY (I nd	FILL),	72		6 5 6				18			
	X	SS-2		Reddish brown an CLAY (CL), with g	d gray, lear ravel and sa	 ו and	67		4 8 8				22	39	21	18
	X	SS-3		Reddish brown an GRAVEL with san (35% passing No.		ey	56		8 19 20				16			
	X	SS-4		Boring terminated	f		100		12 20 21				28			
 Com∣ Date Date Date Engir 	Borin Borin	n Depth g Starte g Comp Geologis	ed: pleted:	10.0 4/1/20 4/1/20 AB 20202013.00	Remarks:	Boring drille Groundwate									PT.	



				RING NO. B-03 n: Monett Regional Airport Relocation o Monett, Missouri	f Run	way '	18-36	8248 Kans	3 NW 1 sas City	chnical 01 Terra y, Misso 7965 (8	ace #5 uri 64	153 🦼		3
Depth, feet	Samples	Sample #	Graphic Log	Surface EI.: Location: See Site and Boring Location Plan	Recovery %	RQD	Penetration Blows Per 6 inches	Hand Penetrometer, Qu TSF	Undrained Shear Strength, TSF	Unit Dry Weight, Ib/cu ft.	Water Content, %	Liquid Limit	Plastic Limit	Disctinity Index
_			<u>1,1,1,1,1</u>	Brown, lean CLAY, with organics										
	X	SS-1		Reddish brown, lean CLAY (CL), trace gravel	100		4 6 9				21			
_ 5 —	X	SS-2		- reddish brown and gray below 3.5 ft.	100		8 9 10				19	31	18	1
	X	SS-3			100		4 8 13				23			
-0	X	SS-4		- with chert gravel and sand below 8.5 ft. Boring terminated at 10.0 ft.	100		9 12 13				21			
)ate)ate ingir	Borin Borin	n Depth Ig Starte Ig Com Geologis	ed: pleted:	10.0 Remarks: Boring dri 4/10/20 Groundwa 4/10/20 AB 20202013.00									<u></u> РТ.	<u> </u>

				RING NO. B-04 n: Monett Regional Airport Relocation o Monett, Missouri	of Run	vay	18-36	8248 Kans	3 NW 1 sas City	chnical 01 Terra y, Misso 7965 (8	ace #5 uri 64	153 🦼		
Depth, feet	Samples	Sample #	Graphic Log	Surface EI.: Location: See Site and Boring Location Plan	Recovery %	RQD	Penetration Blows Per 6 inches	Hand Penetrometer, Qu TSF	Undrained Shear Strength, TSF	Unit Dry Weight, Ib/cu ft.	Water Content, %	Liquid Limit	Plastic Limit	Directionity Index
	X	SS-1		Brown, lean CLAY, with organics Reddish brown, lean CLAY (CL), with chert gravel, sand, and chert fragments	83		50/6"				24			
	X	SS-2		Reddish brown, clayey GRAVEL (GC)	56		24 24 27				16			
	X	SS-3		Reddish brown, fat CLAY (CH), with sand and gravel	100		9 13 12				16	51	27	2
-0	X	SS-4		Boring terminated at 10.0 ft.	100		4 12 20				39			
 115 20 														
Date Date Engir	Borin Borin	n Depth ng Start ng Com Geologi o.:	ed: pleted:	10.0 Remarks: Boring dri 4/10/20 Groundwa 4/10/20 AB 20202013.00									<u> </u> PT.	<u> </u>

				RING NO. B-05 n: Monett Regional Airport Relocation Monett, Missouri	of Run	way '	18-36	8248 Kans	3 NW 1 sas City	chnical 01 Terr y, Misso 7965 (8	ace #5 uri 64	153 🦼		
Depth, feet	Samples	Sample #	Graphic Log	Surface EI.: Location: See Site and Boring Location Plan	Recovery %	RQD	Penetration Blows Per 6 inches	Hand Penetrometer, Qu TSF	Undrained Shear Strength, TSF	Unit Dry Weight, Ib/cu ft.	Water Content, %	Liquid Limit	Plastic Limit	Diactioity Index
	X	SS-1		Brown, lean CLAY, with organics Reddish brown, lean CLAY (CL), trace gravel and sand	100		336				21	35	19	1
_ _ 5 _		SS-2		- with chert gravel below 3.5 ft.	100		7 12 13				19			
_		SS-3			100		8 9 9				25			
- - 10-	X	SS-4		Boring terminated at 10.0 ft.	100		7 6 5				37			
 15 20 														
Date Date Engir	Borin Borin	n Depth Ig Starte Ig Com Geologie	ed: pleted:	10.0 Remarks: Boring d 4/10/20 Groundy 4/10/20 AB 20202013.00	rilled wit water wa	th CN as no	⊥ ∕IE-55 t enco	i0 usi ounte	ing H red c	⊥ SA ar during	⊥ nd au drilli	uto SI ing.	PT.	

				RING NO. B-06 n: Monett Regional Airpor Monett, Missouri	t Relocation of	Runv	vay ′	18-36	8248 Kans	8 NW 1 sas City	chnical 01 Terra /, Misso 7965 (8	ace #5 uri 64	153		
Depth, feet	Samples	Sample #	Graphic Log	Surface El.: Location: See Site and B Location Plan	ION	Recovery %	RQD	Penetration Blows Per 6 inches	Hand Penetrometer, Qu TSF	Undrained Shear Strength, TSF	Unit Dry Weight, Ib/cu ft.	Water Content, %	Liquid Limit	Plastic Limit	Plasticity Index
	X	SS-1		Brown, lean CLAY, with o Brown, lean CLAY (CL), gravel and sand	organics with chert	61		9 11 9				20			
 - 5 -	X	SS-2		Reddish brown, elastic S with chert gravel and sar		100		3 6 9				33	55	30	25
	X	SS-3		Reddish brown, fat CLA chert gravel and sand	/ (CH), with	100		8 8 7				24			
 -10-	X	SS-4		Boring terminated at 10.	D ft.	67		9 8 8				30			
				10.0	eka: Boring drill		b CA	AE 55	-0 ue		SA ar			27	
Com Date Date Engir Proje	Borir Borir neer/(ect No		ed: pleted: st:	10.0 Rema 4/10/20 4/10/20 AB 20202013.00 present approximate strata boundari	Groundwat									PT.	

				RING NO. B-07 n: Monett Regional Air Monett, Missouri	rport Rel	ocation of	Runv	vay ′	18-36	8248 Kans	NW 1 as City	chnical 01 Terra /, Misso 7965 (8	ace #5 uri 64	153 🆌		51
Depth, feet	Samples	Sample #	Graphic Log	Surface El.: Location: See Site ar Location P	lan)	Recovery %	RQD	Penetration Blows Per 6 inches	Hand Penetrometer, Qu TSF	Undrained Shear Strength, TSF	Unit Dry Weight, Ib/cu ft.	Water Content, %	Liquid Limit	Plastic Limit	Plasticity Index
			<u></u>	Brown, lean CLAY, v	vith organ	nics ,										
	X	SS-1		Reddish brown, silty (CL), trace gravel	lean ČLA	Υ	100		3 5 9				26	42	22	2
		SS-2		- reddish brown and to 6.0 ft.	gray from	1 3.5	100		4 7 13				21			
	X	SS-3		- reddish brown with and sand below 6.0 t	chert gra ft.	vel	100		5 10 12				23			
	X	SS-4		Boring terminated at	10.0 ft		89		6 12 15				21			
- – – - – – - 15 – - – - – - – - – - – - – - –																
Date Date	Borin Borin neer/(n Depth Ig Starte Ig Com Geologis	ed: pleted:	4/10/20		Boring drille Groundwate									PT.	

				RING NO. B-08 n: Monett Regional Airport Relocation of Monett, Missouri	of Run	way	18-36	8248 Kans	3 NW 1 sas City	chnical 01 Terr y, Misso 7965 (8	ace #5 uri 64	153 🖌		50
Depth, feet	Samples	Sample #	Graphic Log	Surface EI.: Location: See Site and Boring Location Plan	Recovery %	RQD	Penetration Blows Per 6 inches	Hand Penetrometer, Qu TSF	Undrained Shear Strength, TSF	Unit Dry Weight, Ib/cu ft.	Water Content, %	Liquid Limit	Plastic Limit	Plasticity Index
			<u>x1 1,</u>	Brown, lean CLAY, with organics										
	X	SS-1	\bigotimes	FILL: Brown, fat CLAY (CH), with gravel and sand	100		11 11 11				23	66	28	3
	X	SS-2	\bigotimes	- clayey GRAVEL (GC) below 3.5 ft.	67		8 28 20				28			
	X	SS-3	\bigotimes		72		6 22 12				24			
	X	SS-4		Reddish brown, lean CLAY (CL), with gravel and sand Boring terminated at 10.0 ft.	100		8 12 10				23			
- – - – - 15– - – - – - – - – - – - – - – - – - –														
Date Date Engir Proje	Borir Borir neer/(ect No		ed: pleted: st:	10.0 4/2/20 4/2/20 AB 20202013.00 Present approximate strata boundaries.	illed wit ater wa	th CN as no	/IE-55 t enco	50 usi punte	ing H ered c	SA ar luring	nd au drilli	ito SI ng.	PT.	L

				RING NO. B-09						0040		chnical I		A	M	
Pro	oject	Desc	riptio	n: Monett Regional		ocation of I	Runv	vay 1	18-36	Kans	as City	01 Terra /, Misso	uri 64	153 🖌		
				Monett, Missouri) 599-7	7965 (8	316) 2	83-393	8 FAX	
Depth, feet	Samples	Sample #	Graphic Log	Surface EI.: Location: See Site Location	n Plan	I	Recovery %	RQD	Penetration Blows Per 6 inches	Hand Penetrometer, Qu TSF	Undrained Shear Strength, TSF	Unit Dry Weight, Ib/cu ft.	Water Content, %	Liquid Limit	Plastic Limit	Plasticity Index
			<u>, 1 / /, ·</u>	Brown, lean CLAY	′, with organ	ics	_									
	X	SS-1		FILL: Brown, lean gravel and sand			100		3 8 11				21			
	X	SS-2		Reddish brown, fa gravel and sand	t CLAY (CH), with	100		4 7 9				33	52	27	25
		SS-3					100		7 10 9				38			
	X	SS-4					89		27 12 18				46			
				Boring terminated												
Com Date Date Engir	Borin Borin	n Depth g Starte g Comp Geologis	ed: pleted:	10.0 4/1/20 4/1/20 AB 20202013.00		Boring drille Groundwate									PT.	



				RING NO. B-11 n: Monett Regional Monett, Missouri		cation of I	Runv	vay ′	18-36	8248 Kans	8 NW 1 sas City	chnical 01 Terra y, Misso 7965 (8	ace #5 uri 64	153		51
Depth, feet	Samples	Sample #	Graphic Log	Surface El.: Location: See Site Location	n Plan		Recovery %	RQD	Penetration Blows Per 6 inches	Hand Penetrometer, Qu TSF	Undrained Shear Strength, TSF	Unit Dry Weight, Ib/cu ft.	Water Content, %	Liquid Limit	Plastic Limit	Plasticity Index
				Brown, lean CLAY	, with organic	s	_									
	X	SS-1		Brown, lean CLAY gravel and sand	′ (FILL), with		89		7 6 11				16			
	X	SS-2		Reddish brown, le with gravel and sa	an CLAY (CL) nd),	94		5 12 21				24	48	25	2:
	X	SS-3					100		4 7 7				40			
	X	SS-4		- with coarse cher ft. Boring terminated	-	/ 8.5	89		12 15 11				34			
·																
Date Date	Borin Borin neer/(n Depth Ig Starte Ig Com Geologia	ed: pleted:	10.0 4/2/20 4/2/20 AB 20202013.00		oring drille roundwate									PT.	
				RING NO. B-12 n: Monett Regional Monett, Missouri	Airport Re	location of	Runv	vay ′	18-36	8248 Kans	3 NW 1 sas City	chnical 01 Terra /, Misso 7965 (8	ace #5 uri 64	153		s I
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Depth, feet	Samples	Sample #	Graphic Log	Surface El.: Location: See Site Location	n Plan	g	Recovery %	RQD	Penetration Blows Per 6 inches	Hand Penetrometer, Qu TSF	Undrained Shear Strength, TSF	Unit Dry Weight, Ib/cu ft.	Water Content, %	Liquid Limit	Plastic Limit	Plasticity Index
				Brown, lean CLAY	, with organ	nics										<u> </u>
	X	SS-1		Reddish brown, le trace gravel	an CLAY (C	JL),	100		5 5 7				20	39	21	18
	X	SS-2		- silty below 3.5 ft.			100		4 6 6				20			
	X	SS-3		Reddish brown, cl with sand (GC)	ayey GRAV	ÆL	100		4 13 33				19			
 	X	SS-4		Boring terminated	at 10.0 ft		78		8 21 23				18			
Com Date Date	Borir Borir heer/(n Depth ng Start ng Com Geologi .:	ed: pleted:	10.0 4/2/20 4/2/20 AB 20202013.00	Remarks:	Boring drille Groundwate									PT.	

				RING NO. B-13 n: Monett Regional Airport Relocation of Monett, Missouri	of Runy	way '	18-36	8248 Kans	3 NW 1 sas City	chnical 01 Terra /, Misso 7965 (8	ace #5 uri 64	153		51
Depth, feet	Samples	Sample #	Graphic Log	Surface EI.: Location: See Site and Boring Location Plan	Recovery %	RQD	Penetration Blows Per 6 inches	Hand Penetrometer, Qu TSF	Undrained Shear Strength, TSF	Unit Dry Weight, Ib/cu ft.	Water Content, %	Liquid Limit	Plastic Limit	Diceticity, Index
				Brown, lean CLAY, with organics										
	X	SS-1		Reddish brown, lean CLAY (CL), trace gravel	100		4 7 8				22	38	23	
5	X	SS-2		- reddish brown and gray below 3.5 ft.	100		4 7 9				25			
_	X	SS-3			100		4 7 11				20			
-	X	SS-4		Reddish brown and gray, clayey GRAVEL (GC) Boring terminated at 10.0 ft.	100		14 24 26				16			
 115 20 														
Date Date Engin	Borin Borin	n Depth Ig Starte Ig Com Geologia	ed: pleted:	10.0 Remarks: Boring dr 4/8/20 Groundw 4/8/20 AB 20202013.00									⊔ ⊃T.	1



				RING NO. B-15 n: Monett Regional Airport Rel Monett, Missouri	ocation of	Runv	vay ′	18-36	8248 Kans	NW 1 as City	chnical 01 Terra /, Misso /965 (8	ace #5 uri 64	153 🆌		S I
Depth, feet	Samples	Sample #	Graphic Log	Surface El.: Location: See Site and Boring Location Plan	3	Recovery %	RQD	Penetration Blows Per 6 inches	, Qu	Undrained Shear Strength, TSF	Unit Dry Weight, Ib/cu ft.	Water Content, %	Liquid Limit	Plastic Limit	Dlasticity Index
_	X	SS-1		Brown, lean CLAY, with organ Reddish brown and gray, lean CLAY (CL), trace gravel	nics	100		3 4 8				22	33	19	1
5 —	X	SS-2		-sandy from 3.5 to 5.0 ft. (66% passing No. 200 sieve)		100		4 7 10				22			
- with gravel and sand below 6.0 ft. - with chert gravel below 8.5 ft.												40			
 10 	X	SS-4		- with chert gravel below 8.5 f	l.	100		9 17 15				29			
5-															
- - 20-															
)ate)ate ingin	Borin Borin	n Depth ig Start ig Com Geologi	ed: pleted:		Boring drille Groundwate									 РТ.	

				RING NO. B-16 n: Monett Regional Airport Relocation of Monett, Missouri	of Run	way '	18-36	8248 Kans	3 NW 1 sas City	chnical 01 Terr y, Misso 7965 (8	ace #5 uri 64	153 🦼		5 I
Depth, feet	Samples	Sample #	Graphic Log	Surface El.: Location: See Site and Boring Location Plan	Recovery %	RQD	Penetration Blows Per 6 inches	Hand Penetrometer, Qu TSF	Undrained Shear Strength, TSF	Unit Dry Weight, Ib/cu ft.	Water Content, %	Liquid Limit	Plastic Limit	وتعلموا ويتوقعوا
			× 1,	Brown, lean CLAY, with organics										
	X	SS-1		Brown, lean CLAY (CL), with chert gravel and sand	100		3 5 15				23			
5 -		SS-2		- reddish brown below 3.5 ft.	100		4 8 10				29			
		SS-3			100		5 7 8				36			
-0		SS-4		Boring terminated at 10.0 ft.	100		6 6 9				37			
- - - - - - - - - - - - - - - - - 														
Date Date Engir	Borin Borin	n Depth Ig Start Ig Com Geologi	ed: pleted:	10.0 Remarks: Boring dr 4/8/20 Groundw 4/8/20 AB 20202013.00	illed wit ater wa	th CN is no	/IE-55 t enco	i0 usi ounte	ng H red c	⊥ SA ar Juring	nd au drilli	ito Sl ng.	<u></u> ⊃T.	<u> </u>



				RING NO. B-18 n: Monett Regional Airport Relo Monett, Missouri	ocation of R	unv	vay 1	18-36	8248 Kans	NW 1 as City	chnical 01 Terra /, Misso 7965 (8	ace #5 uri 64	153 🖌		5
Depth, feet	Samples	Sample #	Graphic Log	Surface EI.: Location: See Site and Boring Location Plan		Recovery %	RQD	Penetration Blows Per 6 inches	Hand Penetrometer, Qu TSF	Undrained Shear Strength, TSF	Unit Dry Weight, Ib/cu ft.	Water Content, %	Liquid Limit	Plastic Limit	Plasticity Index
		SS-1		Brown, lean CLAY, with organi Brown, lean CLAY (CL), with c gravel and sand	hert	83		4 6 14				18			
	X	SS-2				56		16 16 13				17			
	X	SS-3		- reddish brown below 6.0 ft.		94		4 8 11				19			
	X	SS-4		- trace gravel and sand below Boring terminated at 10.0 ft.		94		3 5 7				40			
 15 					Doring drillod										
Com Date Date Engir Proje	Borin Borin heer/(ect No		ed: pleted: st:	10.0 4/8/20 4/8/20 AB 20202013.00 present approximate strata boundaries.	Boring drilled Groundwater	l wit ˈwa	h CN s not	IE-55 enco	0 usi ounte	ng H red c	SA ar Iuring	id au drilli	ito SF ng.	PT.	

				n: Monett Regional Monett, Missouri	Airport Re	location of	Runv	vay ′	18-36	8248 Kans	NW 1 as City	chnical 01 Terra /, Misso 7965 (8	ace #5 uri 64	153 🖌		
Depth, feet	Samples	Sample #	Graphic Log	Surface EI.: Location: See Site Location	n Plan	g	Recovery %	RQD	Penetration Blows Per 6 inches	Hand Penetrometer, Qu TSF	Undrained Shear Strength, TSF	Unit Dry Weight, Ib/cu ft.	Water Content, %	Liquid Limit	Plastic Limit	Plasticity Index
		SS-1		Brown, lean CLAY Brown, lean CLAY gravel and sand	′, with orgai ′ (CL), with	nics chert	50		5 6 8				19			
		SS-2		- reddish brown, tr sand below 3.5 ft.	ace gravel	and	100		4 5 9				31			
		SS-3		- with chert gravel 6.0 ft.	and sand b	pelow	72		4 18 16				38			
 -10-	X	SS-4		Boring terminated	at 10.0 ft.		100		4 6 12				47			
	-															
·	-															
_	-															
20-	-															
-	-															
Date Date Engir	Borin Borin	n Depth ng Starte ng Com Geologis	ed: pleted:	10.0 4/8/20 4/8/20 AB 20202013.00	Remarks:	Boring drille Groundwate									PT.	

				RING NO. B-20						0240		hnical I 01 Terra		(A)	M	
Pro	oject	Desc	riptio	n: Monett Regional Monett, Missouri	Airport Relo	cation of F	Runv	vay 1	18-36	Kans	as City	, Missou	uri 64	153 🖌		
				•) 599-7	7965 (8	16) 28	33-393	8 FAX	
Depth, feet	Samples	Sample #	Graphic Log	Surface EI.: Location: See Site Location	Plan		Recovery %	RQD	Penetration Blows Per 6 inches	Hand Penetrometer, Qu TSF	Undrained Shear Strength, TSF	Unit Dry Weight, Ib/cu ft.	Water Content, %	Liquid Limit	Plastic Limit	Plasticity Index
			<u>, 1, 1,</u>	Brown, lean CLAY	, with organi	cs										
		SS-1		Brown, lean CLAY fragments and sar	' (CL), with cl nd	nert	67		3 6 12				18			
 5 -		SS-2		Reddish brown, fa chert gravel	t CLAY (CH)	, with	67		4 7 7				33	67	32	35
		SS-3					28		14 11 11				38			
	X	SS-4					89		4 6 12				40			
	-			Boring terminated						0		SA 00				
Some Com Com Date Some Some Com Some C	Borin Borin	n Depth ng Starte ng Comp Geologis	ed: pleted:	10.0 4/8/20 4/8/20 AB 20202013.00		Boring drille Groundwate									PT.	

				RING NO. B-21 n: Monett Regional Monett, Missouri	Airport Rel	ocation of	Runv	way '	18-36	8248 Kans	NW 1 as City	chnical 01 Terra /, Misso 7965 (8	ace #5 uri 64	153 🖌		
Depth, feet	Samples	Sample #	Graphic Log	Surface El.: Location: See Site Location	n Plan	I	Recovery %	RQD	Penetration Blows Per 6 inches	Hand Penetrometer, Qu TSF	Undrained Shear Strength, TSF	Unit Dry Weight, Ib/cu ft.	Water Content, %	Liquid Limit	Plastic Limit	Plasticity Index
			N 14. N	Brown, lean CLA	(with organ	icc				-						
	X	SS-1		Brown and gray, I trace gravel	ean CLAY (C	DL),	72		3 3 5				23			
 - 5 -		SS-2		- brown, with cher below 3.5 ft.	t gravel and	sand	56		8 11 22				30			
- brown and orange brown, trace gravel and sand below 6.0 ft. 28 8 11 12 24																
-25- Com Date Date Engir Proje	Borin Borin neer/C		ed: oleted: st:	10.0 4/8/20 4/8/20 AB 20202013.00 present approximate strata		Boring drille Groundwate	ed wit er wa	th CN	IE-55 t enco	i0 usi bunte	ng H red d	SA an Iuring	id au drilli	ito SI ng.	PT.	



L	C	OF	BO	RING NO. B-22								hnical I		A	M	
Pro	oject	Desc	riptio	n: Monett Regional		location of I	Runv	vay 1	8-36			01 Terra /, Missou			TS	
				Monett, Missouri) 599-7	7965 (8	16) 28	33-393	B FAX	
Depth, feet	Samples	Sample #	Graphic Log	Surface EI.: Location: See Site Location	n Plan	9	Recovery %	RQD	Penetration Blows Per 6 inches	Hand Penetrometer, Qu TSF	Undrained Shear Strength, TSF	Unit Dry Weight, Ib/cu ft.	Water Content, %	Liquid Limit	Plastic Limit	Plasticity Index
						1) with				±						
		SS-8		Reddish brown, fa chert gravel and s	and	,,,	61		14 4 8				50			
-30-				Boring terminated	at 30.0 ft.		_									
				Boring terminated	at 30.0 ft.											
	-															
-45-	_															
15/20																
BORINGS.GPJ 5/	-															
		n Depth Ig Starte		30.0 4/8/20		Boring drille Groundwate									νT.	
E Date	Borin	ig Comp Geologis	pleted:											5		
	ect No		51.	20202013.00												

				n: Monett Regional Monett, Missouri		of Runw	vay ′	18-36	8248 Kans	3 NW 1 sas City	chnical 01 Terra /, Misso 7965 (8	ace #5 uri 64	153 🖌		, Si
Depth, feet	Samples	Sample #	Graphic Log	Surface El.: Location: See Site Location	Plan	Recovery %	RQD	Penetration Blows Per 6 inches	Hand Penetrometer, Qu TSF	Undrained Shear Strength, TSF	Unit Dry Weight, Ib/cu ft.	Water Content, %	Liquid Limit	Plastic Limit	Plasticity Index
			<u>×1 1,</u>	Brown, lean CLAY	, with organics										
		SS-1		Brown and gray, le with gravel and sa	an CLAY (CL), nd	100		6 15 21				18			
		SS-2		- brown, gray and with coarse chert g from 3.5 to 8.5 ft.	orange brown, gravel and sand	100		10 12 15				19			
		SS-3				61		9 13 13				41			
 -10-		SS-4		- reddish brown an with sand, trace gr	d orange brown, avel below 8.5 ft.	89		5 9 9				39			
				Boring terminated											
Date Date Engir Proje	Borin Borin neer/(ect No		ed: pleted: st:	10.0 4/7/20 4/7/20 AB 20202013.00 present approximate strata b	Ground	drilled wit water wa								PT.	

				RING NO. B-24	Airport Relocation c	of Runv	vay '	18-36	8248	3 NW 1	chnical 01 Terr y, Misso	ace #5		M	-=
				Monett, Missouri					Nalis	599-1 5) 599-1	y, iviisso 7965 (a	316) 2	83-393	8 FAX	
Depth, feet	Samples	Sample #	Graphic Log	Surface El.: Location: See Site Location	n Plan	Recovery %	RQD	Penetration Blows Per 6 inches	Hand Penetrometer, Qu TSF	Undrained Shear Strength, TSF	Unit Dry Weight, Ib/cu ft.	Water Content, %	Liquid Limit	Plastic Limit	Plasticity Index
			× 1, × 7,	Brown, lean CLA	, with organics										
	X	SS-1		Brown, lean CLA	✓ (CL), with gravel	89		3 5 18				21			
	×	SS-2		Brown, clayey GR	AVEL (GC)	83		50/6"				19			
- 5 - 	X	SS-3		Reddish brown, g (MH), with sand	ravelly elastic SILT	100		6 13 38				38	75	37	38
 _ 10_		SS-4		Reddish brown, fa chert fragments a	It CLAY (CH), with nd sand	89		10 13 6				34			
 _ 15 _	-	ST-5				83						35			
 - 20-		SS-6		- trace gravel, sar chert fragments b	d and lignite, no elow 18.5 ft.	100		3 4 4				43			
 	-	ST-7		- with gravel and s and chert fragmer		88					65	52			
Comp Date Date	Borin Borin heer/C	n Depth g Starte g Comp Geologis	ed: pleted:	30.0 4/7/20 4/7/20 AB 20202013.00	Remarks: Boring dri Groundw									PT.	

L	C	OF	BO	RING NO. B-24								chnical I		(A)	M	
Pro	oject	Desc	riptio	n: Monett Regional		ocation of I	Runv	vay 1	8-36	Kans	as City	01 Terra /, Missou	uri 64	153 🖌		
				Monett, Missouri) 599-7	7965 (8	316) 28	33-393	8 FAX	
Depth, feet	Samples	Sample #	Graphic Log	Surface EI.: Location: See Site Location MATERIAL DE	Plan	I	Recovery %	RQD	Penetration Blows Per 6 inches	Hand Penetrometer, Qu TSF	Undrained Shear Strength, TSF	Unit Dry Weight, Ib/cu ft.	Water Content, %	Liquid Limit	Plastic Limit	Plasticity Index
				Reddish brown, fa	t CLAY (CH), with										
		SS-8		Reddish brown, fa gravel and sand		,,	28		2 3 3				53			
-30-				Boring terminated	at 30.0 ft.											
				Boring terminated	at 30.0 ft.											
 -45-																
0																
BORINGS.GPJ 5/15/2	-															
		n Depth		30.0 4/7/20		Boring drille									PT.	
E Date	Borin	ig Starte ig Comj Geologis	pleted:			Groundwate	er wa	5 1101		Junte	red 0	unng	arilli	ng.		
	ect No		51.	20202013.00												

				RING NO. B-25 n: Monett Regional A Monett, Missouri	irport Rel	ocation of	Runv	vay ′	18-36	8248 Kans	8 NW 1 as City	chnical 01 Terra /, Misso 7965 (8	ace #5 uri 64	153		51
Depth, feet	Samples	Sample #	Graphic Log	Surface EI.: Location: See Site a Location	Plan	3	Recovery %	RQD	Penetration Blows Per 6 inches	Hand Penetrometer, Qu TSF	Undrained Shear Strength, TSF	Unit Dry Weight, Ib/cu ft.	Water Content, %	Liquid Limit	Plastic Limit	Plasticity Index
			<u>1,1,1,1</u> ,1	Brown, lean CLAY,	with orgar	nics	_									
· _	X	SS-1		Reddish brown, lea trace gravel	n CLAY (C	CL),	100		3 6 8				19	32	19	1:
		SS-2		- with chert fragmer	nts below 3	3.5 ft.	100		5 9 20				19			
_	X	SS-3		Reddish brown, cla (GC)	yey GRAV	EL	100		16 34 26				24			
- - 10-	X	SS-4		Reddish brown, lea with gravel Boring terminated a		DL),	100		9 12 15				29			
 115 220 																
Date Date Engir	Borin Borin	n Depth ng Start ng Com Geologi .:	ed: pleted:	10.0 4/7/20 4/7/20 AB 20202013.00	Remarks:	Boring drille Groundwate									PT.	1

				RING NO. B-26 n: Monett Regional / Monett, Missouri	Airport Relocatio	n of Run	way	18-36	8248 Kans	3 NW 1 sas City	chnical 01 Terr y, Misso 7965 (8	ace #5 uri 64	153 🦼		51
Depth, feet	Samples	Sample #	Graphic Log	Surface El.: Location: See Site : Location	Plan	Recovery %	RQD	Penetration Blows Per 6 inches	Hand Penetrometer, Qu TSF	Undrained Shear Strength, TSF	Unit Dry Weight, Ib/cu ft.	Water Content, %	Liquid Limit	Plastic Limit	Plasticity Index
				Brown, lean CLAY	with organics										
	X	SS-1		Brown, lean CLAY	(CL), with gravel	100		4 6 8				30			
	X	SS-2		- reddish brown an ft.	d gray below 3.5	100		8 5 7				26			
				Gray, lean CLAY (CL), trace gravel										
		ST-3		(consolidation test)	100				2.70	109	19			
 -10-	X	SS-4				100		4 5 5				25	46	20	26
 - 15- 	-	ST-5		- with fine to coarse 13.0 ft.	e gravel below	100				0.43	104	23			
 - 20 		SS-6		- orange brown an gravel and sand fro	d gray, with om 18.5 to 28.5 ft.	100		3 4 6				22			
 -25-		ST-7		- reddish brown be		100				0.62	110	22			
Date Date	Borin Borin heer/C	n Depth g Start g Com Geologi .:	ed: pleted:	60.0 4/7/20 4/7/20 AB 20202013.00		drilled wit dwater wa								-1.	



				RING NO. B-26								hnical I		A	M	
Pro	oject	Desc	riptio	n: Monett Regional		ion of Ru	INW	/ay 1	8-36			01 Terra v, Missou			ΤĘ	
				Monett, Missouri) 599-7	'965 (8	16) 28	33-393	8 FAX	
Depth, feet	Samples	Sample #	Graphic Log	Surface EI.: Location: See Site Location	i Plan	Deroviary %	Recovery %	RQD	Penetration Blows Per 6 inches	Hand Penetrometer, Qu TSF	Undrained Shear Strength, TSF	Unit Dry Weight, Ib/cu ft.	Water Content, %	Liquid Limit	Plastic Limit	Plasticity Index
		SS-13		Brown, fat CLAY (trace lignite - brown and orang sand, no lignite be		10	00		567				29	53	20	33
-55- 		SS-14				10	00		, 5 5 8				22			
- 60 -				Boring terminated												
Com E Date Date Date Date	73 60.0 Remarks: Boring drilled with CME-550 using HSA and auto SPT. Date Boring Started: 4/7/20 Groundwater was not encountered during drilling. Date Boring Completed: 4/7/20 Groundwater was not encountered during drilling. Engineer/Geologist: AB 20202013.00															

				RING NO. B-27 n: Monett Regional Airport Relocatior Monett, Missouri	n of Run	way	18-36	8248 Kans	3 NW 1 sas City	chnical 01 Terr /, Misso 7965 (8	ace #{ ouri 64	153 🦼		, 51
Depth, feet	Samples	Sample #	Graphic Log	Surface El.: Location: See Site and Boring Location Plan	Recovery %	RQD	Penetration Blows Per 6 inches	Hand Penetrometer, Qu TSF	Undrained Shear Strength, TSF	Unit Dry Weight, Ib/cu ft.	Water Content, %	Liquid Limit	Plastic Limit	Plasticity Index
			<u>x 1, .</u>	Brown, lean CLAY, with organics										
· _	X	SS-1		☑ Brown and gray, lean CLAY (CL), with sand and gravel	78		2 3 4				23			
- 5	X	SS-2		- reddish brown from 3.5 to 6.0 ft.	100		8 6 5				24	35	18	1
_		ST-3		- reddish brown and gray, silty, no sand and gravel below 6.0 ft.	96				0.37	102	23			
10-		SS-4			100		3 5 6				21			
- - 15-	-	ST-5		- gray from 13.0 to 18.5 ft. (consolidaton test)	100				1.12	111	18			
- - 20- -		SS-6		- brown, gray and orange brown, with gravel and sand below 18.5 ft.	100		3 4 4				24			
_ _ 25-		ST-7		- reddish brown with coarse sand and fine gravel below 23.0 ft.	104				0.18		21			
Com Date Date Engir Proje	Borin Borin neer/C		ed: oleted: st:	4/6/20 Ground	drilled wi Iwater wa									

				RING NO. B-27 n: Monett Regional Airport Relocation of Monett, Missouri	of Run	way '	18-36	8248 Kans	3 NW 1 sas City	chnical 01 Terr y, Misso 7965 (8	ace #5 uri 64	153 🖌) 51
Depth, feet	Samples	Sample #	Graphic Log	Surface EI.: Location: See Site and Boring Location Plan	Recovery %	RQD	Penetration Blows Per 6 inches	, Qu	Undrained Shear Strength, TSF		Water Content, %	Liquid Limit	Plastic Limit	Plasticity Index
				Reddish brown, lean CLAY (CL), with sand and gravel			4							
- 30		SS-8		- trace gravel from 28.5 to 45.0 ft.	100		777				29			
 35 -		SS-9		- brown, gray and orange brown from 33.5 to 38.5 ft.	100		3 3 5				25	38	17	2'
- 40- -		SS-10		- gray from 38.5 to 43.5 ft.	100		4 3 4				24			
_ 45- _		SS-11		- brown and gray below 43.5 ft.	100		3 4 5				22			
_ _ 50-	pletior Borin Borin beer/C ect No	SS-12		- with gravel and sand below 48.5 ft.	100		4 4 6				19			
Comp Date Date Engir Proje	pletior Borin Borin neer/C ect No	n Depth g Starte g Com Geologis	ed: pleted: st:	60.0 Remarks: Boring dr 4/6/20 4/6/20 AB 20202013.00 present approximate strata boundaries.	illed wit ater wa	th CN is en	/IE-55 count	i0 usi ered	ng H durir	SA ar ng dril	nd au ling a	ito SI at 1.0	PT. ft.	

LC	C	OF	BO	RING NO. B-27	,							hnical I		A	M	
Pro	oject	Desc	riptio	n: Monett Regional		location of I	Runv	vay 1	8-36			01 Terra /, Missou			ΤS	
				Monett, Missouri								7965 (8				
Depth, feet	Samples	Sample #	Graphic Log	Surface El.: Location: See Site Location	ı Plan	g	Recovery %	RQD	Penetration Blows Per 6 inches	Hand Penetrometer, Qu TSF	Undrained Shear Strength, TSF	Unit Dry Weight, Ib/cu ft.	Water Content, %	Liquid Limit	Plastic Limit	Plasticity Index
 - 55- 		SS-13		Reddish brown, le with sand and grav		CL),	100		7 8 9				20			
		SS-14					100		4 10				23			
				Boring terminated		Boring drille		h Ch	15 55	0.05		SA 20			T	
≤ Com	Borir Borir neer/(n Depth ng Starte ng Comp Geologis	ed: pleted:	60.0 4/6/20 4/6/20 AB 20202013.00	Remarks:	Boring drille Groundwate	ed wit er wa	h CN s end	1E-55 count	0 usi ered	ng H durin	SA an Ig drill	d au ing a	to SF at 1.0	PT. ft.	



				RING NO. B-28					0040		chnical I 01 Terra		1	M	
Pr	oject	Desc	riptio	n: Monett Regional Monett, Missouri		of Runy	vay ′	18-36	Kans	sas City	, Misso	uri 64	153 🆌		
				· ·							7965 (8 	316) 2	83-393	8 FAX	
Depth, feet	Samples	Sample #	Graphic Log	Surface EI.: Location: See Site Location	n Plan	Recovery %	RQD	Penetration Blows Per 6 inches	Hand Penetrometer, Qu TSF	Undrained Shear Strength, TSF	Unit Dry Weight, Ib/cu ft.	Water Content, %	Liquid Limit	Plastic Limit	Plasticity Index
- · ·		SS-8		Reddish brown, le trace gravel		100		2 3 3				24			
		SS-9		Reddish brown, fa chert gravel [✓] - with chert gravel	t CLAY (CH), with below 33.5 ft.	94		8 8 15				17			
		SS-10				72		4 10 9				32			
		SS-11				100		5 7 12				26	58	27	31
B BORINGS.GPJ		SS-12				72		5 10 9				38			
Com HIM Date Date O Eng	Completion Depth: 60.0 Remarks: Boring drilled with CME-550 using HSA and auto SPT. Date Boring Started: 4/2/20 Groundwater was encountered during drilling at 33.5 ft. Date Boring Completed: 4/2/20 Engineer/Geologist: AB Project No.: 20202013.00														

L	.00	G	OF	BO	RING NO. B-28	6							hnical I		<u>(</u>	M	
P	roje	ect	Desc	riptio	n: Monett Regional		location of I	Runv	vay 1	8-36	Kans	as City	01 Terra , Missou	uri 64	153 🖌		Š
					Monett, Missouri) 599-7	'965 (8	16) 2	83-393	8 FAX	
Depth. feet	Compos Compos	samples	Sample #	Graphic Log	Surface El.: Location: See Site Location	n Plan	9	Recovery %	RQD	Penetration Blows Per 6 inches	Hand Penetrometer, Qu TSF	Undrained Shear Strength, TSF	Unit Dry Weight, Ib/cu ft.	Water Content, %	Liquid Limit	Plastic Limit	Plasticity Index
_ _ _ _ _ _ _ _			SS-13 SS-14		Red, fat CLAY (Cł	H), with che	rt	67		4 7 11 5 7				43			
					Boring terminated					9							
YI Co Da Da Da En	mple te Bc te Bc	orin orin er/O	n Depth g Starte g Comp Geologis	ed: pleted:	60.0 4/2/20 4/2/20 AB 20202013.00	Remarks:	Boring drille Groundwate										

				RING NO. B-29 n: Monett Regional Monett, Missouri	Airport Rel	ocation of I	Runv	vay '	18-36	8248 Kans	3 NW 1 sas City	chnical 01 Terr /, Misso 7965 (8	ace #5 uri 64	153 🦼		51
Depth, feet	Samples	Sample #	Graphic Log	Surface El.: Location: See Site Location		3	Recovery %	RQD	Penetration Blows Per 6 inches	Hand Penetrometer, Qu TSF	Undrained Shear Strength, TSF	Unit Dry Weight, Ib/cu ft.	Water Content, %	Liquid Limit	Plastic Limit	Plasticity Index
				MATERIAL DE	ESCRIPTION					Har	S		[
	X	SS-1		Brown, lean CLA Reddish brown, le with gravel and sa	<u>′, with organ</u> an CLAY (C ind	li <u>cs</u> / iL),	100		3 2 4				25			
		SS-2		- with coarse grav	el below 3.5	ft.	50		2 4 5				25			
		ST-3					100						20			
		SS-4		Reddish brown, S sand and coarse	ILT (ML), wi gravel	th	89		5 3 5				41	48	28	20
		ST-5		Reddish brown, le with sand and coa	an CLAY (C Irse gravel	;L),	95						27			
_ _ 20 — _	×	SS-6		Reddish brown, cl (GC), with limesto	ayey GRAV ne fragmen	EL ts	83		25 50/0"				37			
		ST-7		- with sand below (43% passing No.	200 sieve)		92				0.07	77	32			
Comp Date Date Engir Proje	Borin Borin neer/C ct No		ed: pleted: st:	42.7 4/2/20 4/2/20 AB 20202013.00 present approximate strata		Boring drille Groundwate refusal enco	er wa	is no	t enco	ounte						

Γ	LC)G	OF	BO	RING NO. B-29								chnical I		A	M	
	Pro	ject	Desc	riptio	n: Monett Regional		ocation of I	Runv	vay 1	8-36	Kans	as City	01 Terra /, Misso	uri 64	153 🖌	TS	
-					Monett, Missouri) 599-7	7965 (8	316) 2	33-393	8 FAX	
	Depth, feet	Samples	Sample #	Graphic Log	Surface El.: Location: See Site Location	n Plan	I	Recovery %	RQD	Penetration Blows Per 6 inches	Hand Penetrometer, Qu TSF	Undrained Shear Strength, TSF	Unit Dry Weight, Ib/cu ft.	Water Content, %	Liquid Limit	Plastic Limit	Plasticity Index
-							1)				-						
			SS-8		Reddish brown, le with gravel, sand, fragments	and limestor	né'	17		2 2 2				19			
			SS-9		- brown below 33.	5 ft.		0		3 1 WH							
	- 40		SS-10		Boring terminated	at 42.7 ft.		28		WR WR 1				43			
	- 45 - 50				40.7												
Completion Depth: 42.7 Date Boring Started: 4/2/20 Date Boring Completed: 4/2/20 Engineer/Geologist: AB Project No.: 20202013.00										enco	ounte						

			RING NO. B-30 n: Monett Regional Airport Relocation Monett, Missouri	n of Runy	way ′	18-36	8248 Kans	3 NW 1 sas City	chnical 01 Terra y, Misso 7965 (8	ace #5 uri 64	153 🖌		51
Depth, feet Samples	Sample #	Graphic Log	Surface EI.: Location: See Site and Boring Location Plan	Recovery %	RQD	Penetration Blows Per 6 inches	Hand Penetrometer, Qu TSF	Undrained Shear Strength, TSF	Unit Dry Weight, Ib/cu ft.	Water Content, %	Liquid Limit	Plastic Limit	Plasticity Index
	SS-1		Brown, lean CLAY, with organics Reddish brown, lean CLAY (CL), trace gravel	94		3 3 4				21			
5	SS-2		- with gravel and sand below 3.5 ft.	100		5 10 15				20			
	SS-3		- reddish brown below 6.0 ft.	100		11 10 8				33			
10-	SS-4		- with chert gravel below 8.5 ft. Boring terminated at 10.0 ft.	78		6 12 11				34			
20 – – – – 25 Dompletio Date Borir			10.0 Remarks: Boring 4/8/20 Ground 4/8/20	drilled wit	th CN	/IE-55	0 usi	ing H	SA ar	nd au	Ito Sf	PT.	

				RING NO. B-31						0040		chnical I 01 Terra		<i>(</i>	M	
Pr	oject	t Desc	riptio	n: Monett Regional Monett, Missouri		location of I	Runv	vay 1	8-36	Kans	as City	, Missou	uri 64	153 🆌		
	1	1) 599-7	7965 (8	16) 2	83-393	8 FAX	
Depth, feet	Samples	Sample #	Graphic Log	Surface EI.: Location: See Site Location	I Plan	g	Recovery %	RQD	Penetration Blows Per 6 inches	Hand Penetrometer, Qu TSF	Undrained Shear Strength, TSF	Unit Dry Weight, Ib/cu ft.	Water Content, %	Liquid Limit	Plastic Limit	Plasticity Index
		SS-1		Brown, lean CLAY FILL: Reddish bro (CL), with chert gr	′, with orgar wn, lean CL avel and sa	nics _AY nd	83		3 10 21				24			
 - 5 -	X	SS-2					33		16 10 10				24			
		SS-3					83		7 7 8				31	86	29	57
		SS-4					83		3 10 12				30			
		n Deoth		Boring terminated		Boring drille	-d with	h CM	1E-55		ng H	SA an	dau	to Si	PT	
Date Date	e Borin e Borin	n Depth ng Starte ng Comp Geologis D.:	ed: pleted:	10.0 4/8/20 4/8/20 AB 20202013.00	Remarks:	Boring drille Groundwate									ין. 	

				RING NO. B-32 n: Monett Regional Airp Monett, Missouri	ort Relocation	of Runv	vay ′	18-36	8248 Kans	3 NW 1 sas City	chnical 01 Terra /, Misso 7965 (8	ace #5 uri 64	153 🆌		51
Depth, feet	Samples	Sample #	Graphic Log	Surface El.: Location: See Site and Location Pla	n	Recovery %	RQD	Penetration Blows Per 6 inches	Hand Penetrometer, Qu TSF	Undrained Shear Strength, TSF	Unit Dry Weight, Ib/cu ft.	Water Content, %	Liquid Limit	Plastic Limit	Plasticity Index
			<u>1,1,1,1,1</u>	Brown, lean CLAY, wit	h organics										
	X	SS-1		Brown and gray, lean with gravel and sand	CLAY (CL),	100		5 37 25				26			
 - 5	X	SS-2		- brown, with chert frag 3.5 ft.	gments below	67		15 50/3"				10			
	X	SS-3		Brown, fat CLAY (CH) gravel	, with chert	100		4 14 23				17	58	26	3
- - 10-		SS-4				28		10 13 17				25			
-		SS-5		- brown and reddish bi 13.5 ft.	own below	100		12 19				45			
15— 						_		12							
_ 20-		SS-6		Brown and reddish bro GRAVEL (GC)	wn, clayey	68		50/4.5'				21			
- 	-	ST-7		Brown and reddish bro (CH), with chert gravel	own, fat CLAY	100					56	51			
Date Date Engir Proje	Borin Borin neer/C	n Depth Ig Start Ig Com Geologi	ed: pleted: st:	4/9/20	marks: Boring di Groundw									PT.	

L	C	OF	BO	RING NO. B-32		TSi Geotechnical Inc. 8248 NW 101 Terrace #5												
Pro	oject	Desc	riptio	n: Monett Regional		location of I	Runv	way 1	8-36	Kans	as City	, Missou	uri 64	64153 TS I				
				Monett, Missouri) 599-7	7965 (8	16) 28	33-393	B FAX			
Depth, feet	Samples	Sample #	Graphic Log	Surface EI.: Location: See Site Location	n Plan	9	Recovery %	RQD	Penetration Blows Per 6 inches	Hand Penetrometer, Qu TSF	Undrained Shear Strength, TSF	Unit Dry Weight, Ib/cu ft.	Water Content, %	Liquid Limit	Plastic Limit	Plasticity Index		
				Brown and reddis	n brown, fat	CLAY				-								
		SS-8		Brown and reddisl (CH), with chert gr	avel		56		7 8 8				29					
-30-				Boring terminated	at 30.0 ft.													
				Boring terminated	at 30.0 ft.													
 -45-	-																	
BORINGS.GPJ 5/15/20	-																	
		n Depth		30.0 4/9/20	Remarks:	Boring drille									Ϋ́Τ.			
E Date	Borin	ig Starte ig Comj Geologis	pleted:			Groundwate	er wa	15 1101		Junte		uring	ariiil	ng.				
	ect No		σι.	20202013.00														

LOG OF BORING NO. B-33 Project Description: Monett Regional Airport Relocation of Runway 18-36 Monett, Missouri TSi Geotechnical Inc. 8248 NW 101 Terrace #5 Kansas City, Missouri 64153 (816) 599-7965 (816) 283-3938 FAX															51	
Depth, feet	Samples	Sample #	Graphic Log	Surface El.: Location: See Site a Location	Plan	I	Recovery %	RQD	Penetration Blows Per 6 inches	Hand Penetrometer, Qu TSF	Undrained Shear Strength, TSF	Unit Dry Weight, Ib/cu ft.	Water Content, %	Liquid Limit	Plastic Limit	Plasticity Index
		SS-1		Brown, lean CLAY, Brown, lean CLAY and sand	with organ (CL), with g	ics jravel	67		5 10 7				19	34	18	16
 - 5 -	X	SS-2				56		5 6 10				29				
	X	SS-3		- reddish brown bel	- reddish brown below 6.0 ft.								25			
	X	SS-4		- red, brown and gray below 8.5 ft. Boring terminated at 10.0 ft.					6 8 13				21			
		n Dentif				Boring drille	ed wit	h CA	/E-55	i0 usi	ng H	SA ar		to SI	PT	
Comp Date Date Engir Proje	Borin Borin neer/(ect No		ed: pleted: st:	10.0 4/9/20 4/9/20 AB 20202013.00 present approximate strata bo		Boring drille Groundwate	ed wit er wa	h CN s not	IE-55 t enco	i0 usi bunte	ng H red c	SA ar Iuring	nd au drilli	ito SF ng.	PT.	

LOG OF BORING NO. B-34 TSi Geotechnical Inc. Project Description: Monett Regional Airport Relocation of Runway 18-36 TSi Geotechnical Inc. Monett, Missouri Missouri (816) 599-7965 (816) 283-3938 FAX																
Depth, feet	Samples	Sample #	Graphic Log	Surface El.: Location: See Site Location	n Plan		Recovery %	RQD	Penetration Blows Per 6 inches	Hand Penetrometer, Qu TSF	Undrained Shear Strength, TSF	Unit Dry Weight, Ib/cu ft.	Water Content, %	Liquid Limit	Plastic Limit	Plasticity Index
		SS-1		Brown, lean CLAY Reddish brown, le trace gravel and s	′, with organics an CLAY (CL), and		100		4 4 5				21			
 - 5 -		SS-2		- reddish brown ar to 6.0 ft.	reddish brown and gray from 3.5 o 6.0 ft.								18			
	X	SS-3		- reddish brown, n below 6.0 ft. - brown and gray b	reddish brown, no gravel and sand elow 6.0 ft.								27			
 -10-	X	SS-4		Boring terminated			83		4 9 10				18			
 -15																
 -20																
Date Date Engir Proje	Borin Borin neer/C		ed: pleted: st:	10.0 4/9/20 4/9/20 AB 20202013.00 present approximate strata b	Gro	ring drille oundwate									PT.	

				RING NO. B-35 n: Monett Regional Airpor Monett, Missouri	t Relocation o	fRunv	vay '	18-36	8248 Kans	NW 1 as City	chnical 01 Terr /, Misso 7965 (8	ace #5 uri 64	153 🆌		50
Depth, feet	Samples	Sample #	Graphic Log	Surface El.: Location: See Site and B Location Plan	ION	Recovery %	RQD	Penetration Blows Per 6 inches	Hand Penetrometer, Qu TSF	Undrained Shear Strength, TSF	Unit Dry Weight, Ib/cu ft.	Water Content, %	Liquid Limit	Plastic Limit	
_	X	SS-1		Brown, lean CLAY, with Reddish brown, silty lear (CL)	organics I CLAY	100		3 5 5				21	31	19	
- 5 -		ST-2		- reddish brown and gray gravel below 3.0 ft.	v, trace	54				1.19	95	28			
_		SS-3		Reddish brown, fat CLA chert gravel and sand	/ (CH), with	67		5 11 8				16			
- 10-		ST-4				92				1.58	89	37			
- - 15-		SS-5				100		3 4 6				35	68	26	
- 20-		SS-6				100		3 13 7				26			
- - 25-		SS-7				78		4 3 4				14			
Com Date Date Engii	Borin Borin	n Depth g Start g Com Geologi .:	ed: pleted:	30.0 Rema 4/9/20 4/9/20 AB 20202013.00	rks: Boring dril Groundwa									ΡŢ.	

	C	OF	BO	RING NO. B-35		TSi Geotechnical Inc. 8248 NW 101 Terrace #5										
Pro	oject	Desc	riptio	n: Monett Regional		ocation of I	Runv	vay 1	8-36	Kans	as City	, Missou	uri 64	153 🖌	TS	
	1			Monett, Missouri) 599-7	'965 (8	16) 28	33-393	B FAX	
Depth, feet	Samples	Sample #	Graphic Log	Surface EI.: Location: See Site Location	n Plan		Recovery %	RQD	Penetration Blows Per 6 inches	Hand Penetrometer, Qu TSF	Undrained Shear Strength, TSF	Unit Dry Weight, Ib/cu ft.	Water Content, %	Liquid Limit	Plastic Limit	Plasticity Index
				Reddish brown, fa	t CLAY (CH)	. with				_						
				Reddish brown, fa chert gravel and s - reddish brown ar gravel, no sand be	1											
		SS-8		gravel, no sand be	elow 28.5 ft.		100		2 2				26			
- 30 - 	-			Boring terminated												
	-															
-45-																
BORINGS.GPJ 5/15/20	-															
E Date Date Date Engi	S Completion Depth: 30.0 Remarks: Boring drilled with CME-550 using HSA and auto SPT. Date Boring Started: 4/9/20 Groundwater was not encountered during drilling. Date Boring Completed: 4/9/20 Groundwater was not encountered during drilling. Engineer/Geologist: AB AB															
2 Proje	ect No) .:		20202013.00												

												hnical I	#5 M				
Pro	oject	Desc	riptio	 Monett Regional Monett, Missouri 		location of I	Runv	vay 1	8-36	8248 NW 101 Terrace #5 Kansas City, Missouri 64153							
				· · · · · · · · · · · · · · · · · · ·							Í	7965 (8 	16) 2	33-393	8 FAX		
Depth, feet	Samples	Sample #	Graphic Log	Surface EI.: Location: See Site Location	n Plan	g	Recovery %	RQD	Penetration Blows Per 6 inches	Hand Penetrometer, Qu TSF	Undrained Shear Strength, TSF	Unit Dry Weight, Ib/cu ft.	Water Content, %	Liquid Limit	Plastic Limit	Plasticity Index	
			<u>, 1, 1, .</u> . 77877	Brown, lean CLAY	, with orgar	nics											
	X	SS-1		Reddish brown, si (CL), trace gravel	Ity lean CLA	ΑY	100		2 2 4				21				
 - 5 -	X	SS-2					100		5 6 9				24	38	21	17	
	X	SS-3					100		5 8 10				22				
	X	SS-4		Boring terminated	-1 10 0 4		100		7 9 23				19				
Com Date	Borin Borin neer/(n Depth g Starte g Comp Geologis	ed: pleted:	10.0 4/9/20 4/9/20 AB 20202013.00	Remarks:	Boring drille Groundwate									РТ.		






				RING NO. B-40 n: Monett Regional Airport Reloc Monett, Missouri	ation of Runy	way	18-36	8248 Kans	8 NW 1 as City	chnical 01 Terra /, Misso 7965 (8	ace #5 uri 64	153 🦼		
Depth, feet	Samples	Sample #	Graphic Log	Surface El.: Location: See Site and Boring Location Plan	Recovery %	RQD	Penetration Blows Per 6 inches	Hand Penetrometer, Qu TSF	Undrained Shear Strength, TSF	Unit Dry Weight, Ib/cu ft.	Water Content, %	Liquid Limit	Plastic Limit	Plasticity Index
			<u>x 1/2</u> <u>x</u>	Brown, lean CLAY, with organics	3			_						
	X	SS-1		Reddish brown, lean CLAY (CL) with gravel and sand	, 89		4 5 8				25	32	20	12
 - 5 -	X	SS-2			61		10 13 21				37			
	X	SS-3			100		4 11 15				40			
 -10-	X	SS-4		Boring terminated at 10.0 ft.	50		10 15 20				43			
	-													
 -15-	-													
	-													
 -20-	-													
	-													
	-													
Date Date	Borir Borir heer/(n Depth ng Start ng Com Geologi o.:	ed: pleted:	4/9/20 Gr	oring drilled wit oundwater wa	th CN as no	/IE-55 t enco	i0 usi ounte	ng H red c	SA ar Iuring	nd au drilli	ito SI ing.	ΡŢ.	-

				RING NO. B-41 n: Monett Regional Airport Relocation of Monett, Missouri	of Run	way	18-36	8248 Kans	3 NW 1 sas City	chnical 01 Terra /, Misso 7965 (8	ace #5 uri 64	153 🦼		30
Depth, feet	Samples	Sample #	Graphic Log	Surface EI.: Location: See Site and Boring Location Plan	Recovery %	RQD	Penetration Blows Per 6 inches	Hand Penetrometer, Qu TSF	Undrained Shear Strength, TSF	Unit Dry Weight, Ib/cu ft.	Water Content, %	Liquid Limit	Plastic Limit	Plasticity Index
			<u>× 1,</u> 77×77	Brown, lean CLAY, with organics										
	X	SS-1		Reddish brown, lean CLAY (CL), with gravel and sand	100		3 5 6				21			
	X	SS-2		- trace gravel and sand, silty below 3.5 ft.	100		3 7 9				24	45	25	2
	X	SS-3			100		3 7 12				25			
		SS-4		Reddish brown, clayey GRAVEL (GC) Boring terminated at 10.0 ft.	89		5 16 19				18			
 15 20 22														
Date Date	Borin Borin neer/(n Depth ng Starte ng Com Geologis	ed: pleted:	10.0 Remarks: Boring dr 4/9/20 Groundw 4/9/20 AB 20202013.00	illed wit ater wa	th CN as no	/IE-55 t enco	i0 usi punte	ing H red c	SA ar luring	nd au drilli	ito SI	ЪТ.	

				RING NO. B-42 n: Monett Regional Airport Relocation o Monett, Missouri	of Run	way ′	18-36	8248 Kans	3 NW 1 sas City	chnical 01 Terra y, Misso 7965 (8	ace #5 uri 64	153 🦼		
Depth, feet	Samples	Sample #	Graphic Log	Surface El.: Location: See Site and Boring Location Plan	Recovery %	RQD	Penetration Blows Per 6 inches	Hand Penetrometer, Qu TSF	Undrained Shear Strength, TSF	Unit Dry Weight, Ib/cu ft.	Water Content, %	Liquid Limit	Plastic Limit	Plasticity Index
			× 1,	Brown, lean CLAY, with organics										
	X	SS-1		Brown, lean CLAY (CL), with gravel and sand	6		5 8 12				10			
_ 5 —	X	SS-2		- reddish brown below 3.5 ft.	94		10 10 10				33			
_	X	SS-3			100		5 13 9				41			
- 10-	X	SS-4		Boring terminated at 10.0 ft.	89		11 14 30				32			
- - 115 - - - 20 - - - - - -														
Date Date Engir	Borin Borin	n Depth Ig Starte Ig Com Geologia	ed: pleted:	10.0 Remarks: Boring dri 4/9/20 Groundwa 4/9/20 AB 20202013.00	lled wi ater wa	th CN	/E-55 t enco	50 usi ounte	ing H red c	SA ar luring	nd au drilli	uto Sl ing.	PT.	



				RING NO. B-44 n: Monett Regional Airport R Monett, Missouri	Relocation of I	Runv	way '	18-36	8248 Kans	3 NW 1 sas City	chnical 01 Terr y, Misso 7965 (8	ace #5 uri 64	153 🦼		
Depth, feet	Samples	Sample #	Graphic Log	Surface El.: Location: See Site and Bor Location Plan		Recovery %	RQD	Penetration Blows Per 6 inches	Hand Penetrometer, Qu TSF	Undrained Shear Strength, TSF	Unit Dry Weight, Ib/cu ft.	Water Content, %	Liquid Limit	Plastic Limit	Plasticity Index
			<u>x 1,. x</u>	Brown, lean CLAY, with org	anics	_									
	X	SS-1		Brown and reddish brown, I CLAY (CL), with gravel	ean	50		7 20 8				30	37	20	1
_ 5 -	X	SS-2		- with chert and limestone g below 3.5 ft.	jravel	83		3 16 12				24			
_	X	SS-3				100		5 12 12				33			
_ 10-	X	SS-4		Boring terminated at 10.0 ft		100		2 11 17				35			
- - - 115 - - - - 20 - - - - - - - - - - - - - - - - 															
Date Date Engir	Borin Borin	n Depth Ig Starte Ig Com Geologis	ed: pleted:	10.0 Remarks: 4/1/20 4/1/20 DD 20202013.00	Boring drille Groundwate	ed wit er wa	th CN is not	/IE-55 t enco	i0 usi ounte	ng H red c	SA ar luring	nd au drilli	ito SI ng.	⊐T.	<u> </u>





				RING NO. B-47						0240		hnical I 01 Terra		<i>(</i>	M	L .
Pro	oject	Desc	riptio	 Monett Regional Monett, Missouri 		location of	Runv	vay 1	8-36	Kans	as City	, Misso	uri 64	153 🆌	TE	
				· · · · · · · · · · · · · · · · · · ·								7965 (8	316) 2	83-393	8 FAX	
Depth, feet	Samples	Sample #	Graphic Log	Surface El.: Location: See Site Location	n Plan	g	Recovery %	RQD	Penetration Blows Per 6 inches	Hand Penetrometer, Qu TSF	Undrained Shear Strength, TSF	Unit Dry Weight, Ib/cu ft.	Water Content, %	Liquid Limit	Plastic Limit	Plasticity Index
			·			ing				I						
		SS-1		Brown, lean CLAY Brown and reddisl CLAY (CL)			56		3 10 10				17			
 - 5 -	X	SS-2		- with chert gravel	below 3.5 f	t.	83		3 5 5				33			
		SS-3		- with chert and lin below 6.0 ft.	nestone gra	ivel	83		5 15 15				35			
	X	SS-4					83		7 19 15				37			
	-	n Dontifi		Boring terminated		Boring drille	ad wit	b CM	15-55	0 usi		SA an			T	
Com E Date Date Date C Engi	Borir Borir	n Depth ng Starte ng Comp Geologis	ed: pleted:	10.0 4/2/20 4/2/20 DD 20202013.00	Remarks:	Boring drille Groundwate 41.0 ft. towa	er wa	s not								

				RING NO. B-48						0240		chnical I 01 Terra		(A)	M	
Pro	oject	Desc	riptio	n: Monett Regional	Airport Rel	ocation of I	Runv	vay 1	18-36	Kans	as City	, Misso	uri 64	153 🖌		
				Monett, Missouri) 599-7	7965 (8	16) 28	33-393	8 FAX	
Depth, feet	Samples	Sample #	Graphic Log	Surface EI.: Location: See Site Location	I Plan	J	Recovery %	RQD	Penetration Blows Per 6 inches	Hand Penetrometer, Qu TSF	Undrained Shear Strength, TSF	Unit Dry Weight, Ib/cu ft.	Water Content, %	Liquid Limit	Plastic Limit	Plasticity Index
			<u>, 1, 1, 1, 1</u>	Brown, lean CLAY	, with organ	ics	-									
		SS-1		Brown, lean CLAY gravel			72		4 13 13				21			
5 -		SS-2		Reddish brown, fa limestone and che	t CLAY (CH ert gravel), with	100		6 5 5				44	51	26	25
		SS-3					100		5 10 15				44			
	X	SS-4					100		5 10 12				43			
	-			Boring terminated		Boring drille	dwit		AE 55	0.05		SA an			DT	
Com E Date Date 0 Date	Borin Borin	n Depth ng Starte ng Comp Geologis o.:	ed: pleted:	10.0 4/1/20 4/1/20 DD 20202013.00		Boring drille Groundwate									PT.	

				RING NO. B-49 n: Monett Regional Airport Relocation Monett, Missouri	n of Runv	way '	18-36	8248 Kans	3 NW 1 sas City	chnical 01 Terra /, Misso 7965 (8	ace #5 uri 64	153 🦼		50
Depth, feet	Samples	Sample #	Graphic Log	Surface El.: Location: See Site and Boring Location Plan	Recovery %	RQD	Penetration Blows Per 6 inches	Hand Penetrometer, Qu TSF	Undrained Shear Strength, TSF	Unit Dry Weight, Ib/cu ft.	Water Content, %	Liquid Limit	Plastic Limit	Placticity Index
			× 1,, · . 77777	Brown, lean CLAY, with organics										
_	X	SS-1		Reddish brown, lean CLAY (CL), trace gravel	17		4 7 8				27			
5 -	X	SS-2		Reddish brown, fat CLAY (CH), with chert and limestone gravel	100		3 4 7				40			
_	X	SS-3		- brown and reddish brown below 6.0 ft.	100		3 13 16				36			
_ 0_ _	X	SS-4		- with chert gravel, no limestone gravel below 8.5 ft.	83		7 13 13				40			
	X	SS-5		Reddish brown, elastic SILT (MH), with chert gravel	100		6 12 9				41	59	31	2
	X	SS-6		Reddish brown, fat CLAY (CH), with chert	56		2 7 9				37			
20 — — — —				Boring terminated at 20.0 ft.										
ate I ate I ngin	Borin Borin	n Depth g Starte g Comp Geologis	ed: pleted:	4/2/20 Ground	drilled wit Iwater wa								⊔ ⊃Т.	<u>I</u>

				RING NO. B-50						0040		hnical I 01 Terra		<i>M</i>	M	
Pro	oject	Desc	riptio	n: Monett Regional		on of Rur	nwa	y 1	8-36	Kans	as City	, Misso	uri 64	153 🧹	TE	SI
				Monett, Missouri) 599-7	7965 (8	316) 2	83-393	8 FAX	
Depth, feet	Samples	Sample #	Graphic Log	Surface EI.: Location: See Site Location	i Plan	Recovery %		אמח	Penetration Blows Per 6 inches	Hand Penetrometer, Qu TSF	Undrained Shear Strength, TSF	Unit Dry Weight, Ib/cu ft.	Water Content, %	Liquid Limit	Plastic Limit	Plasticity Index
			× 1, 77822	Brown, lean CLAY												
	X	SS-1		Brown, lean CLAY gravel	′ (CL), trace	56			2 7 7				26	32	18	14
 - 5 -	X	SS-2		- reddish brown, w below 3.5 ft.	ith chert gravel	56			3 6 8				27			
	X	SS-3		- reddish brown ar ft.	nd gray below 6.0	72			3 5 7				23			
	X	SS-4				100			3 5 6				40			
10				Boring terminated	at 10.0 ft.											
- 15- 	-															
	-															
	-															
-20-																
26-25- 1 Com		n Depth		10.0	Remarks: Boring	g drilled w	/ith (СМ	F-55	0 1191	na H	SA an	d ai	Ito SF		
Date	Borin Borin neer/(ig Starte ig Comp Geologis	ed: pleted:	4/1/20		ndwater w										

Pro	oject	Desc	riptio	n: Monett Regional Airport Relocation of Monett, Missouri	of Run	way '	18-36	Kans	sas City	01 Terr /, Misso 7965 (8	uri 64	153		31
Depth, feet	Samples	Sample #	Graphic Log	Surface El.: Location: See Site and Boring Location Plan	Recovery %	RQD	Penetration Blows Per 6 inches	Hand Penetrometer, Qu TSF	Undrained Shear Strength, TSF	Unit Dry Weight, Ib/cu ft.	Water Content, %	Liquid Limit	Plastic Limit	Plasticity Index
			× 14	Brown, lean CLAY, with organics				-						
_	X	SS-1		Brown and reddish brown, lean CLAY (CL), trace gravel	83		3 3 5				36			
- 5 -	X	SS-2		Reddish brown, fat CLAY (CH), with chert gravel	56		5 5 6				40			
_	X	SS-3			56		4 9 15				29	53	25	2
- 10-		SS-4		- with chert and limestone gravel below 8.5 ft.	56		8 16 18				33			
_ 15		SS-5			72		4 9 14				38			
_ _ 20-		SS-6			33		3 4 4				19			
				Boring terminated at 20.0 ft.										
Date Date Engir	Borin Borin	n Depth Ig Starti Ig Com Geologi	ed: pleted:	20.0 Remarks: Boring dri 4/2/20 Groundwa 4/2/20 DD 20202013.00	lled wit ater wa	th CN Is no	/IE-55 t enco	50 usi Dunte	ing H ered c	SA ar Iuring	nd au drilli	ito SI ng.	<u>Р</u> Т.	<u> </u>

				RING NO. B-52 n: Monett Regional Airport Relo Monett, Missouri	cation of Ru	INW	ay 1	18-36	8248 Kans	NW 1 as City	chnical 01 Terra /, Misso 7965 (8	ace #5 uri 64	153 🖌		51
Depth, feet	Samples	Sample #	Graphic Log	Surface El.: Location: See Site and Boring Location Plan	Recovery %	Necuvery /0	RQD	Penetration Blows Per 6 inches	Hand Penetrometer, Qu TSF	Undrained Shear Strength, TSF	Unit Dry Weight, Ib/cu ft.	Water Content, %	Liquid Limit	Plastic Limit	Placticity Index
			× 1,	Brown, lean CLAY, with organic	cs										
	X	SS-1		Brown and reddish brown, lean CLAY (CL)		2		1 2 6				21	28	18	1
- 5 -	X	SS-2		- brown, trace limestone gravel below 3.5 ft.	7:	2		5 9 8				41			
 		SS-3		- reddish brown and brown, wit chert gravel below 6.0 ft.	h 10	00		3 3 5				29			
-	X	SS-4		Boring terminated at 10.0 ft.	10	00		3 5 10				44			
Date Date Engir	Borin Borin	n Depth ng Starte ng Com Geologia	ed: pleted:	4/2/20	Boring drilled v Groundwater v									PT.	<u> </u>

				RING NO. B-53						0240		hnical I 01 Terra		(A)	M	L .
Pro	oject	Desc	riptio	n: Monett Regional Monett, Missouri		ition of F	Runv	vay 1	8-36	Kans	as City	, Missou	uri 64	153 🖌		Sī
										-		7965 (8	16) 28	33-393	8 FAX	
Depth, feet	Samples	Sample #	Graphic Log	Surface EI.: Location: See Site Location	I Plan		Recovery %	RQD	Penetration Blows Per 6 inches	Hand Penetrometer, Qu TSF	Undrained Shear Strength, TSF	Unit Dry Weight, Ib/cu ft.	Water Content, %	Liquid Limit	Plastic Limit	Plasticity Index
			<u>., 1, .</u>	Brown, lean CLAY	with organics					_						
	X	SS-1		Brown and reddish CLAY (CL), trace	n brown, lean gravel		22		2 4 4				24			
 - 5 -		SS-2		Reddish brown, fa chert gravel, trace			56		2 7 12				45			
 		SS-3		- reddish brown ar ft.	nd gray below 6	.0	83		2 6 9				40			
10-	X	SS-4					33		3 7 7				46			
				Boring terminated												
Com Date Date Engir	Borin Borin	n Depth g Starte g Comp Geologis	ed: pleted:	10.0 4/1/20 4/1/20 DD 20202013.00		ing drille oundwate									PT.	

				RING NO. B-54 n: Monett Regional Ai Monett, Missouri	irport Rel	ocation of	Run	way ′	18-36	8248 Kans	NW 1 as City	chnical 01 Terr /, Misso 7965 (8	ace #5 uri 64	153 🖌		
Depth, feet	Samples	Sample #	Graphic Log	Surface El.: Location: See Site an Location F	Plan	9	Recovery %	RQD	Penetration Blows Per 6 inches	Hand Penetrometer, Qu TSF	Undrained Shear Strength, TSF	Unit Dry Weight, Ib/cu ft.	Water Content, %	Liquid Limit	Plastic Limit	Plasticity Index
			<u>, 1,</u>	Brown, lean CLAY, v	with orgar	nics										
	X	SS-1		Brown and reddish b CLAY (CL), with grav	prown, lea vel	an	22		2 15 9				26			
- 5 -		SS-2		- with chert and lime below 3.5 ft.	stone gra	ivel	72		3 9 13				34	44	24	2
_	X	SS-3					56		6 10 10				39			
- 10-	X	SS-4		Boring terminated at	100ft		83		4 9 4				40			
 115 20 																
Date Date Engir	Borin Borin	n Depth Ig Starte Ig Com Geologis	ed: pleted:	4/2/20	Remarks:	Boring drille Groundwate	ed wit er wa	th CN Is not	/E-55 t enco	0 usi ounte	ng H red c	SA ar Iuring	id au drilli	ito SI ng.	PT.	





				RING NO. C-02						0040		chnical I 01 Terra		(A)	M	
Pro	oject	Desc	riptio	n: Monett Regional Monett, Missouri		location of I	Runv	vay 1	18-36	Kans	as City	, Misso	uri 64	153 🖌	TE	
				· ·) 599-7	7965 (8	16) 28	33-393	8 FAX	
Depth, feet	Samples	Sample #	Graphic Log	Surface EI.: Location: See Site Location	ı Plan	g	Recovery %	RQD	Penetration Blows Per 6 inches	Hand Penetrometer, Qu TSF	Undrained Shear Strength, TSF	Unit Dry Weight, Ib/cu ft.	Water Content, %	Liquid Limit	Plastic Limit	Plasticity Index
				CONCRETE 7.0"												
		SS-1		BASE GRAVEL 5. Brown, fat CLAY (and sand		gravel	78		6 10 10				27			
 - 5 -	X	SS-2	\bigotimes				83		5 17 15				28			
		SS-3	\bigotimes				100		8 4 8				46	82	34	48
	X	SS-4	\bigotimes				94		12 10 11				28			
	-			Boring terminated		Poring drillo	d wit		1 55	0.00				SDT		
≤ Com E Date Date C Engii	Borin Borin	n Depth g Starte g Com Geologis	ed: pleted:	10.0 4/1/20 4/1/20 AB 20202013.00	Remarks:	Boring drille Groundwate	ed wit er wa	h CN s not	1E-55 : enco	i0 usi ounte	ng F/ red d	A and luring	auto drilli) SPT ng.		

				RING NO. C-03 n: Monett Regional Airport Relocation o Monett, Missouri	of Run	way '	18-36	8248 Kans	3 NW 1 sas City	chnical 01 Terra y, Misso 7965 (8	ace #5 uri 64	153 🖌		Si
Depth, feet	Samples	Sample #	Graphic Log	Surface El.: Location: See Site and Boring Location Plan	Recovery %	RQD	Penetration Blows Per 6 inches	Hand Penetrometer, Qu TSF	Undrained Shear Strength, TSF	Unit Dry Weight, Ib/cu ft.	Water Content, %	Liquid Limit	Plastic Limit	Dontioth - Indon
			2. 5. A.	CONCRETE 7.0"										
-	-		° XXX	BASE GRAVEL 5.0"										
_	X	SS-1	\bigotimes	Brown, lean CLAY (FILL), with gravel and sand	44		9 16 15				12			
_	×	SS-2		Reddish brown, clayey GRAVEL with sand (GC)	83		50/2.5	•			7			
5 -				Reddish brown, fat CLAY (CH), with sand and gravel	-		6							
_		SS-3			100		8 15				48			
_	Y	SS-4		Reddish brown, sandy lean CLAY (CL), with gravel	92		31				13			
					02		50/5"							
10- - -				Boring terminated at 10.0 ft.										
5-														
_														
_														
_														
20-														
_														
_														
25														
omp ate ate	Borin Borin	n Depth g Starte g Com Geologis	ed: pleted:	10.0Remarks:Boring dri4/1/20Groundwa4/1/20AB	lled wit ater wa	th CN Is no	/IE-55 t enco	i0 usi ounte	ing F. ered c	A and luring	auto drilli	o SP1 ng.	Γ.	

				RING NO. C-04						0040		hnical I		1	M	
Pro	oject	Desc	riptio	n: Monett Regional		ocation of	Runv	vay 1	18-36			01 Terra /, Missou			ΤĘ	5
				Monett, Missouri								7965 (8			8 FAX	_
Depth, feet	Samples	Sample #	Graphic Log	Surface El.: Location: See Site Location	n Plan	9	Recovery %	RQD	Penetration Blows Per 6 inches	Hand Penetrometer, Qu TSF	Undrained Shear Strength, TSF	Unit Dry Weight, Ib/cu ft.	Water Content, %	Liquid Limit	Plastic Limit	Plasticity Index
				CONCRETE 7.0"												
		SS-1		BASE GRAVEL 4. Brown, lean CLAY gravel and sand	′ (FILL), with		72		5 5 6				32			
5 -		SS-2		Reddish brown, fa gravel and sand	t CLAY (CH	I), with	83		6 3 4				35	51	26	25
 		SS-3		- brown and gray b	pelow 6.0 ft.		100		7 3 5				17			
		SS-4		Boring terminated	1 40 0 6		100		7 9 11				18			
	-															
Com Date Date Ω Engii	Completion Depth: 10.0 Remarks: Boring drilled with CME-550 using FA and auto SPT. Date Boring Started: 4/1/20 Groundwater was not encountered during drilling. Date Boring Completed: 4/1/20 Engineer/Geologist: AB Project No.: 20202013.00															

				RING NO. C-05					0040		chnical I		<i>(</i>	M	
Proj	ject	Desc	riptio		Airport Relocation o	of Runv	vay 1	8-36	Kans	as City	01 Terra /, Missou	uri 64	153 🦨		SI
				Monett, Missouri) 599-7	7965 (8	316) 2	83-393	8 FAX	
Depth, feet	Samples	Sample #	Graphic Log	Surface EI.: Location: See Site Location	n Plan	Recovery %	RQD	Penetration Blows Per 6 inches	Hand Penetrometer, Qu TSF	Undrained Shear Strength, TSF	Unit Dry Weight, Ib/cu ft.	Water Content, %	Liquid Limit	Plastic Limit	Plasticity Index
			8. K.A	CONCRETE 7.0"					±						
	X	SS-1		BASE GRAVEL 3. Brown, lean CLAY gravel and sand	′ (FILL), with	72		5 10 7				27	40	13	27
 - 5 -	X	SS-2		Reddish brown, le with gravel and sa	an CLAY (CL), nd	78		5 4 5				31			
	X	SS-3				100		6 4 5				32			
10 10	X	SS-4		- reddish brown ar ft.		67		6 6 9				17			
				Boring terminated											
25 Completion Depth: 10.0 Remarks: Boring drilled with CME-550 using FA and auto SPT. Date Boring Started: 4/1/20 Groundwater was not encountered during drilling. Date Boring Completed: 4/1/20 Engineer/Geologist: AB Project No.: 20202013.00															

				RING NO. C-06 n: Monett Regional Airport Relocation Monett, Missouri	n of Run	way	18-36	8248 Kans	8 NW 1 as City	chnical 01 Terra y, Misso	ace #5 uri 64	153 🖌		SI
Depth, feet	Samples	Sample #	Graphic Log	Surface El.: Location: See Site and Boring Location Plan	Recovery %	RQD	Penetration Blows Per 6 inches	, Qu	Undrained Shear Strength, TSF 662	Unit Dry Weight, 5962 Ib/cu ft.	Water Content, % 5 (91	Liquid Limit	Plastic Limit X	
				CONCRETE 6.5"										
_	X	SS-1			72		5 4 5				21			
- 5 -		SS-2	\bigotimes		78		7 5 5				16			
_		SS-3		Reddish brown, lean CLAY (CL), with gravel and sand	100		4 7 3				28			
- - 10-		SS-4		- reddish brown and gray below 8.5 ft. Boring terminated at 10.0 ft.	83		9 9 7				18			
Date Date Engir	Borin Borin	n Depth Ig Starte Ig Com Geologis	ed: pleted:	4/1/20 Ground	drilled wi Iwater wa									<u> </u>

LC	C	OF	BO	RING NO. C-07	,							hnical I 01 Terra		(h)	M	
Pro	oject	Desc	riptio	n: Monett Regional		location of I	Runv	vay 1	8-36	Kans	as City	, Missou	uri 64	153 🖌	TE	
				Monett, Missouri) 599-7	7965 (8	16) 28	33-393	8 FAX	
Depth, feet	Samples	Sample #	Graphic Log	Surface El.: Location: See Site Location	n Plan	9	Recovery %	RQD	Penetration Blows Per 6 inches	Hand Penetrometer, Qu TSF	Undrained Shear Strength, TSF	Unit Dry Weight, Ib/cu ft.	Water Content, %	Liquid Limit	Plastic Limit	Plasticity Index
				MATERIAL DE	SCRIPTION					Ï						
			0	CONCRETE 6.5" BASE GRAVEL 5.	0"		_									
		SS-1		Brown, fat CLAY (and sand		gravel	100		6 6 14				35			
 - 5 -	X	SS-2	\bigotimes				67		7 10 10				22			
		SS-3					56		9 14 13				28			
	X	SS-4		Reddish brown, le with chert gravel a		;L),	100		5 5 6				34			
	-			Boring terminated		Poring drillo										
S Com E Date Date C Engin	Borin Borin	n Depth g Starte g Comp Geologis	ed: pleted:	10.0 4/1/20 4/1/20 AB 20202013.00	Remarks:	Boring drille Groundwate										

				RING NO. C-08 n: Monett Regional Airport Relocation o Monett, Missouri	fRun	way	18-36	8248 Kans	3 NW 1 sas City	chnical 01 Terr y, Misso 7965 (8	ace #5 uri 64	153		
Depth, feet	Samples	Sample #	Graphic Log	Surface El.: Location: See Site and Boring Location Plan	Recovery %	RQD	Penetration Blows Per 6 inches	Hand Penetrometer, Qu TSF	Undrained Shear Strength, TSF	Unit Dry Weight, Ib/cu ft.	Water Content, %	Liquid Limit	Plastic Limit	Diceticity, Index
				CONCRETE 7.0"										
_	X	SS-1		BASE GRAVEL 5.0" Reddish brown, elastic SILT (FILL), with gravel and sand	100		4 5 5				36	54	30	2
	X	SS-2		Reddish brown, fat CLAY (CH), with gravel and sand	50		11 18 13				30			
_	X	SS-3		- no sand below 6.0 ft.	100		10 15 13				33			
- - 10-	X	SS-4		Boring terminated at 10.0 ft.	94		5 11 15				45			
- - 115 - - - 20 - - - -														
Date Date Engin	Borin Borin	n Depth Ig Starte Ig Com Geologis	ed: pleted:	10.0 Remarks: Boring dri 4/1/20 Groundwa 4/1/20 AB 20202013.00									<u> </u> Γ.	<u> </u>

				RING NO. C-09						0210		hnical I 01 Terra		ſ	M	
Pro	oject	Desc	riptio	n: Monett Regional Monett, Missouri		location of I	Runv	vay 1	18-36	Kans	as City	, Missou	uri 64	153 🖌		
									1) 599-7	7965 (8	16) 28	33-393	8 FAX	
Depth, feet	Samples	Sample #	Graphic Log	Surface EI.: Location: See Site Location	n Plan	9	Recovery %	RQD	Penetration Blows Per 6 inches	Hand Penetrometer, Qu TSF	Undrained Shear Strength, TSF	Unit Dry Weight, Ib/cu ft.	Water Content, %	Liquid Limit	Plastic Limit	Plasticity Index
					SCRIPTION					I						
			0	CONCRETE 8.0" BASE GRAVEL 3.	E "											
	X	SS-1	\bigotimes	Brown and gray, le with gravel and sa		FILL),	56		6 7 13				18			
	X	SS-2	\bigotimes				72		3 3 4				34			
	X	SS-3					50		8 8 6				18			
	X	SS-4		Reddish brown, le with chert gravel a		SL),	44		4 5 5				22			
				Boring terminated		Doring drillo										
Com E Date Date C Engir	Borin Borin	n Depth Ig Starte Ig Comp Geologis	ed: pleted:	10.0 4/1/20 4/1/20 AB 20202013.00	Remarks:	Boring drille Groundwate										

				RING NO. C-10 n: Monett Regional Airport Relocation of Monett, Missouri	Run	way	18-36	8248 Kans	3 NW 1 sas City	chnical 01 Terra y, Misso 7965 (8	ace #5 uri 64	153 🦼		
Depth, feet	Samples	Sample #	Graphic Log	Surface EI.: Location: See Site and Boring Location Plan	Recovery %	RQD	Penetration Blows Per 6 inches	Hand Penetrometer, Qu TSF	Undrained Shear Strength, TSF	Unit Dry Weight, Ib/cu ft.	Water Content, %	Liquid Limit	Plastic Limit	Discticity, Index
				CONCRETE 8.5"										
_			$\overset{\circ}{\times}$	BASE GRAVEL 3.5"	\vdash		6							
	Å	SS-1	\bigotimes	Brown, and gray, lean CLAY (FILL), with gravel and sand	39		7 9				21			
- 5 -	X	SS-2		Reddish brown, lean CLAY (CL), with gravel and sand	100		6 4 5				33	42	21	2
	X	SS-3			50		8 12 10				21			
_ 10_	X	SS-4		- with coarse chert gravel below 8.5 ft. Boring terminated at 10.0 ft.	100		9 15 15				35			
- - 15 - - - 20 - - - - -														
Date Date Engir	Borin Borin	n Depth Ig Starte Ig Com Geologis	ed: pleted:	10.0 Remarks: Boring drill 4/1/20 Groundwa 4/1/20 AB 20202013.00									<u> </u> T.	<u> </u>

GENERAL NOTES

The number of borings is based on: topographic and geologic factors; the magnitude of structure loading; the size, shape, and value of the structure; consequences of failure; and other factors. The type and sequence of sampling are selected to reduce the possibility of undiscovered anomalies and maintain drilling efficiency. Attempts are made to detect and/or identify occurrences during drilling and sampling such as the presence of water, boulders, gas, zones of lost circulation, relative ease or resistance to drilling progress, unusual sample recovery, variation in resistance to driving split-spoon samplers, unusual odors, etc. However, lack of notation regarding these occurrences does not preclude their presence.

Although attempts are made to obtain stabilized groundwater levels, the levels shown on the Logs of Boring may not have stabilized, particularly in more impermeable cohesive soils. Consequently, the indicated groundwater levels may not represent present or future levels. Groundwater levels may vary significantly over time due to the effects of precipitation, infiltration, or other factors not evident at the time indicated.

Unless otherwise noted, soil classifications indicated on the Logs of Boring are based on visual observations and are not the result of classification tests. Although visual classifications are performed by experienced technicians or engineers, classifications so made may not be conclusive.

Generally, variations in texture less than one foot in thickness are described as layers within a stratum, while thicker zones are logged as individual strata. However, minor anomalies and changes of questionable lateral extent may appear only in the verbal description. The lines indicating changes in strata on the Logs of Boring are approximate boundaries only, as the actual material change may be between samples or may be a gradual transition.

Samples chosen for laboratory testing are selected in such a manner as to measure selected physical characteristics of each material encountered. However, as samples are recovered only intermittently and not all samples undergo a complete series of tests, the results of such tests may not conclusively represent the characteristics of all subsurface materials present.

NOTATION USED ON BORING LOGS

APPROXIMAT	E PROPORTIONS			PARTICLE SIZE
TRACE WITH MODIFIER	<15% 15-30% >30%	BOULI Cobbi Gravi	LES	>12 Inches 12 Inches – 3 Inches
-		SAND	Coarse Fine	3 Inches – ¾ Inch ¾ Inch – No. 4 Sieve (4.750 mm)
	Ũ	SILT CLAY	Coarse Medium Fine	No. 4 – No. 10 Sieve (2.000 mm) No. 10 – No. 40 Sieve (0.420 mm) No. 40 – No. 200 Sieve (0.074 mm) No. 200 Sieve - 0.002 mm < 0.002 mm

PENETRATION – BLOWS

Number of impacts of a 140-pound hammer falling a distance of 30 inches to cause a standard split-barrel sampler, 1 3/8 inches I.D., to penetrate a distance of 6 inches. The number of impacts for the first 6 inches of penetration is known as the seating drive. The sum of the impacts for the last 12 inches of penetration is the Standard Penetration Test Resistance or "N" value, blows per foot. For example, if blows = 6-8-9, "N" = 8+9 or 17.

OTHER NOTATIONS

Recovery % – length of recovered soil divided by length of sample attempted.

- 50/2" Impacts of hammer to cause sampler to penetrate the indicated number of inches
- Sampler penetrated under the static loading of the weight of the drill rods WR
- Sampler penetrated under the static loading the weight of the hammer and drill rods WH
- HSA Hollow stem auger drilling method
- Flight auger drilling method FA
- Rotary wash drilling methods with drilling mud RW
- Automatic hammer used for Standard Penetration Test sample AH
- SH Safety hammer with rope and cathead used for Standard Penetration Test sample

GRAPHIC SYMBOLS

- ∇ Depth at which groundwater was encountered during drilling
- T Depth at which groundwater was measured after drilling
- X Standard Penetration Test Sample, ASTM D1586
 - 3-inch diameter Shelby Tube Sample, ASTM D1587
- G Sample grabbed from auger
- NX Size rock core sample

UNIFIED SOIL CLASSIFICATION SYSTEM, (ASTM D-2487)

Maj	or Divis	sions	Gre Symi		Typical Names	L	abo	oratory Classification C	Criteria
	on is)	Clean gravels (Little or no fines)	G	W	Well-graded gravels, gravel- sand mixtures, little or no fines	coarse-		$C_u = \underline{D_{60}}$ greater than 4; $C_c = (\underline{D}_{10})$	$(30)^2$ between 1 and 3 $(x D_{60})^2$
iize)	urse fracti sieve size	Clean g (Little or	G	Р	Poorly graded gravels, gravel- sand mixtures, little or no fines	e size), e	ial symbo	Not meeting all gradation rec	quirements for GW
Coarse-grained soils (More than half of materials is larger than No. 200 sieve size)	Gravels (More than half of coarse fraction is larger than No. 4 sieve size)	Gravels with fines (Appreciable amount of fines)	GM ^a	d	Silty gravels, gravel-sand-silt mixtures	Determine percentages of sand and gravel from grain-size curve. Depending on percentage of fines (fraction smaller than No. 200 sieve size), coarse- Grained soils are classified as follows: Less than 5 per cent GW, GP, SW, SP More than 12 per cent GM, GC, SM, SC 5 to 12 per cent Borderline coses requiring dual cumbolob	es requiring di	Atterberg limits below "A" line or P.1. less than 4	Above "A" line with P.1. between 4 and 7 are <i>borderline</i>
ned soils arger than N	(More th larg	Gravels (Apprecia of f	G	u C	Clayey gravels, gravel-sand- clay mixtures	Determine percentages of sand and gravel from grain-size curve. Depending on percentage of fines (fraction smaller than No. 200 Grained soils are classified as follows: Less than 5 per cent GW, GP, SW, SP More than 12 per cent GM, GC, SM, SC 5 to 12 per cent	derline cas	Atterberg limits below "A" line with P.1. greater than 7	cases requiring use of dual symbols
Coarse-grained soils aterials is larger thar	ion is e)	Clean sands ttle or no fines)	SV	N	Well-graded sands, gravelly sands, little or no fines	i (fraction (ows: GW GN Boy		$C_u = \frac{D_{60}}{D_{10}}$ greater than 6; $C_c = (D)$	$(30)^2$ between 1 and 3 x D ₆₀
C half of ma	s oarse fract 4 sieve siz	Clean sands (Little or no fines)	S	Р	Poorly graded sands, gravelly sands, little or no fines	of sand an ge of fines fied as foll	N	Not meeting all gradation requir	rements for SW
More than	Sands (More than half of coarse fraction is smaller than No. 4 sieve size)	Sands with fines (Appreciable amount of fines)	SM ^a	d	Silty sands, sand-mix mixtures	Determine percentages of sand and gra Depending on percentage of fines (frac Grained soils are classified as follows: Less than 5 per cent More than 12 per cent	cent	Atterberg limits about "A" line or P.I. less than 4	Limits plotting in hatched zone with P.I. between 4 and
)	∕lore tha small€	Sands with fines ppreciable amou of fines)		u		ending en	12 per cent	Atterberg limits about "A"	7 are <i>borderline</i> cases requiring use
	(J	Sa (App	S	С	Clayey sands, sand-clay mixtures	Deter Depe Grair Less More		line with P.I. greater than 7	of dual symbols
	lays)	М	L	Inorganic silts and very fine sands, rock flour, silty or clayey fine sands, or clayey silts with slight plasticity				
00 sieve size)	Silts and clays	than 50)	C	Ĺ	Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clays	60 For c	classif	floation of fine-grained sails	
1 No. 20			0	L	Organic silts and organic silty clays of low plasticity	H 50 - Soils Equat X Horiz W then	<u>s</u> tion of zontal n PI=(<u>ELINE</u>
Fine-grained soils erials is smaller that	lys	learer	М	Н	Inorganic silts, micaceous or diatomaceous fine sandy or silty soils, elastic silts	then D	tion of ical at n PI = (f A - line at PI=4 to LL=25.5, or the Or3 (LL-20) f U'-line t LL =16 to PI=7 O.9 (LL-8) O'	
Fine-grained soils (More than half of materials is smaller than No. 2	Silts and clays	(Luquu mm greater than 50)	C	H	Inorganic clays of medium to high plasticity, organic silts	10- 7		CH MH OR	ОН
1 half of 1			0	Н	Organic clays of medium to high plasticity, organic silts	°, K	10 16	I I I I I I I I I I I I I I I I I I I	80 90 100 110
(More that	Highly organic	soils	Р	t	Peat and other highly organic soils				
		1.014		. 1 1	visions of d and u are for roads and		<u>a</u> 1	1	

^aDivision of GM and SM groups into subdivisions of d and u are for roads and airfields only. Subdivision is based on Atterberg limits; suffix d used when L.L. is 26 or less and the P.1. is 6 or less; the suffix u used when L.L. is greater than 28.

^bBorderline classifications, used for soils possessing characteristics of two groups, are designated by combinations of group symbols. For example: GW-GC, well-graded gravel-sand mixture with clay binder.

T:\Geotechnical Group\Notes for Geotech Reports\Unified Soil Classifications System2.doc

APPENDIX C



LAB BORINGS.GPJ US

TSI ATTERBERG LIMITS



ДA BORINGS.GPJ US

TSI ATTERBERG LIMITS



TSI ATTERBERG LIMITS BORINGS.GPJ US_LAB.GDT


L L L L L L 2 ġ BORINGS **GRAIN SIZE**



UNCONFINED COMPRESSION TEST: BORING B-26 / ST-5

PROJECT NAME: Monett Regional Airport Relocation of Runway 18-36 PROJECT LOCATION: Monett, Missouri PROJECT NUMBER: 20202013.00

BORING NO.	B-26
Sample No.	ST-5
Sample Depth (ft)	14
Visual Classification (USCS)	CL

ASTM DESIGNATION	
Unconfined Compression	ASTM D2166
Atterberg Limits	ASTM D4318
Visual Classification (USCS)	ASTM D2488

ATTERBERG LIMITS	
Liquid Limit	
Plastic Limit	
Plasticity Index	
Classification (USCS)	

DENSITY & MOISTURE	
Wet Unit Weight (pcf)	128.1
Moisture Content (%)	22.9
Dry Unit Weight (pcf)	104.3

STRENGTH]
Undrained Shear Strength, S ((tsf)	0.43
% Strain at q _u	2.7





UNCONFINED COMPRESSION TEST: BORING B-26 / ST-7

PROJECT NAME: Monett Regional Airport Relocation of Runway 18-36 PROJECT LOCATION: Monett, Missouri PROJECT NUMBER: 20202013.00

BORING NO.	B-26
Sample No.	ST-7
Sample Depth (ft)	24
Visual Classification (USCS)	CL

ASTM DESIGNATION	
Unconfined Compression	ASTM D2166
Atterberg Limits	ASTM D4318
Visual Classification (USCS)	ASTM D2488

ATTERBERG LIMITS]
Liquid Limit	
Plastic Limit	
Plasticity Index	
Classification (USCS)	

DENSITY & MOISTURE	
Wet Unit Weight (pcf)	134.5
Moisture Content (%)	22.2
Dry Unit Weight (pcf)	110.0

STRENGTH	
Undrained Shear Strength, S _u (tsf)	0.62
% Strain at q _u	15.0





UNCONFINED COMPRESSION TEST: BORING B-27 / ST-3

PROJECT NAME: Monett Regional Airport Relocation of Runway 18-36 PROJECT LOCATION: Monett, Missouri PROJECT NUMBER: 20202013.00

BORING NO.	B-27
Sample No.	ST-3
Sample Depth (ft)	7
Visual Classification (USCS)	CL

ASTM DESIGNATION	
Unconfined Compression	ASTM D2166
Atterberg Limits	ASTM D4318
Visual Classification (USCS)	ASTM D2488

ATTERBERG LIMITS	
Liquid Limit	
Plastic Limit	
Plasticity Index	
Classification (USCS)	

DENSITY & MOISTURE	
Wet Unit Weight (pcf)	125.7
Moisture Content (%)	23.3
Dry Unit Weight (pcf)	102.0

STRENGTH]
Undrained Shear Strength, S ((tsf)	0.37
% Strain at q _u	2.5





UNCONFINED COMPRESSION TEST: BORING B-27 / ST-7

PROJECT NAME: Monett Regional Airport Relocation of Runway 18-36 PROJECT LOCATION: Monett, Missouri PROJECT NUMBER: 20202013.00

B-27
ST-7
23.5
CL

ASTM DESIGNATION	
Unconfined Compression	ASTM D2166
Atterberg Limits	ASTM D4318
Visual Classification (USCS)	ASTM D2488

ATTERBERG LIMITS	
Liquid Limit	
Plastic Limit	
Plasticity Index	
Classification (USCS)	

DENSITY & MOISTURE	
Wet Unit Weight (pcf)	128.7
Moisture Content (%)	21.2
Dry Unit Weight (pcf)	106.2

STRENGTH	
Undrained Shear Strength, S _u (tsf)	0.18
% Strain at q $_{\rm u}$	11.1





UNCONFINED COMPRESSION TEST: BORING B-28 / ST-3

PROJECT NAME: Monett Regional Airport Relocation of Runway 18-36 PROJECT LOCATION: Monett, Missouri PROJECT NUMBER: 20202013.00

BORING NO.	B-28
Sample No.	ST-3
Sample Depth (ft)	7
Visual Classification (USCS)	CL

ASTM DESIGNATION	
Unconfined Compression	ASTM D2166
Atterberg Limits	ASTM D4318
Visual Classification (USCS)	ASTM D2488

ATTERBERG LIMITS	
Liquid Limit	
Plastic Limit	
Plasticity Index	
Classification (USCS)	

DENSITY & MOISTURE	
Wet Unit Weight (pcf)	124.7
Moisture Content (%)	23.1
Dry Unit Weight (pcf)	101.3

STRENGTH	1
Undrained Shear Strength, S ((tsf)	0.84
% Strain at q _u	5.3





UNCONFINED COMPRESSION TEST: BORING B-28 / ST-5

PROJECT NAME: Monett Regional Airport Relocation of Runway 18-36 PROJECT LOCATION: Monett, Missouri PROJECT NUMBER: 20202013.00

BORING NO.	B-28
Sample No.	ST-5
Sample Depth (ft)	14
Visual Classification (USCS)	CL

ASTM DESIGNATION	
Unconfined Compression	ASTM D2166
Atterberg Limits	ASTM D4318
Visual Classification (USCS)	ASTM D2488

ATTERBERG LIMITS]
Liquid Limit	38
Plastic Limit	21
Plasticity Index	17
Classification (USCS)	

DENSITY & MOISTURE	
Wet Unit Weight (pcf)	126.2
Moisture Content (%)	22.9
Dry Unit Weight (pcf)	102.7

STRENGTH	1
Undrained Shear Strength, S ((tsf)	0.69
% Strain at q _u	5.0





UNCONFINED COMPRESSION TEST: BORING B-35 / ST-2

PROJECT NAME: Monett Regional Airport Relocation of Runway 18-36 PROJECT LOCATION: Monett, Missouri PROJECT NUMBER: 20202013.00

BORING NO.	B-35
Sample No.	ST-2
Sample Depth (ft)	4
Visual Classification (USCS)	CL

ASTM DESIGNATION	
Unconfined Compression	ASTM D2166
Atterberg Limits	ASTM D4318
Visual Classification (USCS)	ASTM D2488

ATTERBERG LIMITS	
Liquid Limit	
Plastic Limit	
Plasticity Index	
Classification (USCS)	

DENSITY & MOISTURE	
Wet Unit Weight (pcf)	121.5
Moisture Content (%)	28.5
Dry Unit Weight (pcf)	94.6

STRENGTH]
Undrained Shear Strength, S ((tsf)	1.19
% Strain at q _υ	6.6





UNCONFINED COMPRESSION TEST: BORING B-35 / ST-4

PROJECT NAME: Monett Regional Airport Relocation of Runway 18-36 PROJECT LOCATION: Monett, Missouri PROJECT NUMBER: 20202013.00

BORING NO.	B-35
Sample No.	ST-4
Sample Depth (ft)	9
Visual Classification (USCS)	СН

ASTM DESIGNATION	
Unconfined Compression	ASTM D2166
Atterberg Limits	ASTM D4318
Visual Classification (USCS)	ASTM D2488

ATTERBERG LIMITS	
Liquid Limit	
Plastic Limit	
Plasticity Index	
Classification (USCS)	

DENSITY & MOISTURE	
Wet Unit Weight (pcf)	121.5
Moisture Content (%)	36.5
Dry Unit Weight (pcf)	89.0

STRENGTH	
Undrained Shear Strength, S ((tsf)	1.58
% Strain at q _u	6.9



Job Num	ber: 20202	2013			EOTECHNOLO	GY Summary of Test Results
Monett Regional Airport Runway			FROM THE	GROUND UP		
Boring Number	Sample Number	Sample Depth	Moisture Content	Dry Density	Unconfined Compressive Strength	Visual Classification & Remarks
B-26	ST-3	6.0' - 8.0'	19.4	109	2.70tsf	CLAY- gray-brown and yellow
B-27	ST-5	13.0' - 15.0'	17.9	111	1.12tsf	CLAY - red-brown & gray-brown, trace rock rock fragments
B-28	ST-7	23.0' - 25.0'	22.6	102.4	0.75tsf	CLAY - red-brown , trace rock and rock fragments
B-29	ST-5	13.0' - 13.8'	27.4	N/A	N/A	CLAY - red-brown with rock fragments

 \ast B-29, ST-3 was bent and 5" was cut off from the bottom of the shelby tube







Project Name: Monett Regional Airport

			Original Height	Final Heigth of	
Boring	Sample	Depth	of Soil (mL)	Soil (mL)	% Swell
B-49	Bulk 1	1.0-5.0	10	14.5	45%
B-53	Bulk 1	1.0-5.0	10	15	50%
C-3	Bulk 1	1.0-5.0	10	16	60%
C-7	Bulk 1	1.0-5.0	10	14.8	48%
			÷		

Project Number: 20202013

Project Name:	Monett Regional Airport	Project Number:	20202013.00
Boring Number:	B-49	Tested by:	SLY 5/13/2020
Sample Number:	Bulk 1	Calculated by:	SLY 5/13/2020
Sample Depth:	1.0 - 5.0'	Checked by:	FHH 5/15/2020
Visual Description:	Brownish-red, fat CLAY (CH), with sand, trace gravel		

Sieve	Particle Size, mm	Percent Finer
1"	25.40	98
1/2"	12.70	95
3/8"	9.53	94
No.4	4.75	91
No.10	2.00	86
No.20	0.85	82
No.40	0.425	79
No.60	0.250	77
No.100	0.150	76
No.200	0.075	74
	0.0278	59
'sis	0.0184	52
laly	0.0111	45
Ar	0.0080	43
ter	0.0058	38
Hydrometer Analysis	0.0041	35
drc	0.0030	33
Hy	0.0020	31
	0.0012	29

Particle Size Description Soil Classification System

Particle	Size Range, mm	Percent of Specimen
Gravel	4.75 to 76.4	9
Coarse Sand	2.00 to 4.75	5
Medium Sand	0.43 to 2.00	7
Fine Sand	0.075 to 0.43	5
Silt	0.005 to 0.075	37
Clay	< 0.005	36

Project Name:	Monett Regional Airport	
Boring Number: B-49		
Sample Number:	Bulk 1	
Sample Depth:	1.0 - 5.0'	
Visual Description:	Brownish-red, fat CLAY (CH), with sand, trace grave	

Project Number:	20202013.00
Tested by:	SLY 5/13/2020
Calculated by:	SLY 5/13/2020
Checked by:	FHH 5/15/2020



Project Name:	Monett Regional Airport	Project Number:	20202013.00
Boring Number:	B-53	Tested by:	SLY 5/13/2020
Sample Number:	Bulk 1	Calculated by:	SLY 5/13/2020
Sample Depth:	1.0 - 5.0'	Checked by:	FHH 5/15/2020
Visual Description:	Brownish-red, fat CLAY (CH), with gravel, trace sand		

Sieve	Particle Size, mm	Percent Finer
1"	25.40	97
1/2"	12.70	93
3/8"	9.53	90
No.4	4.75	86
No.10	2.00	83
No.20	0.85	79
No.40	0.425	77
No.60	0.250	76
No.100	0.150	75
No.200	0.075	74
	0.0257	66
'sis	0.0168	62
laly	0.0101	57
Ar	0.0073	53
ter	0.0052	52
Hydrometer Analysis	0.0037	51
drc	0.0026	50
Hy	0.0018	49
	0.0011	47

Particle Size Description Soil Classification System

Particle	Size Range, mm	Percent of Specimen
Gravel	4.75 to 76.4	14
Coarse Sand	2.00 to 4.75	4
Medium Sand	0.43 to 2.00	6
Fine Sand	0.075 to 0.43	3
Silt	0.005 to 0.075	22
Clay	< 0.005	52

Project Name:	Monett Regional Airport	
Boring Number:	B-53	
Sample Number:	Bulk 1	
Sample Depth:	1.0 - 5.0'	
Visual Description:	Brownish-red, fat CLAY (CH), with gravel, trace	

Project Number:	20202013.00
Tested by:	SLY 5/13/2020
Calculated by:	SLY 5/13/2020
Checked by:	FHH 5/15/2020



Project Name:	Monett Regional Airport	Project Number:	20202013.00
Boring Number:	C-3	Tested by:	SLY 5/13/2020
Sample Number:	Bulk 1	Calculated by:	SLY 5/13/2020
Sample Depth:	1.0 - 5.0'	Checked by:	FHH 5/15/2020
Visual Description:	Brownish-red, sandy fat CLAY (CH), with gravel		

Sieve	Particle Size, mm	Percent Finer
1"	25.40	99
1/2"	12.70	94
3/8"	9.53	90
No.4	4.75	79
No.10	2.00	69
No.20	0.85	59
No.40	0.425	55
No.60	0.250	53
No.100	0.150	51
No.200	0.075	50
	0.0292	41
'sis	0.0188	39
laly	0.0111	36
Ar	0.0080	34
ter	0.0057	32
ime	0.0041	31
Hydrometer Analysis	0.0029	29
Hy	0.0019	28
	0.0012	27

Particle Size Description Soil Classification System

Particle	Size Range, mm	Percent of Specimen
Gravel	4.75 to 76.4	21
Coarse Sand	2.00 to 4.75	10
Medium Sand	0.43 to 2.00	14
Fine Sand	0.075 to 0.43	5
Silt	0.005 to 0.075	18
Clay	< 0.005	32

Project Name:	Monett Regional Airport	
Boring Number:	C-3	
Sample Number:	Bulk 1	
Sample Depth:	1.0 - 5.0'	
Visual Description:	Brownish-red, sandy fat CLAY (CH), with gravel	

Project Number:	20202013.00
Tested by:	SLY 5/13/2020
Calculated by:	SLY 5/13/2020
Checked by:	FHH 5/15/2020



Project Name:	
Boring Number:	
Sample Number:	
Sample Depth:	
Visual Description:	

Monett Regional Airport
C-7
Bulk 1
1.0 - 5.0'
Red, sandy fat CLAY (CH), with gravel

Project Number:	20202013.00
Tested by:	SLY 5/13/2020
Calculated by:	SLY 5/13/2020
Checked by:	FHH 5/15/2020

Sieve	Particle Size, mm	Percent Finer
1"	25.40	96
1/2"	12.70	90
3/8"	9.53	87
No.4	4.75	82
No.10	2.00	75
No.20	0.85	69
No.40	0.425	66
No.60	0.250	65
No.100	0.150	63
No.200	0.075	62
	0.0266	55
'sis	0.0174	52
laly	0.0101	51
Ar	0.0072	50
ter	0.0051	49
Hydrometer Analysis	0.0037	47
drc	0.0026	46
Hy	0.0017	45
	0.0011	44

Particle Size Description Soil Classification System

Particle	Size Range, mm	Percent of Specimen
Gravel	4.75 to 76.4	18
Coarse Sand	2.00 to 4.75	7
Medium Sand	0.43 to 2.00	9
Fine Sand	0.075 to 0.43	4
Silt	0.005 to 0.075	13
Clay	< 0.005	49

20202013.00

SLY 5/13/2020

SLY 5/13/2020

FHH 5/15/2020

Project Name:	Monett Regional Airport	Project Number:
Boring Number:	C-7	Tested by:
Sample Number:	Bulk 1	Calculated by:
Sample Depth:	1.0 - 5.0'	Checked by:
Visual Description:	Red, sandy fat CLAY (CH), with gravel	





PROJECT NAME: Monett Airport

PROJECT No.:	20202013		_			
SAMPLE NUMBER:			-			
SAMPLE LOCATION:	-					_
DEPTH:			<u>.</u>			
VISUAL CLASS. (USCS):	Brownish-red,	rat CLAY (CH), w	ith sand, trace o	gravel		
TYPE OF COMPACTION	MOD	D1557]			
SIEVE ANALYSIS RESULTS	3/4	3/8	No. 4	PROCEDURE		
% Retained(cumulative)	0.0	6.3	0.0	В		
SOIL WEIGHT DATA						
Determination Number	1	2	3	4	5	6
Weight- Soil + Mold (wet),g	6049.7	6146.2	6149.3	6122.6		
Weight of Mold,g	4276.0	4279.3	4276.0	4279.3		
Weight Wet Soil,g	1773.7	1866.9	1873.3	1843.3		
Volume of Mold (ft ³)	0.0333	0.0333	0.0333	0.0333		
MOISTURE DATA						1
Weight- Soil + Tare (wet),g	490.0	381.5	554.0	505.7		
Weight- Soil + Tare (dry),g	442.4	339.4	478.6	429.2		
Weight- Tare,g	89.1	85.3	84.2	84.3		
COMPUTED DATA						
Wet unit weight (pcf)	117.4	123.6	124.0	122.1		
Moisture content (%)	13.5	16.6	19.1	22.2		
Dry unit weight (pcf)	103.5	106.0	104.1	99.9		

Maximum Dry Density (pcf)	106.1
Optimum Moisture Content (%)	16.2
Natural Moisture Content (%)	29.8

Corr. Max. Dry Density (pcf)	108.6
Corr. Optimum Moist. Cont. (%)	

Liquid Limit	56
Plastic Limit	20
Plasticity Index	36
CLASSIFICATION. (USCS)	СН

	Date
Tested by : SLY	5/11/2020
Calculated by: SLY	5/12/2020
Checked by: FHH	5/15/2020



B-49 Proctor



PROJECT NAME: Monett Airport

PROJECT No.:	20202013					
SAMPLE NUMBER:	Bulk 1		-			
SAMPLE LOCATION:	B-53		-			
DEPTH:	1-5 ft.					_
VISUAL CLASS. (USCS):	Brownish-red,	fat CLAY (CH), w	ith gravel, trace	sand		
			-			
TYPE OF COMPACTION	MOD	D1557				
SIEVE ANALYSIS RESULTS	3/4	3/8	No. 4	PROCEDURE		
% Retained(cumulative)	0.0	9.9	0.0	В		
SOIL WEIGHT DATA						
Determination Number	1	2	3	4	5	6
Weight- Soil + Mold (wet),g	6144.8	6178.6	6114.0	6068.2		
Weight of Mold,g	4276.0	4279.3	4276.0	4279.3		
Weight Wet Soil,g	1868.8	1899.3	1838.0	1788.9		
Volume of Mold (ft ³)	0.0333	0.0333	0.0333	0.0333		
MOISTURE DATA]			1		
Weight- Soil + Tare (wet),g	391.7	575.7	494.1	336.6		
Weight- Soil + Tare (dry),g	341.3	485.3	410.9	280.5		
Weight- Tare,g	85.0	84.3	84.2	83.8		
COMPUTED DATA						
Wet unit weight (pcf)	123.7	125.8	121.7	118.5		
Moisture content (%)	19.7	22.6	25.5	28.5		
Dry unit weight (pcf)	103.4	102.6	97.0	92.2		

Maximum Dry Density (pcf)	104.0
Optimum Moisture Content (%)	20.8
Natural Moisture Content (%)	26.9

Corr. Max. Dry Density (pcf)	108.1
Corr. Optimum Moist. Cont. (%)	

Liquid Limit	87
Plastic Limit	24
Plasticity Index	63
CLASSIFICATION. (USCS)	СН

	Date
Tested by : SLY	5/8/2020
Calculated by: SLY	5/11/2020
Checked by: FHH	5/15/2020
NOTE:	



B-53 Proctor



PROJECT NAME: Monett Airport

PROJECT No.:	20202013		_			
SAMPLE NUMBER:			-			
SAMPLE LOCATION:						_
DEPTH:						
VISUAL CLASS. (USCS):	Brownish-red,	sandy fat CLAY (CH), with gravel			
TYPE OF COMPACTION	MOD	D1557				
SIEVE ANALYSIS RESULTS	3/4	3/8	No. 4	PROCEDURE		
% Retained(cumulative)	0.0	16.6	0.0	В		
SOIL WEIGHT DATA						
Determination Number	1	2	3	4	5	6
Weight- Soil + Mold (wet),g	6068.8	6113.3	6111.7	6054.9		
Weight of Mold,g	4279.3	4276.0	4279.3	4276.0		
Weight Wet Soil,g	1789.5	1837.3	1832.4	1778.9		
Volume of Mold (ft ³)	0.0333	0.0333	0.0333	0.0333		
MOISTURE DATA				1		
Weight- Soil + Tare (wet),g	319.5	331.1	317.1	354.7		
Weight- Soil + Tare (dry),g	288.0	293.8	276.9	302.0		
Weight- Tare,g	85.9	84.0	85.0	83.3		
COMPUTED DATA		1	1			
Wet unit weight (pcf)	118.5	121.7	121.3	117.8		
Moisture content (%)	15.6	17.8	21.0	24.1		
Dry unit weight (pcf)	102.5	103.3	100.3	94.9		

Maximum Dry Density (pcf)	103.3
Optimum Moisture Content (%)	17.3
Natural Moisture Content (%)	32.1

Corr. Max. Dry Density (pcf)	110.4
Corr. Optimum Moist. Cont. (%)	

Liquid Limit	66
Plastic Limit	21
Plasticity Index	45
CLASSIFICATION. (USCS)	СН

	Date
Tested by : SLY	5/8/2020
Calculated by: SLY	5/11/2020
Checked by: FHH	5/15/2020



C-3 Proctor



PROJECT NAME: Monett Airport

PROJECT No.:	20202013		_			
SAMPLE NUMBER:	Bulk 1		-			
SAMPLE LOCATION:	C-7					
DEPTH:	1-5 ft.		-			
VISUAL CLASS. (USCS):	Red, sandy fat	CLAY (CH), with	gravel			
TYPE OF COMPACTION	MOD	D1557]			
SIEVE ANALYSIS RESULTS	3/4	3/8	No. 4	PROCEDURE		
% Retained(cumulative)	0.0	16.1	0.0	В		
SOIL WEIGHT DATA						
Determination Number	1	2	3	4	5	6
Weight- Soil + Mold (wet),g	6090.0	6161.8	6157.3	6119.0		
Weight of Mold,g	4276.0	4279.3	4276.0	4279.3		
Weight Wet Soil,g	1814.0	1882.5	1881.3	1839.7		
Volume of Mold (ft ³)	0.0333	0.0333	0.0333	0.0333		
MOISTURE DATA						1
Weight- Soil + Tare (wet),g	433.1	561.7	273.0	366.8		
Weight- Soil + Tare (dry),g	387.3	488.7	240.9	312.6		
Weight- Tare,g	83.1	83.7	85.3	82.6		
COMPUTED DATA						
Wet unit weight (pcf)	120.1	124.7	124.6	121.8		
Moisture content (%)	15.1	18.0	20.6	23.6		
Dry unit weight (pcf)	104.4	105.6	103.3	98.6		

Maximum Dry Density (pcf)	105.8
Optimum Moisture Content (%)	17.2
Natural Moisture Content (%)	22.1

112.0

Liquid Limit	96
Plastic Limit	26
Plasticity Index	70
CLASSIFICATION. (USCS)	СН

	Date
Tested by : SLY	5/12/2020
Calculated by: SLY	5/13/2020
Checked by: FHH	5/15/2020
NOTE:	



CALIFORNIA BEARING RATIO

at Optimum Water Content







CALIFORNIA BEARING RATIO

at Optimum Water Content

Project Name	Monett Regional Airport
Project Number	20202013.00
Sample Number	Bulk 1
Sample Location	B-53
Soil Description	Brownish-red, fat CLAY (CH), trace sand and gravel



Calc. by	SLY 05/18/2020
Checked by	MD 05/19/2020

4.3

Maximum Dry Density104Optimum Moisture Content20.8Method of CompactionASTM D1557Density at 90% Compaction93.6





CALIFORNIA BEARING RATIO

at Optimum Water Content









APPENDIX D

Monett Regional Airport Improvements Slope Stability Analysis, Section C-C, Left Slope Depth of FILL: 20 ft. Boring: B-26

Material Model: Mohr-Coulomb . Analysis Type: Drained

Material Properties Used:

FILL: Cohesion (drained)= 100 psf; Total Unit Wt.= 120 pcf; Phi: 28 deg. Native Clay: Cohesion (drained)= 50 psf; Total Unit Weight= 120 pcf; Phi: 26 deg Runway Pavement: C= 10000 psf, Unit Wt.= 150 pcf



Monett Regional Airport Improvements Slope Stability Analysis, Section C-C, Right Slope Depth of FILL: 20 ft. Boring: B-26

Material Model: Mohr-Coulomb . Analysis Type: Drained



Monett Regional Airport Improvements Slope Stability Analysis, Left Slope Depth of FILL: 35 ft. Boring: B-26

Material Model: Mohr-Coulomb . Analysis Type: Drained Material Properties Used: FILL: Cohesion (drained)= 100 psf; Total Unit Wt.= 120 pcf; Phi: 28 deg. Native Clay: Cohesion (drained)= 50 psf; Total Unit Weight= 120 pcf; Phi: 26 deg Runway Pavement: C= 10000 psf, Unit Wt.= 150 pcf



Monett Regional Airport Improvements Slope Stability Analysis, Right Slope Depth of FILL: 35 ft. Boring: B-26

Material Model: Mohr-Coulomb . Analysis Type: Drained



Distance

Monett Regional Airport Improvements Slope Stability Analysis, Section C-C, Left Slope Depth of FILL: 20 ft. Boring: B-27

Material Model: Mohr-Coulomb . Analysis Type: Drained

Material Properties Used: FILL: Cohesion (drained)= 100 psf; Total Unit Wt.= 120 pcf; Phi: 28 deg. Native Clay: Cohesion (drained)= 50 psf; Total Unit Weight= 120 pcf; Phi: 26 deg Runway Pavement: C= 10000 psf, Unit Wt.= 150 pcf


Material Model: Mohr-Coulomb . Analysis Type: Drained



Material Model: Mohr-Coulomb . Analysis Type: Drained Material Properties Used: FILL: Cohesion (drained)= 100 psf; Total Unit Wt.= 120 pcf; Phi: 28 deg. Native Clay: Cohesion (drained)= 50 psf; Total Unit Weight= 120 pcf; Phi: 26 deg Runway Pavement: C= 10000 psf, Unit Wt.= 150 pcf



Material Model: Mohr-Coulomb . Analysis Type: Drained



Material Model: Mohr-Coulomb . Analysis Type: Drained

Material Properties Used:

FILL: Cohesion (drained)= 100 psf; Total Unit Wt.= 120 pcf; Phi: 28 deg. Native Clay: Cohesion (drained)= 50 psf; Total Unit Weight= 120 pcf; Phi: 26 deg Runway Pavement: C= 10000 psf, Unit Wt.= 150 pcf



Material Model: Mohr-Coulomb . Analysis Type: Drained



Material Model: Mohr-Coulomb . Analysis Type: Drained Material Properties Used: FILL: Cohesion (drained)= 100 psf; Total Unit Wt.= 120 pcf; Phi: 28 deg. Native Clay: Cohesion (drained)= 50 psf; Total Unit Weight= 120 pcf; Phi: 26 deg Runway Pavement: C= 10000 psf, Unit Wt.= 150 pcf



Material Model: Mohr-Coulomb . Analysis Type: Drained

Material Properties Used:



Material Model: Mohr-Coulomb . Analysis Type: Undrained

Material Properties Used:

FILL: Cohesion (undrained)= 1000 psf; Total Unit Wt.= 120 pcf Native Clay: Cohesion (undrained)= 1500 psf; Total Unit Weight= 125 pcf; Runway Pavement: C= 10000 psf, Unit Wt.= 150 pcf



Material Model: Mohr-Coulomb . Analysis Type: Undrained



Material Model: Mohr-Coulomb . Analysis Type: Undrained Material Properties Used: FILL: Cohesion (undrained)= 1000 psf; Total Unit Wt.= 120 pcf; Native Clay: Cohesion (undrained)= 1500 psf; Total Unit Weight= 120 pcf; Runway Pavement: C= 10000 psf, Unit Wt.= 150 pcf



Material Model: Mohr-Coulomb . Analysis Type: Undrained

Material Properties Used: FILL: Cohesion (undrained)= 1000 psf; Total Unit Wt.= 120 pcf Native Clay: Cohesion (undrained)=1500 psf; Total Unit Weight= 125 pcf;



Material Model: Mohr-Coulomb . Analysis Type: Undrained

Material Properties Used:

FILL: Cohesion (undrained)= 1000 psf; Total Unit Wt.= 120 pcf Native Clay: Cohesion (undrained)= 1500 psf; Total Unit Weight= 125 pcf; Runway Pavement: C= 10000 psf, Unit Wt.= 150 pcf



Elevation (x 1000)

Material Model: Mohr-Coulomb . Analysis Type: Undrained



Material Model: Mohr-Coulomb . Analysis Type: Undrained Material Properties Used: FILL: Cohesion (undrained)= 1000 psf; Total Unit Wt.= 120 pcf; Native Clay: Cohesion (undrained)= 1500 psf; Total Unit Weight= 120 pcf; Runway Pavement: C= 10000 psf, Unit Wt.= 150 pcf



Material Model: Mohr-Coulomb . Analysis Type: Undrained

Material Properties Used: FILL: Cohesion (undrained)= 1000 psf; Total Unit Wt.= 120 pcf Native Clay: Cohesion (undrained)=1500 psf; Total Unit Weight= 125 pcf; 11.206 - 30 **Relative Depth** FILL **Native Clay**

Material Model: Mohr-Coulomb . Analysis Type: Undrained

Material Properties Used:

FILL: Cohesion (undrained)= 1000 psf; Total Unit Wt.= 120 pcf Native Clay: Cohesion (undrained)= 1500 psf; Total Unit Weight= 125 pcf; Runway Pavement: C= 10000 psf, Unit Wt.= 150 pcf



Elevation (x 1000)

Material Model: Mohr-Coulomb . Analysis Type: Undrained









Material Model: Mohr-Coulomb . Analysis Type: Undrained

Material Properties Used: FILL: Cohesion (undrained)= 1000 psf; Total Unit Wt.= 120 pcf Native Clay: Cohesion (undrained)=1500 psf; Total Unit Weight= 125 pcf; - 40 11.206 **Relative Depth** FILL Native Clay Distance