



CITY OF FOUNTAINS

Project Number <u>89005559 STP-3376(403)</u>

Project Title Paseo Bridge over Brush Creek

KANSAS CITY MISSOURI

Bid No. 2

ISSUE DATE: <u>3-17-2017</u>

Bidders are hereby notified that the Bidding and Contract Documents for the above project, for which Bids are to be received on <u>March 21, 2017</u>, are amended as follows:

The Bid date for this Project stated in Document 00130 - Invitation to Bid shall be changed to: 2:00 PM, on March 28, 2017.

Information to Bidders The following is provided to Bidders for information only:

Q1.	Will lane drops be allowed on Paseo to access the work?
À1.	Yes
Q2. A2.	What will be the limitations on days and hours?
A2.	Normal work hours will apply. We ask that the contractor give us 48 hour notice if there is to be planned weekend work.
Q3.	Will this project have any Trainee Hours required?
A3.	No
Q4.	In the Technical Specifications there is a 05000 Safety Rail section calling for a metal safety rail, I do not see this noted on the plans anywhere. Can you please clarify where this metal safety rail is to be installed?
A4.	There isn't any safety rail included on this project. This specification can be disregarded/deleted.
Q5.	On plan sheet No. 2 in the general notes it states, plans of the existing structure are included in the project specifications. These don't seem to be in the specs, can you please provide these asbuilts of the existing structure and precast panels
A5.	Already provided.
Q6.	Is all the rebar shown on plan Sheet No. 8 in the stamped and stained sidewalk intended to be epoxy coated as stated in the general notes on Sheet No. 2?
A6.	Yes
Q7.	On plan Sheet No. 7 there is a note stating all work on this sheet is included in bid alternate group A, Is this note correct? The Paver Bricks Cap bid line item is under Base Bid on the bid form. If it is supposed to be in Alternate A, which line item is it under?
A7.	Disregard the not on Sheet 7. The Pavers are in the base bid.

Q8.	Is the intent to remove the existing concrete fill at the bottom of the median planters shown on the Bridge Removal/Repair Sections, Plan Sheet No. 4? Or can this fill be left in place?
A8.	The intent is that only the trees and vegetation are to be removed. The soil fill and concrete are to be left in place.
Q9.	Is the sidewalk outside the bridge, Alternate A, to be stamped and stained as well?
A9.	The sidewalk off the bridge is not stamped or stained.

Specifications

1. Revisions from previous bid

Drawings:

1. Paseo Intersections As-builts

NOTE: Bidders must acknowledge receipt of this Addendum by listing the number and date, where provided, on the Bid Form - Document 00410.





INVITATION TO BID

Project No.: <u>89005559 STP-3376(403)</u>

Project Title: Paseo Bridge over Brush Creek

Bid No. 2

The **General Services Department** of Kansas City, Missouri will receive sealed Bids until 2:00 PM, on March 28, 2017 at City Hall, 414 East 12th Street, First Floor, Room 102W, Kansas City, Missouri, 64106, for <u>Project</u> **No. 89005559 – Paseo Bridge over Brush Creek - Fed No. STP 3376(403)**. Bids will be opened after that time at the same location.

Bidding Documents will be available online to all interested parties at the Kansas City, Missouri Plan Room, <u>http://www.kcmoplanroom.org</u>. <u>All addenda will be posted at this location</u>. Any document or plan may be viewed or downloaded from this location.

This project is funded in part with funds offered by FHWA and administered by MoDOT. The City of Kansas City, Missouri, hereby notifies all bidders that it will affirmatively ensure that in any contract entered into pursuant to this advertisement, businesses owned and controlled by socially and economically disadvantaged individuals will be afforded full opportunity to submit bids in response to this invitation and will not be discriminated against on the grounds of race, color, religion, creed, sex, age, ancestry, or national origin in consideration for an award.

For this Project the DBE goal is (11%).

By submitting this bid, the bidder certifies that the bidder is familiar with the Training Provision in the Missouri Highways and Transportation Commission's "General Provisions and Supplement Specifications" which are available on the Missouri Department of Transportation web page at www.modot.mo.gov under "Business with MoDOT" "Standards and Specifications". The number of trainee hours provided under this contract will be 0 slots at 1000 hours per slot 0 hours.

<u>Prior to Bidding.</u> Prime Contractors must have a fully responsive contractor questionnaire on file with the Missouri Highways and Transportation Commission (MHTC) at least seven (7) days prior to the bid opening date. Subcontractors utilized for DBE Goals must appear on the MoDOT/Kansas City Approved DBE List and on the MoDOT DBE Directory located at <u>http://www.modot.mo.gov/business/contractor_resources/bid opening info/bidGenInfo.shtml.</u>

Each bidder must certify that it is not presently debarred, suspended, proposed for debarment, declared ineligible or voluntarily excluded from covered transactions by any federal agency and they have not been convicted or had civil judgment rendered within the past 3 years.

The contract will be awarded to the lowest, responsive, responsible bidder.

Bidding Documents will be available online to all interested parties at the Kansas City, Missouri Plan Room, <u>http://www.kcmoplanroom.org</u>. <u>All addenda will be posted at this location</u>. Any document or plan may be viewed or downloaded from this location.

A **Pre-Bid Conference** will be held on <u>March 9, 2017</u> at <u>9:00 AM</u> in the <u>18th Floor Conference Room</u>, located at 414 East 12th Street, City Hall, Kansas City, Missouri, 64106. Attendance at the pre-Bid conference is *encouraged* for all Bidders on this Project

Forward all questions in writing to the following Project Manager and Contract Administrator.

Project Manager: <u>Chad Thompson, P.E.</u> Phone Number: <u>(816)</u> 513-2738 E-mail: Terry: <u>Chad.Thompson@kcmo.org</u>

Procurement Officer: <u>Darrell Everette</u> Phone Number: <u>(816) 513-0798</u> E-mail: <u>Darrell.everette@kcmo.org</u>

View all procurement and contracting opportunities at http://www.kcmo.org



Memorandum Kansas City, MO 64108

2400 Pershing Road, Suite 400

T 816-329-8600 F 816-329-8601

www.transystems.com

Paseo Bridge over Brush Creek – Project # 89005559

Revisions from previous bid plans and specs:

- I. The project scope was separated into a base bid and three groups of prioritized alternates.
 - a. The base bid focuses on the bridge repairs.
 - b. Alternate Group A covers slope protection repair and the improvements related to the right turn lane at the southeast corner of the project.
 - c. Alternate Group B covers the floodlights on the west side of the bridge.
 - d. Alternate Group C covers the landscaping off the bridge and the irrigation system.
- 2. The green roof system and concrete base was deleted and replaced with a landscaped planter. This allows the existing dirt to remain in place and eliminated the need for manhole rings and covers and steps.
- 3. The planter in the median will just be capped with paver bricks from the sidewalk removals, or with new bricks at the Contractor's option, thus eliminating the concrete cap and balustrade originally proposed.
- 4. Slope protection repair, in addition to making it an alternate bid item, provides the option for the Contractor to raise the existing slab. The detail for replacement was also modified to be less complicated.

CITY OF KANSAS CITY, MISSOURI DEPARTMENT OF PUBLIC WORKS THE PASEO INTERSECTION COMPLEX PHASE A (BRIDGES)

LIST OF DRAWINGS

DWG. NO.

S2

ROADWAY

<u>DWG. NO.</u>

G1

R1

R2

R3

R4

R5

Re

R7

R

R9

11

12

TITLE

TITLE SHEET GENERAL INFORMATION SITE PLAN ROADWAY CONSTRUCTION ROADWAY CONSTRUCTION TRAFFIC CONTROL TRAFFIC CONTROL TRAFFIC CONTROL ROADWAY DETAILS SITE GRADING/UTILITY PLAN TRAFFIC SIGNALS AMENITY PLAN PAVER AND PLANTER DETAILS

DESIGN CRITERIA

22600
24400
2725
0
40 MPH

1993

2	DRIDGE FEAN AND FROHEL
3	CONSTRUCTION LAYOUT
4	NORTH ABUTMENT-PLAN AND ELEVATION
5	SOUTH ABUTMENT-PLAN AND ELEVATION
6	NORTH ABUTMENT- REINFORCEMENT
7	SOUTH ABUTMENT-REINFORCEMENT
8	ABUTMENT DETAILS
9	PIER PLAN AND ELEVATION
10	PIER REINFORCEMENT
11	SLAB AND GIRDER PLAN
*	
12	ABUTMENT DIAPHRAGM
13	PIER DIAPHRAGM
14	DIAPHRAGM DETAILS
	· •
15	SLAB REINFORCEMENT
16	BRIDGE CROSS SECTION
17	APPROACH WALLS
18	APPROACH WALL DETAILS 1
19	APPROACH WALL DETAILS 2
20	BARRIER CURB DETAILS
21	APPROACH SLAB DETAILS
22	VERTICAL CURVE CORRECTION AND DE
23	PRECAST PRESTRESSED CONCRETE GIR
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27	CHANNEL WALLS-PLAN AND PROFILE (
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30	WATER PIPE DETAILS
31	GAS PIPE DETAILS
32	EXCAVATION
33	BORING LOGS
34	BILL OF REINFORCEMENT 1
35	BILL OF REINFORCEMENT 2
36	BILL OF REINFORCEMENT 3
37	BILL OF REINFORCEMENT 4
38	BILL OF REINFORCEMENT 5
39	BILL OF REINFORCEMENT 6
40	PRECAST DECK PANELS-ALTERNATE A
1	PLANTER AND SIDEWALK SLAB PLANS
2	PLANTER AND SIDEWALK ELEVATIONS
3	EMBEDDED PLATE LOCATIONS
4	PLANTER AND SIDEWALK SECTIONS
5	RAILING AND PANEL SECTIONS
6	BILL OF REINFORCEMENT 7
1	CAST STONE ELEVATIONS
2	CAST STONE SECTIONS AND DETAILS
1	ELECTRICAL LAYOUT

BRIDGE

TITLE

GENERAL NOTES AND QUANTITIES

BRIDGE PLAN AND PROFILE

BRUSH CREEK HYDRAULIC DATA

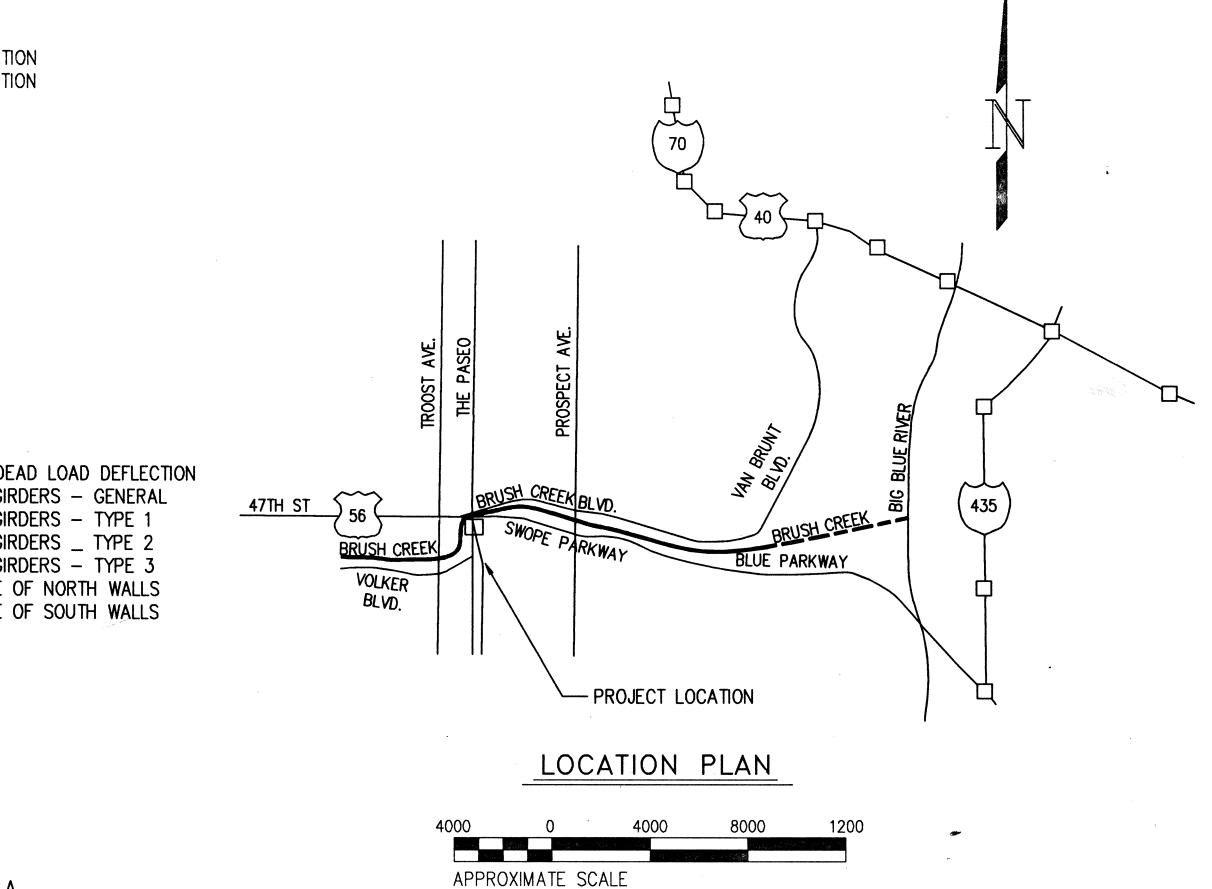
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100 YEAR FLOOD	21,600 CFS
10 YEAR FLOOD	12,900 CFS
UNDRALILIO DATA COTADUCUED	DV KANCAC OF

HYDRAULIC DATA ESTABLISHED BY KANSAS CITY DISTRICT CORPS OF ENGINEERS AUTHORIZED PROJECT REPORT "BRUSH CREEK CHANNEL MODIFICATION", OCTOBER 1988 ON BASIS OF MODEL STUDY WITH 2060 SQUARE FEET NET CLEAR WATERWAY AND "LOW STEEL" ELEVATION 803.0 AT THE PASEO BRIDGES.

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UTILITY LOCATION MARKING
STREETS & TRAFFIC
WATER DEPARTMENT
GAS SERVICE COMPANY
K.C.P.& L. COMPANY
SOUTHWESTERN BELL TELEPHONE COMPANY
AMERICAN CABLEVISION
POLLUTION CONTROL
WESTERN UNION COMPANY
1

FEDERAL PROJECT NO. STP-DSB-3471(401)





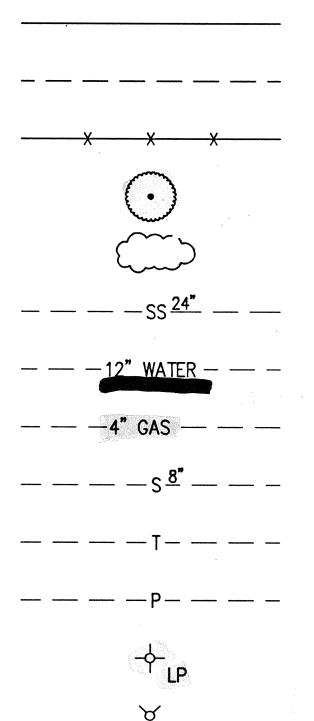
September 1, 1993



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LEGEND



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LENGTH OF PROJECT - PHASE A

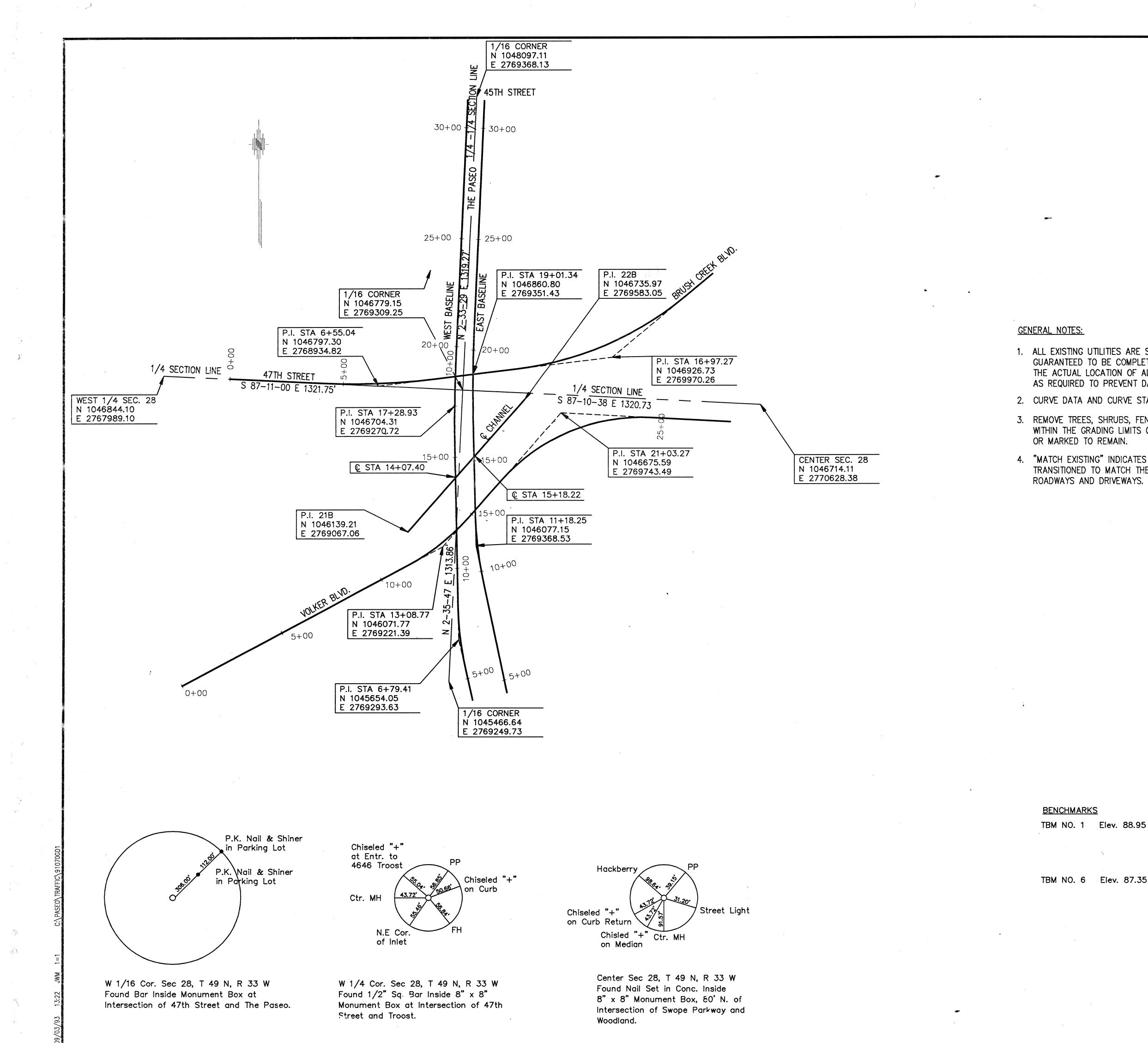
BEGINNING OF PROJECT END OF PROJECT PROJECT LENGTH BRIDGE LENGTH



EAST ALIGNMENT STA 11+61.20 STA 16+71.62 510.42' 192.33'

WEST ALIGNMENT STA 11+67.73 STA 16+53.73 486.00' 192.33'

I ADALON
Approved Chales Cule Assistant City Engineer
Gurnie C. Gunter
City Engineer
Adopted this 29 day of September 1993
George L. Satterlee Suprace Satter Cel
Director of Public Works
Terry Dopson Temp R Dorson 9/20/93
Director of Parks Recreation & Boulevards
Entry No. 930588
Project No. <u>381-0897556-5357</u> C.D. No. <u>5</u>
Sheet



BENCHMARKS

TBM NO. 6 Elev. 87.35

1. ALL EXISTING UTILITIES ARE SHOWN AT LOCATION OF RECORD ONLY AND ARE NOT GUARANTEED TO BE COMPLETE OR CORRECT AS SHOWN. CONTRACTOR SHALL VERIFY THE ACTUAL LOCATION OF ALL UTILITIES WITH THEIR RESPECTIVE OWNERS AS REQUIRED TO PREVENT DAMAGE BY CONTRACTOR'S OPERATIONS.

By Date

No.

ENGINEER

ASSOCIATES

00

A.C. KIRKWOOD a Division of Shafer Kline & Wai

Designed By BSB Drawn By LIF Checked By BSB Scale1"=20'H 1"=10'V Job No. 9107 Corvtract No.

CITY MO. PUBLIC WORKS DEPT. PASEO INTERSECTION COMPLEX

KANSAS

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Dwg. No. G1/

INFORMATION

GENERAL

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PROJECT ENGINEER Date 7-3-73 NOTE: This drawing is PRELIMINARY until approved by project en

2. CURVE DATA AND CURVE STATIONING ARE SHOWN BY ARC DEFINITION.

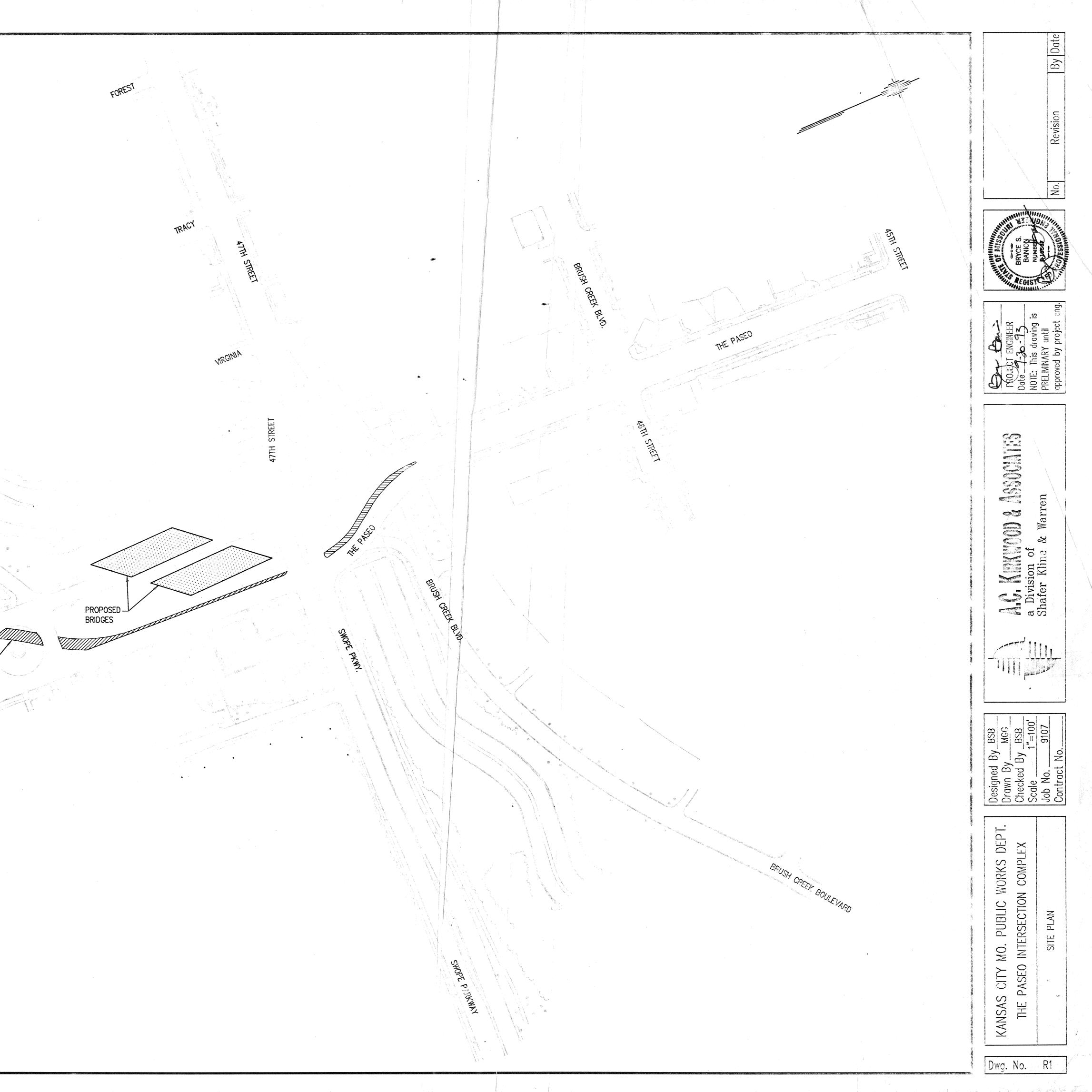
3. REMOVE TREES, SHRUBS, FENCES, AND OTHER EXISTING IMPROVEMENTS WITHIN THE GRADING LIMITS OF THIS CONTRACT EXCEPT THOSE INDICATED,

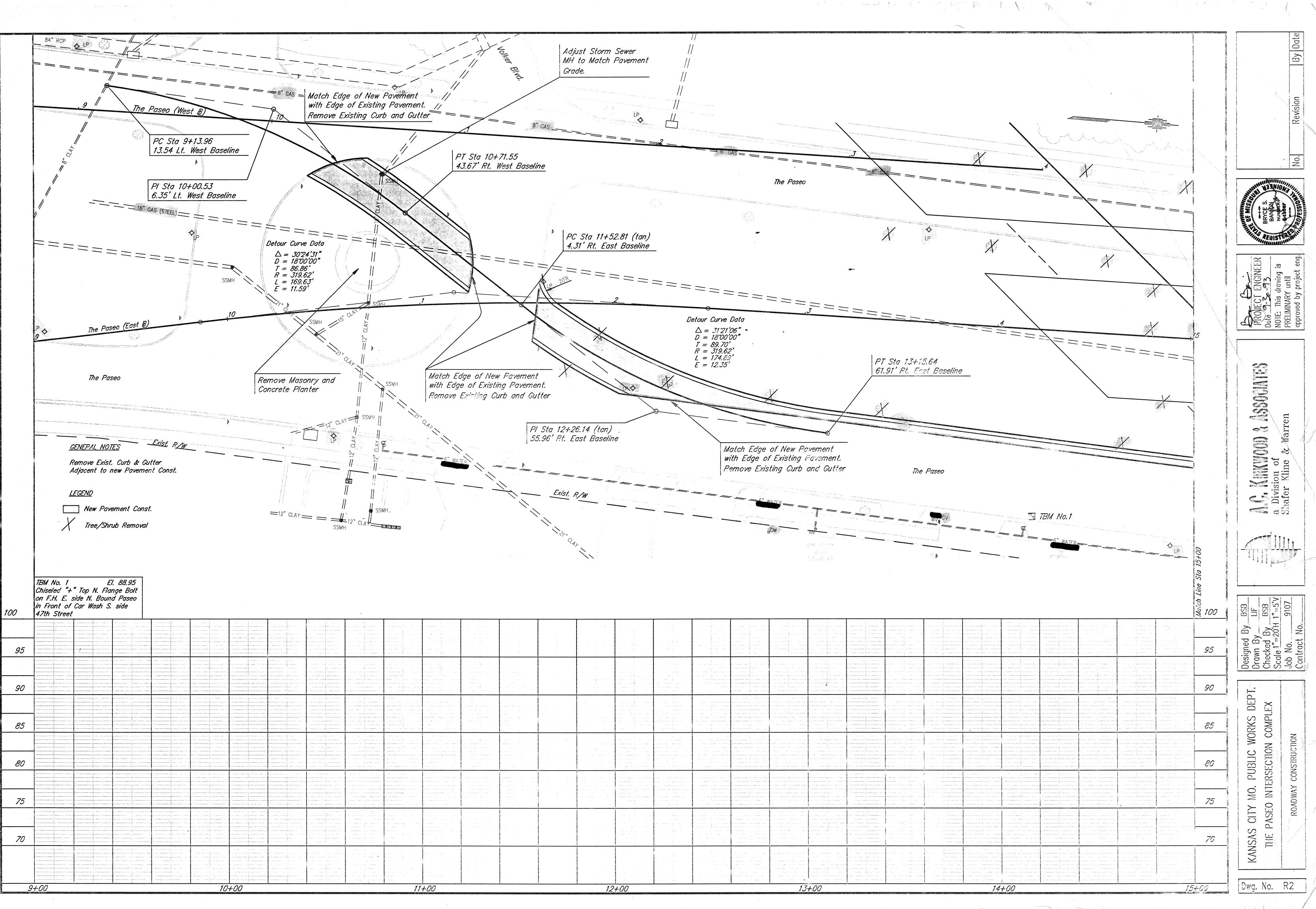
"MATCH EXISTING" INDICATES THAT THE NEW CONSTRUCTION IS TO BE TRANSITIONED TO MATCH THE EXISTING GRADE AND SECTION OF EXISTING ROADWAYS AND DRIVEWAYS.

Chiseled "+" on top of Northeast Bolt on Fire Hydrant East of Northbound The Paseo in front of car wash at 4717 The Paseo

Chiseled "+" on top of Southeast flange bolt on fire hydrant on East side of AMOCO station and West side of South bound The Paseo

/ 26ji VOLKER BOULEVARA L'3 PROPOSED TEMP. PAVEMENT -6,5-4-45 49TH STREET



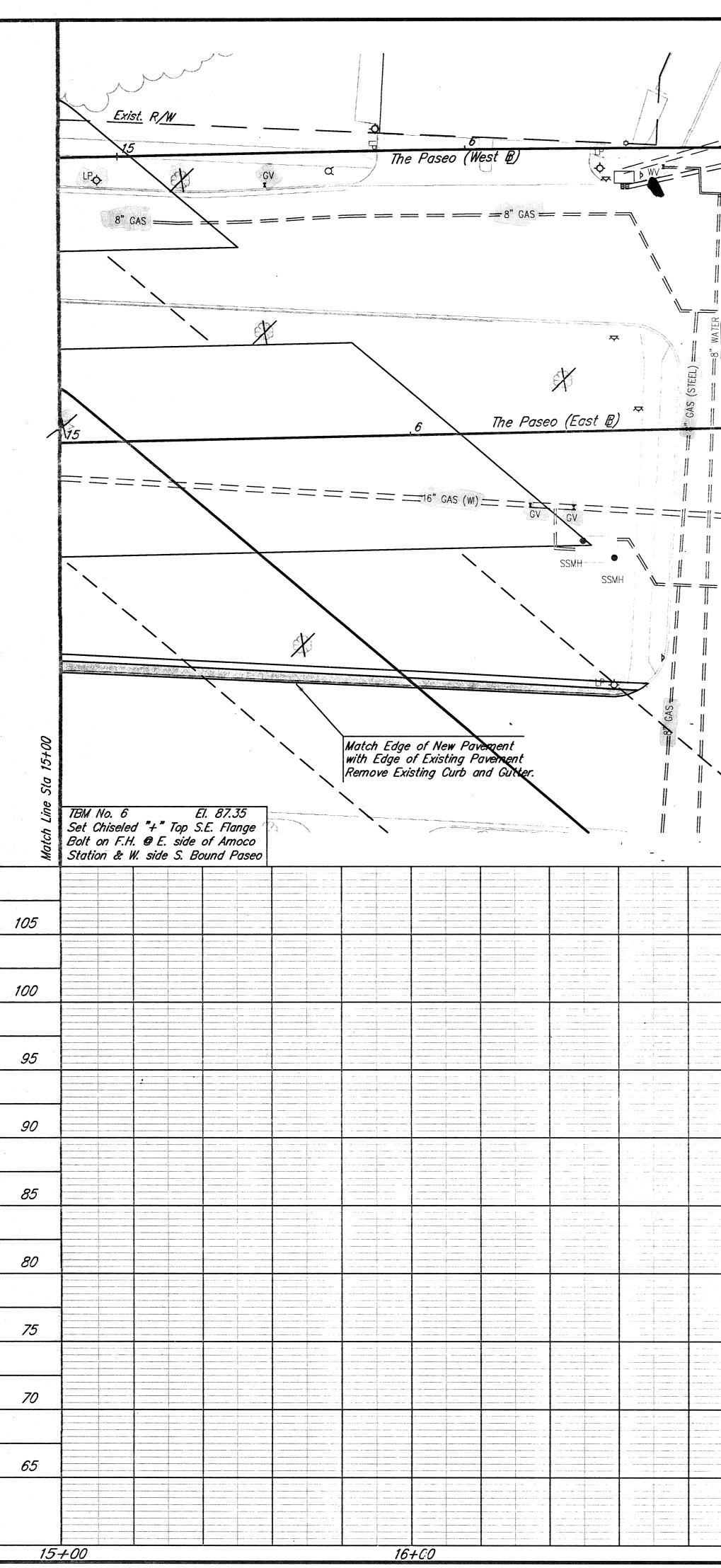


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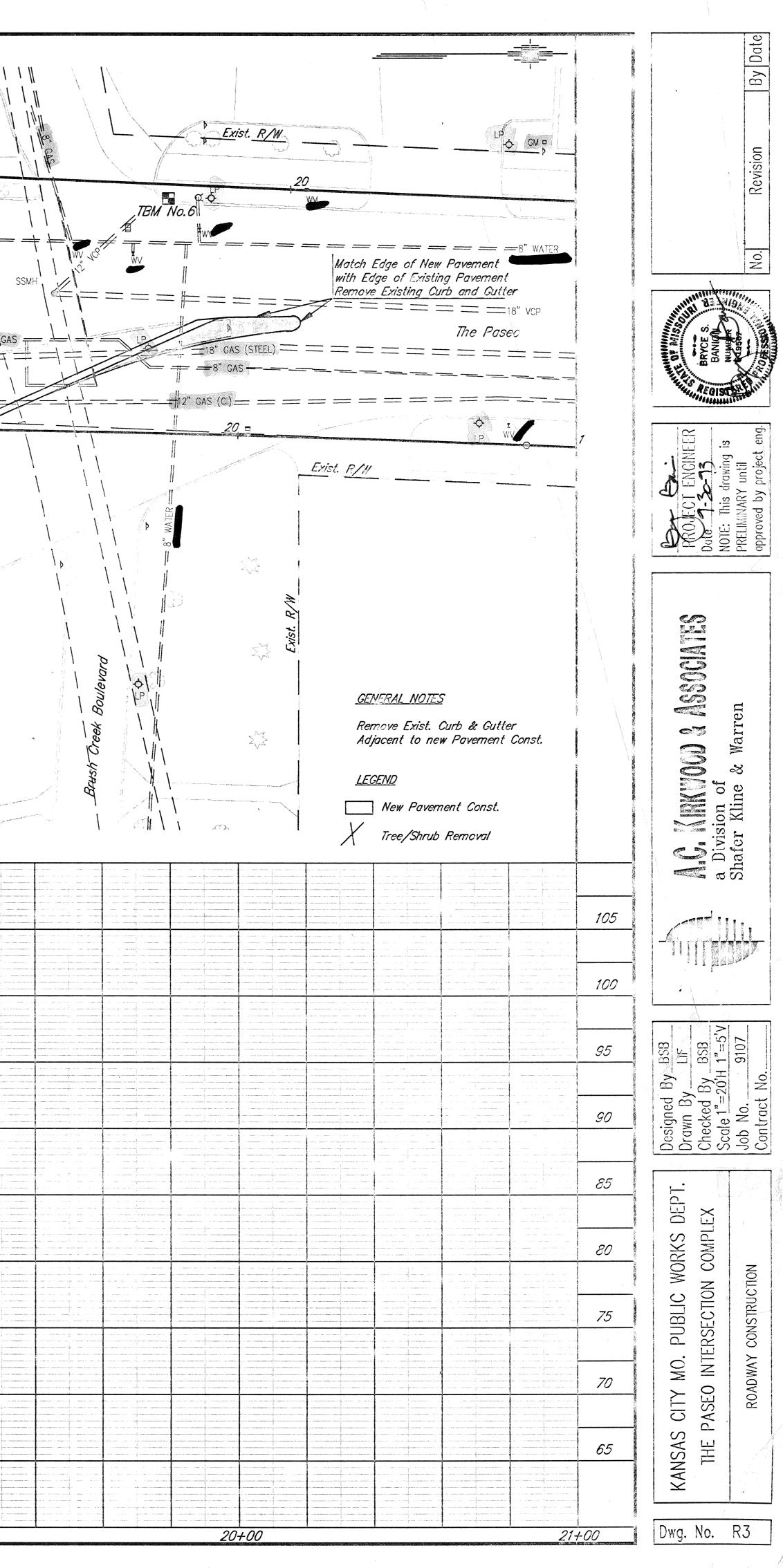
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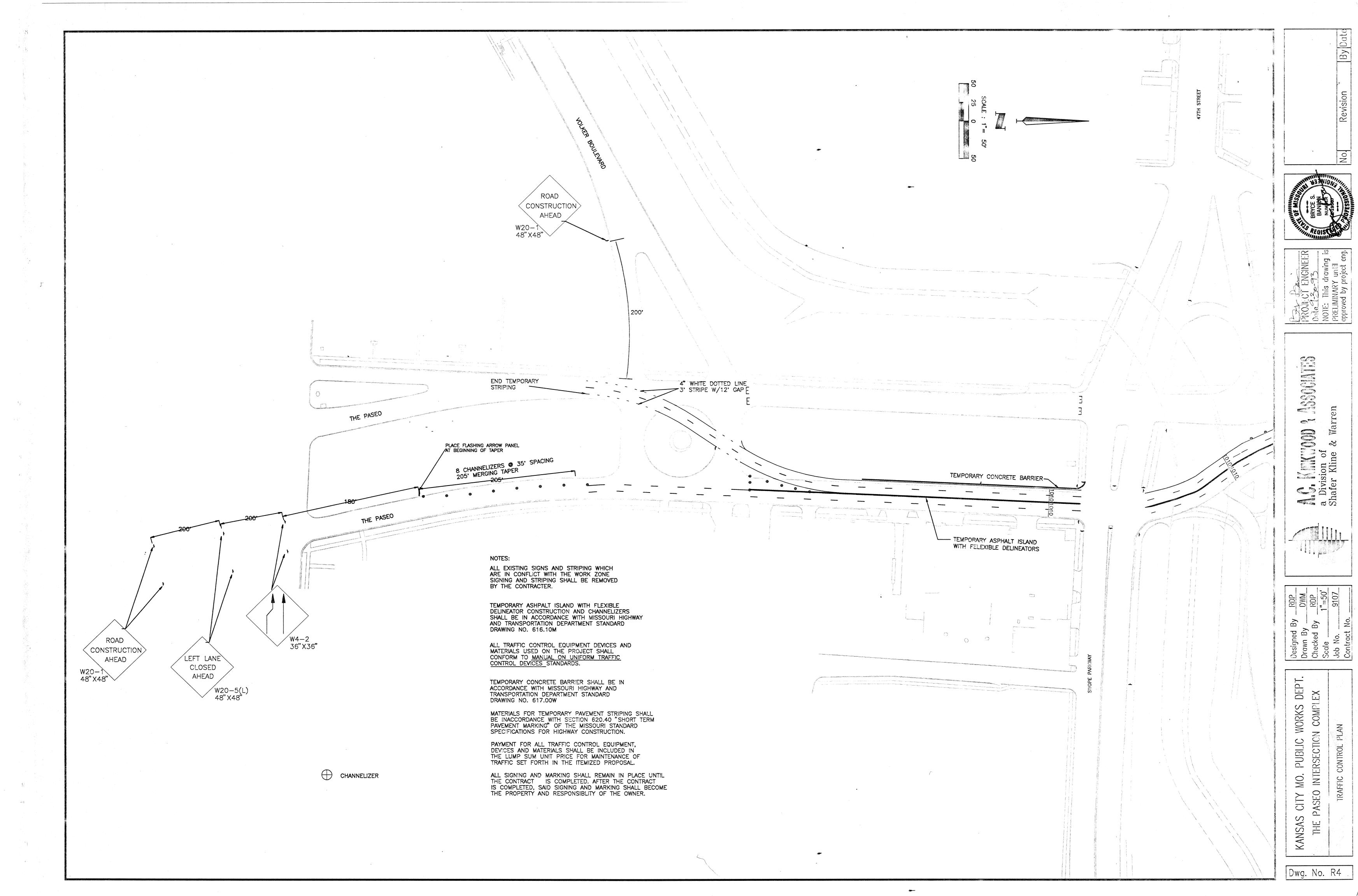
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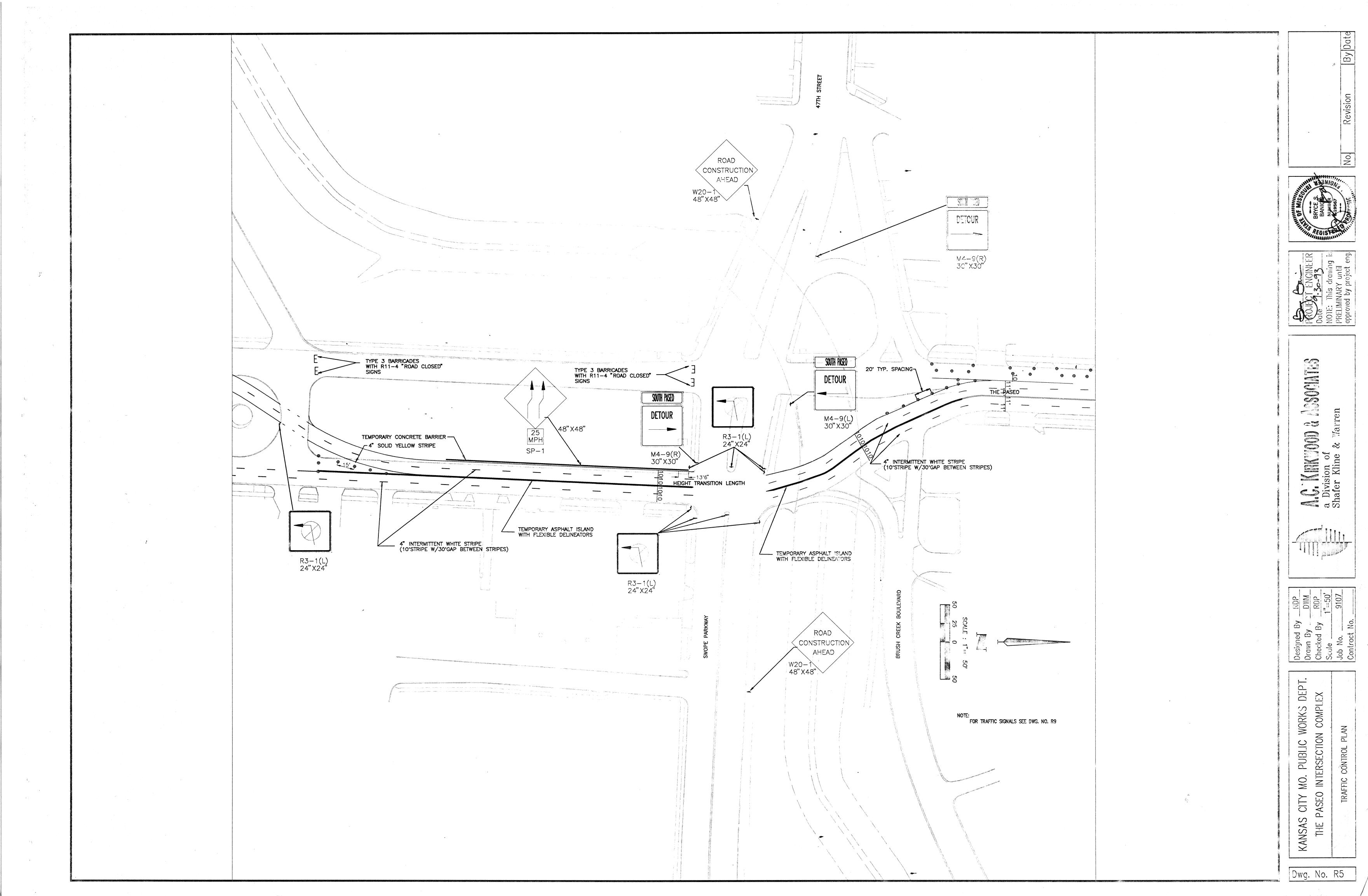
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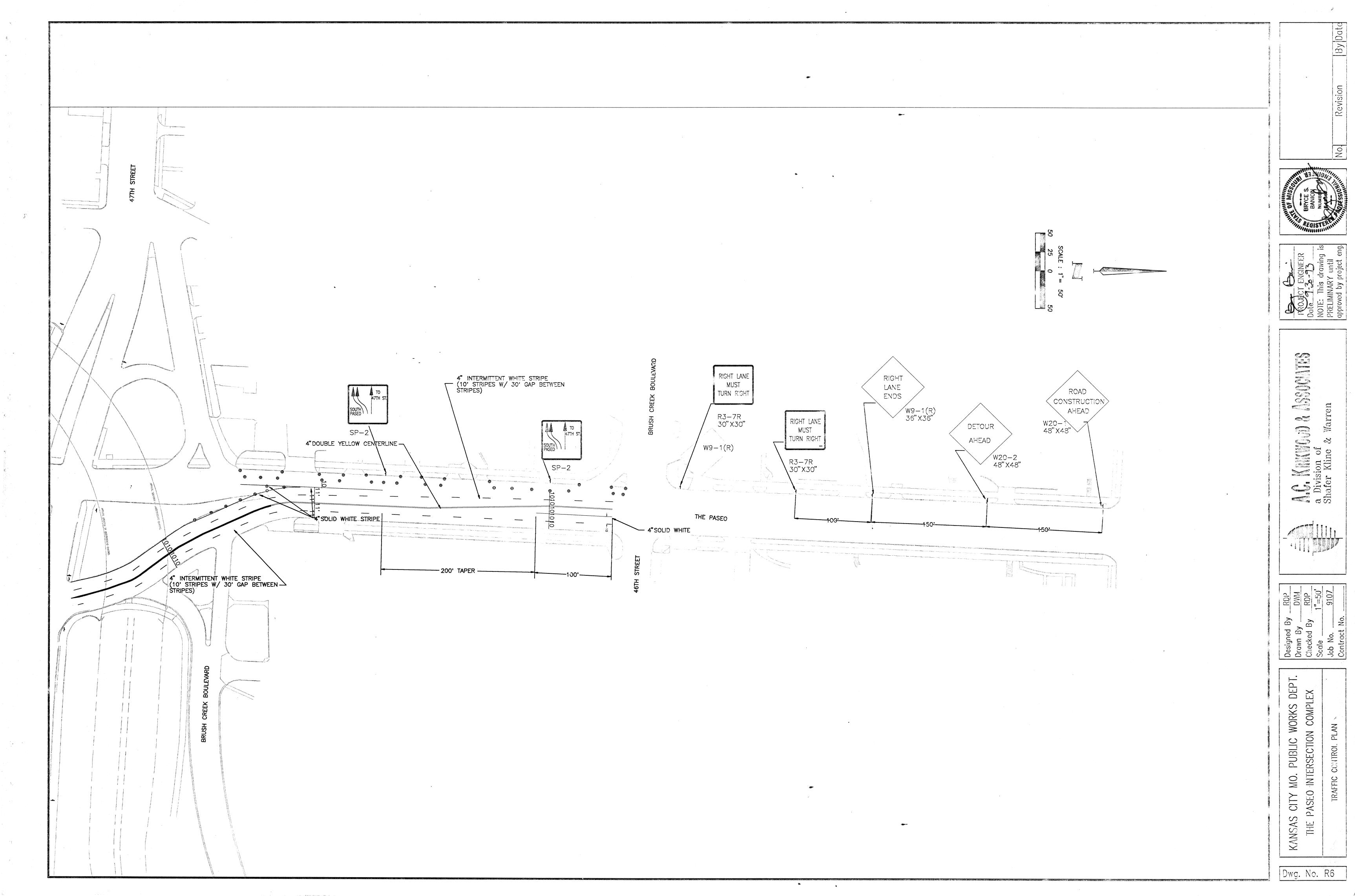
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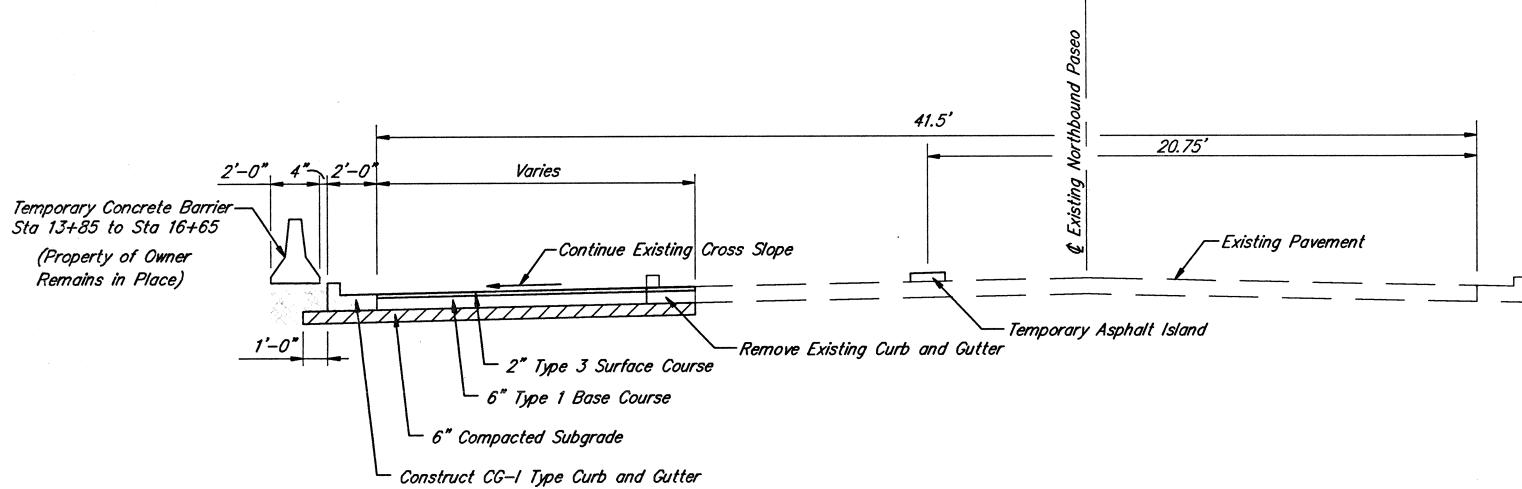








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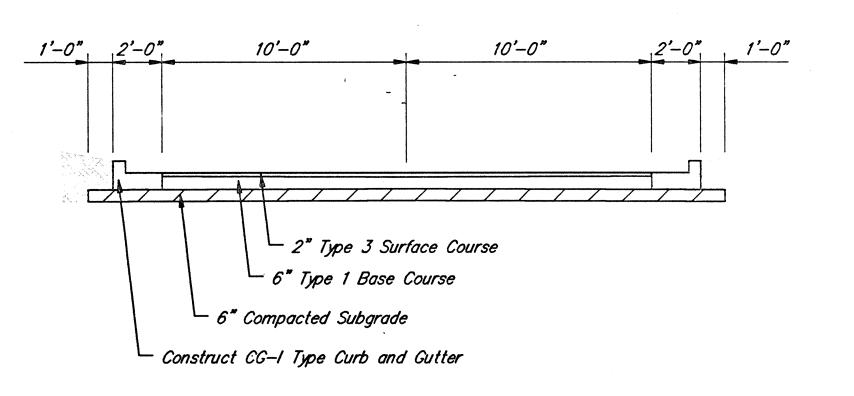
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TYPICAL STREET SECTION Sta 13+15.64 to Sta 20+17.80 The Paseo East B

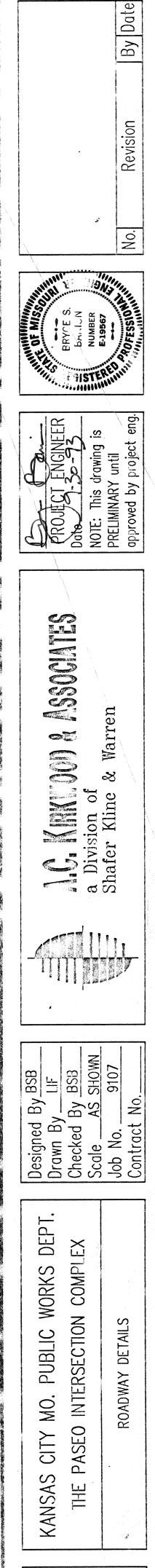


TYPICAL STREET SECTION Sta 10+50 to Sta 12+00 (East @)

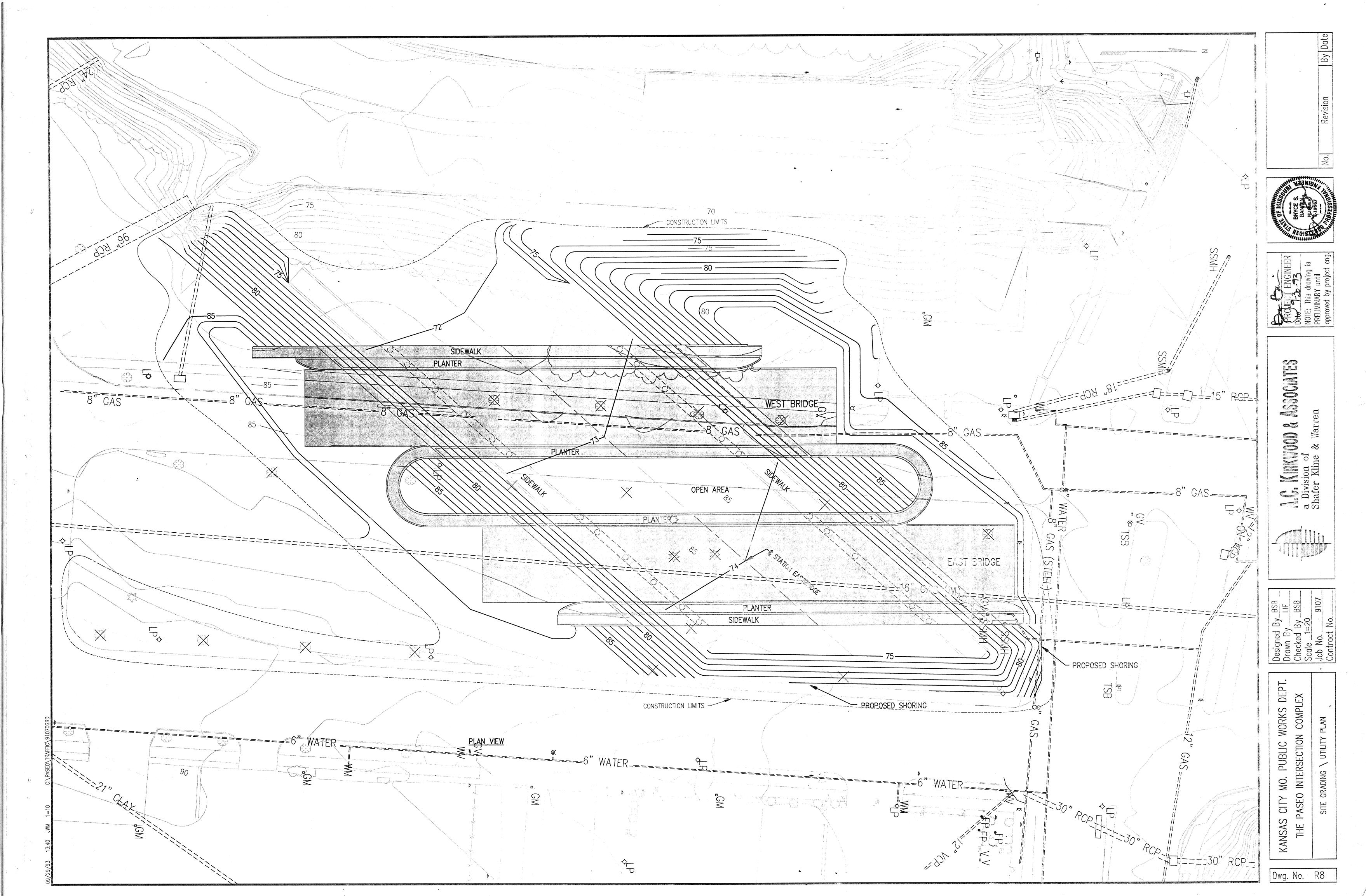
STANDARD DESIGN DETOUR SIGNING SEE TRAFF SP-1 SP-2 SEE TRAFF DETOUR LE M4-9L ROAD CON W20-1 W20-2 DETOUR AF PAVEMENT W4-2(L) W20-5(L) LEFT LANE R3-7R RIGHT LANE R3–1(L) NO LEFT RIGHT LAN W9-1(R) M4-9R DETOUR RIC -TOTAL CON TYPE III BARRICADES WI CHANNELIZERS TEMPORARY CONCRETE . 4" YELLOW MARKING 4" SOLID WHITE MARKIN TEMPORARY ASPHALT TRAFFIC SIGNALS (SEE

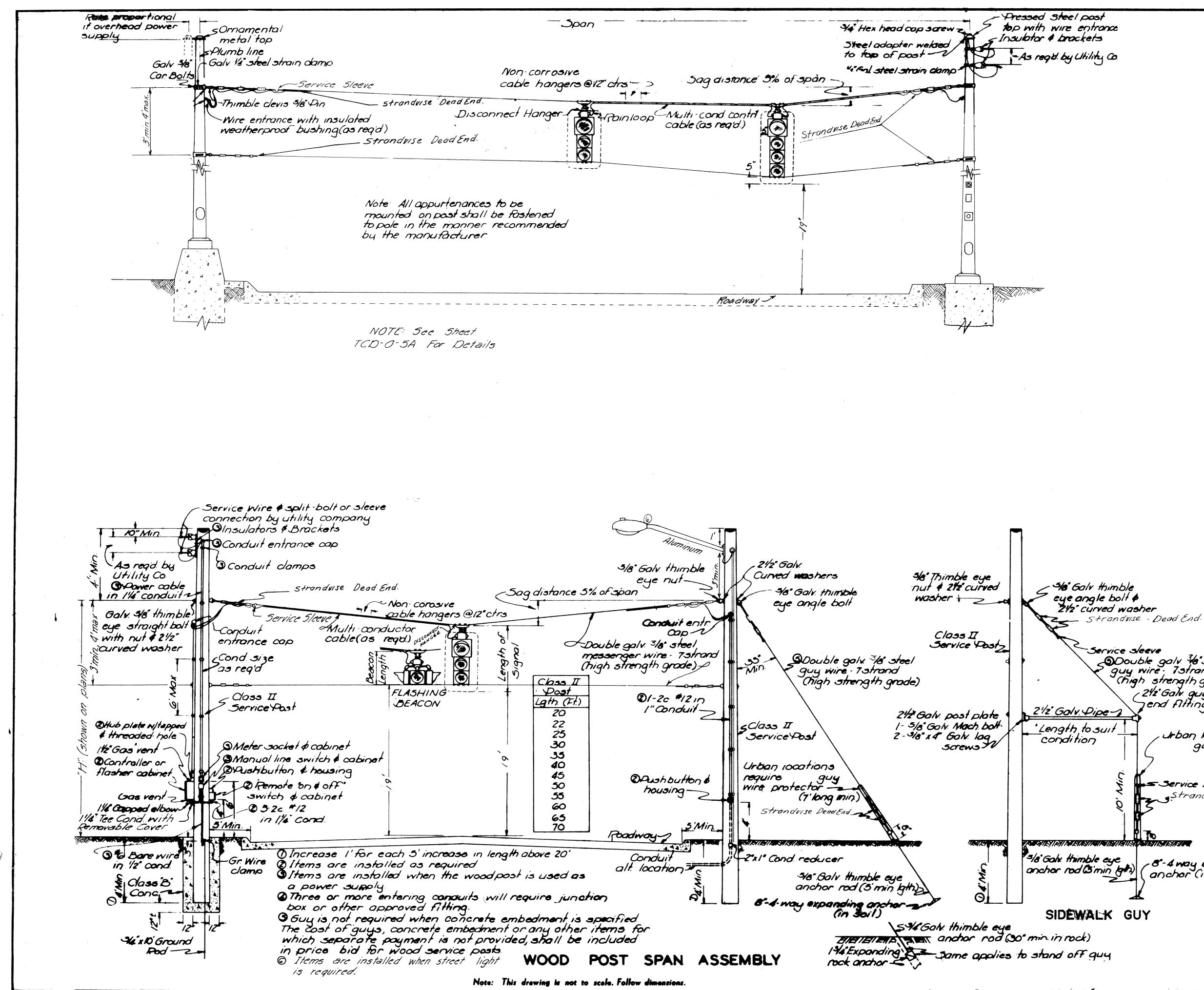
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MESSAGE	WIDTH	HEIGHT	REQD.	
FIC CONTROL PLAN	30"	30"	1	
FIC CONTROL PLAN	30"	30"	2	
EFT ARROW	30"	30"	1	
STRUCTION AHEAD	48"	48"	6	
HEAD	48"	48"	1	
WIDTH TRANSITION	36"	36"	2	
CLOSED AHEAD	48"	48"	2	
E MUST TURN RIGHT	30"	30"	2	
URN	24"	24"	10	
E ENDS	36"	36"	1	
IGHT ARROW	30"	30"	2	
NSTRUCTION SIGNS			30	
ITH TYPE A FLASHING LI	GHTS		4	
			51	
BARRIER			280 LF	
			900 LF	
NG			1350 LF	
ISLANDS WITH FLEXIBLE	DELINEATOR		725 LF	
DWG. NO. R9)			1	



Dwg. No. R7

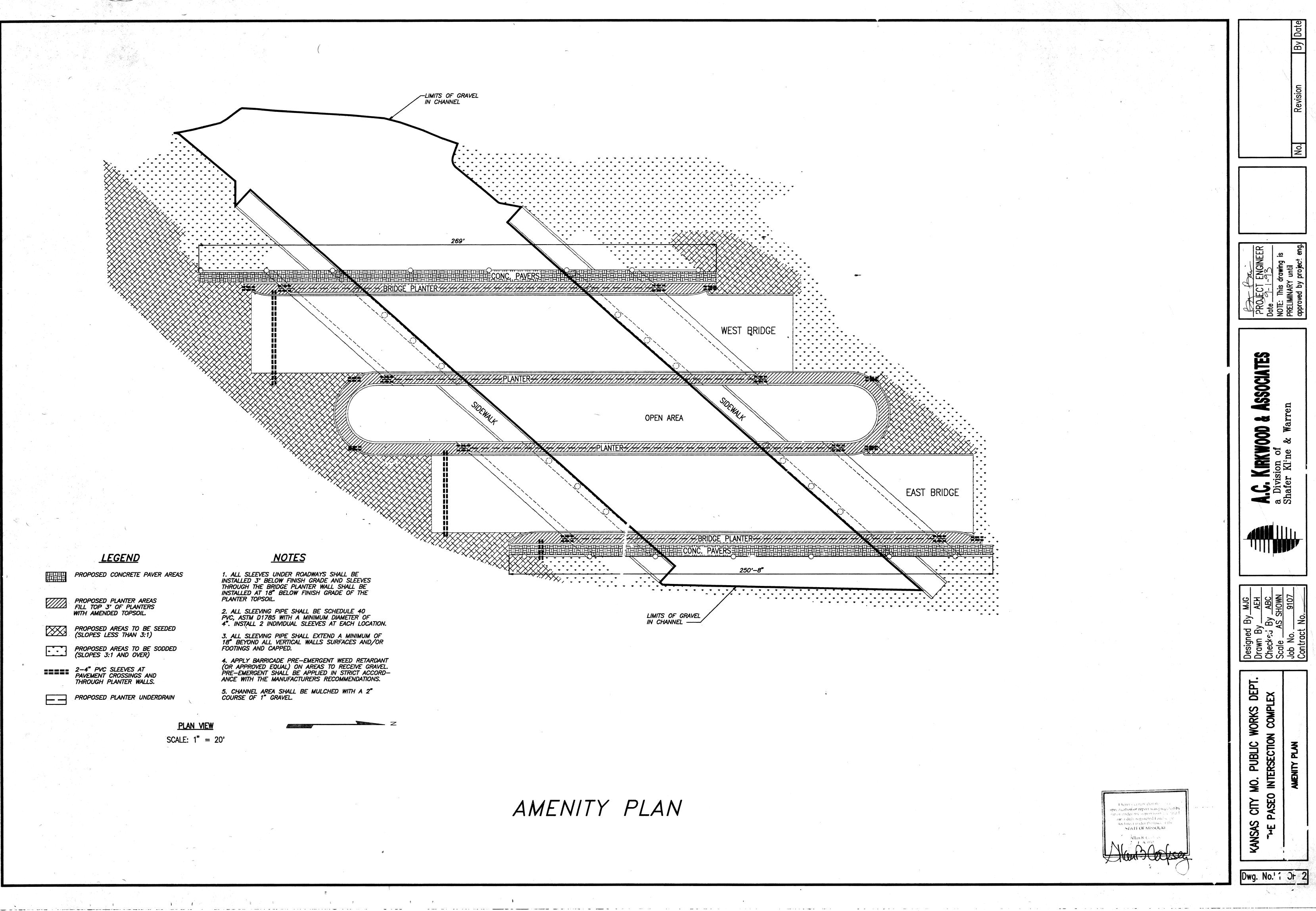


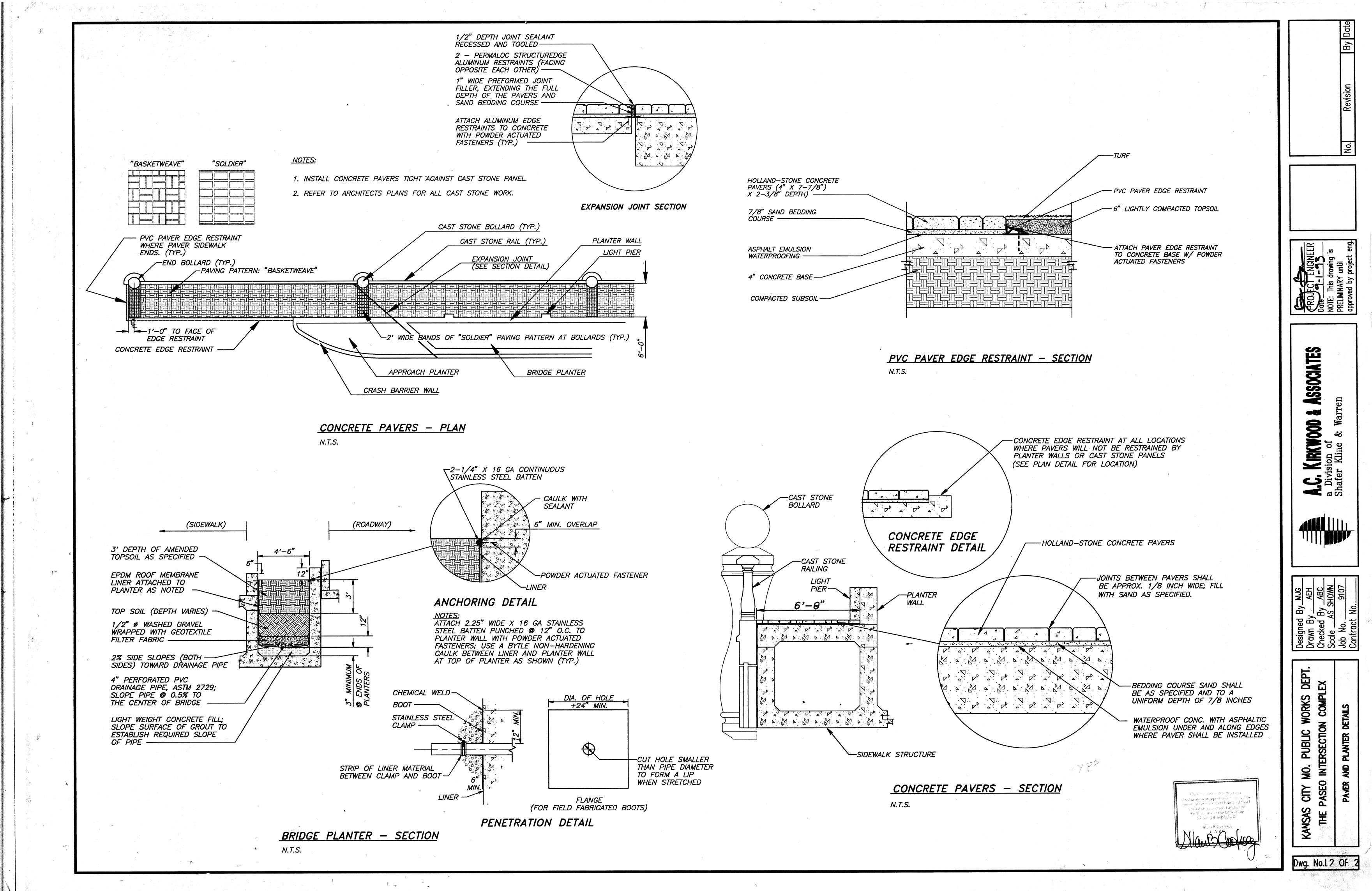


5.4 ···

Double galv 3/6" steel guy wire - Tstrand (high strength grade) 242' Galv guy connector . Urban locations require guy wire protector (T'Min) Service sleeve Strandwise Dead End. , 8-4 way expanding (anchor (in soil)

> TRAFFIC SIGNALS TCD-O-5 B Dwg. No. R9





PARED SLOPE PROTECTION SQ. YDS. 245 245 245 245 980 MAINTENANCE OF TRAFFIC L.S. L.S. L VERTICAL DRAIN AT ABUTMENT L.S. 180	TEN UNITS WEST BROOC LAST BROOC WEST BROOC TOTAL SERVERSMENT CL DD 221 65 65 220 75 201 MEDIA MEDIA <td< th=""><th>an anna an an an anna an anna an anna anna anna anna an an</th><th>n i dan va maa ku ja maa ku aa maa aa aa</th><th>an an a</th><th>nen - San Maggangangangangan (second) analysista. Se Analysista sanakang (second second second</th><th>n anna - an an anna an anna an anna an an an anna an an</th><th>ag stör – aller samta samta anna anna anna anna anna anna anna</th><th></th><th>MMAR</th><th>Y OF</th><th>QUAN</th><th></th><th>a da a como en este anos en este a como en este a c</th><th>дар и проставите с с и преста и пологовата с даран иле</th><th></th><th>SUMMARY OF QU</th><th></th></td<>	an anna an an an anna an anna an anna anna anna anna an an	n i dan va maa ku ja maa ku aa maa aa	an a	nen - San Maggangangangangan (second) analysista. Se Analysista sanakang (second second	n anna - an an anna an anna an anna an an an anna an an	ag stör – aller samta samta anna anna anna anna anna anna anna		MMAR	Y OF	QUAN		a da a como en este anos en este a como en este a c	дар и проставите с с и преста и пологовата с даран иле		SUMMARY OF QU	
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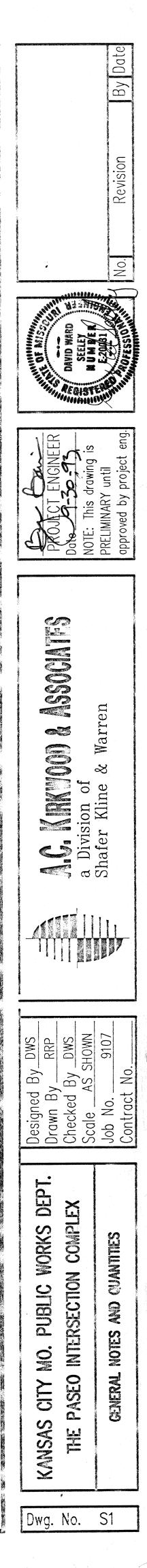
SUMMARY OF QU	JANTITIE	S
NON-PARTICIPATIN	G UTILI	TIES
ITEM	UNITS	TOTAL
WATER MAIN IMPROVEMENTS	L.S.	L.S.
GAS PIPE INSTALLATION	L.S.	L.S.

SUMMARY OF QUANTITIES (ALT. 1)

PRESTRESSED CONCRETE PANELS

ITEM	UNITS TOTAL	\$
DEDUCT TRAP ROCK CONCRETE	CU. YDS. 64	
DEDUCT EPOXY COATED REINFORCING STEEL	LB. 27747	
ADD PRESTRESSED CONCRETE PANELS	SQ. FT. 6930	

*



GENERAL NOTES

SPECIFICATIONS

DESIGN SPECIFICATIONS: AASHTO STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES 1992. CONSTRUCTION SPECIFICATIONS: CITY OF KANSAS CITY, MISSOURI SPECIFICATIONS, SPECIAL PROVISIONS AND MISSOURI STANDARD SPECIFICATIONS FOR HIGHWAY CONSTRUCTION, 1993 AND AMENDMENTS. ALL "MHTD" REFERENCES HEREIN WILL PERTAIN TO THE MISSOURI STANDARD STANDARD SPECIFICATIONS LIST ABOVE.

LOADING

- A DESIGN LIVE LOAD = HS20-44
- B FUTURE WEARING SURFACE = 25LBS./SQ. FT.
- C OTHER LOADS AASHTO 1992

STEEL H PILES

ALLOWABLE LOAD = 62 TONS PER PILE ALLOWABLE BEARING = 10,000 PSI

PEDESTAL PILES

ALLOWABLE BEARING = 50,000 PSF

DIMENSIONS

ALL PLAN DIMENSIONS ARE HORIZONTAL AND ARE MEASURED AT 60° F.

DATUM

ALL ELEVATIONS ARE REFERRED TO THE CITY OF KANSAS CITY, MISSOURI DATUM.

BENCH MARK

ALL BENCH MARK DATA WILL BE AS SHOWN ON THE PLANS.

ELEV. 0.00

<u>NGVD</u> 722.30 ____

EXISTING UTILITIES

THE CONTRACTOR SHALL BE RESPONSIBLE FOR LOCATING ALL EXISTING UTILITIES WITHIN THE WORK AREA, AND PROVIDING PROTECTION FOR THE VARIOUS UTILITIES AFFECTED, BEFORE PROCEEDING WITH THE WORK.

CAST IN PLACE CONCRETE

ALL CAST-IN-PLACE CONCRETE EXCEPT AS NOTED SHALL BE CLASS 1 (F'C= 4000 PSI A.E.). BRIDGE DECK, BARRIER CURBS AND APPROACH SLAB SHALL BE TRAP ROCK CONCRETE (F'c = 4500 PSI A.E.). THE SIDEWALK AND PLANTER STRUCTURES SHALL BE SEMI LIGHTWEIGHT CONCRETE (F'c = 4000 PSI A.E.). SURFACE PIERS AND ABUTMENTS WHICH ARE EXPOSED SHALL BE FREE OF ANY FORIEGN MATERAILS WHICH MIGHT CAUSE STAINING OF THE CONCRETE. ALL EXPOSED CORNERS SHALL BE BEVELED 34" UNLESS OTHERWISE NOTED.

PRECAST CONCRETE

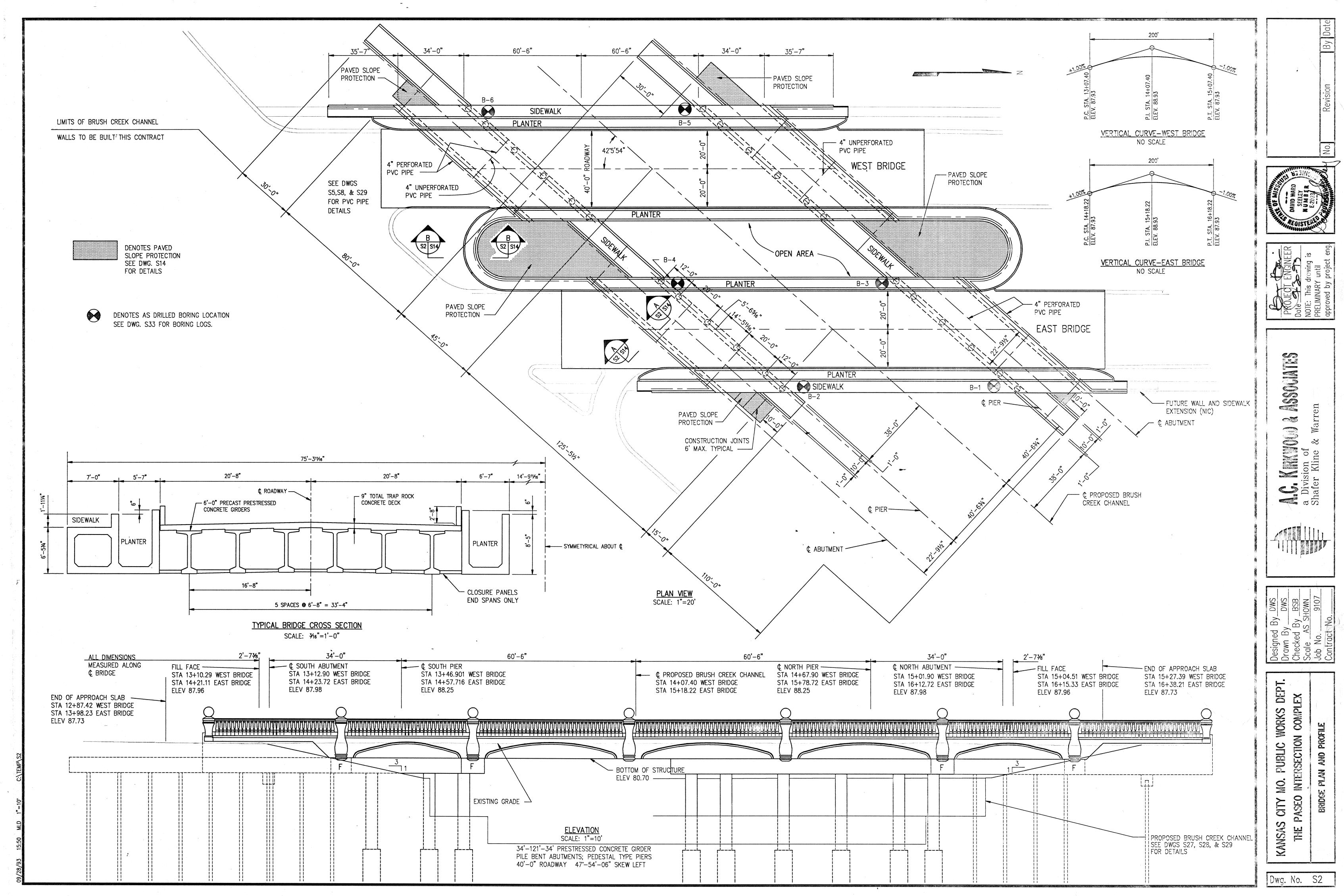
ULTIMATE COMPRESSIVE STRENGTH = 6000 PSI

CONSTRUCTION CLEARANCE

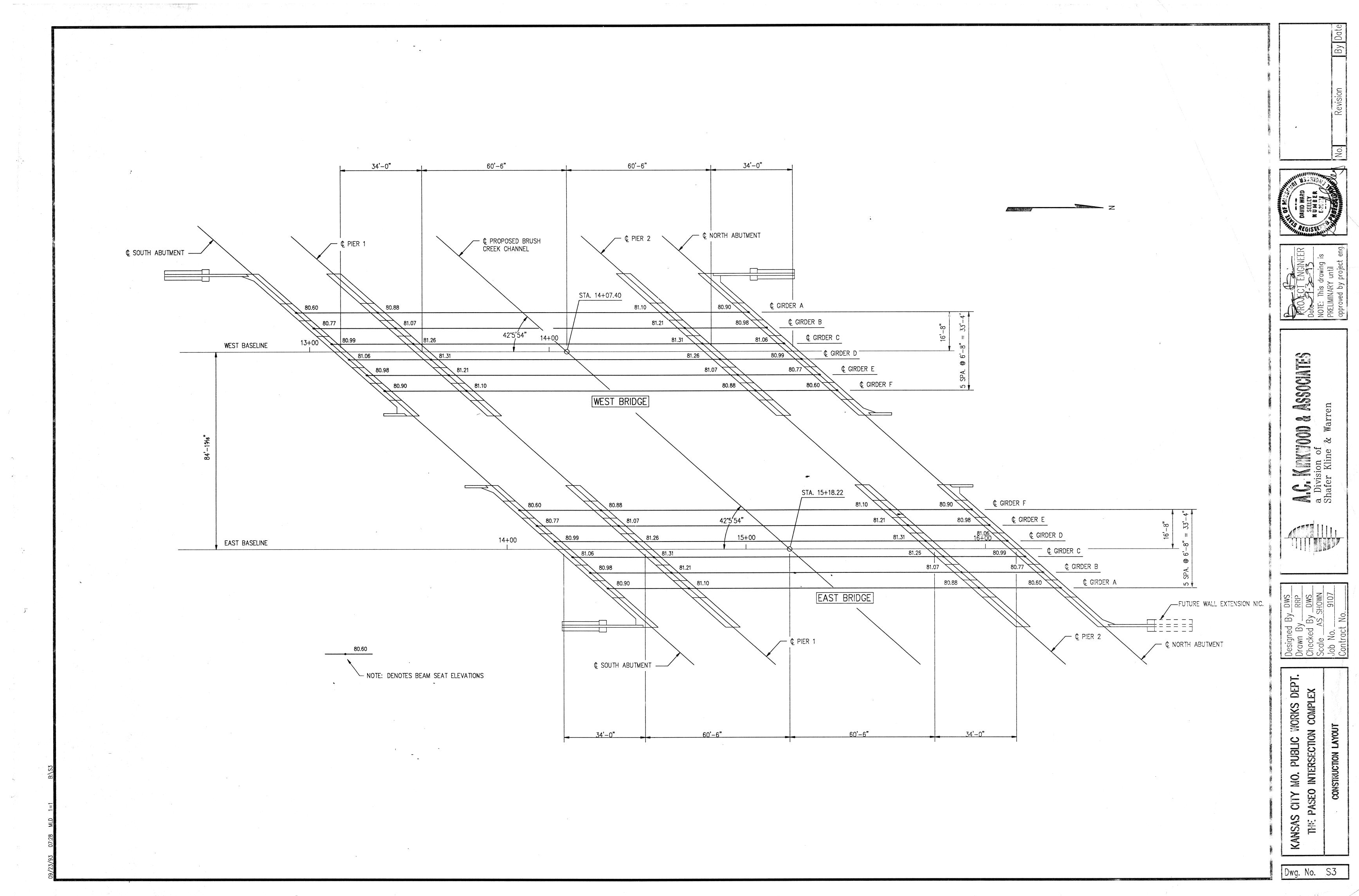
REINFORCING BARS SHALL BE DEFORMED NEW BILLET STEEL CONFORMING TO ASTM A615-GRADE 60. REINFORCING BARS TO BE WELDED SHALL BE OF A WELDABLE GRADE. MINIMUM CLEARANCE TO REINFORCING STEEL SHALL BE 2" UNLESS OTHERWISE NOTED. ALL REINFORCING BENDS SHALL BE DETAILED IN ACCORDANCE WITH THE MANUAL OF STANDARD PRACTICE OF THE CONCRETE REINFORCING STEEL INSTITUTE, UNLESS OTHERWISE SHOWN ON THE PLANS. BAR DIMENSIONS ARE GIVEN OUT TO OUT OF BAR.

NON-PARTICIPATING ITEMS/UTILITIES

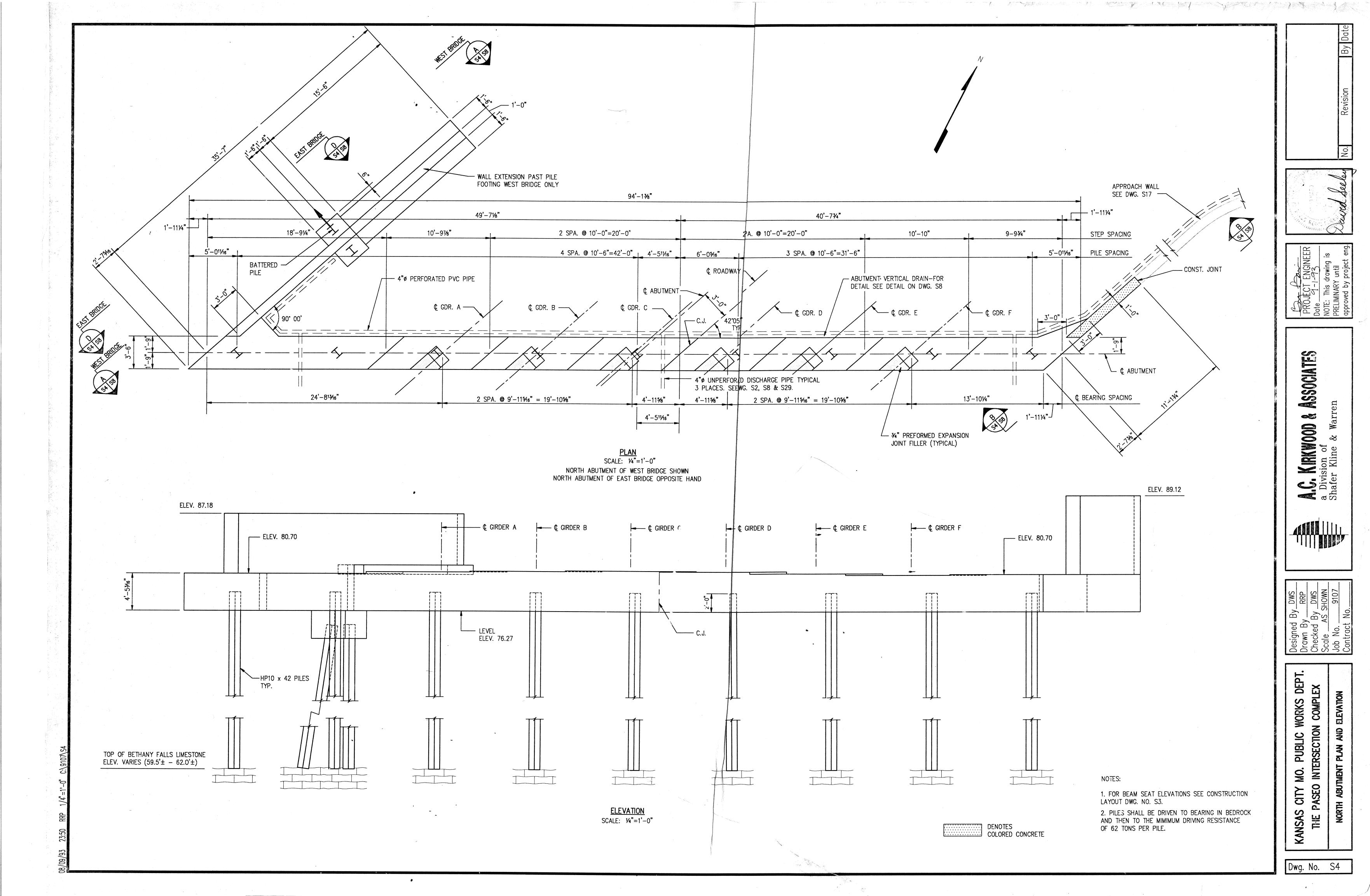
THESE ITEMS DO NOT RECEIVE FEDERAL PARTICIPATING FUNDS.

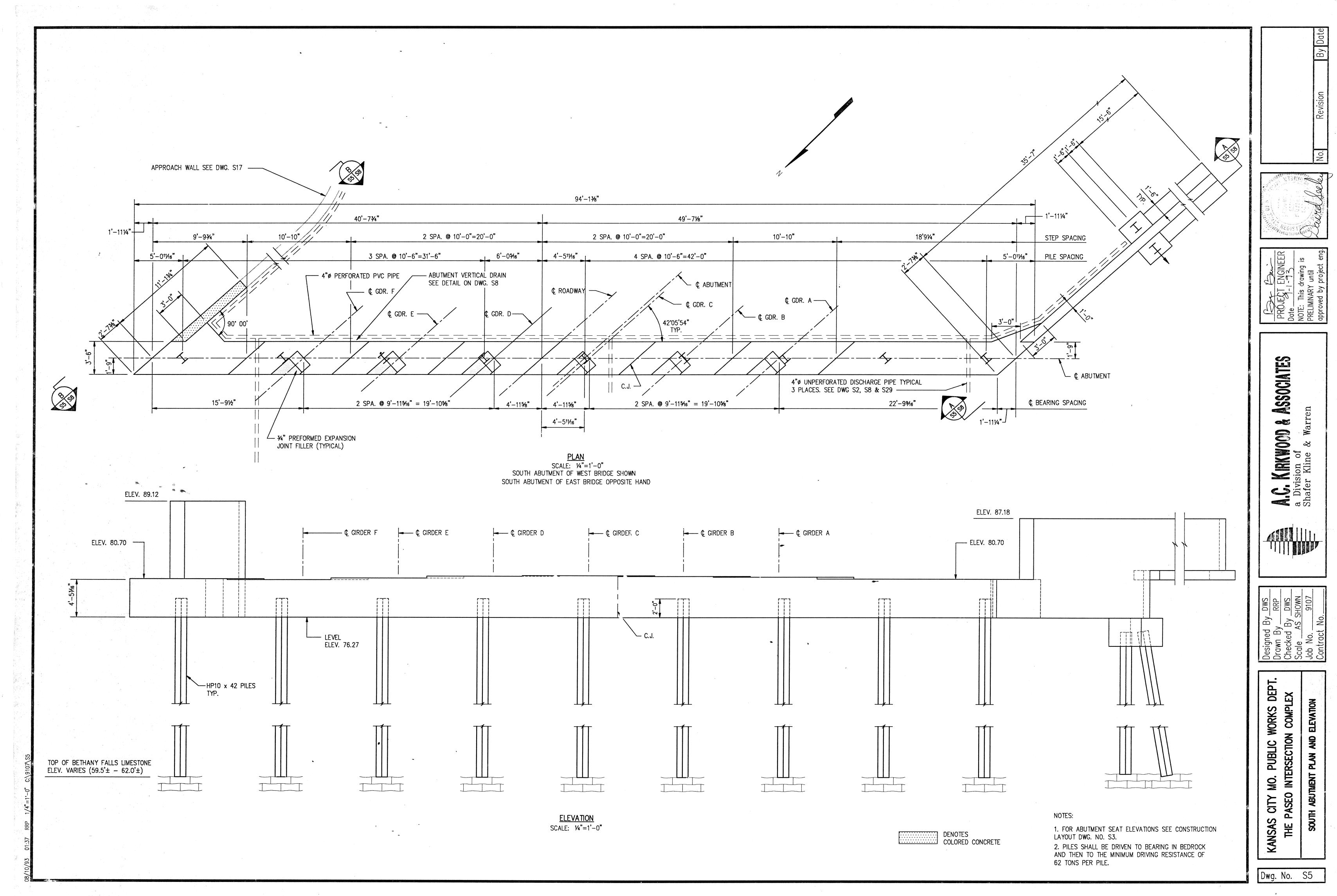


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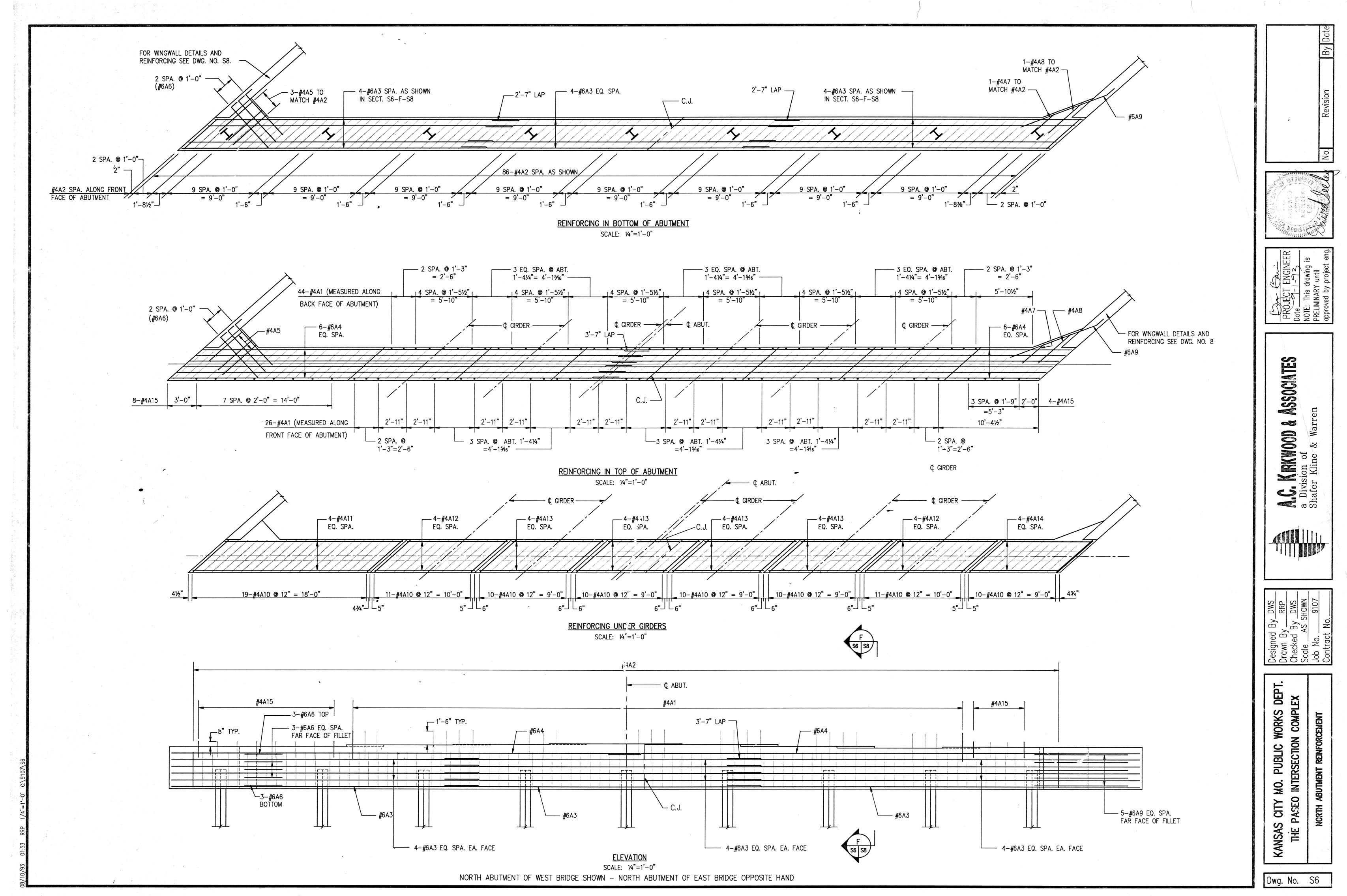


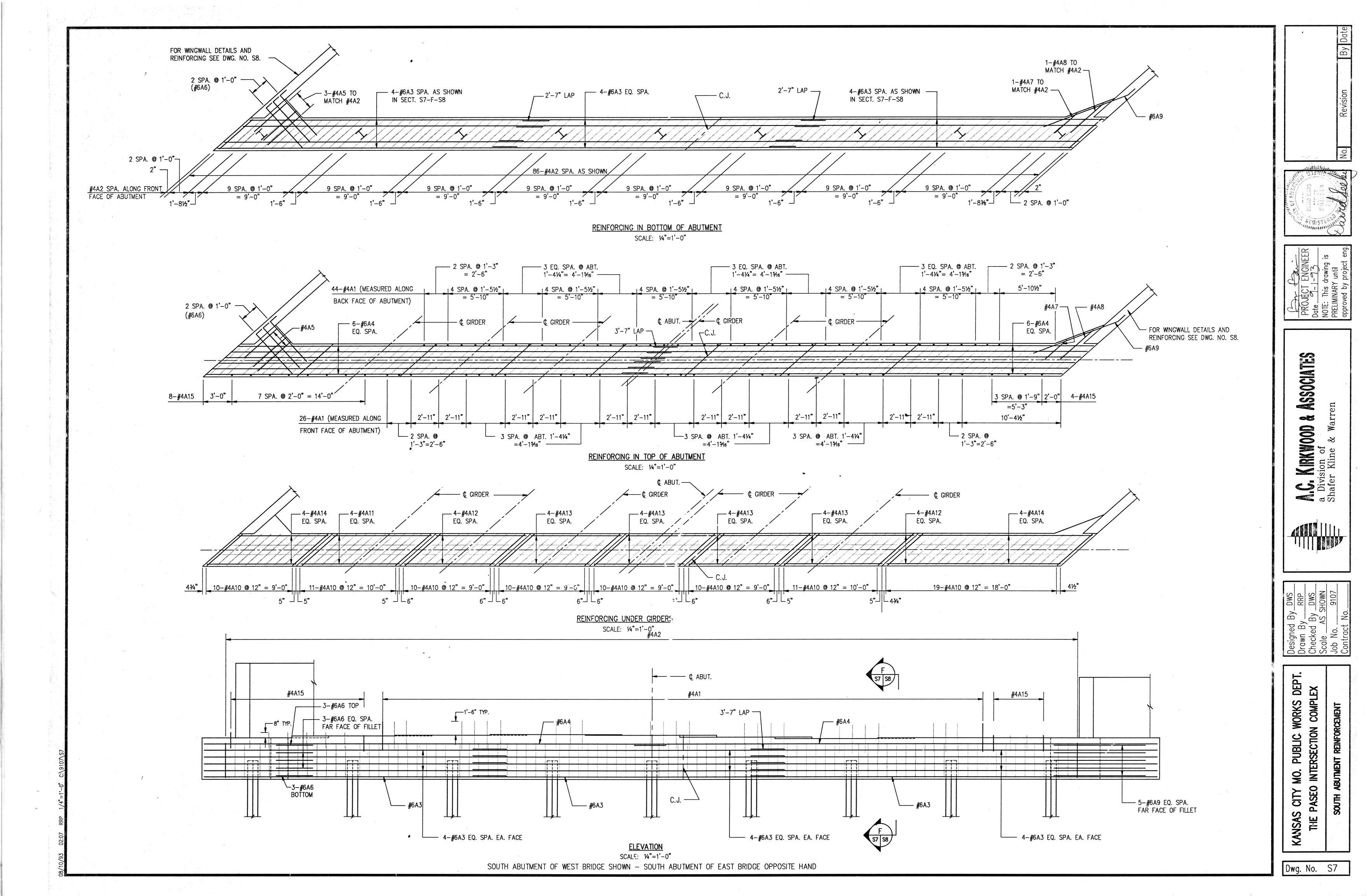
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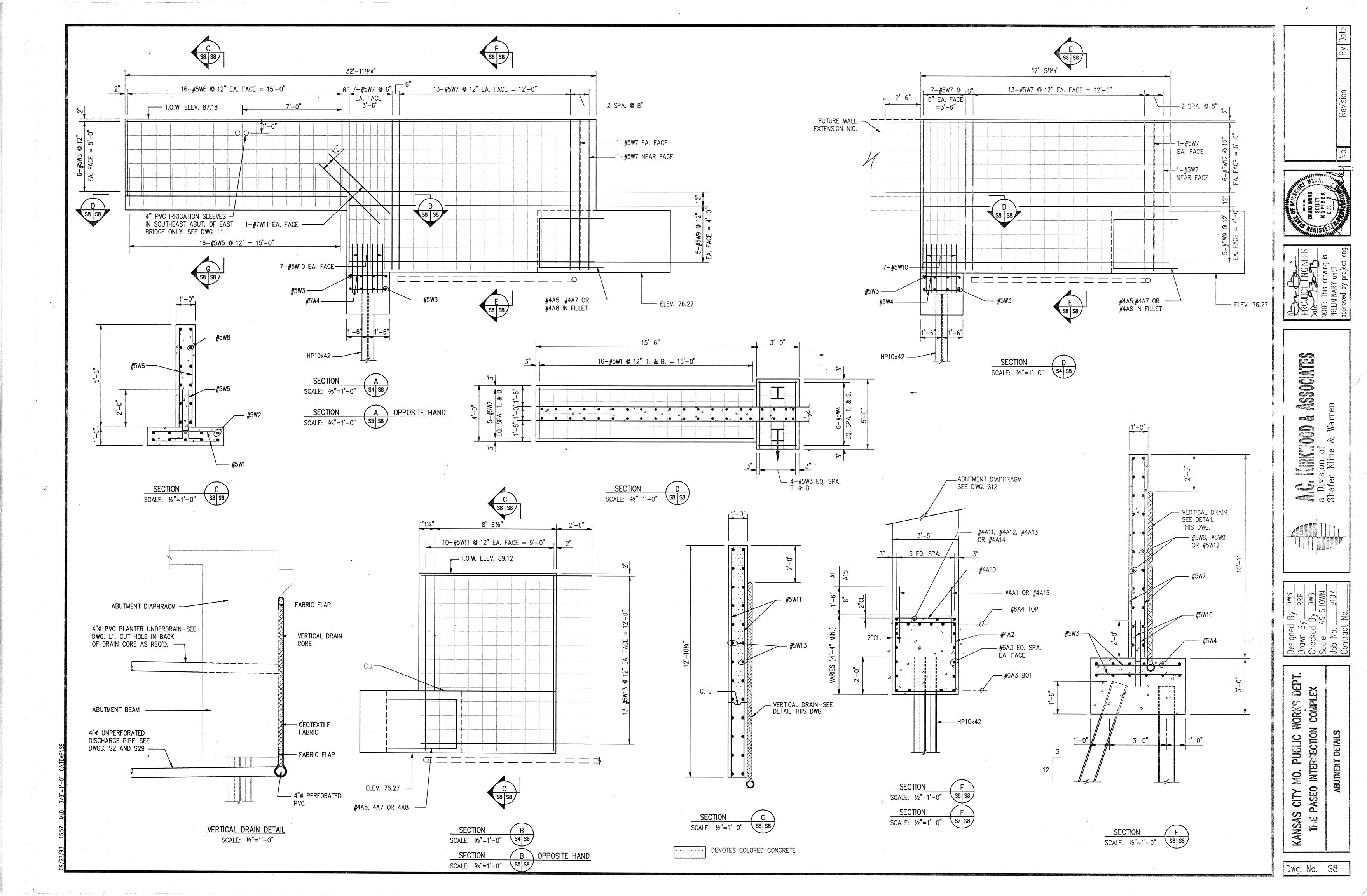


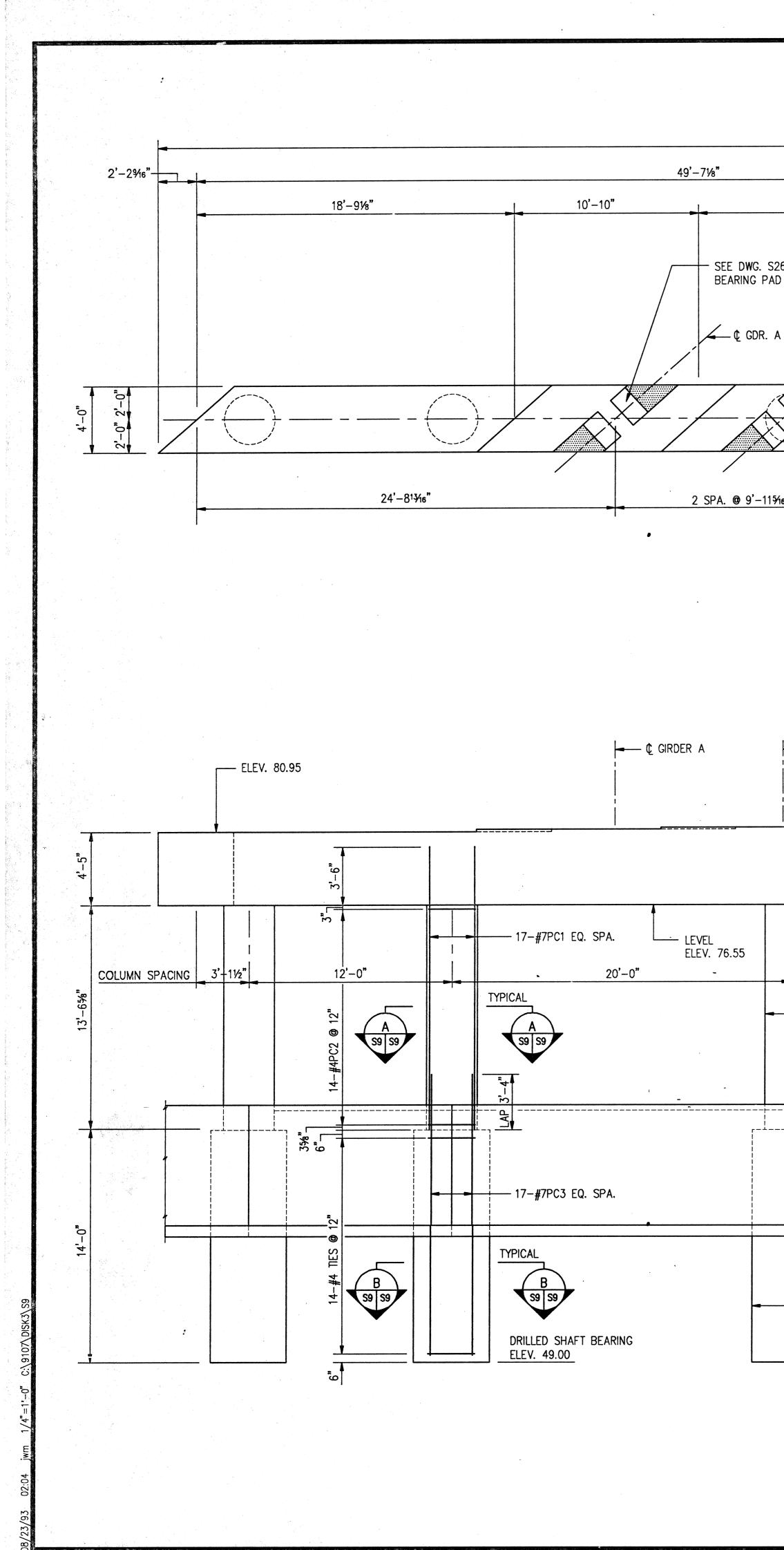








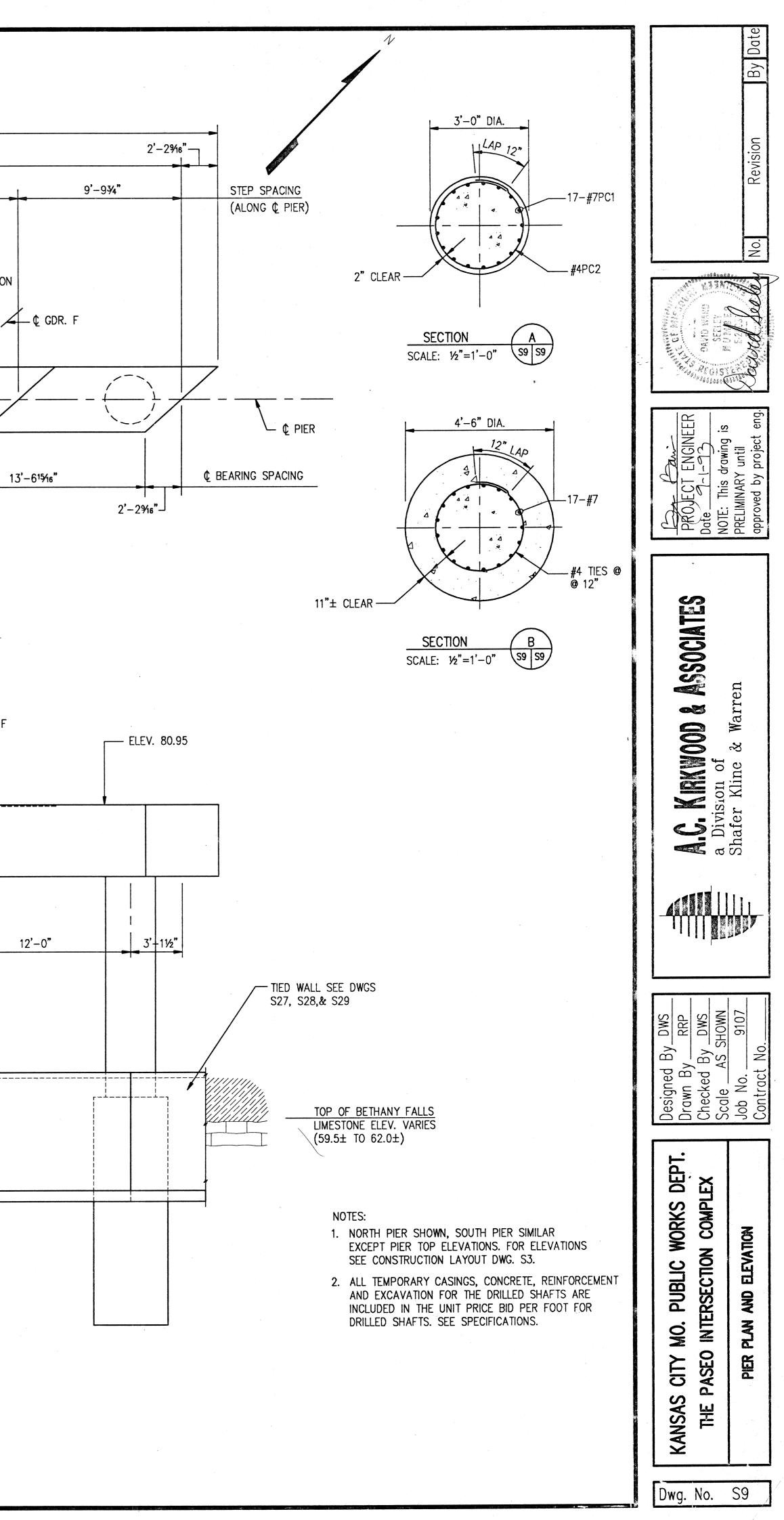


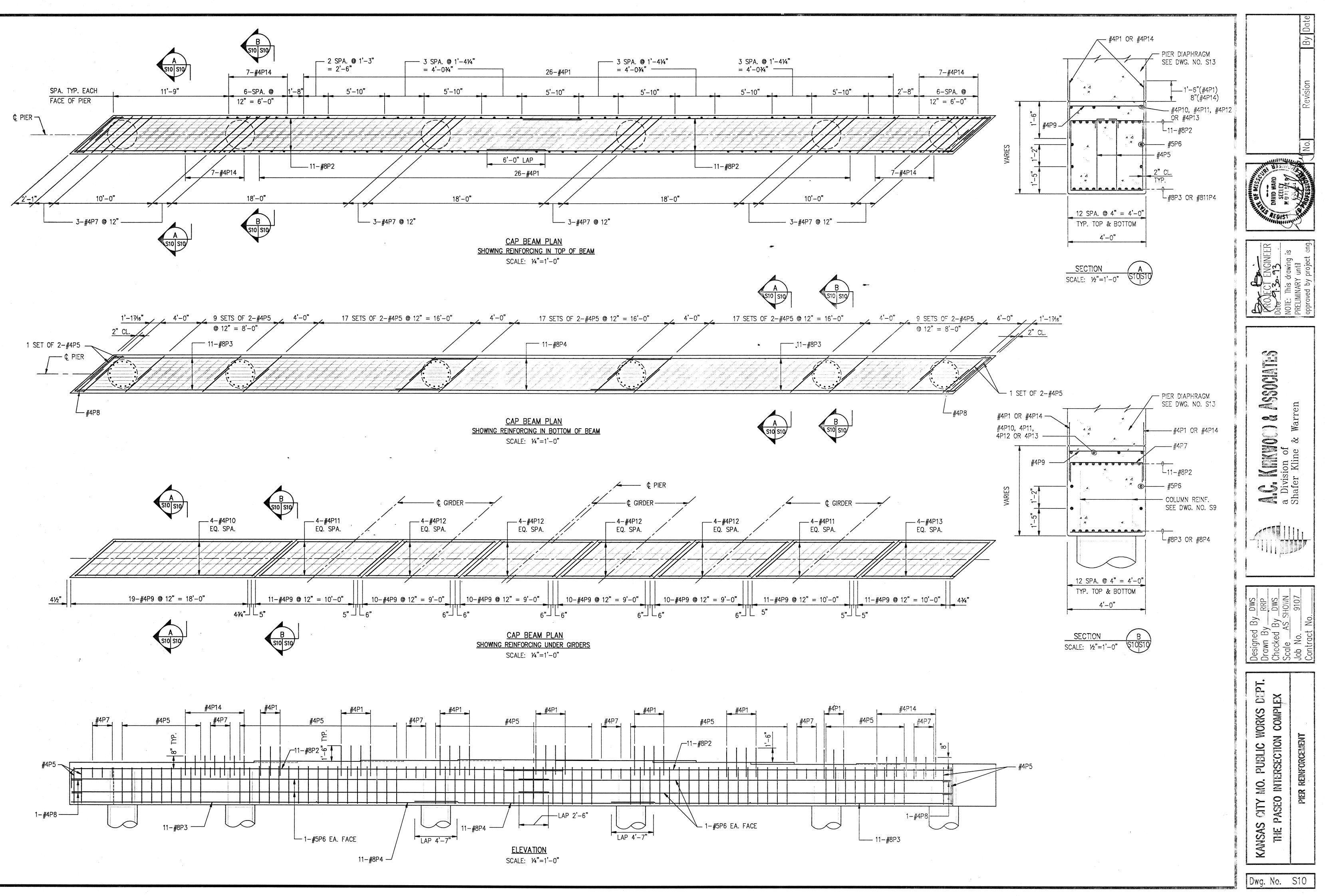


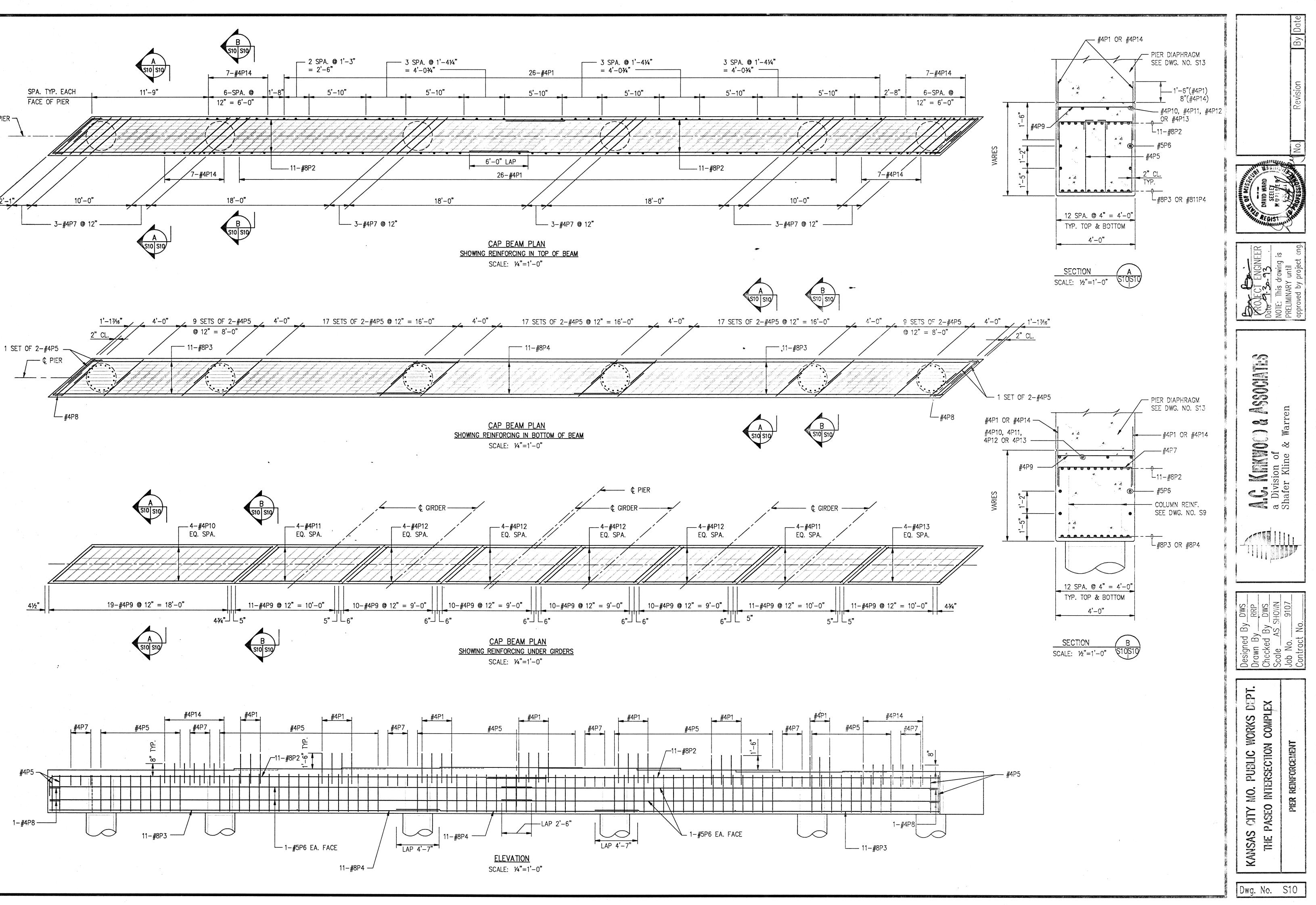
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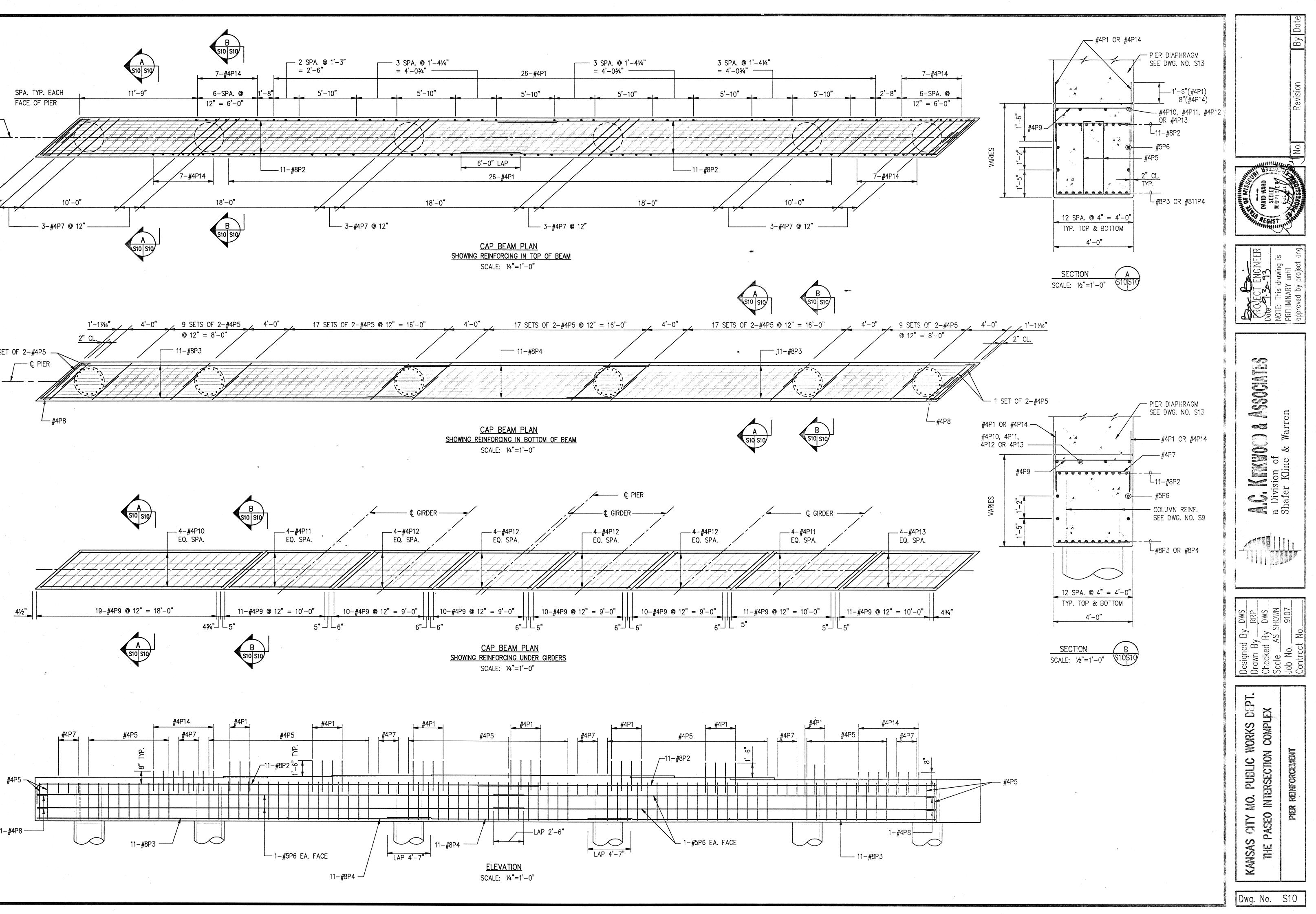
94'-8" 40'-7¾" 10'-10" 2 SPACES @ 10'-0" = 20'-0"2 SPACES @ 10'-0" = 20'-0" 🖞 ROADWAY — — SEE DWG. S26 FOR BEARING PAD LOCATIONS ¢ PIER -¢ GDR. D ← ¢ GDR. E ¢ GDR. B 🕻 GDR. C — 42.05'54" 13'-6¹⁵⁄16" 2 SPA. @ 9'-115/16 = 19'-105%" 4'-115%" 2 SPA. @ 9'-115/16 = 19'-105/8''4'-115%" 4'-511/16" PLAN (LOOKING ALONG STATIONING) (WEST BRIDGE PIERS SHOWN EAST BRIDGE PIERS OPPOSITE HAND) SCALE: 1/4"=1'-0" C PIER C ROADWAY GIRDER F GIRDER E GIRDER B GIRDER D . . 12'-0" 10'-0" 20'-0" 4'-511⁄16" 5'-6516' 3'-0" DIA. COLUMN - SANDBLAST ALL EXPOSED SURFACES TYPICAL TOP OF WALL & SIDE WALK -ELEV. 63.00 - BOTTOM OF PROPOSED CHANNEL - 54"ø DRILLED SHAFT 54" DIA. DRILLED SHAFT TYPICAL

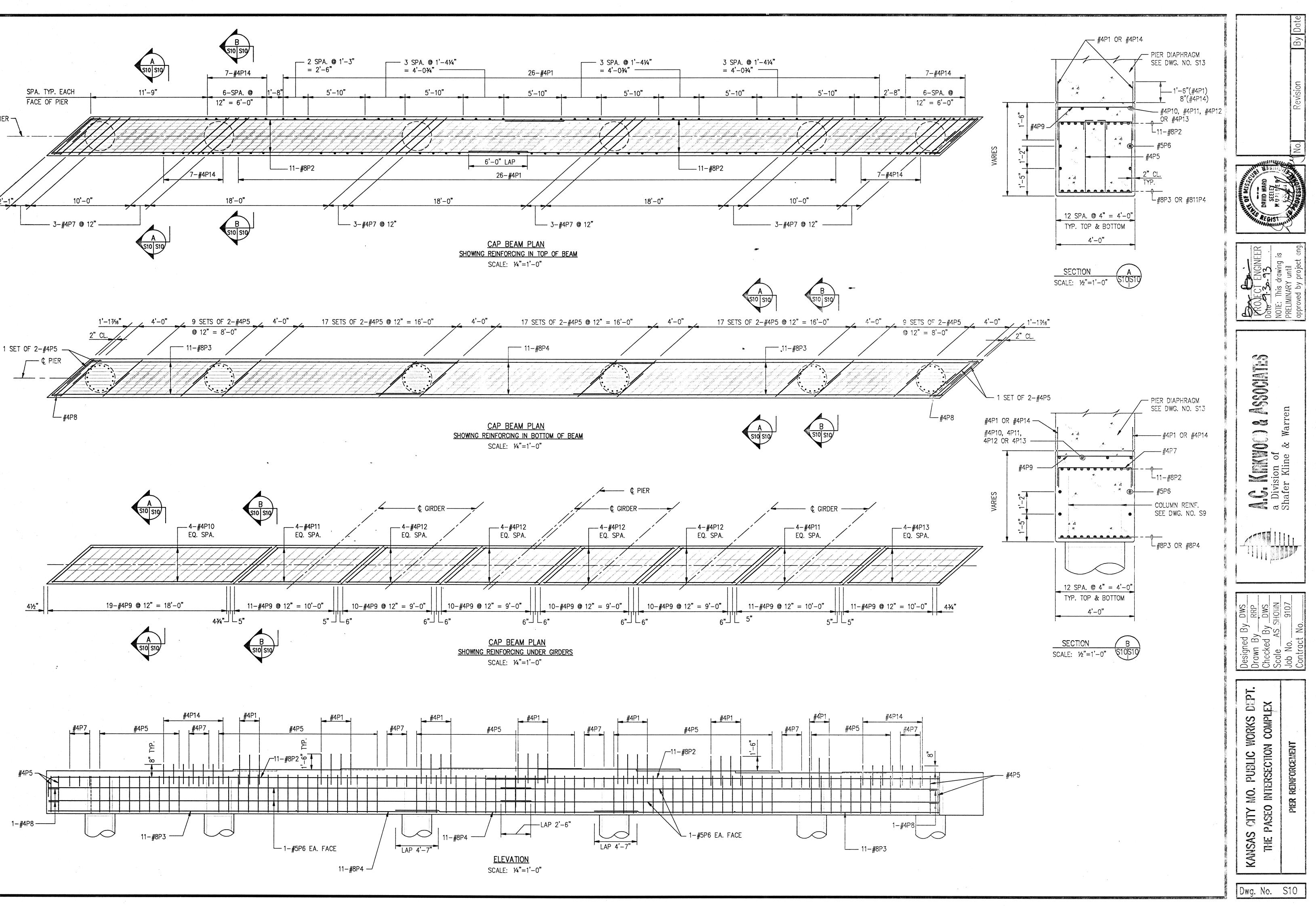
ELEVATION SCALE: 1/4"=1'-0"

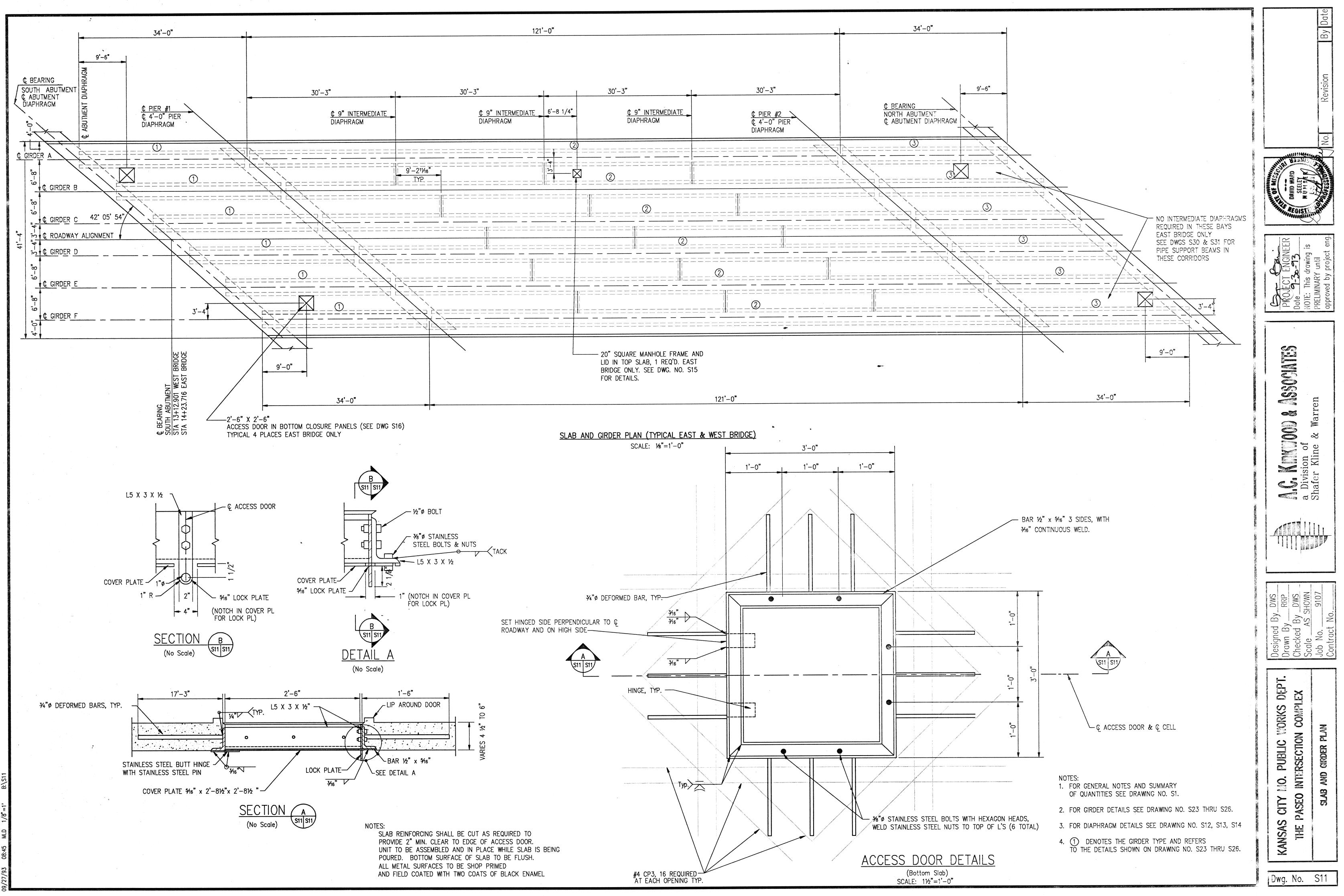


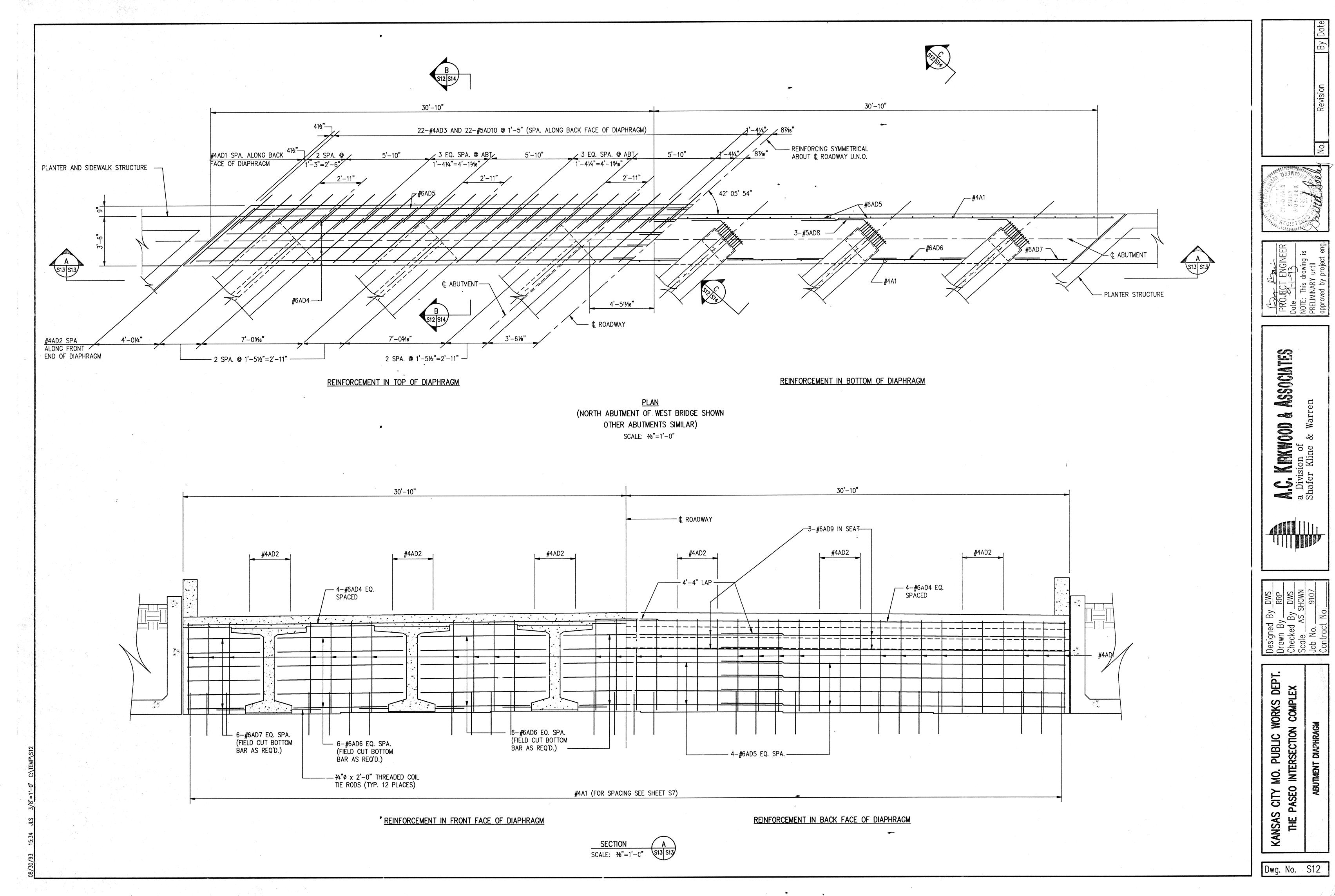


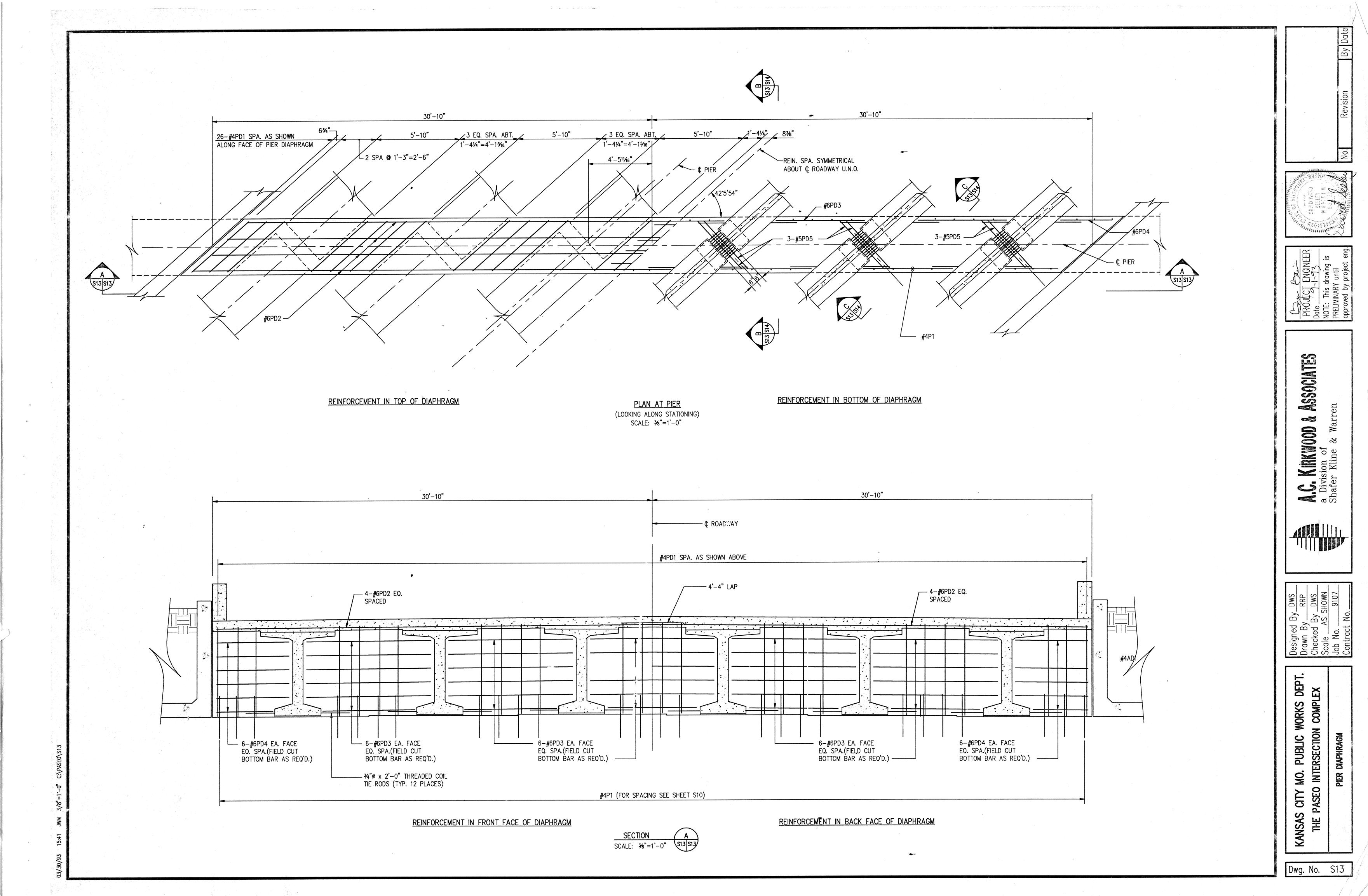


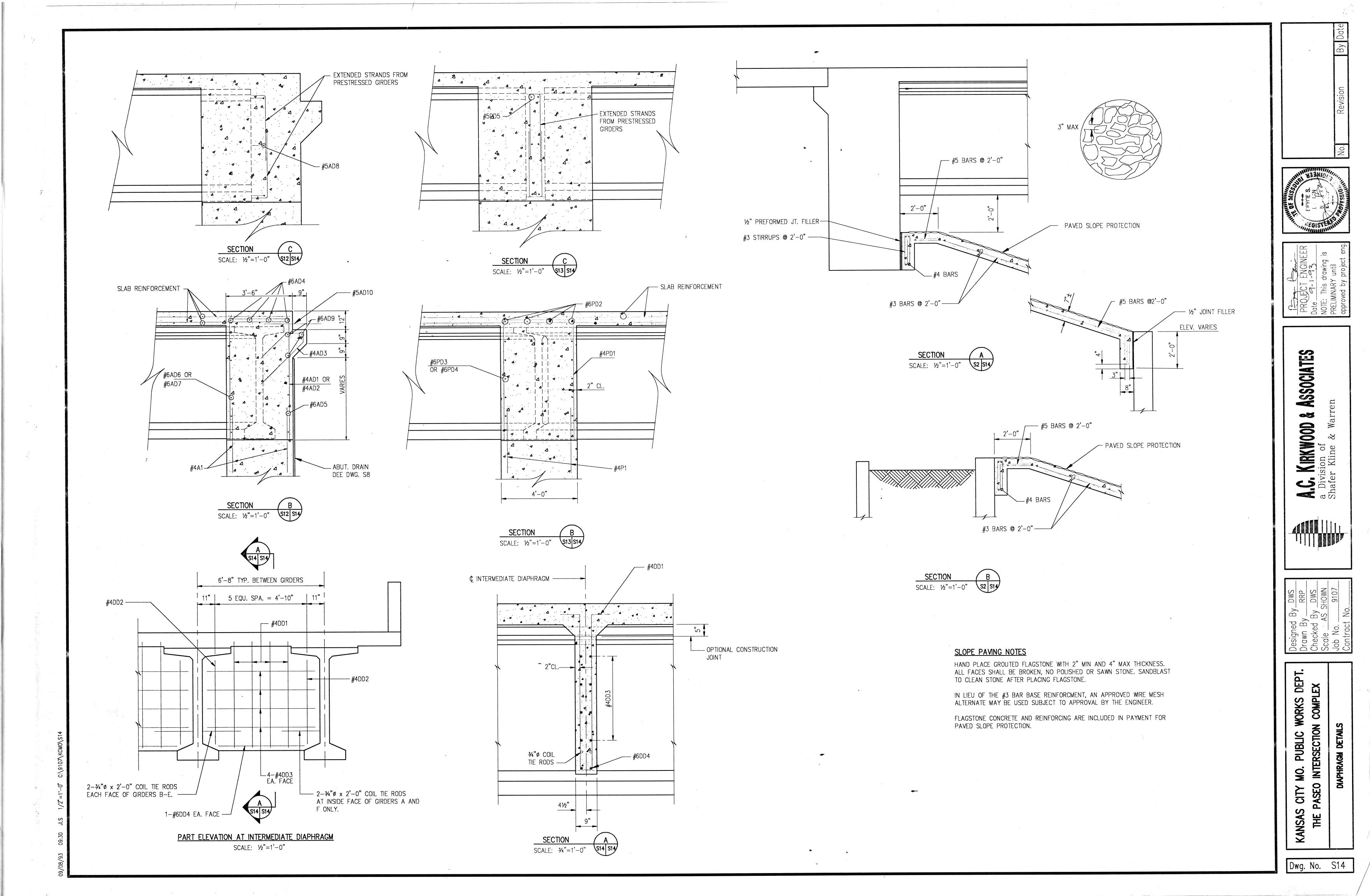


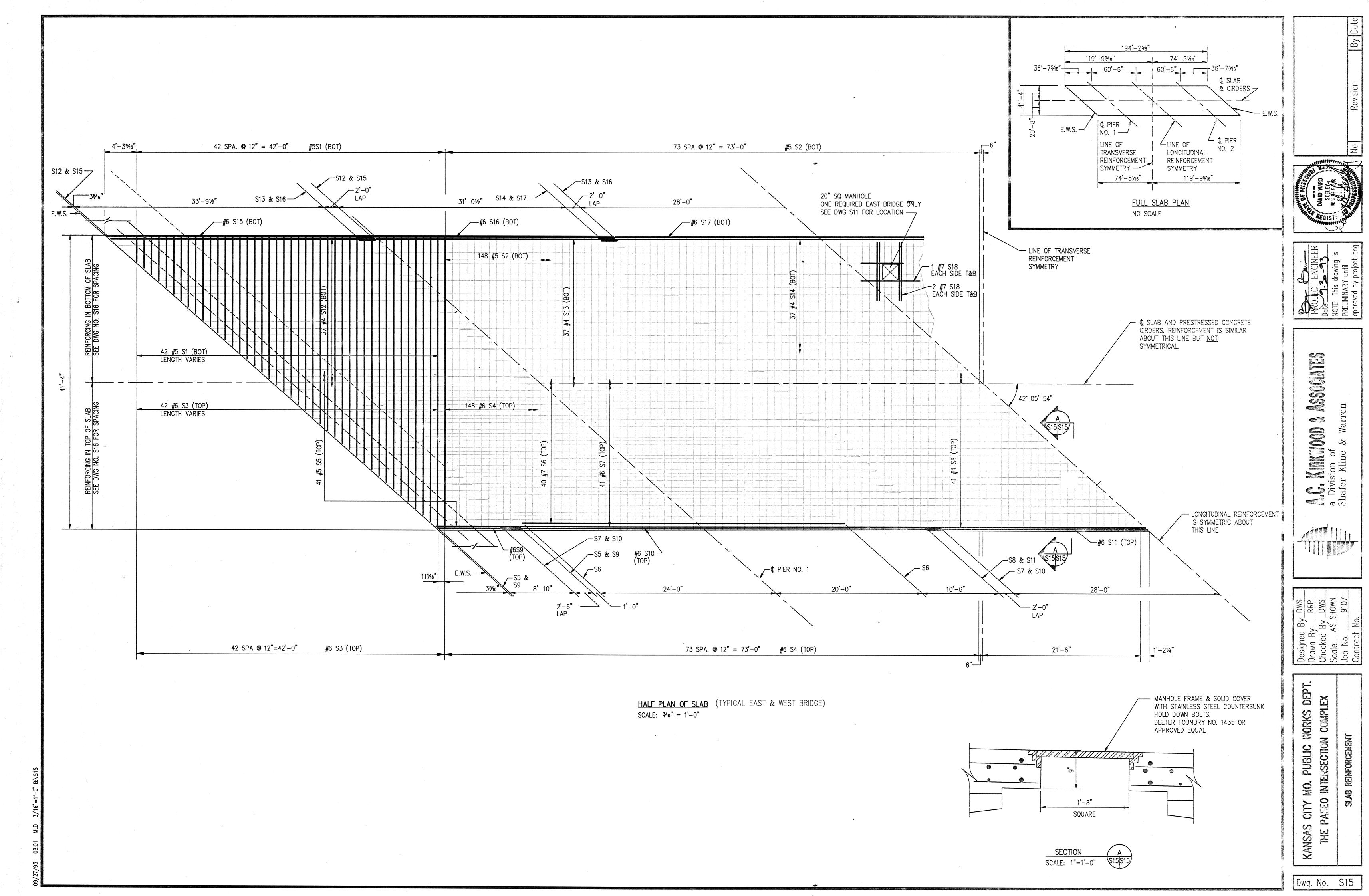


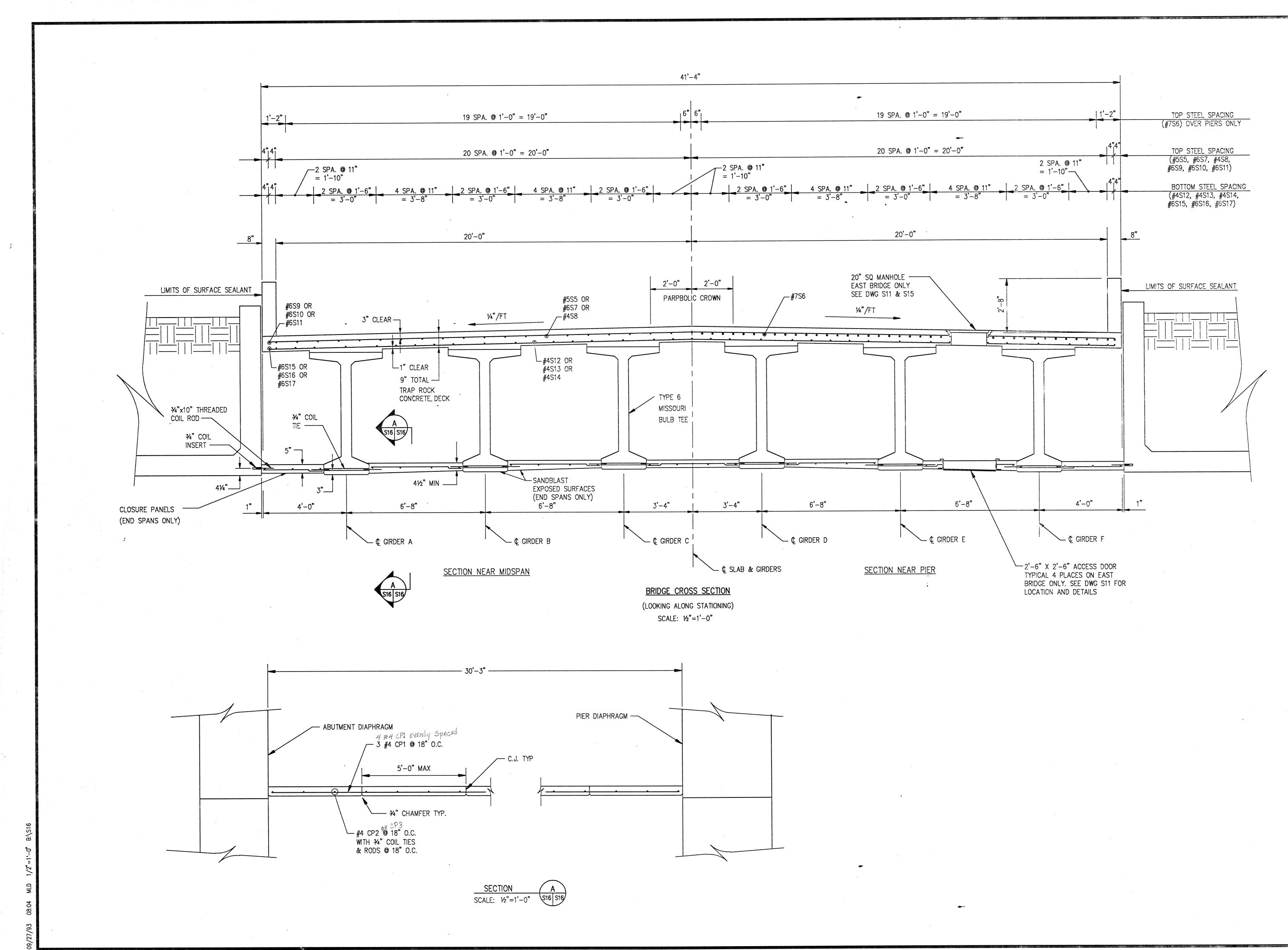


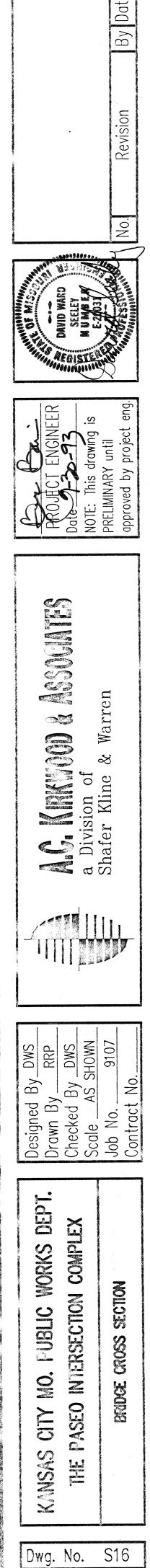


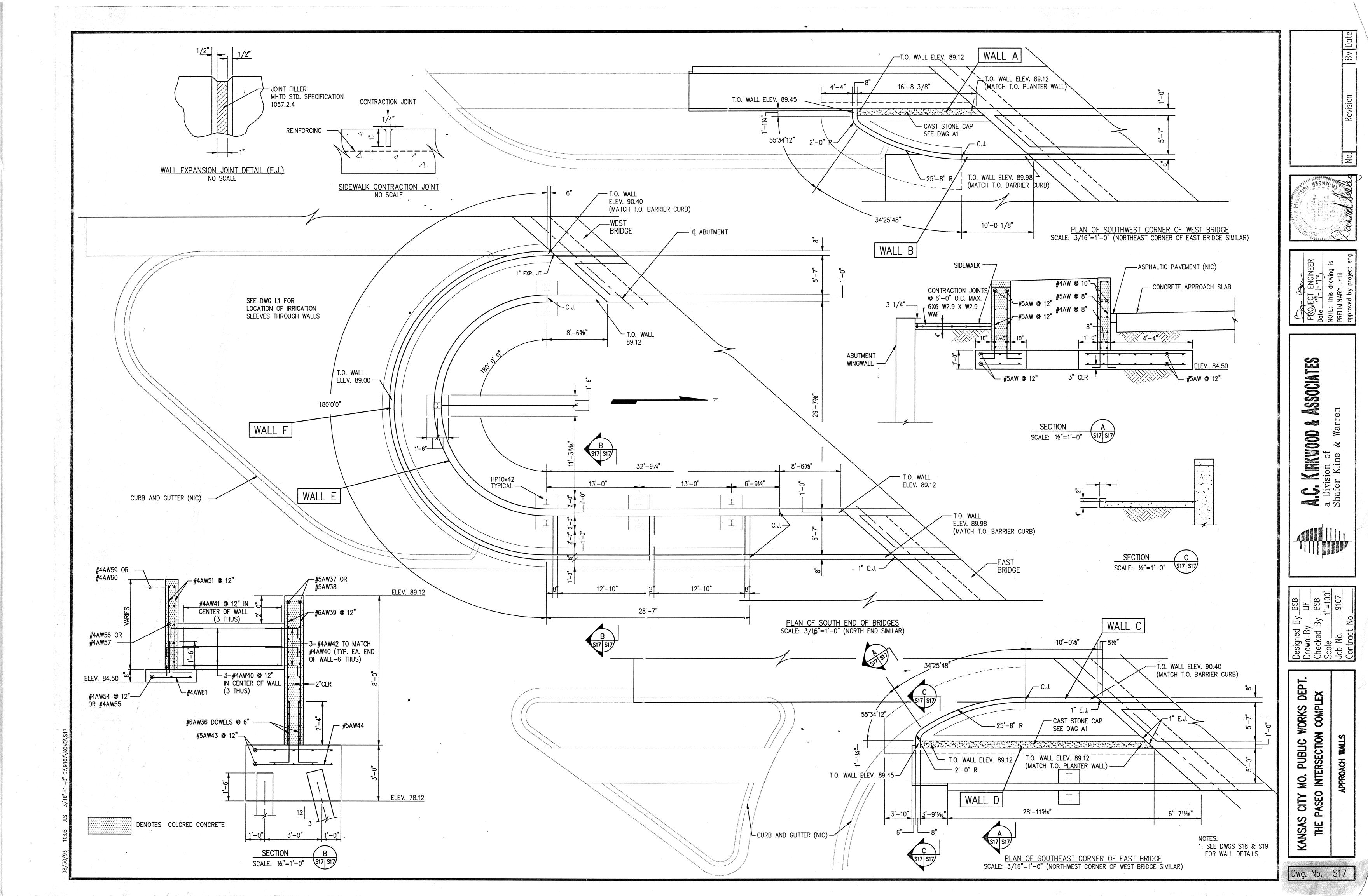


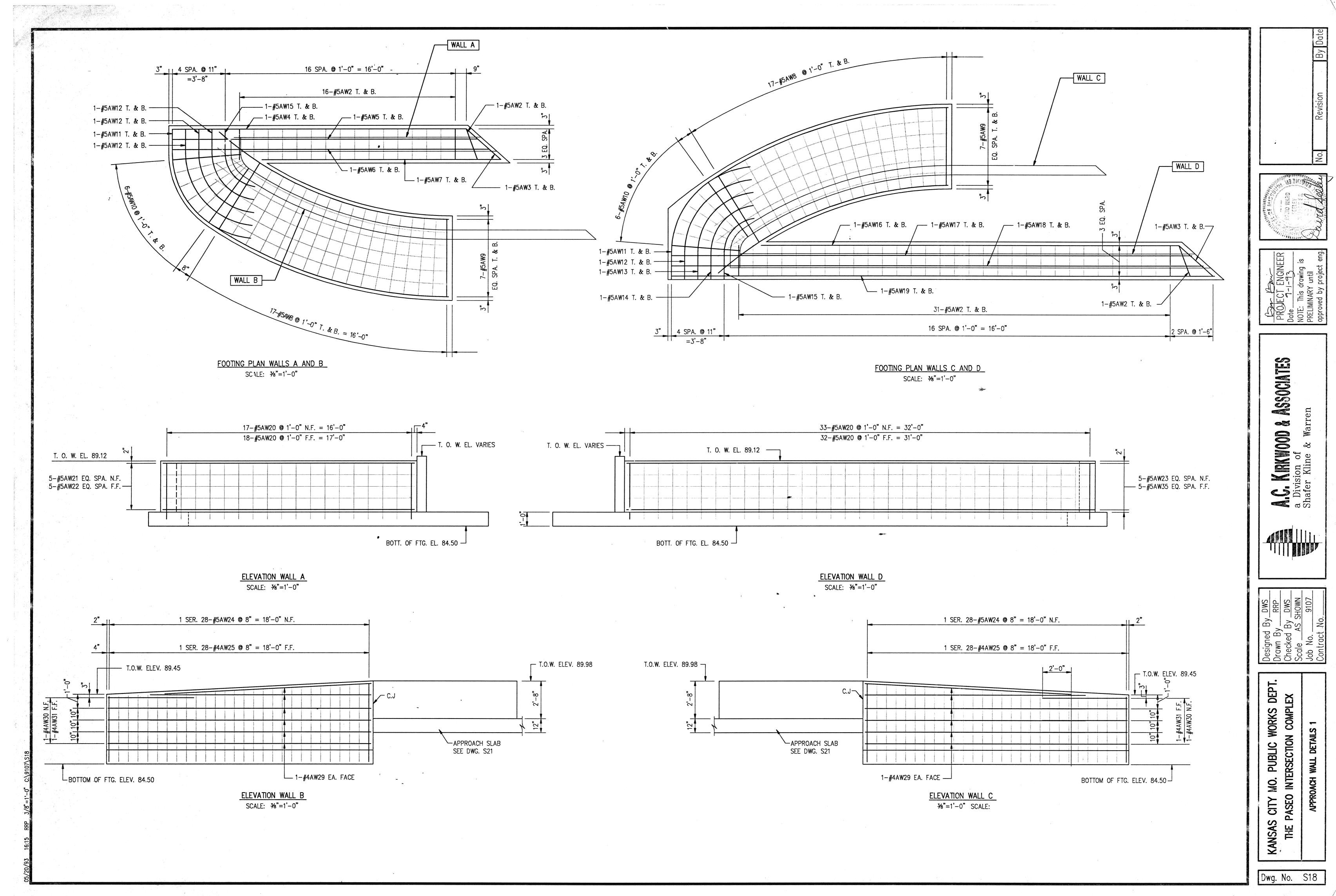


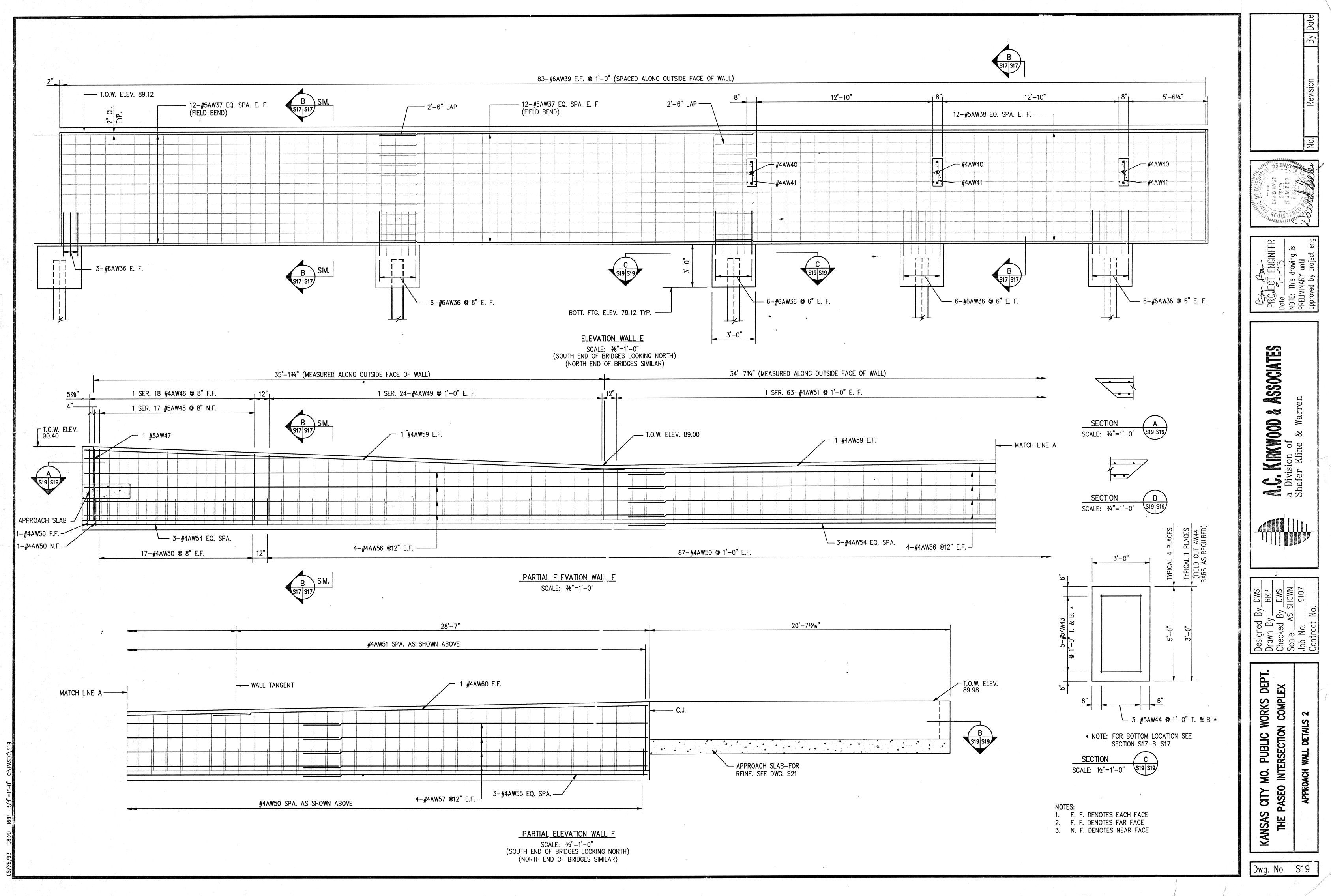




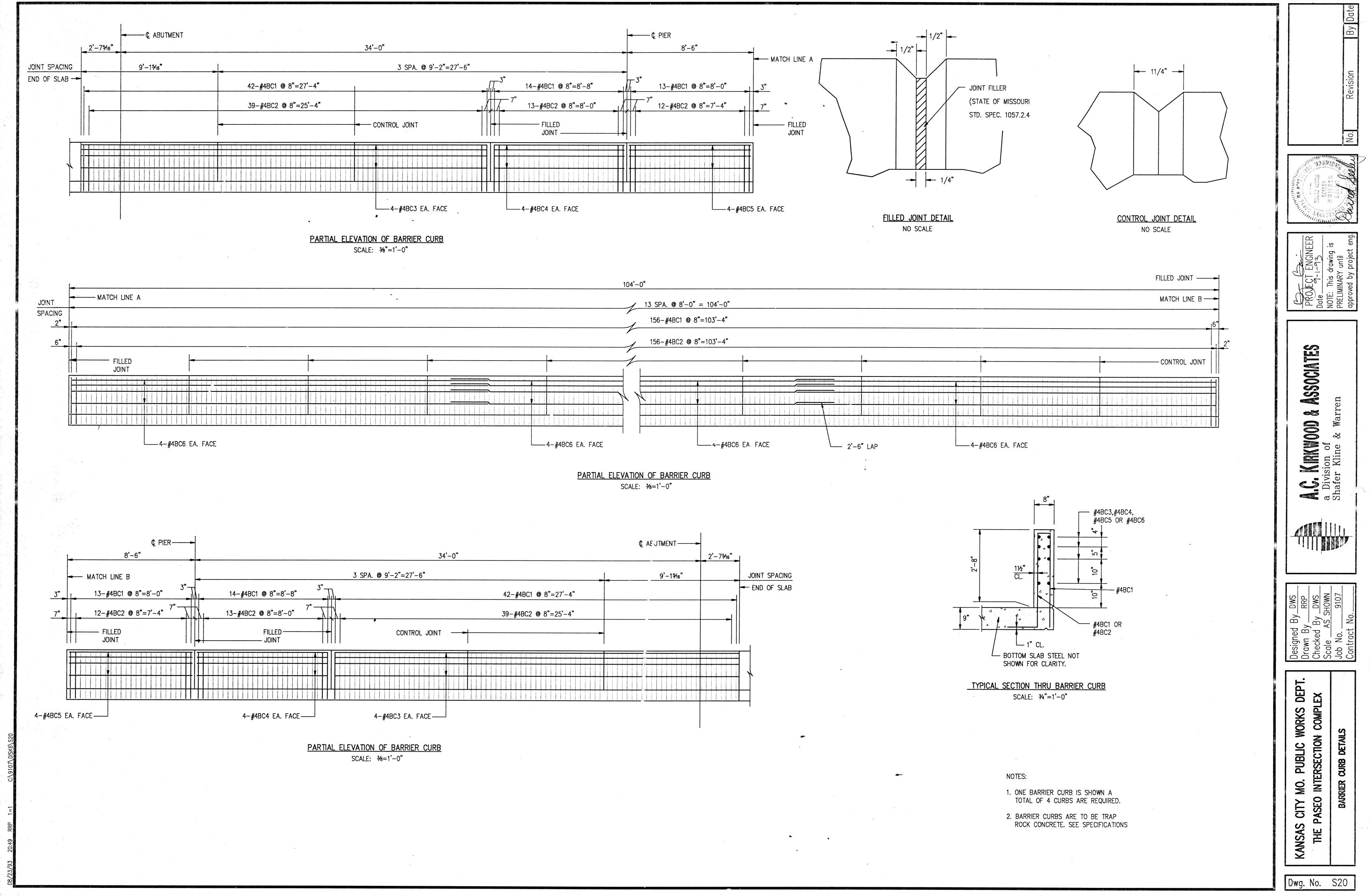


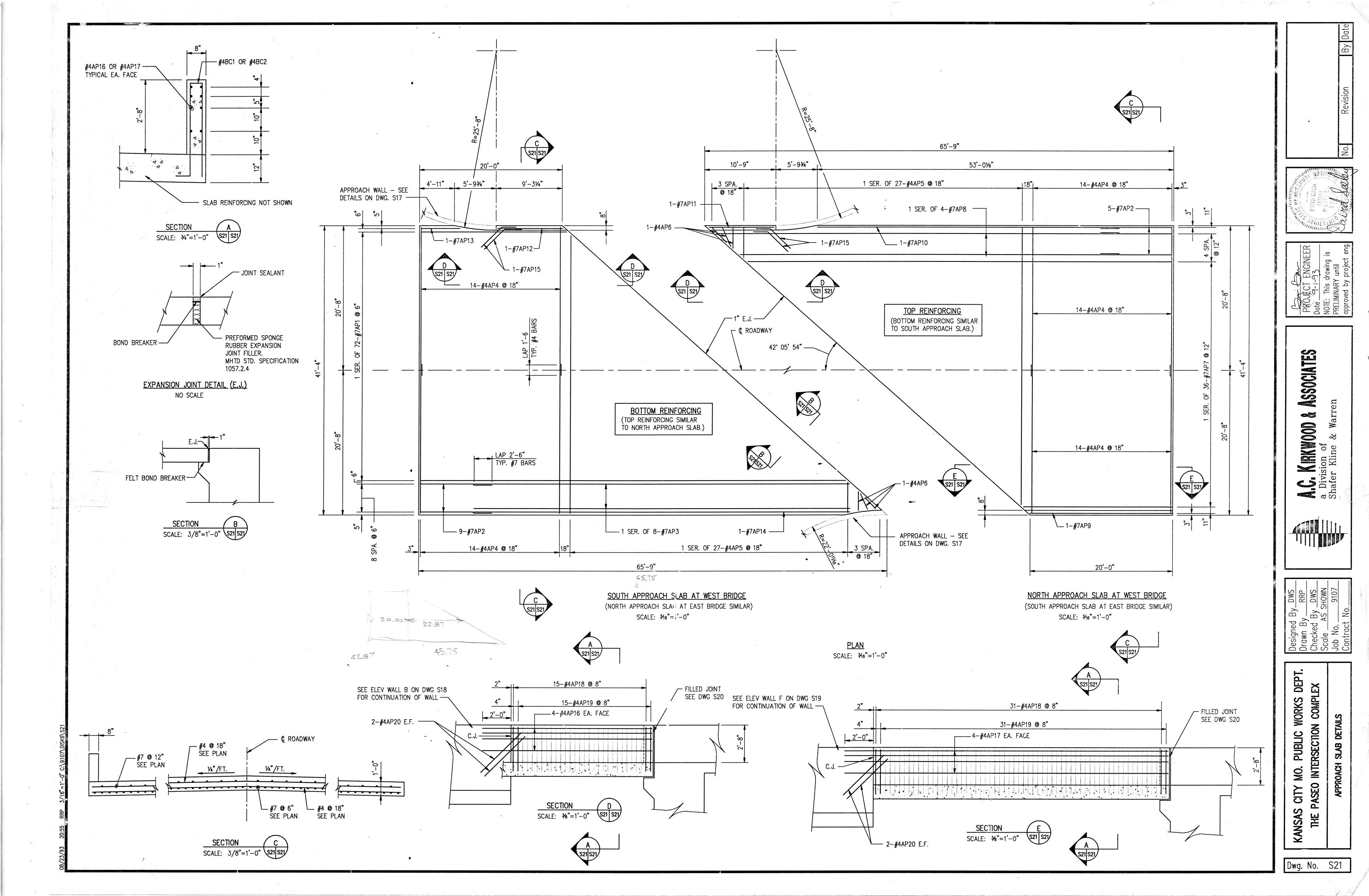






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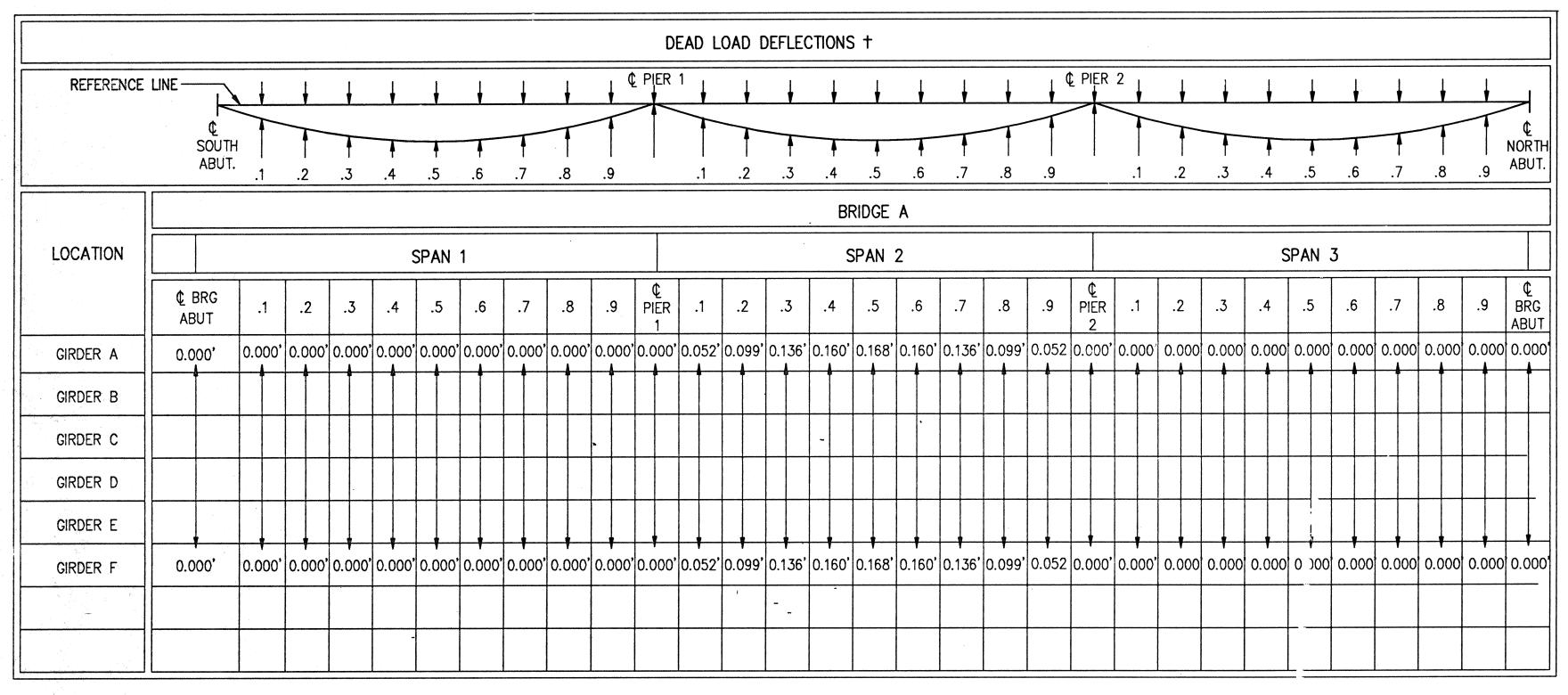




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	¢ BRG ABUT	.1	.2	.3	.4	.5	.6	.7	.8	.9	¢ PIER 1	.1	.2	.3	.4
GIRDER A	0.043'	0.050'	0.056'	0.062'	C.068'	0.074'	0.078'	0.081'	0.083'	0.084'	0.083'	0.149'	0.200'	0.237	0.25
GIRDER B	0.083'	0.090'	0.094'	0.097'	0.099'	0.103'	0.099'	0.097'	0.095'	0.091'	P		T		
GIRDER C		0.088'	0.092'	0.095'	0.096'	0.097'	0.096'	0.095'	0.092'	0.088'					
GIRDER D															
GIRDER E															
GIRDER F	0.083'	0.088'	0.092'	0.095'	0.096'	0.097'	0.096'	0.095'	0.092'	0.088'	0.083'	0.149'	0.200'	0.237'	0.25

* INCLUDES CORRECTION FOR, SLOPE OF GIRDERS, AND VERTICAL CURVE.



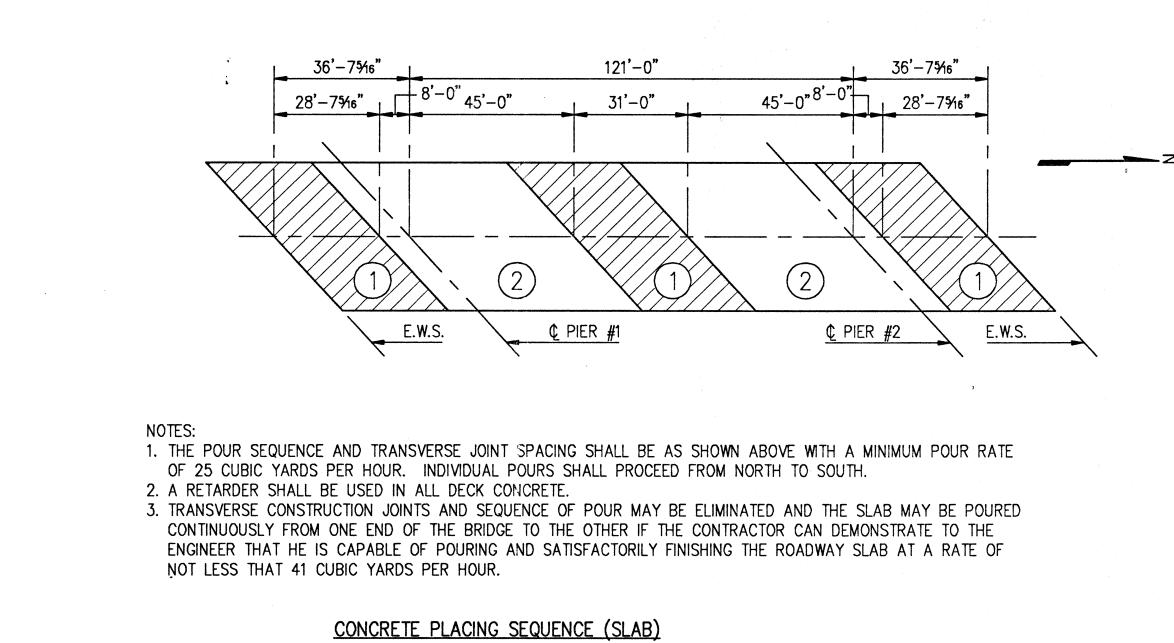
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+ DEFLECTION CAUSED BY WEIGHT OF SLAB, HAUNCHES, AND DLAPHRAGMS.

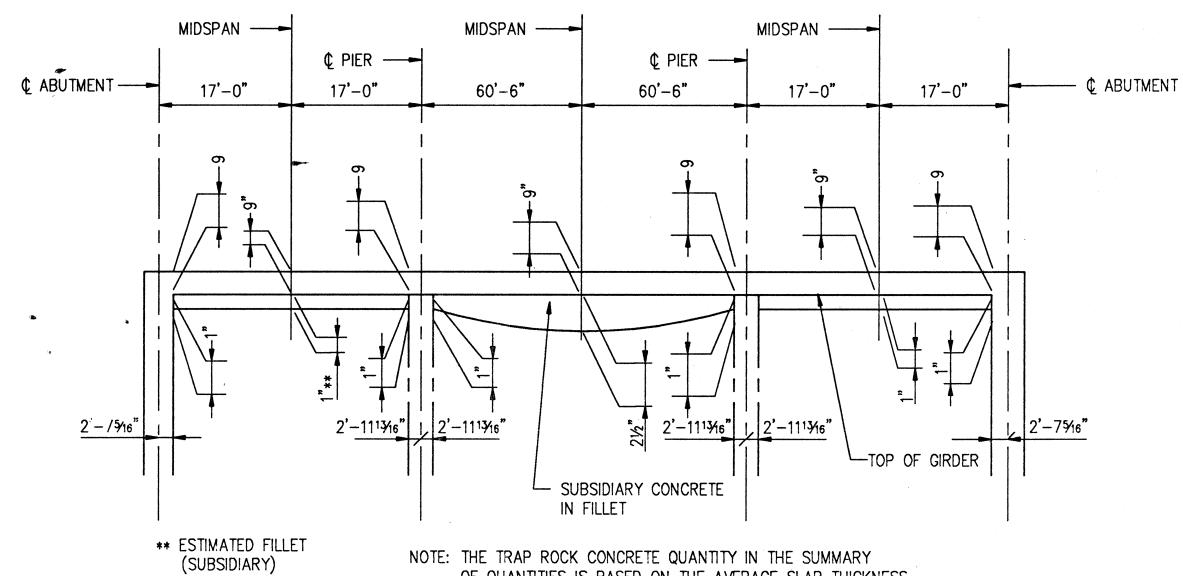
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LOCATION	х х х х х х х х				Q	SPAN	1							•	S
	¢ BRG ABUT	.1	.2	.3	.4	.5	.6	.7	.8	.9	¢ PIER 1	.1	• .2	.3	.4
GIRDER A	0.043'	0.038'	0.047'	0.050'	0.054'	0.059'	0.064'	0.069'	0.074'	0.079'	0.083	0.128	0.165'	0.191'	0.206'
GIRDER B	0.083'	0.085'	0.085'	0.085'	0.085'	0.088'	0.085'	0.085'	0.086'	0.086'					
GIRDER C		0.083'	0.083'	0.083'	0.083'	0.083'	0.083'	0.083'	0.083'	0.083'					
GIRDER D									•	F					
GIRDER E															
GIRDER F	0.083'	0.083'	0.083'	0.083'	0.083'	0.083'	0.083'	0.083'	0.083'	0.083' (0.083	' 0.128'	0.165'	0.191'	0.206'

* ESTIMATE ONLY CONTRACTOR SHALL CALCULATE HAUNCH BASED ON MEASURED CAMBER

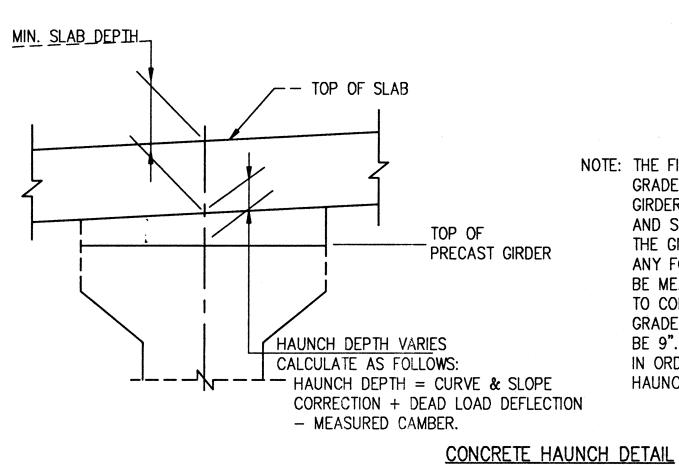
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ßF	RIDGE	ΞA																												
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	.5		.6	.7		.8		.9	PIE 2	R		1	.2	2	.3		.4	1	.5	5	.6	5	.7	,	.8	3	.9)	¢ BR ABI	G
9'	0.26	6'0	.259	0.23	37'	0.200)' O.	149'	0.0	83'	0.0	88'	0.0	92'	0.09	95'	0.0	96'	0.0	97'	0.0	96'	0.0	95'	0.0	92'	0.0	88'	0.0	33'
											0.0	88'	0.0	92'	0.09	95'	0.0	96'	0.0	97'	0.0	96'	0.0	95'	0.0	92'	0.0	88'	0.0	B3'
											0.0	91'	0.0	95'	0.09	97 '	0.0	99'	0.10	03'	0.0	99'	0.0	97'	0.0	94'	0.0	90'	0.0	36'
ə'	0.26	6'0	.259	'0.23	37'	0.200)' 0.	149'	0.0	83'	0.0	84'	0.0	83'	0.08	81'	0.0	78'	0.0	74'	0.0	68'	0.0	62'	0.0	56'	0.0	50'	0.04	43'



NO SCALE



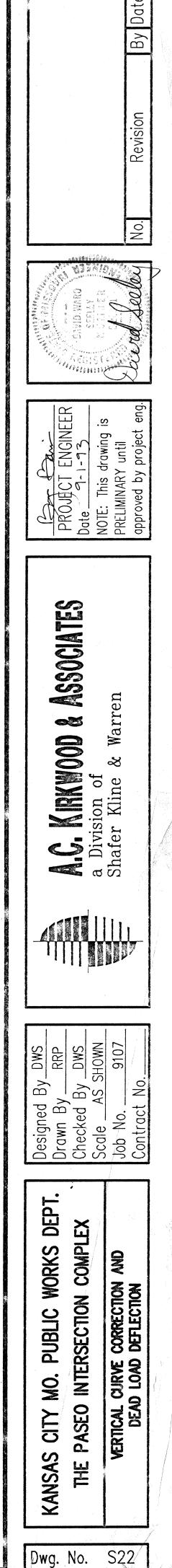
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3'	0.2	211'	0.2	206'	0.1	91'	0.1	65'	0.1	28'	0.0	83'	0.0	83'	0.0)83'	0.08	33'	0.08	3'	0.083'	0.083'	0.0	83'	0.0	83'	0.0	83'	0.0	83'
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													0.0	83'	0.0	•)83'	0.08	33'	0.08	3'	0.083'	0.083'	0.0	83'	0.0	83'	0.0	83'		-
													0.0	86'	0.0	86'	0.08	35'	0.08	5'	0.088'	0.085'	0.0	85'	0.08	35'	0.0	85'	0.0	83'
,, ,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	0.2	211'	0.2	:06'	0.1	91'	0.1	65'	0.1	28'	0.0	83'	0.0	79'	0.0)74'	0.06	<u>;</u> 9'	0.06	4'	0.059'	0.054'	0.0	50'	0.04	47'	0.0	38'	0.0	43'



OF QUANTITIES IS BASED ON THE AVERAGE SLAB THICKNESS OF 9". APPROXIMATELY 14 CUBIC YARDS OF ADDITIONAL CONCRETE WILL BE REQUIRED DUE TO THE VARIANCE IN THE HAUNCH THICKNESS. THE CONCRETE QUANTITY NEEDED TO COMPENSATE FOR BEAM CAMBER IN ORDER TO PROPERLY CONSTRUCT THE BRIDGE TO CROWN GRADE SHALL NOT BE PAID FOR DIRECTLY, BUT SHALL BE SUBSIDIARY TO THE UNIT PRICE BID FOR TRAP ROCK CONCRETE.

SKETCH SHOWING SUBSIDIARY CONCRETE IN HAUNCH NO SCALE

NOTE: THE FINISHED DECK SLAB SHALL BE CONSTRUCTED TO PLAN GRADE BY VARYING THE DEPTH OF THE HAUNCH OVER THE GIRDER TO PROVIDE FOR PRESTRESS CAMBER, VERTICAL CURVE AND SLOPE, AND CONCRETE DEAD LOAD DEFLECTION. AFTER THE GIRDERS HAVE BEEN ERECTED, AND PRIOR TO PLACING ANY FORMWORK, THE ACTUAL CAMBER IN EACH GIRDER SHALL BE MEASURED IN THE FIELD. ADJUST THE HAUNCH THICKNESS TO COMPENSATE FOR GIRDER CAMBER AND OBTAIN THE PROPER GRADE LINE. THE MINIMUM DEPTH OVER THE GIRDERS SHALL BE 9". IF NECESSARY, THE PLAN GRADE SHALL BE ADJUSTED IN ORDER TO OBTAIN THE MINIMUM SLAB DEPTH. SEE CONCRETE HAUNCH DETAIL.



THE MANUFACTURE OF PRECAST PRESTRESSED CONCRETE BEAMS SHALL CONFORM TO THE MISSOURI HIGHWAY AND TRANSPORTATION COMMISSION SPECIFICATIONS.

THE ULTIMATE COMPRESSIVE STRENGTH OF THE CONCRETE AS DETERMINED BY CYLINDER TESTS AT THE AGE OF 28 DAYS IS 6,000 PSI.

GIRDER LENGTH SHALL REASONABLY CONFORM TO THE LINES AND DIMENSIONS SHOWN ON THE DESIGN PLANS AND BE WITHIN THE TOLERANCES SPECIFIED IN THE LATEST PUBLICATION OF A.A.S.H.T.O., "TENTATIVE STANDARDS FOR PRESTRESSED PILES, SLABS, I-BEAMS, AND BOX BRIDGES AND AN INTERIM MANUAL FOR INSPECTION OF SUCH CONSTRUCTION", EXCEPT AS MODIFIED BY THIS SHEET OR AS MODIFIED BY THE M.H.T.C SPECIFICATIONS.

ALL EXPOSED EDGES OF BEAMS EXCEPT THE TOP AND ENDS SHALL BE BEVELED WITH A 34-INCH TRIANGULAR MOULDING OR ROUNDED TO A 34-INCH RADIUS. THE ANGLE OF INTERSECTION BETWEEN WEB AND FLANGE SHALL BE ROUNDED.

TOPS OF BEAMS ARE TO BE STRUCK OFF LEVEL AND GIVEN A WIRE BRUSH OR STIFF BROOM FINISH. APPLIED IN THE DIRECTION TRANSVERSE TO THE LENGTH OF THE GIRDER. AT APPROXIMATELY THE TIME OF INITIAL SET, THE TOPS OF THE BEAM SHALL BE BRUSHED TRANSVERSELY WITH A COARSE WIRE BRUSH TO REMOVE ALL LAITANCE.

THE PRESTRESSING STEEL SHALL BE 1/2-INCH NOMINAL DIAMETER, GRADE 270 "UNCOATED SEVEN WIRE STRESS-RELEIVED STRAND FOR PRESTRESSED CONCRETE", ASTM DESIGNATION A416, LOW RELAXATION STRANDS. MINIMUM ULTIMATE STRENGTH OF STRANDS SHALL BE 41,300 POUNDS.

ULTIMATE COMPRESSIVE CYLINDER STRENGTH OF THE CONCRETE SHALL BE 5,000 PSI MINIMUM BEFORE DETENSIONING OF PRESTRESSING STRANDS.

AN INITIAL TENSILE FORCE OF 1,000 TO 3,000 POUNDS SHALL BE APPLIED TO EACH STRAND TO TAKE UP ANY SLACK IN THE CABLES. A TENSILE FORCE OF 30,983 POUNDS SHALL BE APPLIED TO EACH STRAND. STRANDS WHICH ARE TO BE DEFLECTED SHALL BE STRESSED TO A MAGNITUDE SUCH THAT AFTER DEFLECTION, THEY ARE TENSIONED TO 30,983 POUNDS.

ALL MILD STEEL REINFORCEMENT SHALL BE ASTM A615, GRADE 60. ALL CHAIRS AND SPACERS IN PRECAST, PRESTRESSED GIRDERS SHALL BE GALVANIZED. COIL TIES SHALL BE HELD IN PLACE IN THE FORMS BY SLOTTED WIRE-SETTING-STUDS PROJECTING THROUGH THE FORMS. STUDS ARE TO BE LEFT IN PLACE OR REPLACED WITH TEMPORARY PLUGS UNTIL GIRDERS ARE ERECTED AND THEN REPLACED BY COIL TIE RODS.

NOTE: TOLERANCE FOR CAMBER IS 1/8" PER 10 FEET OF SPAN (1" MAXIMUM) VARIATION IN CAMBER BETWEEN ADJACENT GIRDERS; AND 1/8" PER TEN FEET OF SPAN DEVIATION FROM THE SPECIFIED CAMBER, BUT NOT GREATER THAN 1/2" DEVIATION.

TOLERANCE FOR SWEEP IS 1/8" PER TEN FEET OF SPAN, BUT NOT GREATER THAN 1".

PLAN SPA. (±1")

AREA-

- BRG. P. OR BRG. / DEVIATION FROM F SLIRFACE

(#¥1)

PRESTRESSED CONCRETE GIRDER FABRICATION TOLERANCES

TRAPPED AIR HOLES AND SURFACE VOIDS ON THE EXTERIOR INCLINED SURFACE OF THE BOTTOM FLANGE OF ALL EXTERIOR BEAMS SHALL BE FILLED WITH CONCRETE GROUT SO AS TO PRODUCE A NON-POROUS SURFACE.

DETENSIONING OF STRANDS SHALL BE PERFORMED IN A SEQUENCE TO MINIMIZE LATERAL ECCENTRICITY. METHOD AND SEQUENCE OF RELEASE SHALL BE SHOWN IN SHOP DETAILS.

EXTREME CARE SHALL BE EXCERCISED IN LIFTING, HANDLING, STORAGE, AND TRANSPORTATION OF THE BEAM TO PREVENT DAMAGE. THEY SHALL BE LIFTED BY MEANS OF THE DEVICE PROVEDED IN AN UPRIGHT POSITION AT ALL TIMES AND SHALL BE SUPPORTED ON BEARING POINTS POSITIONED BELOW THE DESIGNATED LIFTING POINTS OR BELOW THE DESIGNATED BEARING POINTS.

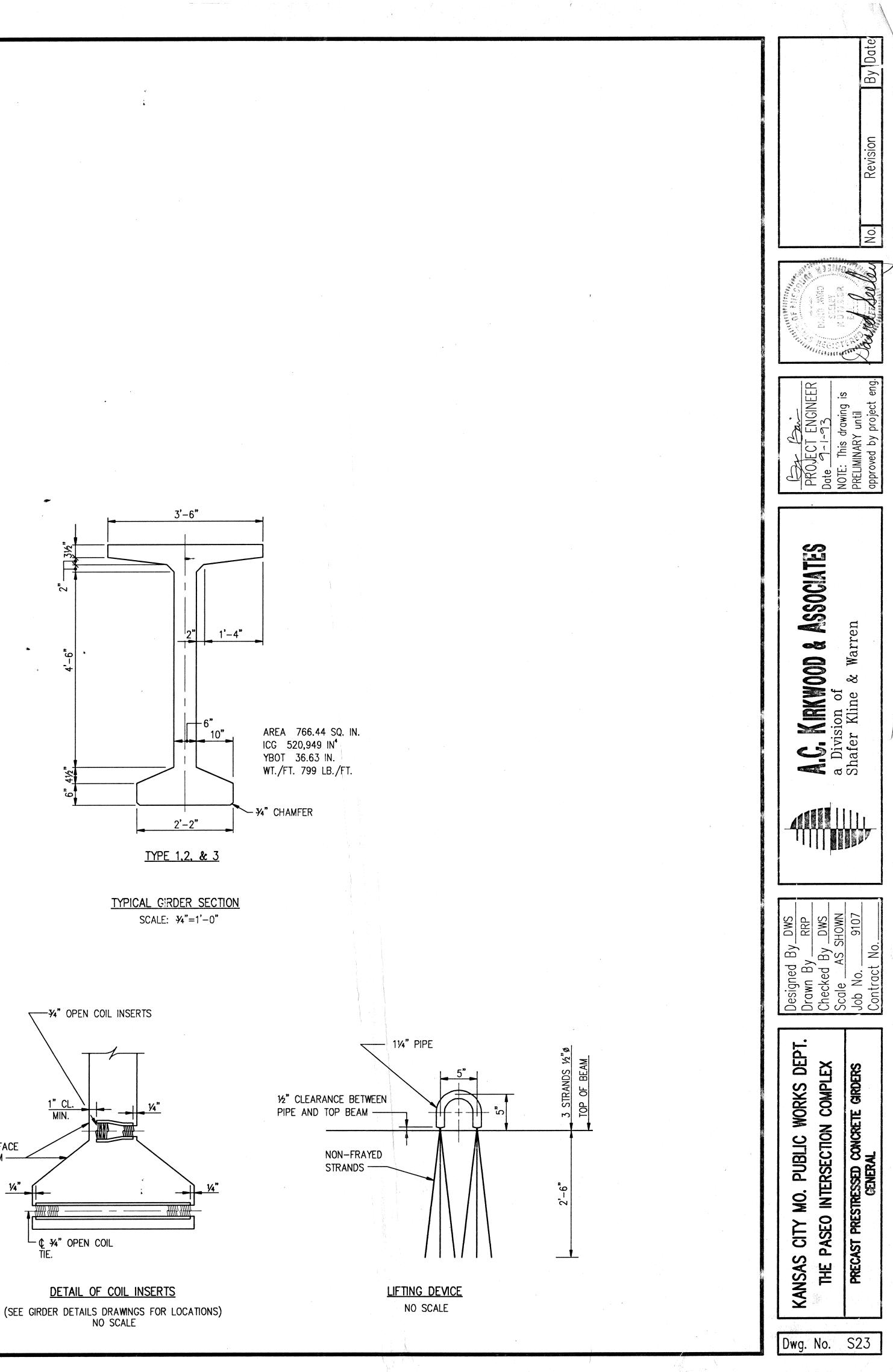
DURING TRANSPORTATION ONLY, THE BEAMS MAY BE SUPPORTED BY BEARING POINTS BELOW THE GIRDERS AT A MAXIMUM OF 4'-O" FROM THE BEAM END. THE GIRDERS SHALL HAVE A MINIMUM AGE OF 35 DAYS BEFORE PLACING OF THE BRIDGE SLAB. THE DIAPHRAGMS SHALL BE POURED AS NOTED ON THE

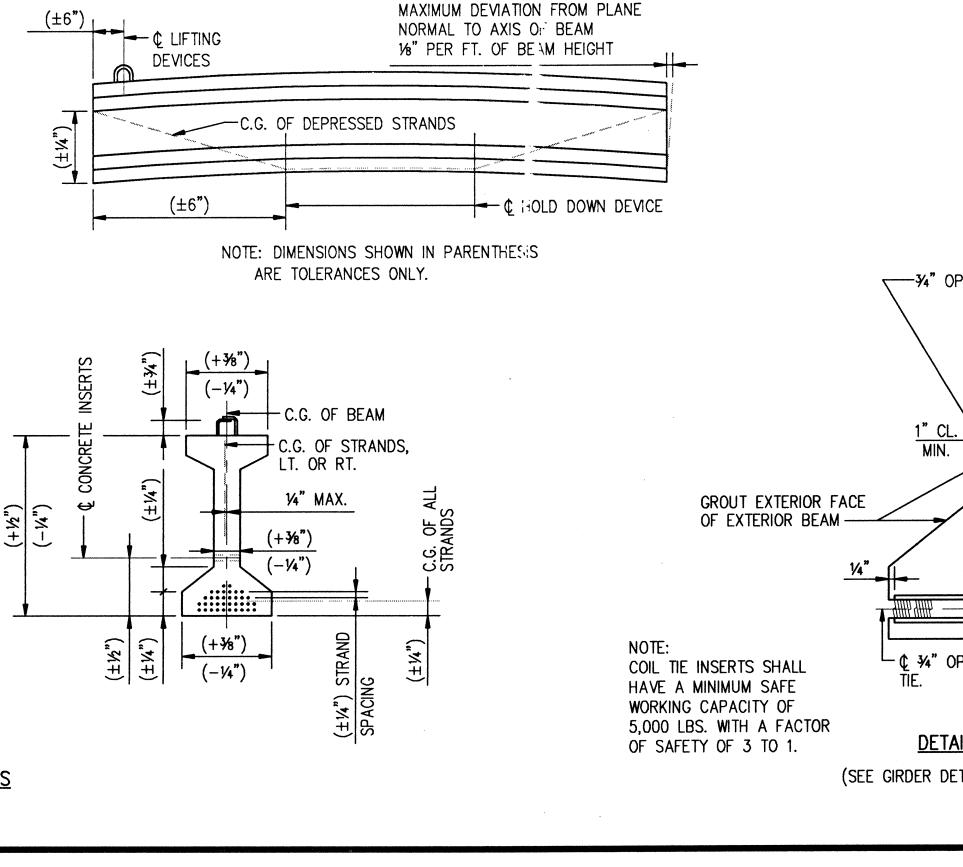
DESIGN PLANS. ELASTOMERIC BEARING PADS SHALL CONFORM TO M.H.T.C. SPECIFICATIONS. THE PADS WILL NOT BE PAID FOR DIRECTLY, BUT SHALL BE CONSIDERED SIBSIDIARY TO THE ITEM "PRESTRESSED CONCRETE GIRDERS". COIL TIES AND BOLTS SHALL HAVE AN ULTIMATE STRENGTH OF 200 PERCENT IN EXCESS OF THE MANUFACTURER'S SAFE LOAD AND SHALL BE APPROVED BY THE ENGINEER. COIL TIES AND BOLTS WILL NOT BE PAID FOR DIRECTLY, BUT

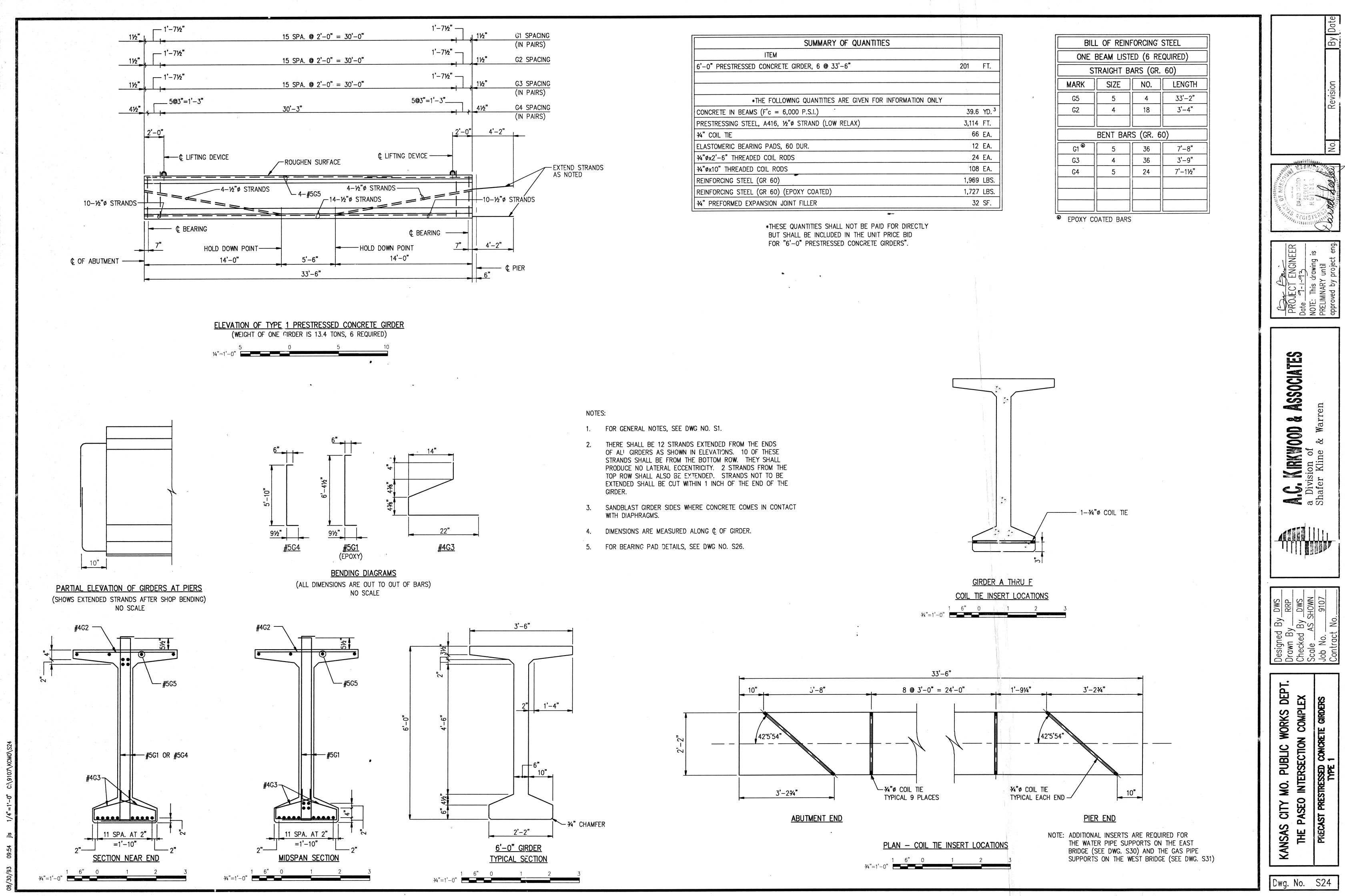
SHALL BE CONSIDERED SUBSIDIARY TO ITEM "PRESTRESSED CONCRETE GIRDERS" THE FINISHED DECK SLAB SHALL BE CONSTRUCTED TO PLAN GRADE BY VARYING THE DEPTH OF THE CONCRETE FILLETS OVER THE BEAMS TO PROVIDE FOR PRESTRESS CAMBER, CONCRETE DEAD LOAD DEFLECTION, AND VERTICAL CURVE. AFTER THE GIRDERS HAVE BEEN ERECTED AND PRIOR TO PLACING ANY FORMWORK. THE ACTUAL CAMBER IN EACH BEAM SHAL BE MEASURED IN THE FIELD. ANY VARIATION BETWEEN THE ACTUAL CAMBER AND THE ERECTION CAMBER SHOWN ON THE DESIGN PLANS SHALL BE CORRECTED BY VARYING THE FILLET DEPTH.

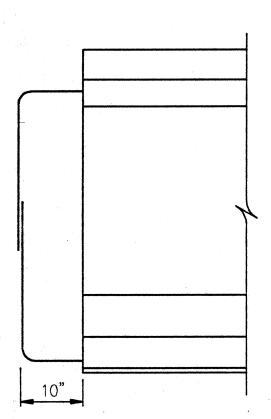
ALL LIFTING DEVICES SHALL BE REMOVED AFTER ERECTION AND BEFORE SLAB PLACEMENT.

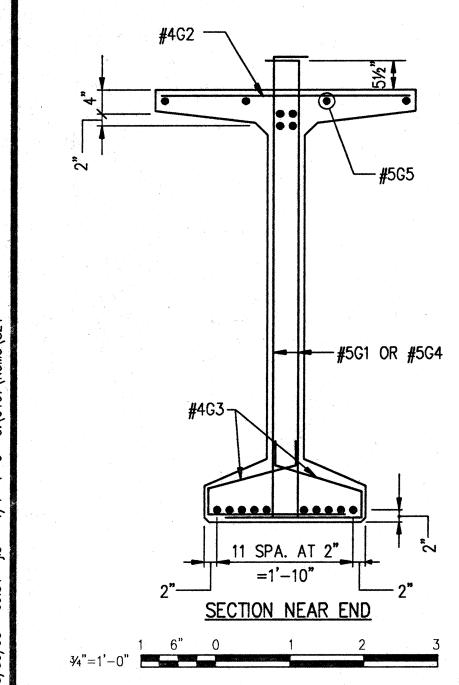
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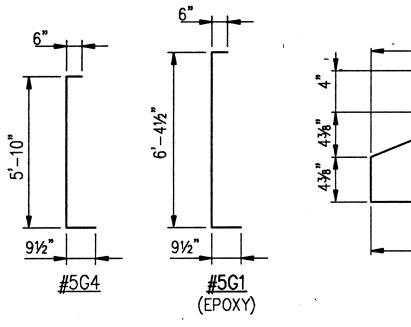


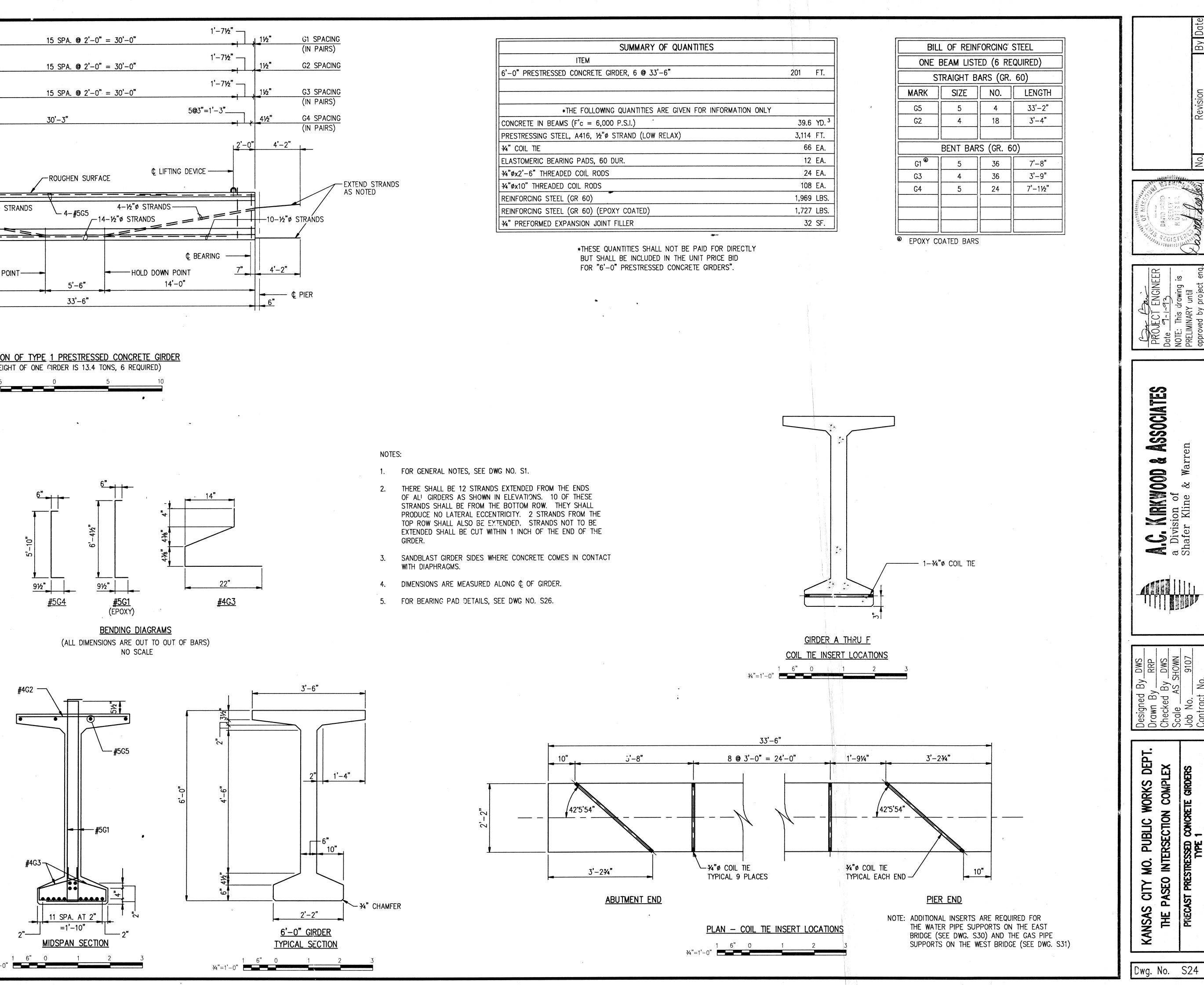






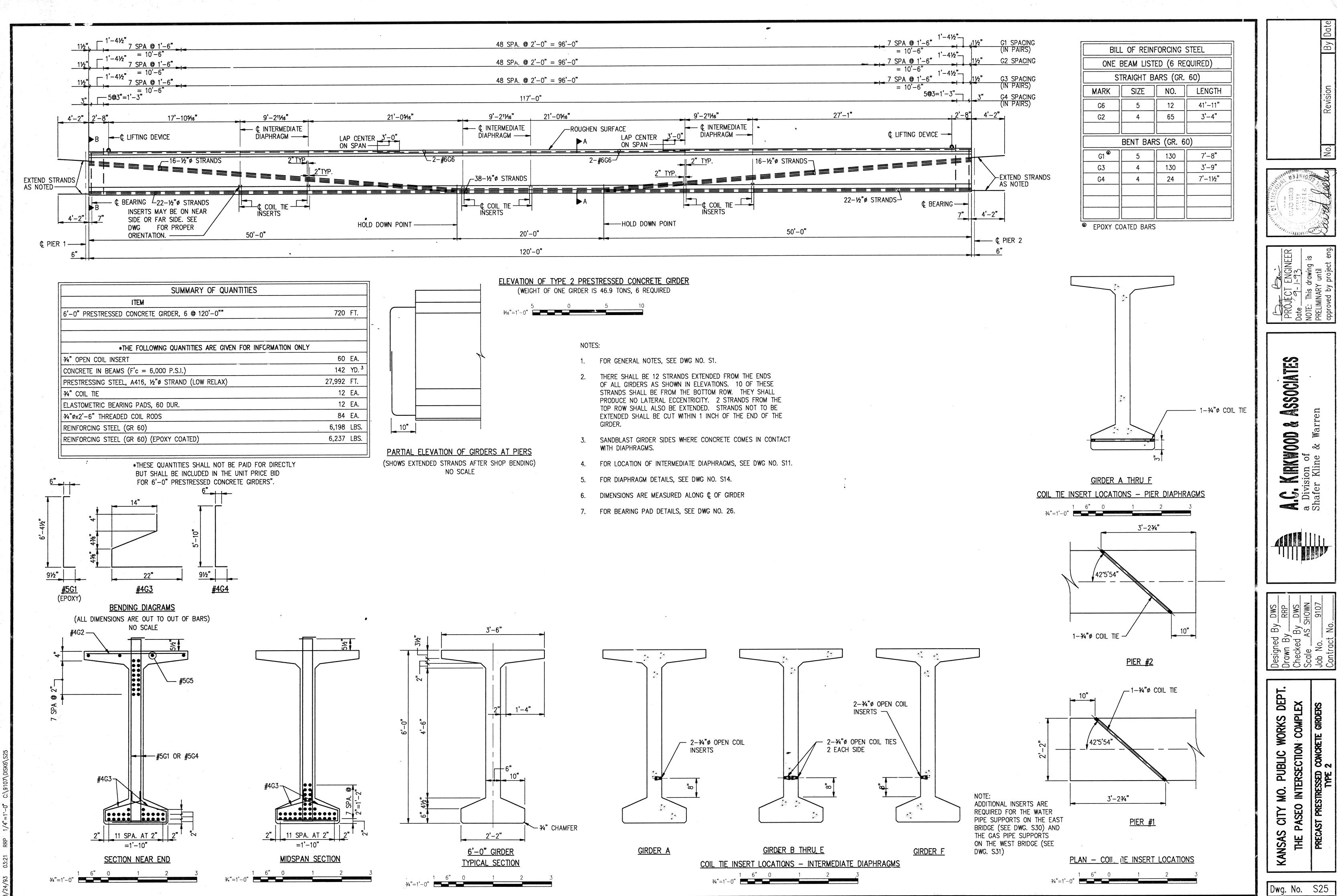


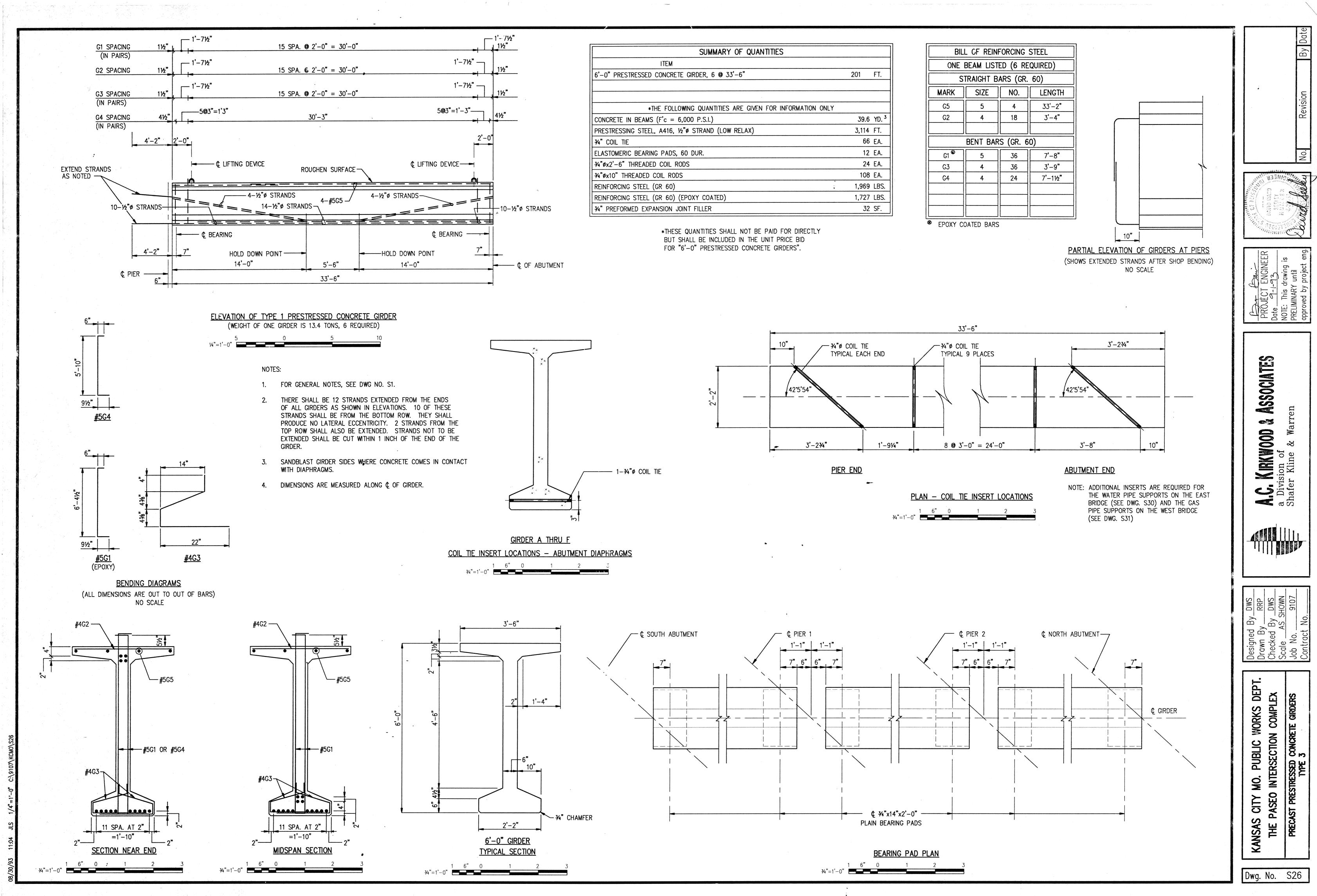


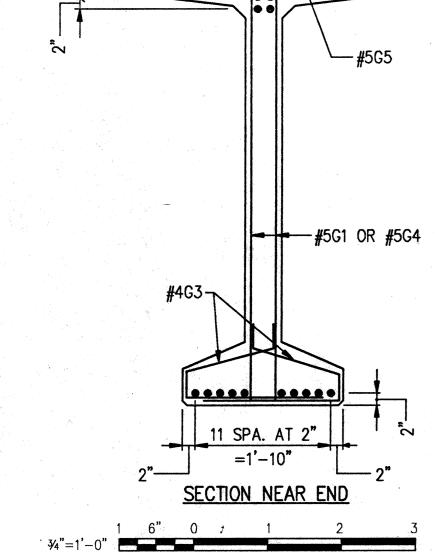


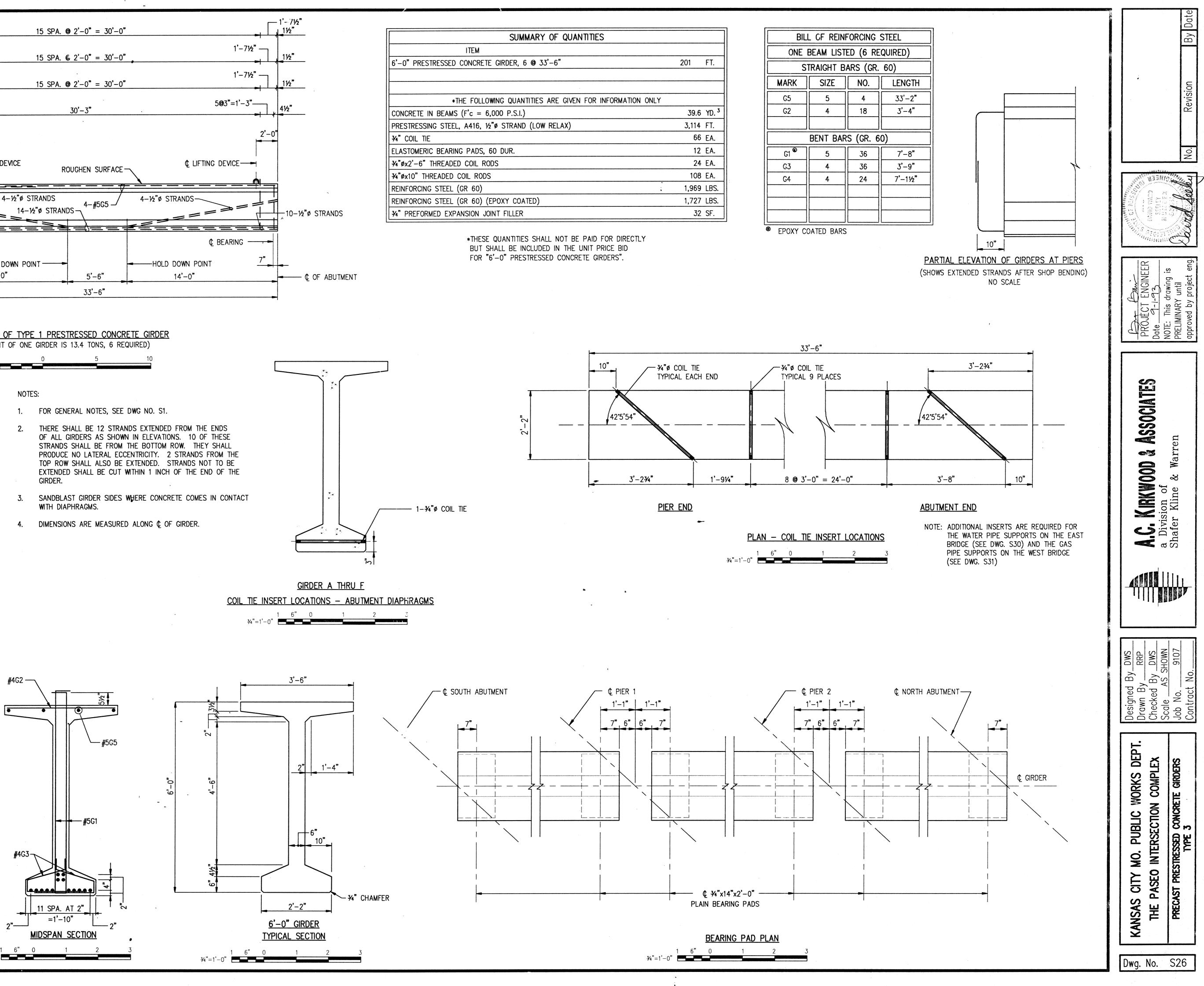


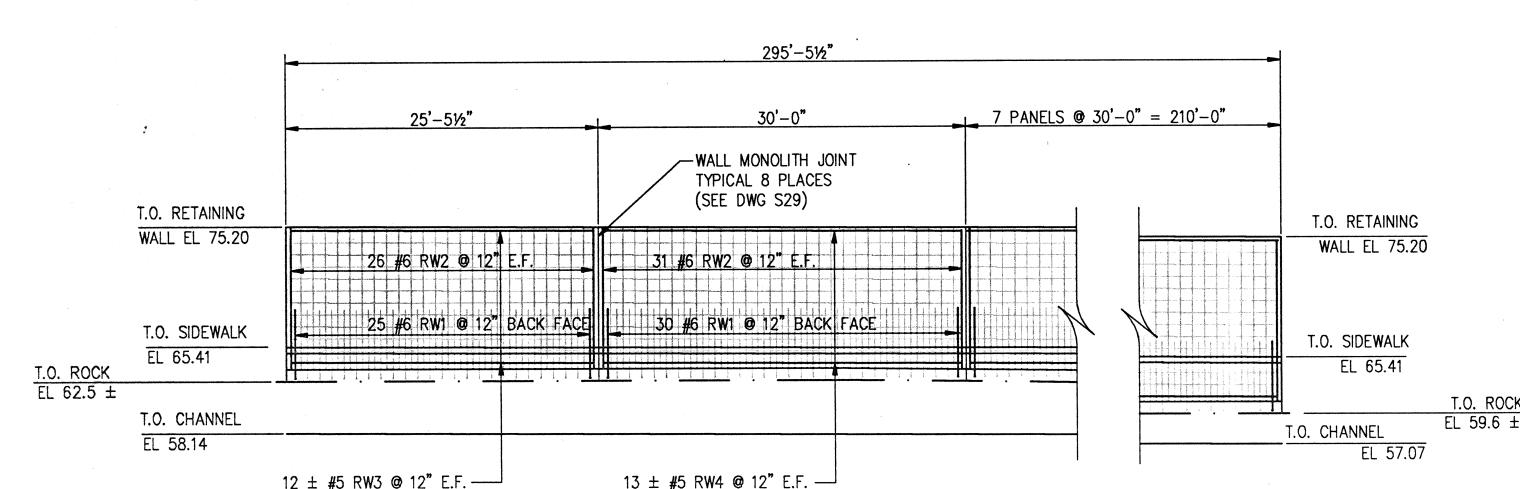


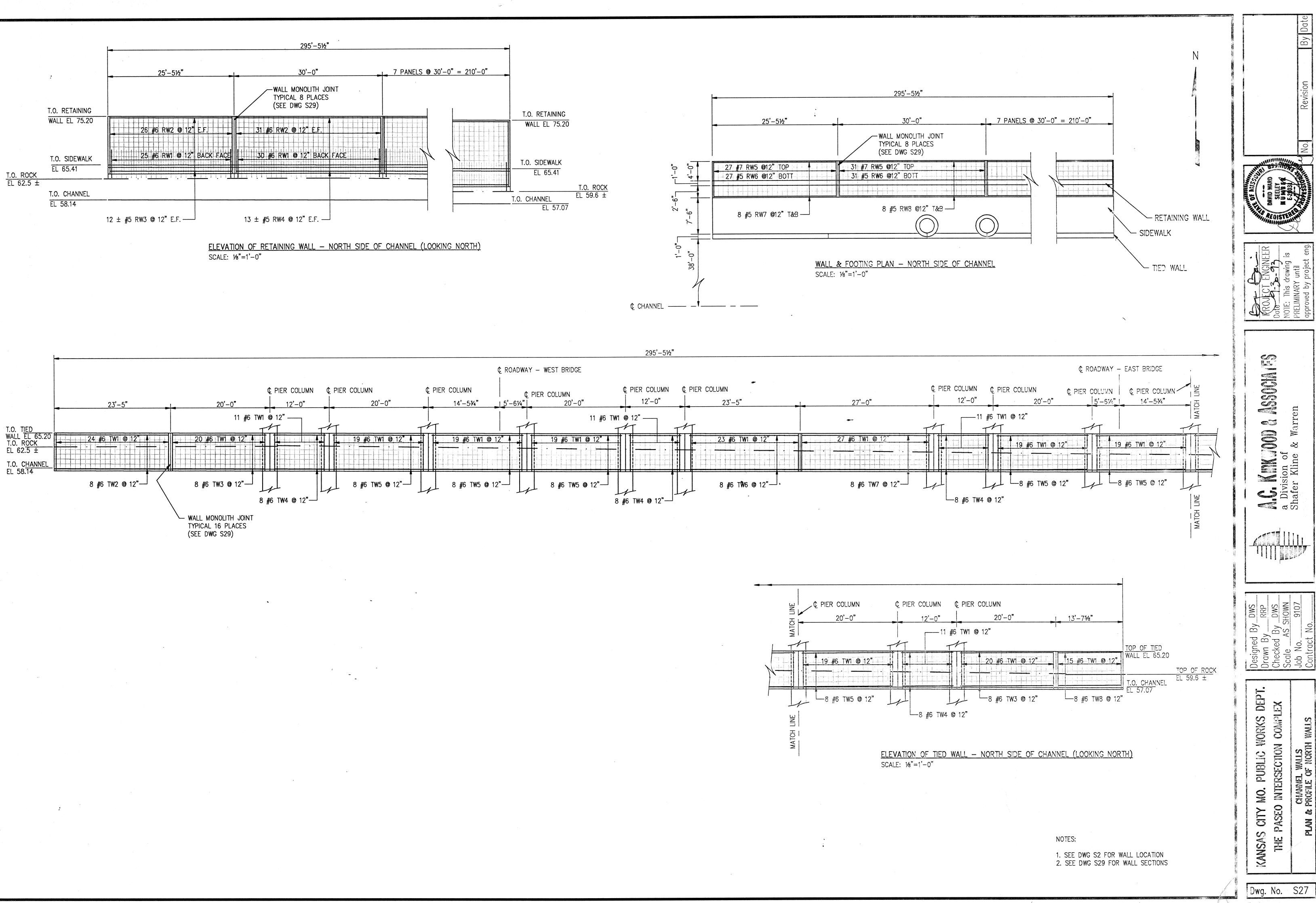




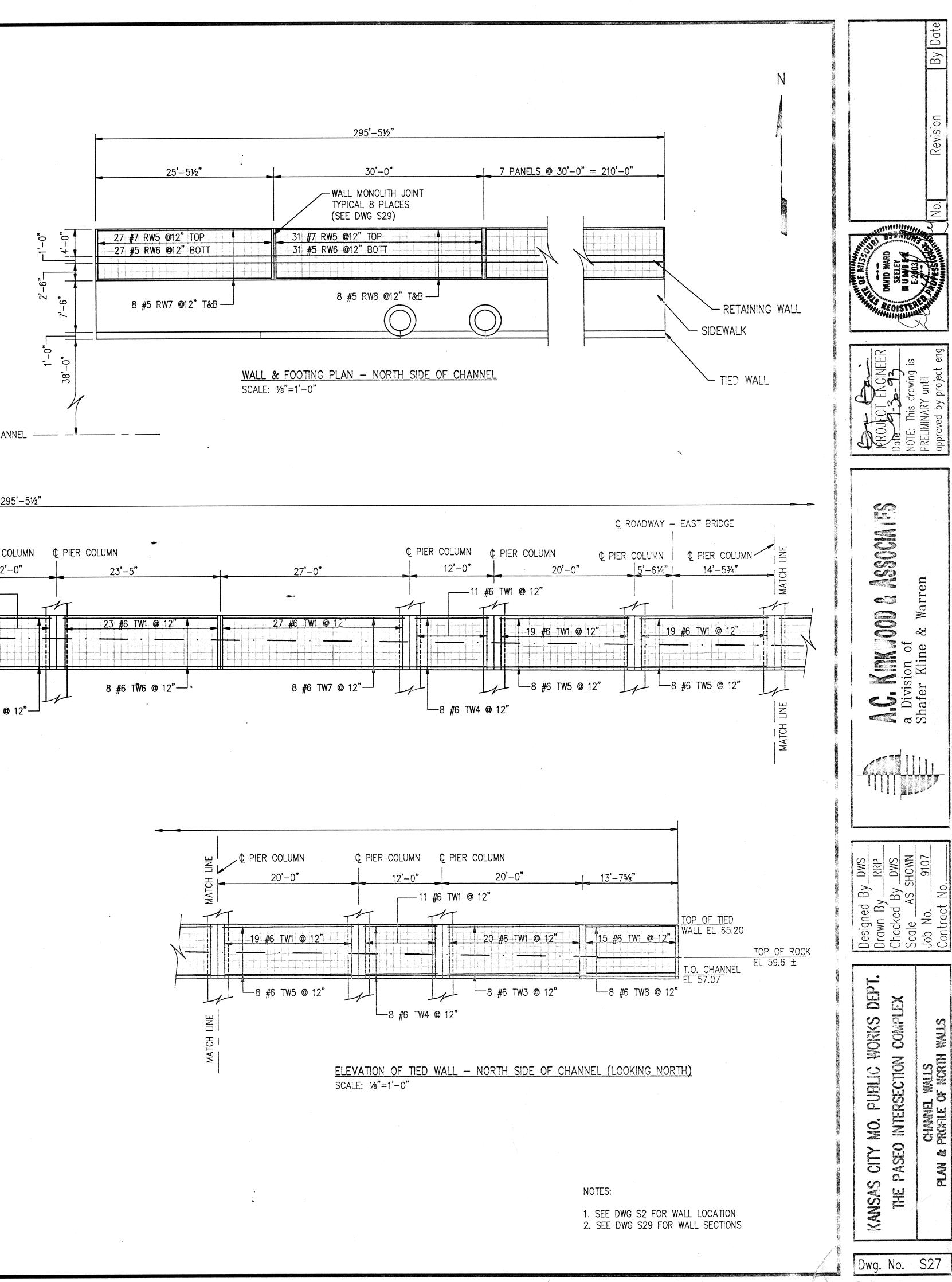


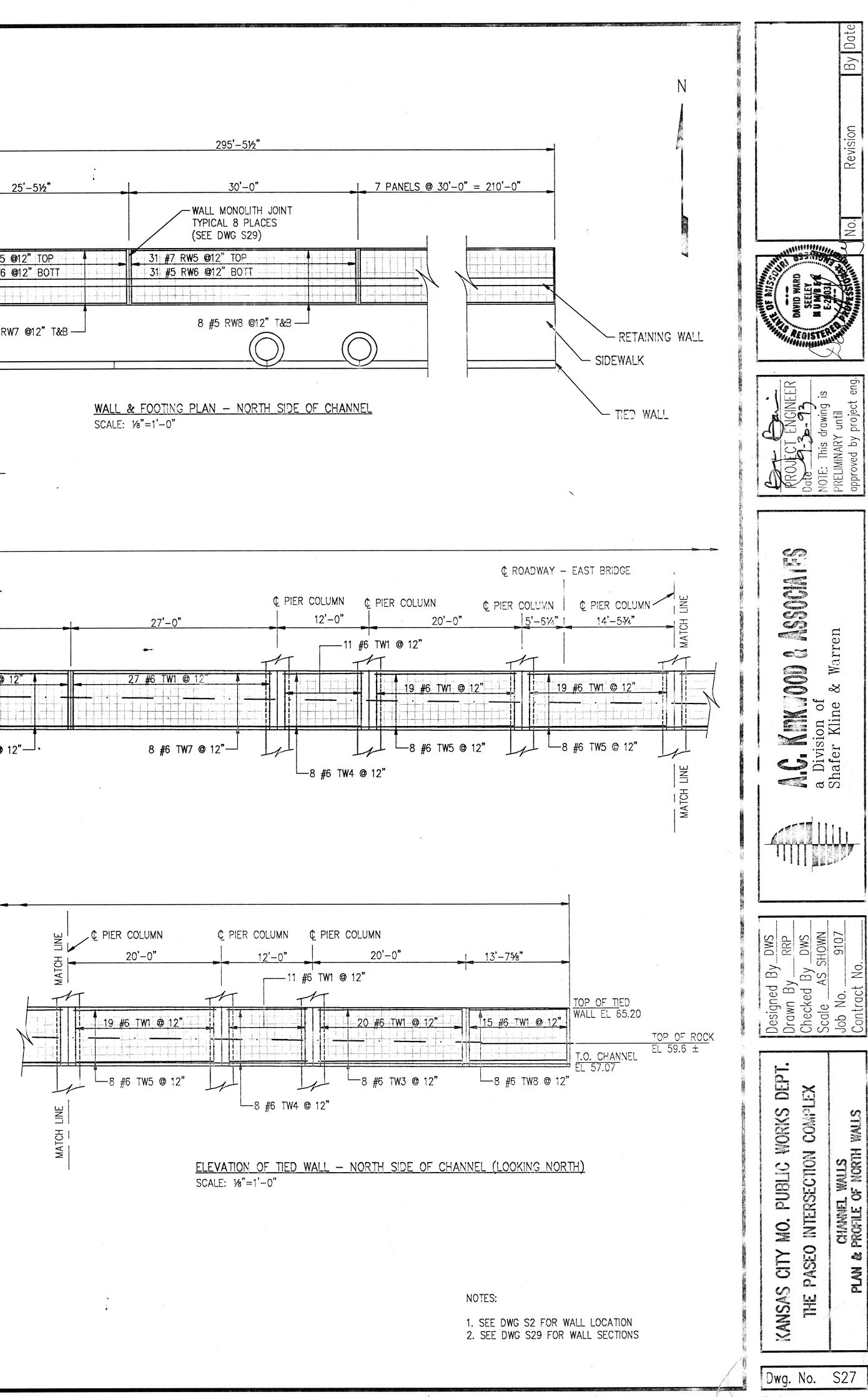


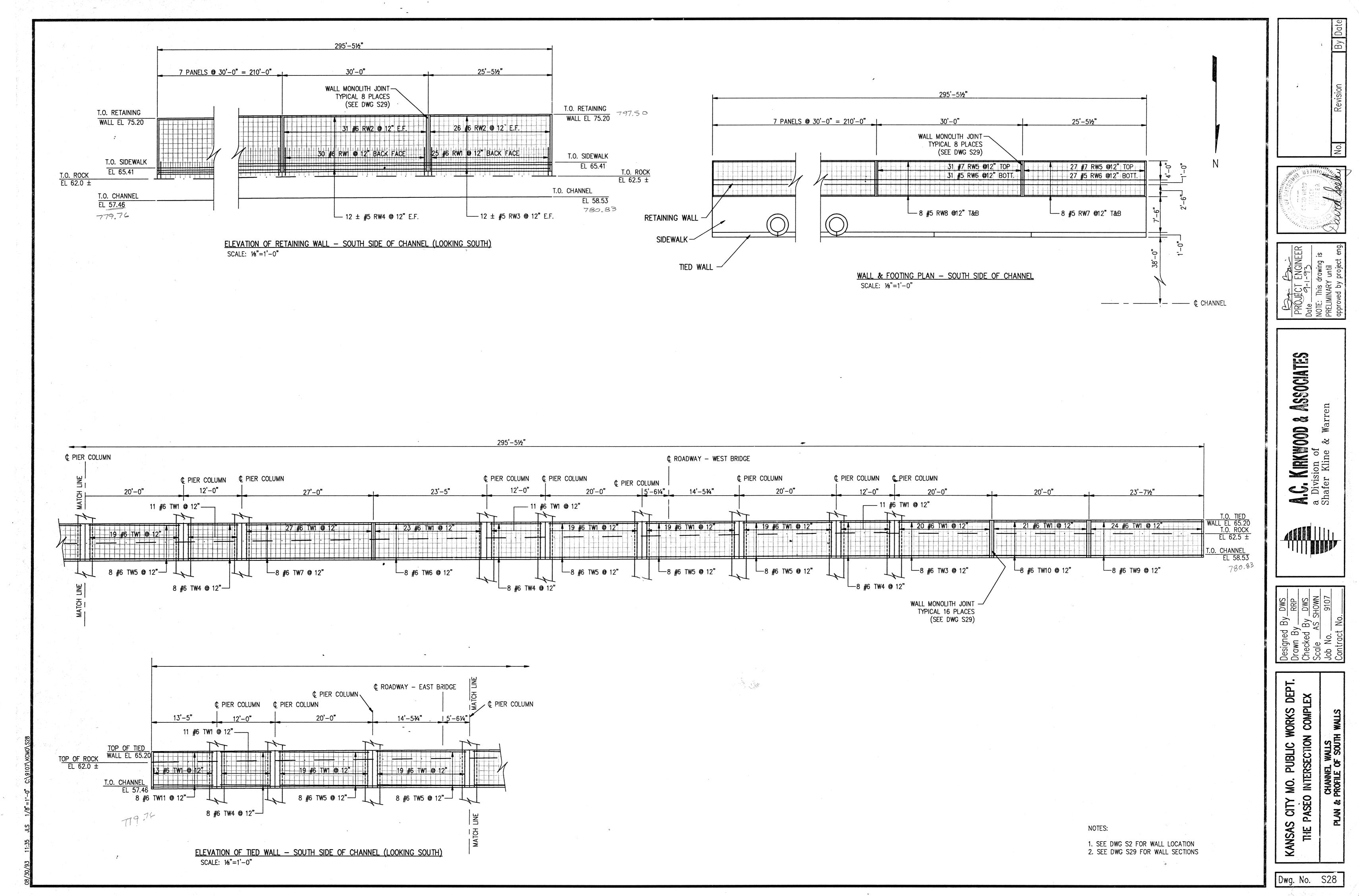


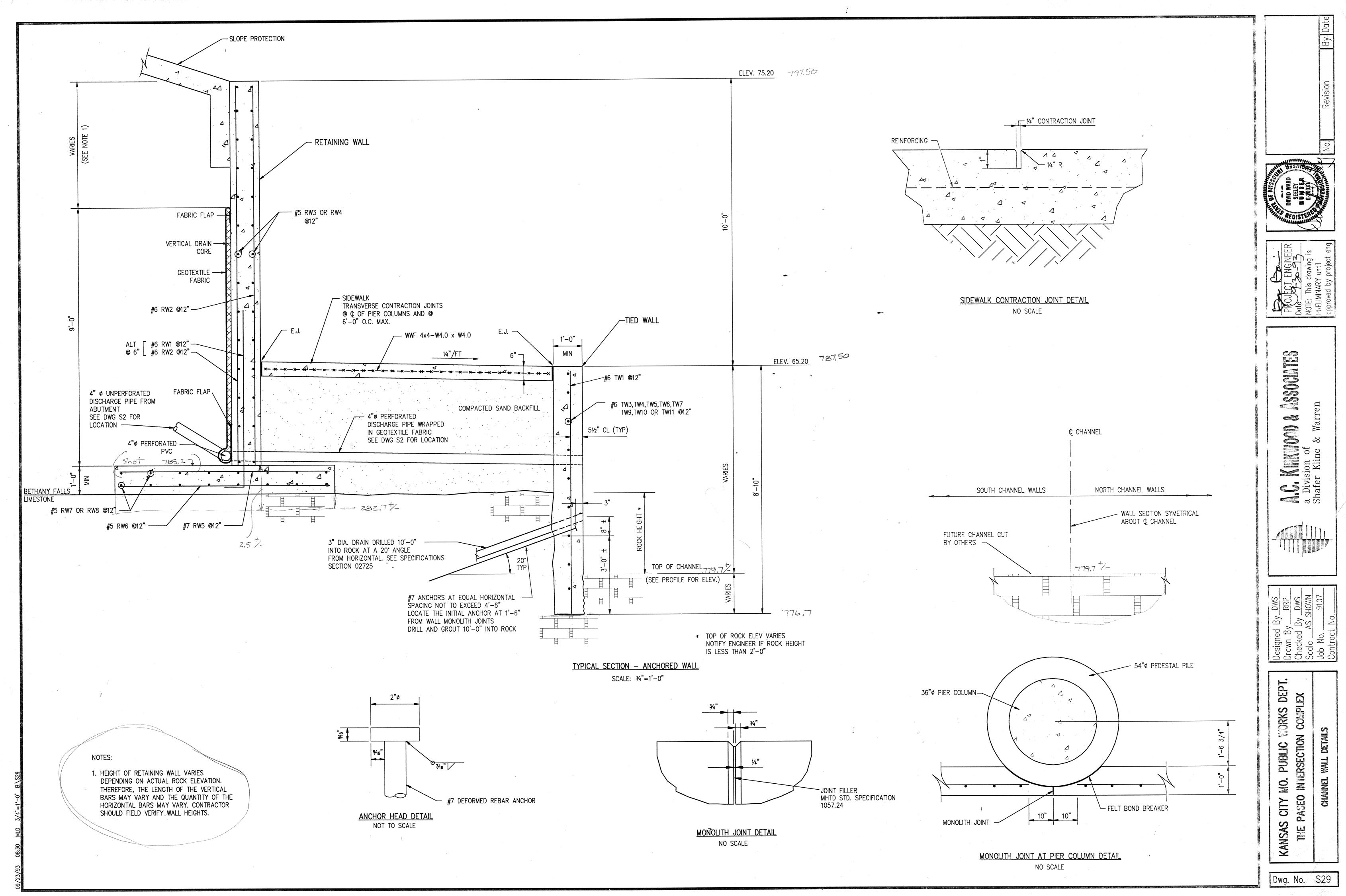


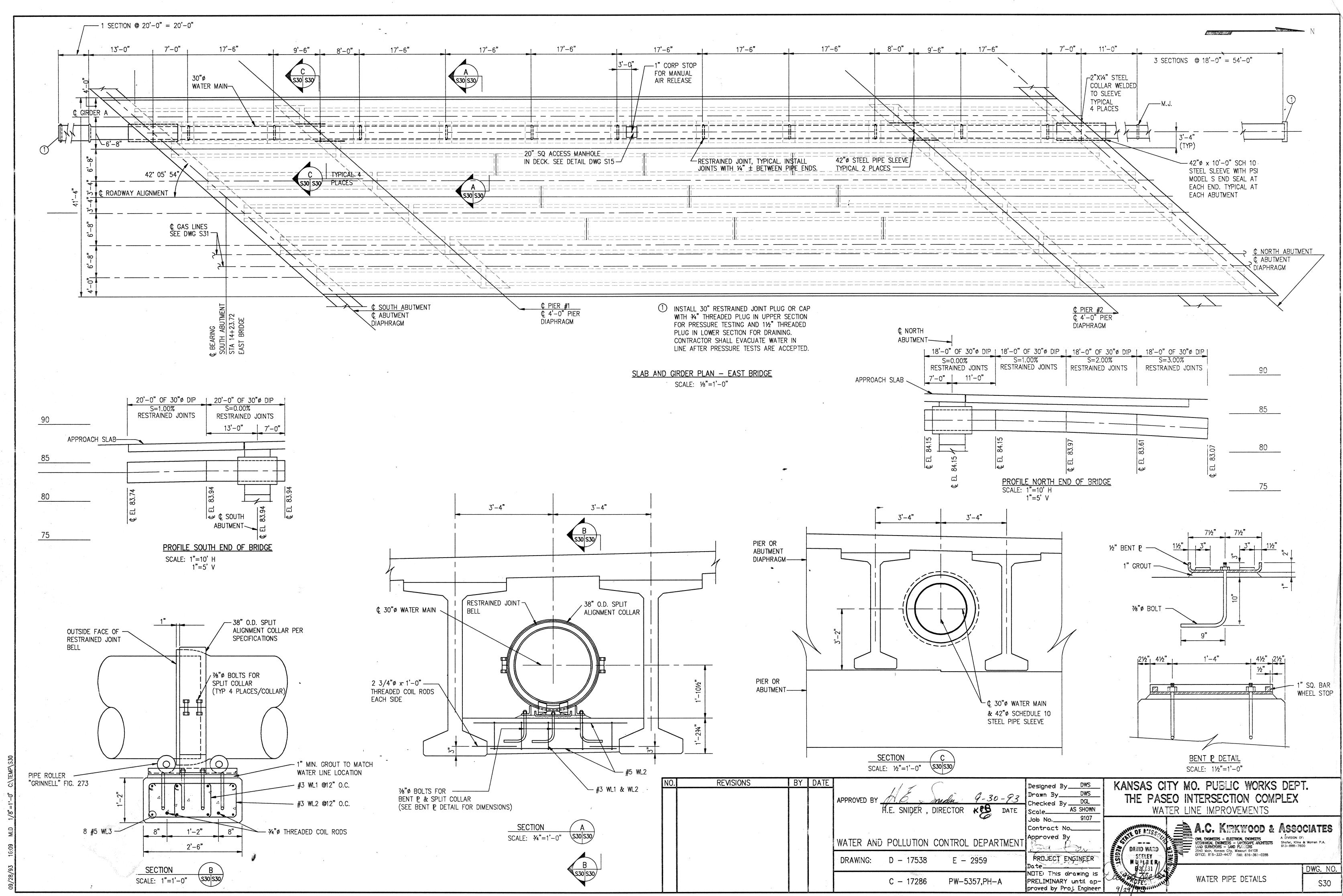
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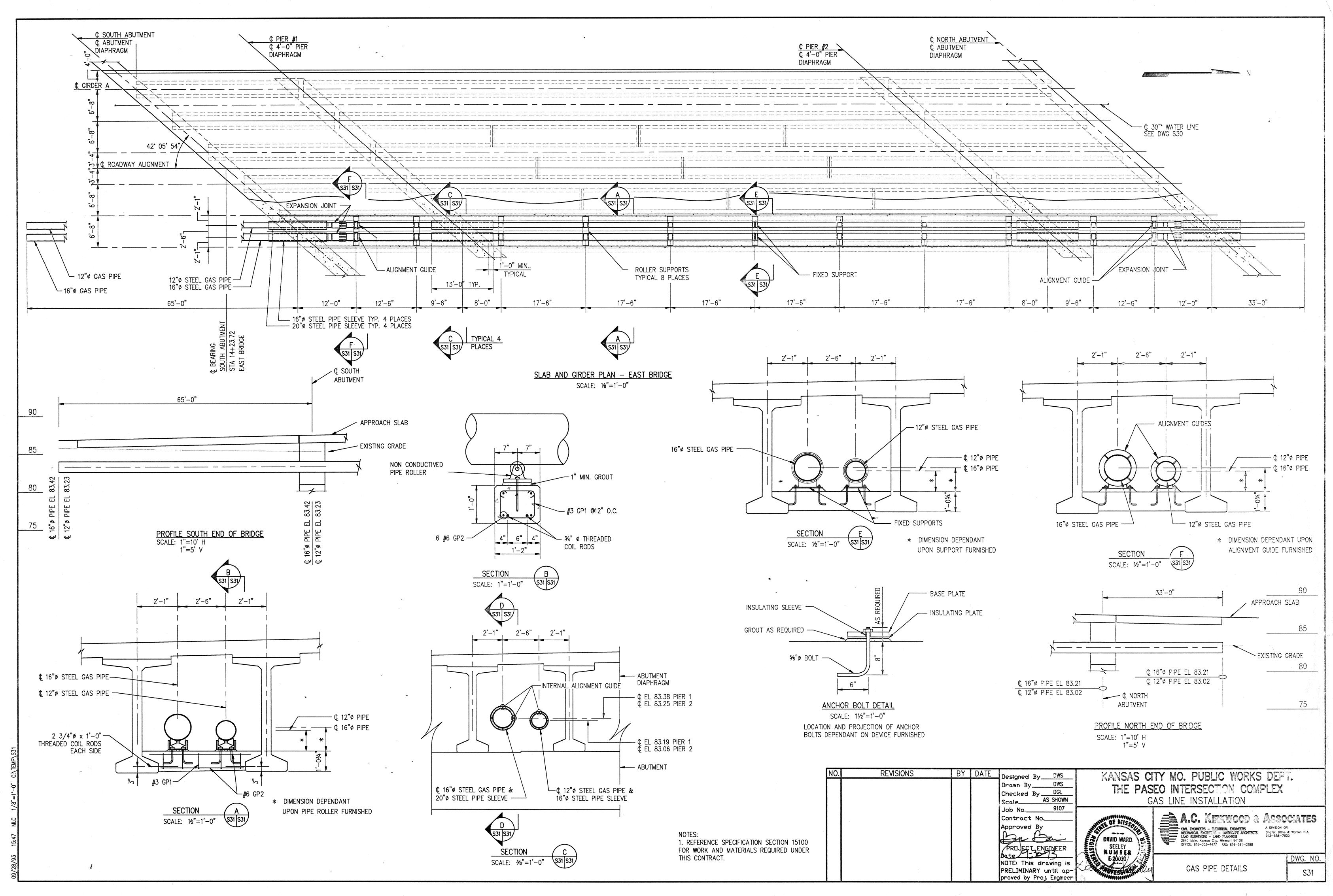




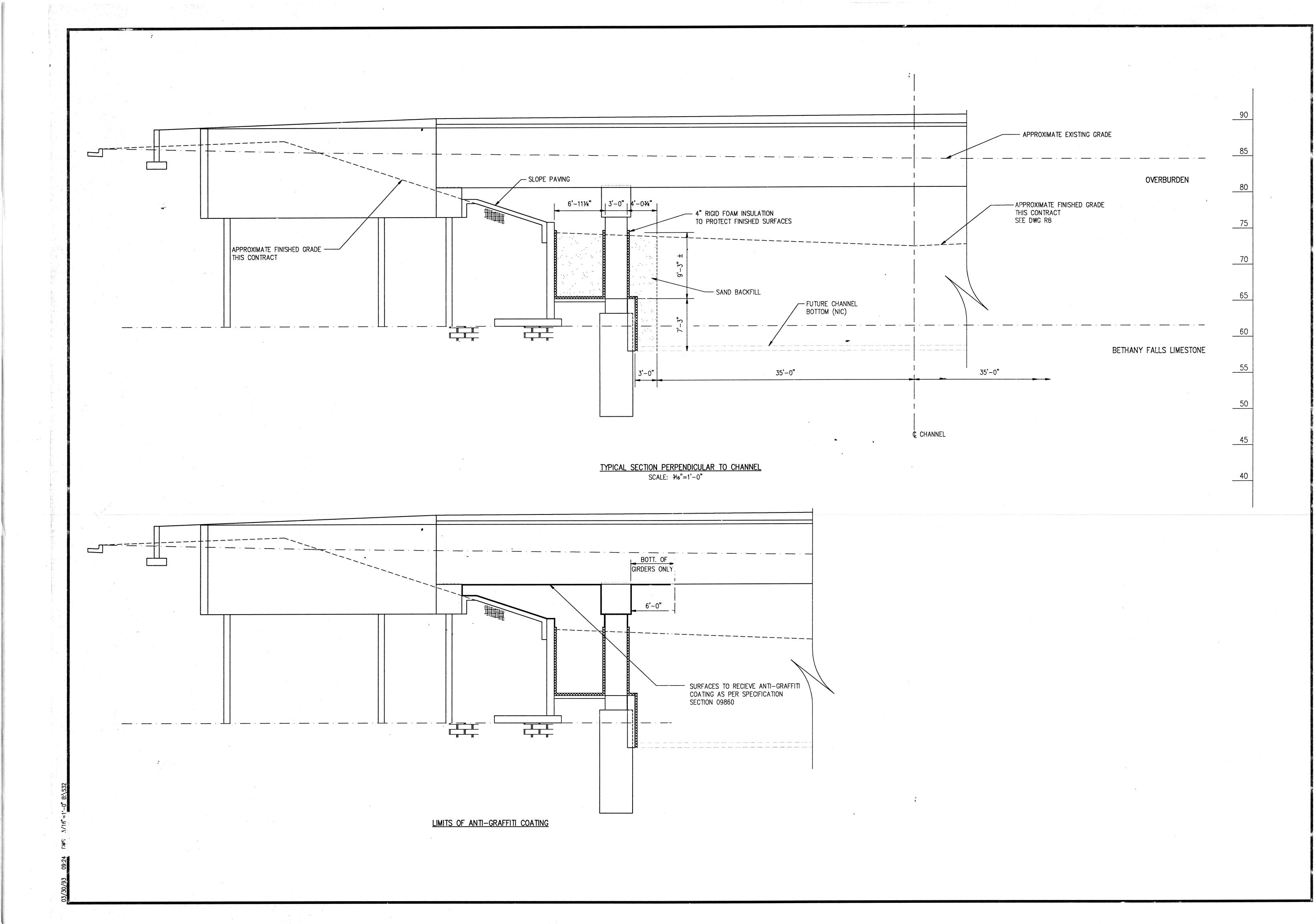


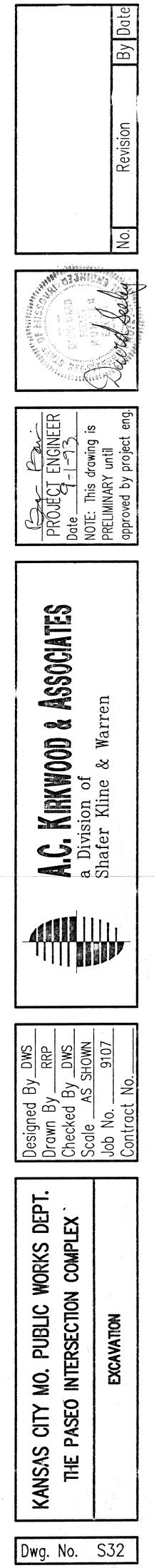


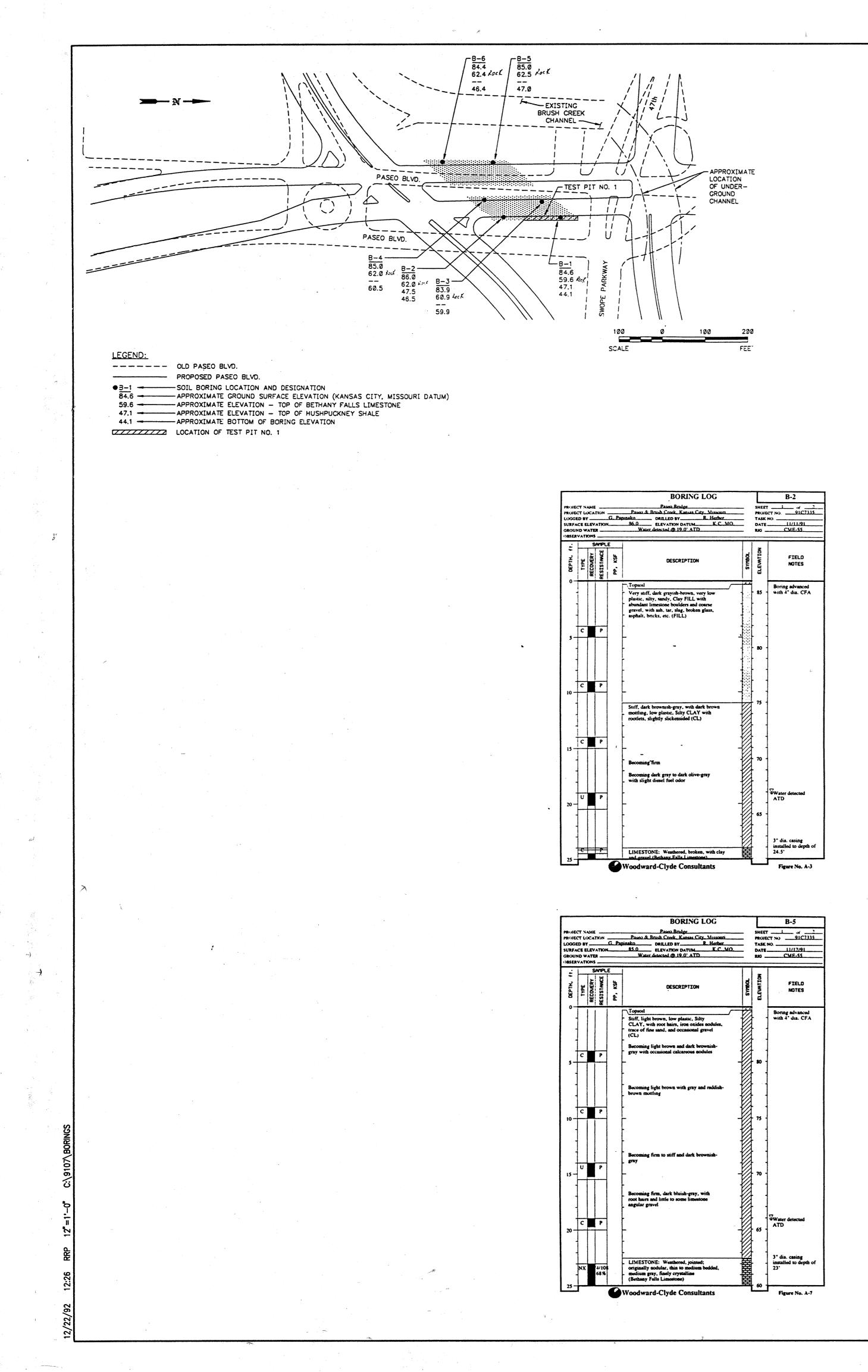
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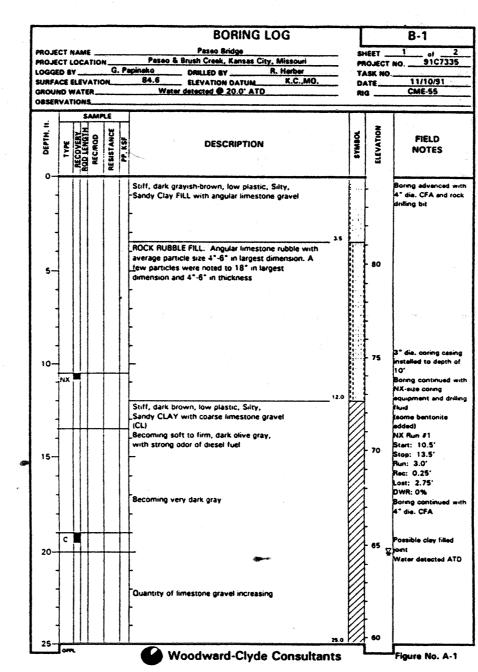


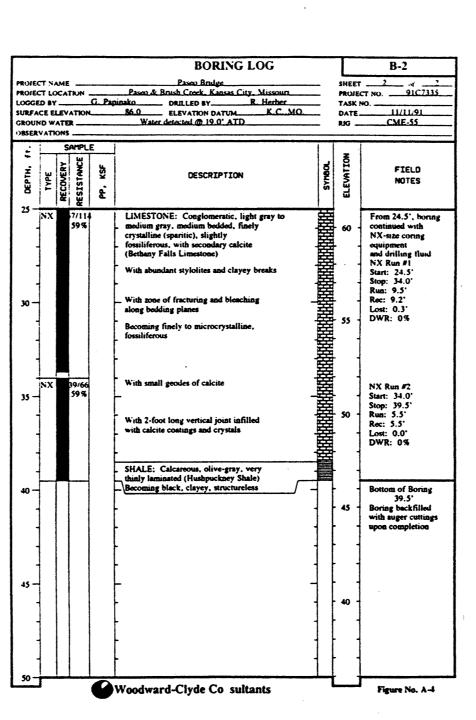
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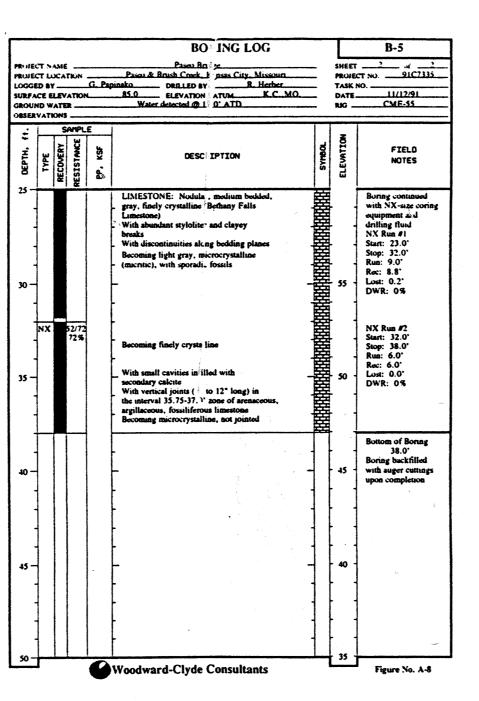


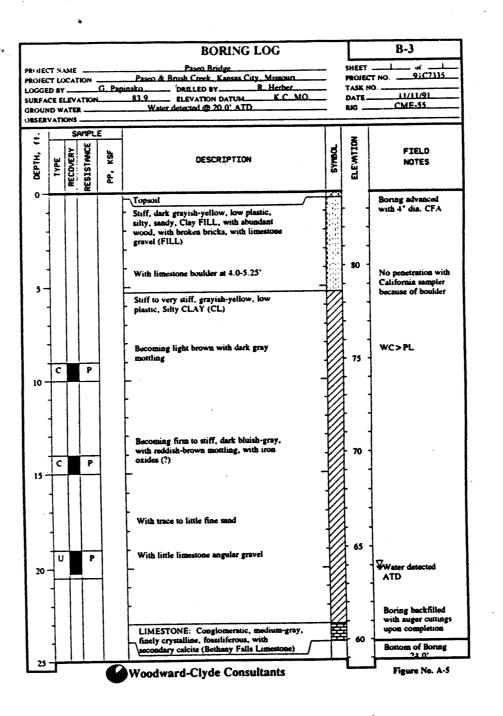


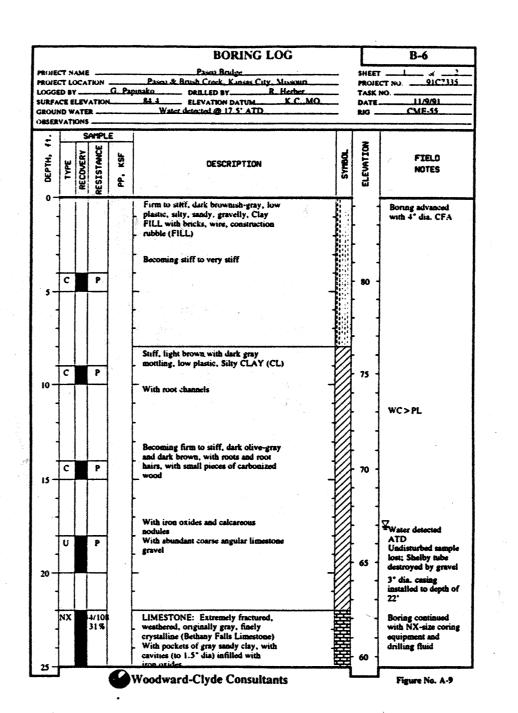


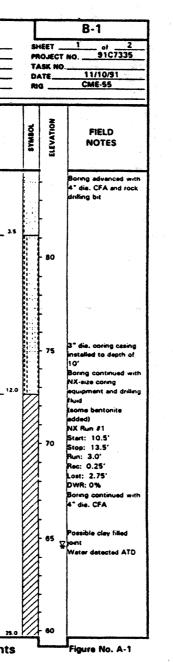


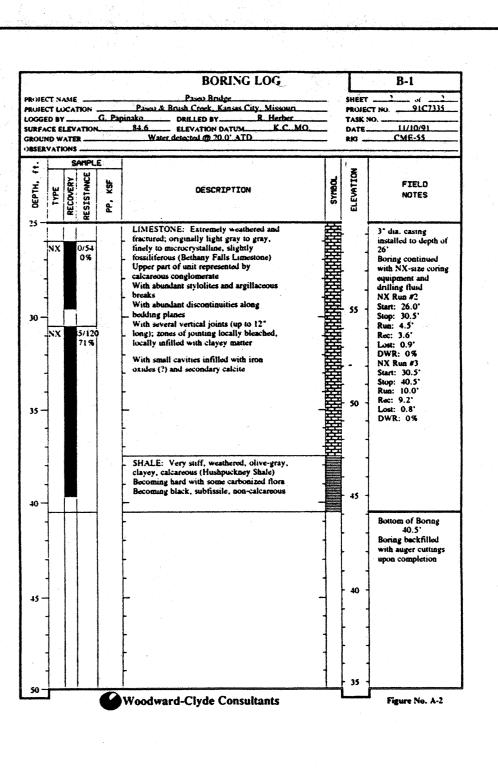
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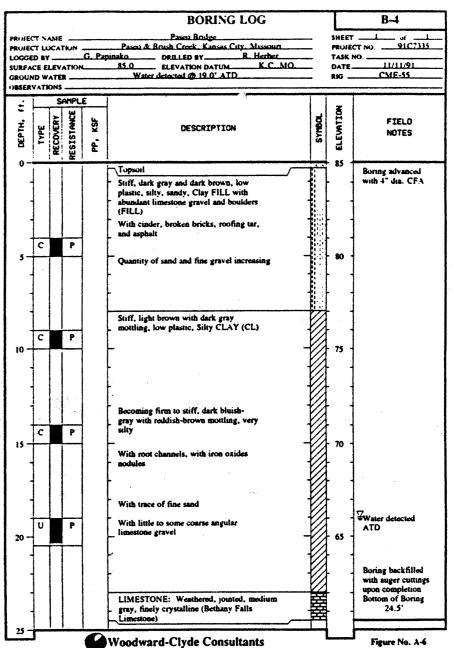


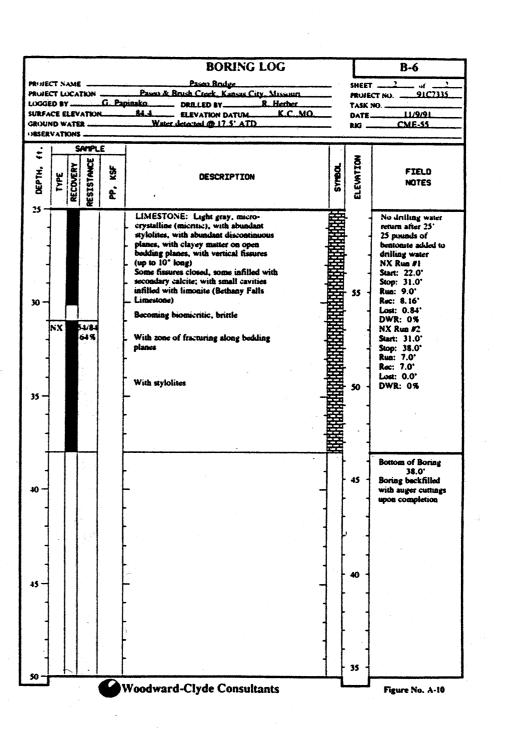


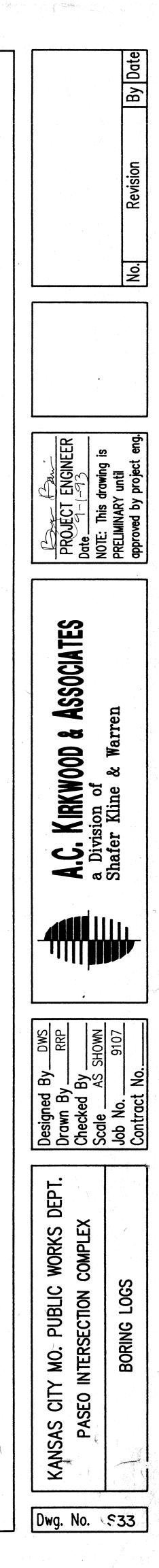




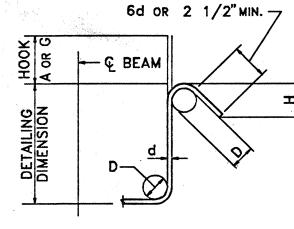


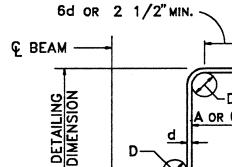






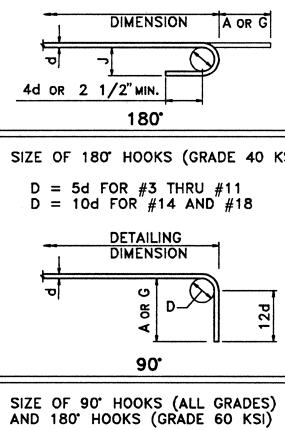
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W3		20	4	<u> </u>								4' 6		ļ		20		0, 9,					+
and the second se	WEST WINGWALL	20	2									2' 6		§		20	1	7' 0'	7		•		1
W5	WEST WINGWALL	19	2			10"				-		3' 7	120	14 5 W10		20		3' 6'		5			1
	WEST WINGWALL	20	5		· · · · · · · · · · · · · · · · · · ·						·	5' 4	1/0	20 5 W12	EAST WINGWALL	20		9' 11'				· · · · ·	
	WEST WINGWALL	20										<u>10' 9</u>		12 5 W11 26 5 W13	WEST WINGWALL	20		2' 4' 1' 10'					+
	WEST WINGWALL	20	3									<u>32'6</u> 17'0	["] 407 " 177	26 5 W13	WEST WINGWALL			1 10					+
	and the state of the	20	3	<u>, </u>								3' 6											+
	unit eritari de la constancia de la constancia Constancia de la constancia																						
	EAST WINGWALL	20	1		2							12' 4	" 257										_
W13	EAST WINGWALL		_ 1 [.]	<u>1' 10"</u>							I	11' 10	" 321								I ABUTMI		
	SUBSTR			·	• 1		TH ABUIME	NT WEST BRI		-	Т	3' 0	" 140	70 4 41	SUBSTRU			3' 0'	7				A31 T
A1 A2	ABUT. BEAM	20		<u>0"</u> 4 1⁄2"		0" 4'	8" ·					<u> </u>	" 1039	70 4 A1 86 4 A2	ABUT. BEAM	34		<u> </u>)" 4'	8"		+
A3	ABUT. BEAM	20	3				<u> </u>					31' 9		36 6 A3	ABUT. BEAM	20	-	31' 8'					+
A4	ABUT. BEAM	20	4	6' 9"	9							46' 9	" 843	12 6 A4	ABUT. BEAM	20	4	ł6' 9'					
A5	ABUT. BEAM	10			4'	6" 3'	7"					12' 7	25	3 4 A5	ABUT. BEAM	10				s" 3'	7"		4
A6	ABUT. BEAM	14	<u> 1'</u>	0"	['] 5'	6" 0" 7'						$\frac{6'}{0'}$ $\frac{6'}{7}$	" <u>88</u>	9 6 A6	ABUT. BEAM	14	1	· 0 [·]		5")" 3'	-7 ⁿ		_
A7 A8	ABUT. BEAM ABUT. BEAM	10				0" 3' 6" 3'	/ 7"		-			<u>9 7</u> 12' 7	р " 8	1 4 A7 1 4 A8	ABUT. BEAM ABUT. BEAM	10				5 ["] 3'	/ 7"		+
A0 A9	ABUT. BEAM	14			9'	6" 1'	0"			41⁄4"	111/4"		" 79	5 6 A9		10				5" 1'	0"		+
A10	ABUT. BEAM	20	4	' 10"								4' 10	294	91 4 A10	ABUT. BEA 1	20	2	i 10"	,				
A11	ABUT. BEAM	20	18									18 ' 5		4 4 A11	ABUT. BEFM	20		8' 5'					
A12	ABUT. BEAM	20	1(10' 6'		8 4 A12	ABUT. BE M	20		$\frac{0}{2}$ 6"					
A13	ABUT. BEAM	20	9									9'8 9'5		16 4 A13 4 4 A14	ABUT. BE \M ABUT. BE \M	20	<u> </u>						
A14 A15	ABUT. BEAM ABUT. BEAM	20 20	1'									<u> </u>		12 4 A14	ABUT. BEAM	20		' <u> </u>					+
W1	ABUT. BEAM	20	3									3' 6'		32 5 W1		20		5' 6"					<u>†</u>
W2	WEST WINGWALL	20	1!	5' 0"	,						•	15 ' 0'	" 156		EAST WING VALL	20	1	5 ' 0")				
W3	WEST WINGWALL	20	4	~					`			4' 6'			EAST WINC FALL	20	4	6"					
W4	· · · · · · · · · · · · · · · · · · ·	20	2			4.0.19	`					$\frac{2'}{2'}$ 6 ³			EAST WING YALL	20	2			277			
W5	WEST WINGWALL	19 20	2			10"					•	<u>5 /</u> 5' 4'	" <u>120</u> " 178		EAST WING VALL	<u> 19 </u> 20	2		1 0	,			+
	and the second	20	1(<u> </u>				20		0' 9")				†
W6 W7		20	3									32' 6'				20		52 ' 6"			· · · · ·		
W6 W7		20	17	7' 0 "								17 ' 0'		10 5 W 9	EAST WIN WALL	20		7' 0"					
W6 W7 W8 W9		20	3	6"								3' 6'	" 51	14 5 W10	EAST WINGWALL	20	27	5' <u>6</u> "	,				_
W6 W7 W8					1		-	-	1	1	1		· ·							1	I	•	1
₩6 ₩7 ₩8 ₩9 ₩10	WEST WINGWALL		11	· · A"								12' "	° 257	20 5 W11	FAST WINDWALL	2∩	1	2' [^]	9				+
 ₩6 ₩7 ₩8 ₩9 ₩10 ₩11 	WEST WINGWALL	20	12									12' 4' 11' 10'	²⁵⁷ 321	20 5 W11 26 5 W13	EAST WIN WALL	20 20		2' 4" 1' 10"	,				

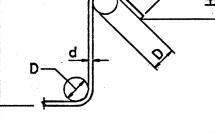




90.

	STIRF	RUP HOOK	DIMENSION	S								
	G	RADES 40-5	0–60 KSI									
BAR	D	90° HOOK	135*	HOOK								
SIZE (IN.) HOOK HOOK APPROX.												
SIZE (IN.) A OR G A OR G H												
#3	11⁄2"	4"	4"	21⁄2"								
#4	2"	41⁄2"	41⁄2"	3"								
# 5	21⁄2"	6"	51⁄2"	33⁄4"								
# 6	41⁄2"	8"	7"	41⁄2"								
		OTHERWISE N										







180

SIZE OF 180" HOOKS (GRADE 40 KSI)

DETAILING DIMENSION

90.

D = 6d FOR #3 THRU #8 D = 8d FOR #9, #10 AND #11 D = 10d FOR #14 AND #18

	END	HOOK DIM	ENSIONS	
	_	180 ° H	IOOKS	90° HOOKS
BAR SIZE	D (IN.)	ALL	GRADES	ALL GRADES
0.22		A OR G	J	A OR G
#3	21⁄4"	5"	3"	6"
#4	3"	6"	4"	8"
# 5	33⁄4"	7"	5"	10"
# 6	41⁄2"	8"	6"	12"
# 7	51⁄4"	10"	7"	14"
# 8	6"	11"	8"	16"
# 9	91⁄2"	15"	113⁄4"	19"
#10	10 3⁄ 4"	17"	131⁄4"	22"
#11	12"	19"	143⁄4"	2'-0"
#14	181⁄4"	2'-3"	213⁄4"	2'-7"
<u></u> #18	24"	3'-0"	2'-41⁄2"	3'-5"

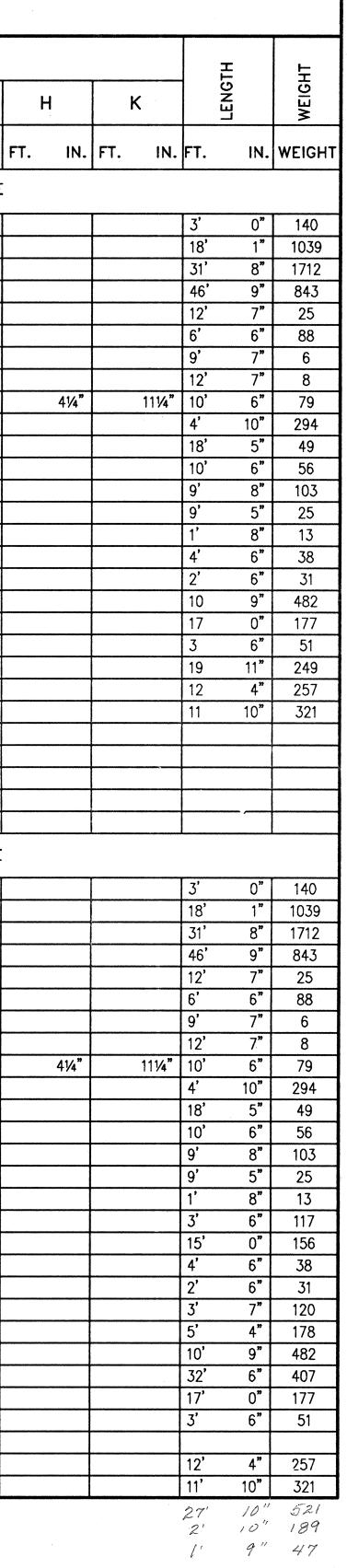
NOTES:

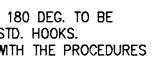
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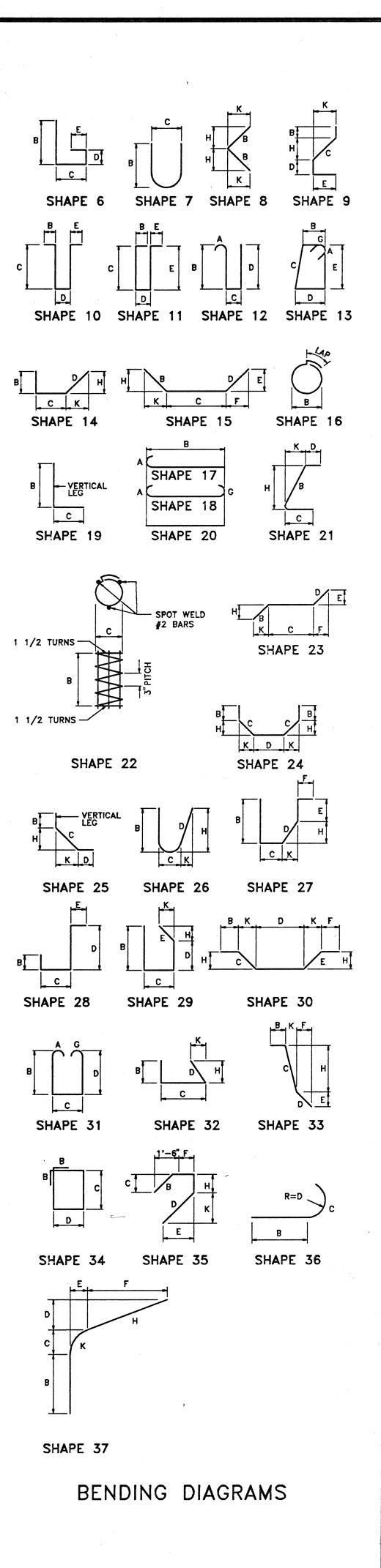
ALL STANDARD HOOKS AND BENDS OTHER THAN 180 DEG. TO BE BENT WITH SAME PROCEDURE AS FOR 90 DEG. STD. HOOKS. HOOKS AND BENDS SHALL BE IN ACCORDANCE WITH THE PROCEDURES AS SHOWN ON THIS SHEET. E - EPOXY COATED REINFORCEMENT.

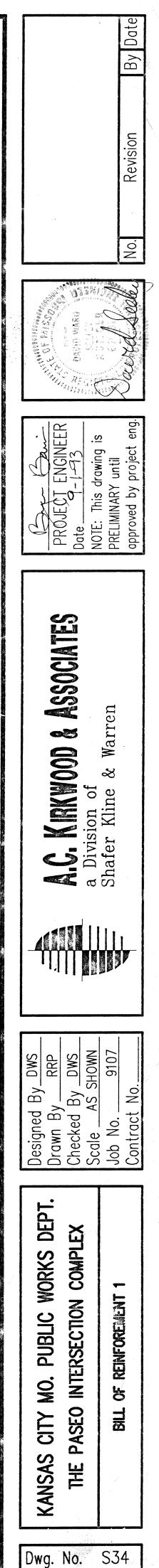
S – STIRRUP. V - BAR DIMENSIONS VARY IN EQUAL INCREMENTS BETWEEN DIMENSIONS SHOWN ON THIS LINE AND THE FOLLOWING LNE. NO. EA. - NUMBER OF BARS OF EACH LENGTH. NOMINAL LENGTHS - ARE BASED ON OUT TO OUT DIMENSIONS SHOWN IN BENDING DIAGRAMS AND ARE LISTED FOR FABRICATORS USE. (NEAREST INCH) ACTUAL LENGTHS - ARE MEASURED ALONG CENTERLINE OF BAR TO

THE NEAREST INCH. PAYWEIGHTS ARE BASED ON ACTUAL LENGTHS.





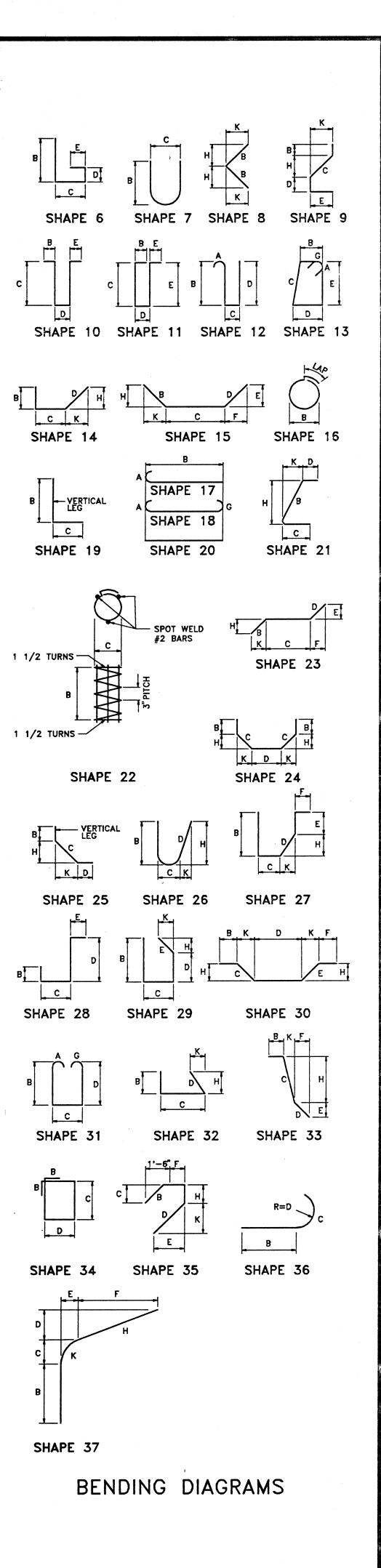




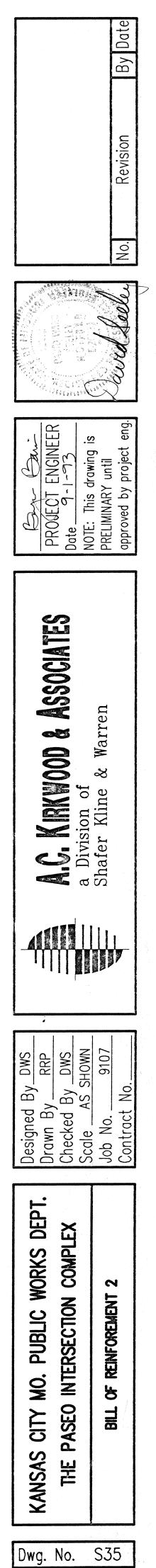
NOTE: This drawing is not to scale. Follow dimensions.

СОМ	PLETE BILL OF REINFORCING ST	EEL				COMPLETE I	BILL OF REINF	FORCING ST	EEL			
MARK NO.	DIMENSIONS	エ 上	. N	ARK	CH NO. E			DIMENSIONS				TH
	B C D E	MEIGH H K H H	REQ'D	LOCATION	EAC	B	C D	E	F	н	er 11	LENGT
SIZE MARK EPOX SHAPI SHAPI NO.	T. IN. FT. IN. FT. IN. FT. IN. FT.	IN. FT. IN. FT. IN. FT. IN. WEIGHT	NO. SIZE	MAR	SHAF VARI NO.	FT. IN. FT.	IN. FT. IN	I. FT. IN.	FT. IN. FT	. IN. FT.	IN. FT.	IN. W
SUBSTRUCTURE	NORTH PIER WEST BRIDGI		I	SUBSTRU	JCTURE	<u></u>	SOUTH F	PIER EAST BR	IDGE	_		
2 4 P1 PIER CAP 20 3	0"	3' 0" 104	52 4	P1 PIER CAP	20	3' 0"					3'	0"
2 8 P2 PIER CAP 17 4 2 8 P3 PIER CAP 20 3	3' 0" 7' 0"	48' 0" 2820 35' 6" 2173	22 8 22 8	P2 PIER CAP P3 PIER CAP	20	48' 0" 37' 0"					48' 35'	0" 6"
1 8 P4 PIER CAP 20 2		25' 0" 734 15' 7" 1520	11 8 146 4	P4 PIER CAP P5 PIER CAP	20 34	25' 0" 4½" 3'	11" 3' 6	n			25' 15'	0" 7"
6 4 P5 PIER CAP 34 34 3 5 P6 PIER CAP 20 4	6' 3"	46' 3" 386	8 5	P6 PIER CAP	20	46' 3"		22			46'	3"
B 4 P7 PIER CAP 24 1' 4 P8 PIER CAP 21 5'	6" 5' 5½" 5½" 1' 0" 1' 0"	8' 5" 101 3' 8" 4' 011/16" 7' 5" 20	18 4 4 4	P7 PIER CAP P8 PIER CAP	24 21	1' 6" 5' 5½" 1'	5' 5½ 0" 1' 0	22	3'	8"4'	8' 011/16" 7'	5" 5"
2 4 P9 PIER CAP 20 5 4 P10 PIER CAP 20 18	5" 2' 5"	5' 5" 333 18' 5' 49	92 4	P9 PIER CAP P10 PIER CAP	20 20	5' 5" 18' 5"					5' 18'	5" 5'
3 4 P11 PIER CAP 20 10)' 6"	10' 6' 56	8 4	P11 PIER CAP	20	10' 6"					10'	6' 8"
6 4 P12 PIER CAP 20 9 4 P13 PIER CAP 20 9	8" 6"	9' 8" 103 9' 6" 25	16 4 4 4	P12 PIER CAP P13 PIER CAP	20 20	9 8 9' 6"	σ				9'	6"
8 4 P14 PIER CAP 20 2 92 7 PC1 PIER COLUMN 20 16	2" 5' 10"	2' 2" 41 16' 10" 3510	28 4 102 7	P14 PIER CAP PC1 PIER COLUMN	20 20	2' 2" 16' 10"					2' 16'	2" 10"
44PC2PIER COLUMN201044PC2PIER COLUMN1621	8'	9' 5" 528	84 4		16	2' 8'					9'	5"
SUBSTRUCTURE	SOUTH PIER WEST BRIDGI		<u>_</u>	SUPERST			NORTH A	BUTMENT DIA		ST BRIDGE	_	*
SUBSTRUCTURE 2 4 P1 PIER CAP 20 3'			26 4	AD1 ABUT. DIAPHRAGM		6'	5" 4' 8¾				17'	7"
8 P2 PIER CAP 17 4		48' 0" 2820 35' 6" 2173	18 4	AD2 ABUT. DIAPHRAGM AD3 ABUT. DIAPHRAGM	E 19	6' 5" 4' 1' 9¥4" 1'	9" 9"	" 2' 6"	111/2"	6" 1'	8" 6'	2" 4"
8 P4 PIER CAP 20 2	5' 0"	25 ' 0" 734	86	AD4 ABUT. DIAPHRAGM	20	32' 10"				' '	32'	10"
6 4 P5 PIER CAP 34 5 P6 PIER CAP 20 44	4½" 3' 11" 3' 6" 5' 3" 11 11"	15' 7" 1520 46' 3" 386	30 6	AD5 ABUT. CIAPHRAGM AD6 ABUT. DIAPHRAGM	20	23' 4" 8' 9"					23' 8'	4" 9"
3 4 P7 PIER CAP 24 1' 4 P8 PIER CAP 21 5'	6" 5' 5½" 5½" 1' 0"	8' 5" 101 3' 8" 4' 0 ¹ 1⁄16" 7' 5" 20		AD7 ABUT. DIAPHRAGM AD8 ABUT. DIAPHRAGM	20	5' 0" 2'	0" 2' 3'	»	1'	61/2" 1'	5' 8½" 4'	0" 3"
2 4 P9 PIER CAP 20 5	5"	5' 5" 333	96	AD9 ABUT. DIAPHRAGM	E 20	23' 4"					23'	4" 0"
4 P10 PIER CAP 20 18 4 P11 PIER CAP 20 10	b' 6"	18' 5' 49 10' 6' 56	44 5	AD10 ABUT. DIAPHRAGM]5 0]	SOUTH A	BUTMENT DIA	PHRAGM WES	ST BRIDGE		0
6 4 P12 PIER CAP 20 9' 4 P13 PIER CAP 20 9'	8" 6"	9' 8" 103 9' 6" 25	26 4	AD1 ABUT. DIAPHRAGM		6'	5" 4' 8¾'				17'	7 "
3 4 P14 PIER CAP 20 2'	2" 5' 10"	2' 2' 41 16' 10" 3510	18 4	AD2 ABUT. DIAPHRAGM I AD3 ABUT. DIAPHRAGM I	E 19 E 35	6' 5" 4' 1' 9¥4" 1'	<u>9"</u> 0" 3' 0'	" 2' 6"	111/2"	6" 1'	8" 6'	2" 4"
2 7 PC1 PIER COLUMN 20 16 4 4 PC2 PIER COLUMN 16 2'	8'	9' 5" 528	86	AD4 ABUT. DIAPHRAGM	E 20	32' 10"					32'	4" 10"
				AD5 ABUT. DIAPHRAGM AD6 ABUT. DIAPHRAGM		23' 4" 8' 9"					23' 8'	4" 9"
SUBSTRUCTURE	NORTH PIER EAST BRIDGE	3' 0" 104		AD7 ABUT. DIAPHRAGM AD8 ABUT. DIAPHRAGM		5' 0" 2'	0" 2' 3'	n	 1'	61⁄2" 1'	5' 8½" 4'	0" 3"
2 8 P2 PIER CAP 17 4		48' 0" 2820	96	AD9 ABUT. DIAPHRACM	E 20	23' 4" 3' 0"					23'	4" 0"
2 8 P3 PIER CAP 20 3 1 8 P4 PIER CAP 20 25	5' 0"	- <u>35' 6" 2173</u> - <u>25' 0" 734</u>	44 5	AD10 ABUT. DIAPHRAGM		<u> </u> 5 0	NORTH A	BUTMENT DIA	PHRAGM EAS	T BRIDGE]	0
6 4 P5 PIER CAP 34 5 P6 PIER CAP 20 44	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	15' 7" 1520 46' 3" 386	26 4	AD1 ABUT. DIAPHR/ 3M		6'	5" 4' 8¾'			T	17'	7"
3 4 P7 PIER CAP 24 1' 4 P8 PIER CAP 21 5'	6" 5' 5½" 5½" 1' 0" 1' 0"	8' 5" 101 3' 8" 4' 0 ¹ ½16" 7' 5" 20	18 4	AD2 ABUT. DIAPHR GM AD3 ABUT. DIAPHR GM		6' 5" 4' 1' 9¥4" 1'	9" 0" 3' 0'	" 2' 6"	111/2"	6" 1'	8" 6'	2" 4"
2 4 P9 PIER CAP 20 5'	5"	5' 5" 333	86	AD4 ABUT. DIAPHR GM	E 20	32' 10"					32'	10" 4"
4 P10 PIER CAP 20 18 4 P11 PIER CAP 20 10		18' 5' 49 10' 6' 56	ğ	AD5 ABUT. DIAPHR 4GM AD6 ABUT. DIAPHR \GM		23' 4" 8' 9"					8'	9"
6 4 P12 PIER CAP 20 9' 4 P13 PIER CAP 20 9'	8" 6"	9' 8" 103 9' 6" 25	And a second	AD7 ABUT. DIAPHR \GM AD8 ABUT. DIAPHR \GM	20	5' 0" 2'	0" 2' 3'	77	1'	61⁄2" 1'	5' 8½" 4'	0" 3"
3 4 P14 PIER CAP 20 2'	2" 5' 10"	2' 8' 41 16' 10" 3510	96	AD9 ABUT. DIAPHE AGM I		23' 4" 3' 0"					23'	4" 0"
2 7 PC1 PIER COLUMN 20 16 4 PC2 PIER COLUMN 16 2'	8'	9' 5" 528	44 5	AD10 ABUT. DIAPHF AGM I	20		······				J	
					+							
· · · · ·		DETAILING HOOK DIMENSION A OR G						:				
	6d or 2 1/2" MIN.											
6d OR 2 1/2" MIN. 7		4d or 2 1/2" MIN.										
NOOH & BEAM		180				1	NOTES:	HOOKS AND BEND				
		SIZE OF 180° HOOKS (GRADE 40 KSI)			K DIMENSIONS	90' HOOKS	BENT WITH SAM HOOKS AND BE	IE PROCEDURE AS NDS SHALL BE IN	FOR 90 DEG. STE). HOOKS.		
DIMENSION	90.	D = 5d FOR #3 THRU #11 D = 10d FOR #14 AND #18		BAR D SIZE (IN.) A o	ALL GRADES	ALL GRADES A OR G	AS SHOWN ON			-		
		DETAILING DIMENSION		#3 21/4"	5" <u>3</u> "	6" 8"	V – BAR DIMEN		WIN ON THIS LINE		OWING LNE.	
135°	GRADES 40-50-60 KSI			#4 3 #5 3 3 ¥4"	7" 5"	10"		IBER OF BARS OF THS - ARE BASED	EACH LENGTH. ON OUT TO OUT	DIMENSIONS		
135 BAR D SIZE (IN	90° HOOK 135° HOOK HOOK HOOK APPROX.	A 0	-	#6 4½" 8 #7 5¼" 1	B" 6" 0" 7"	12" 14"	ACTUAL LENGTH	FABRICATORS US IS - ARE MEASUR		CH)		
#3 11/	A OR G A OR G H	90°		#8 6" 1	1" 8" 5" 1134"	16" " 19"		THE NEAREST IN RE BASED ON ACTU	ICH.			
#4 2 [°] #5 2½	° 4½° 4½° 3°	SIZE OF 90° HOOKS (ALL GRADES) AND 180° HOOKS (GRADE 60 KSI)		#10 10 3 4" 1	7" 131⁄4"	" 22"						
#6 47	2 ["] 8" 7" 4½"	D = 6d FOR #3 THRU #8 D = 8d FOR #9, #10 AND #11	ŀ	#14 18¼" 2'·	9" 14¾' -3" 21¾'	" 2'-7"						
NOTE: UNU	ESS OTHERWISE NOTED DIAMETER "D" IS FOR ALL BENDS AND HOOKS ON A BAR.	D = 10d FOR #14 AND #18	Γ	#18 24" 3'	-0" 2'-41	2" 3'-5"						

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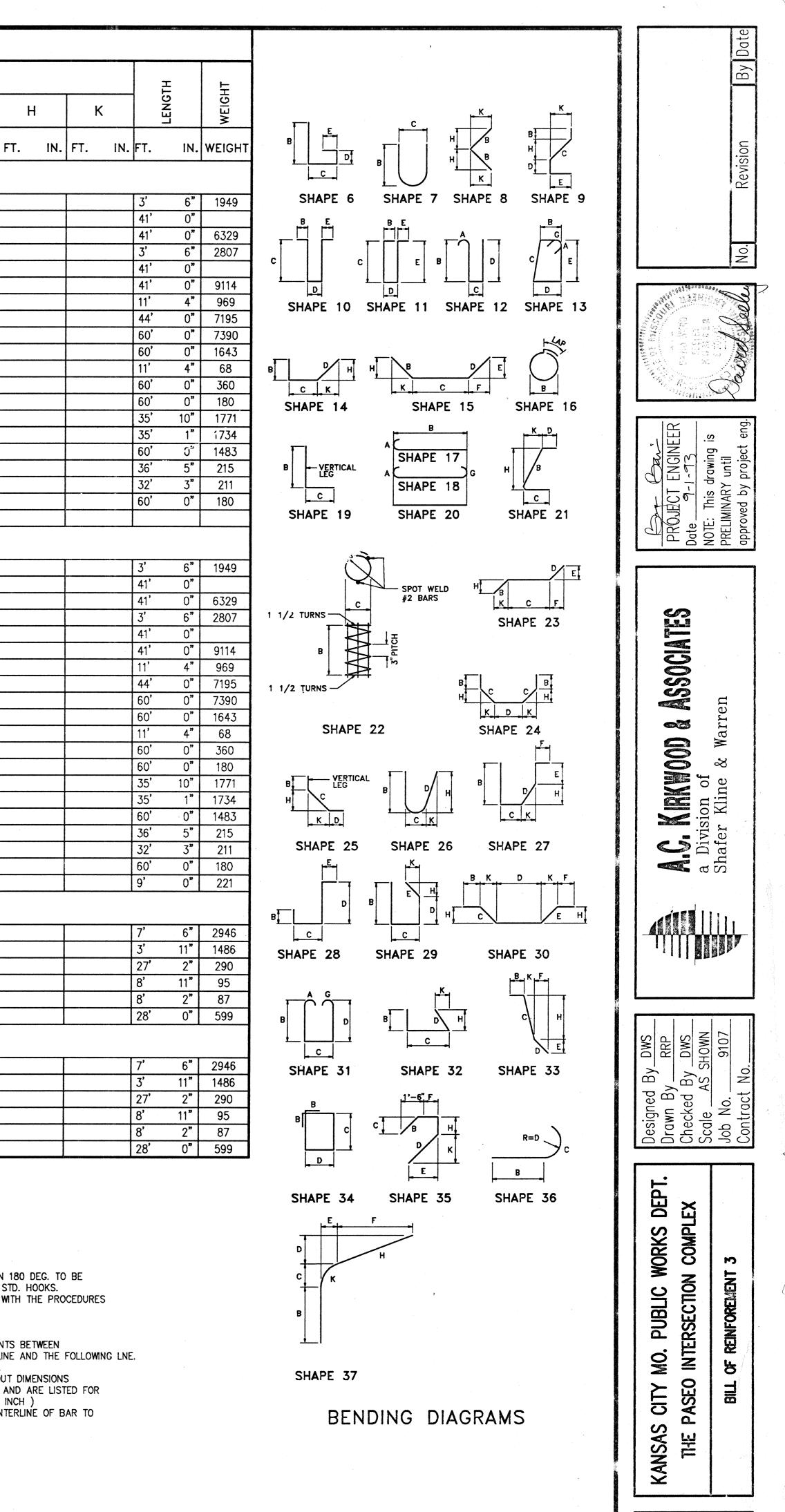
1.1



NOTE: This drawing is not to scale. Follow dimensions.

- ۲	COMPLETE BILL OF REINFORCING STEE		COMPLETE	BILL OF REINFORCING STEEL
	MARK DIMENSIONS		MARK	DIMENSIONS
REQ'D.		WEIGHT H		C D E F
NO. R		N. FT. IN. FT. IN. FT. IN. WEIGHT	NO. RE SIZE MARK SHAPE SHAPE NO. L	T. IN. FT. IN. FT. IN. FT. IN. FT.
	SUPERSTRUCTURE SOUTH ABUTMENT DIAPHRAGN		SUPERSTRUCTURE	SLAB WEST BRIDGE
26	4 AD1 ABUT. DIAPHRAGM E 10 6' 5" 4' 834"	17' 7" 305	84 5 S1 TRANS. BOTTOM E 20 V 3' 6"	
18			148 5 S2 TRANS. BOTTOM E 20 41' 0"	
8		32' 10" 395 23' 4" 421 8' 0" 704		
30	6 AD7 ABUT. DIAPHRAGM 20 5' 0"	8' 9" 394 5' 0" 90	148 6 S4 TRANS. TOP E 20 41' 0" 82 5 S5 LONGIT. TOP E 20 11' 4" 80 7 S6 LONGIT. TOP E 20 44' 0"	е с
	6 AD9 ABUT. DIAPHRAGM E 20 23' 4"	1' 6½" 1' 8½" 4' 3" 80 23' 4" 315 3' 0" 138	82 6 S7 LONGIT. TOP E 20 60' 0"	
44	5 AD10 ABUT. DIAPHRAGM E 20 3' 0" NORTH PIER DIAPHRAGM WES		4 6 S9 LONGIT. TOP E 20 11' 4"	
	4 PD1 PIER DIAPHRAGM E 10 6' 5" 5' 5½"	18' 3" 317 32' 10" 395	4 6 S10 LONGIT. TOP E 20 60' 0" 2 6 S11 LONGIT. TOP E 20 60' 0" 74 4 S12 LONGIT. BUTTOM E 20 35' 10"	
8	6 PD3 PIER DIAPHRAGM 20 8' 0"	32 10 393 8' 0" 721 4' 10" 174	74 4 S12 LONGIT. BOTTOM E 20 33 10 74 4 S13 LONGIT. BOTTOM E 20 35' 1" 37 4 S14 LONGIT. BOTTOM E 20 60' 0"	
	6 PD4 PIER DIAPHRAGM 20 4' 10" 5 PD5 PIER DIAPHRAGM 20 4' 10"	4' 10' 174 4' 10" 181	4 6 S15 LONGIT. BOTTOM E 20 35' 10" 4 6 S16 LONGIT. BOTTOM E 20 35' 10" 4 6 S16 LONGIT. BOTTOM E 20 35' 1"	
	SUPERSTRUCTURE SOUTH PIER DIAPHRAGM WES	T BRIDGE	4 6 S16 LONGIT. BOTTOM E 20 35 1 2 6 \$17 LONGIT. BOTTOM E 20 60' 0"	· · · · · · · · · · · · · · · · · · ·
Section 201	4 PD1 PIER DIAPHRAGM E 10 6' 5" 5' 5½"	18' 3" 317 32' 10" 395	SUPERSTRUCTURE	SLAB EAST BRIDGE
60	6 PD3 PIER DIAPHRAGM 20 8' 0"	32 10 393 8' 0" 721 4' 10" 174	84 5 S1 TRANS. BOTTOM E 20 V 3' 6"	
19 million	6 PD4 PIER DIAPHRAGM 20 4' 10" 5 PD5 PIER DIAPHRAGM 20 4' 10"	4' 10' 174 4' 10" 181	148 5 S2 TRANS. BOTTOM E 20 41' 0" 84 6 S3 TRANS. TOP E 20 V 3' 6"	
	SUPERSTRUCTURE NORTH PIER DIAPHRAGM EAS	T BRIDGE	04 0 35 TRANS. TOP E 20 V 5 0 148 6 S4 TRANS. TOP E 20 41' 0"	
g	4 PD1 PIER DIAPHRAGM E 10 6' 5" 5' 5½" 6 PD2 PIER DIAPHRAGM E 20 32' 10" 6' 5" 5' 5½"	18' 3" 317 32' 10" 395	82 5 S5 LONGIT. TOP E 20 11' 4" 80 7 S6 LONGIT. TOP E 20 44' 0"	
60	6 PD3 PIER DIAPHRAGM 20 8' 0" 6 PD4 PIER DIAPHRAGM 20 4' 10" 10"	8' 0" 721 4' 10" 174	82 6 S7 LONGIT. TOP E 20 60' 0" 41 4 S8 LONGIT. TOP E 20 60' 0"	
i i i i i i i i i i i i i i i i i i i	5 PD5 PIER DIAPHRAGM 20 4' 10"	4' 10" 181	4 6 S9 LONGIT. TOP E 20 11' 4" 4 6 S10 LONGIT. TOP E 20 60' 0"	
	SUPERSTRUCTURE SOUTH PIER DIAPHRAGM EAS	T' BRIDGE	2 6 S11 LONGIT. TOP E 20 60' 0" 74 4 S12 LONGIT. BOTTOM E 20 35' 10"	
26 8		18' 3" 317 32' 10" 395	74 4 S13 LONGIT. BOTTOM E 20 35' 1" 37 4 S14 LONGIT. BOTTOM E 20 60' 0"	
60 24	6 PD3 PIER DIAPHRAGM 20 8' 0"	8' 0" 721 4' 10" 174	4 6 S15 LONGIT. BOTTOM E 20 35' 10" 4 6 S16 LONGIT. BOTTOM E 20 35' 1"	
36		4' 10" 181	2 6 S17 LONGIT. BOTTOM E 20 60' 0" 12 7 S18 AT MANHOLE E 20 9' 0"	
	SUPERSTRUCTURE INTERMEDIATE DIAPHRAGM WE	ST BRIDGE	SUPERSTRUCTURE	BARRIER CURBS WEST BRIDGE
48 24		12' 9" 409 10' 1" 162	588 4 BC1 STIRRUPS E 28 3' 2½" 568 4 BC2 STIRRUPS E 19 3' 2½"	5" 3' 2½" 8" 8"
96 24	4 DD3 INTER. DIAPHRAGM 20 5' 10" 6 DD4 INTER. DIAPHRAGM 20 4' 6"	5' 10" 374 4' 6" 162	16 4 BC3 LONGIT. E 20 27' 2" 16 4 BC4 LONGIT. E 20 8' 11"	
			16 4 BC5 LONGIT. E 20 8' 2" 32 4 BC6 LONGIT. E 20 28' 0"	
	SUPERSTRUCTURE INTERMEDIATE DIAPHRAGM EA		SUPERSTRUCTURE	BARRIER CURBS EAST BRIDGE
24		12' 9" 409 10' 1" 162	588 4 BC1 STIRRUPS E 28 3' 2½" 568 4 BC2 STIRRUPS E 19 3' 2½"	5" 3' 2½" 8" 8" '
96 24	4 DD3 INTER. DIAPHRAGM 20 5' 10" 6 DD4 INTER. DIAPHRAGM 20 4' 6"	5' 10" 374 4' 6" 162	16 4 BC3 LONGIT. E 20 27' 2" 16 4 BC4 LONGIT. E 20 8' 11"	
			16 4 BC5 LONGIT. E 20 8' 2" 32 4 BC6 LONGIT. E 20 28' 0"	
	6d or 2 1/2" MIN.	DETAILING HOOK DIMENSION A OR G		
	6d OR 2 1/2" MIN7			
		4d or 2 1/2" MIN. 180°		NOTES:
536		SIZE OF 180° HOOKS (GRADE 40 KSI)	END HOOK DIMENSIONS	ALL STANDARD HOOKS AND BENDS OTHER THAN 180 BENT WITH SAME PROCEDURE AS FOR 90 DEG. STD.
/kcmo/		D = 5d FOR #3 THRU #11 D = 10d FOR #14 AND #18	BAR D SIZE (IN.) A OR G J A OR G	E – EPOXY COATED REINFORCEMENT.
C:\9107	STIRRUP HOOK DIMENSIONS		$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	S – STIRRUP. V – BAR DIMENSIONS VARY IN EQUAL INCREMENTS E DIMENSIONS SHOWN ON THIS LINE A
	GRADES 40-50-60 KSI		$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	NO. EA NUMBER OF BARS OF EACH LENGTH. NOMINAL LENGTHS - ARE BASED ON OUT TO OUT D SHOWN IN BENDING DIAGRAMS AND
	SIZE (IN.) HOOK HOOK APPROX. A OR G A OR G H	< <u> </u>	#0 472 0 0 12 #7 $51/4$ " 10" 7" 14" #8 6" 11" 8" 16"	FABRICATORS USE. (NEAREST INCH ACTUAL LENGTHS – ARE MEASURED ALONG CENTERL THE NEAREST INCH.
) JLS	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	SIZE OF 90° HOOKS (ALL GRADES) AND 180° HOOKS (GRADE 60 KSI)	$\pi \circ$ σ $\pi \circ$ $\pi \circ$ $\pi \circ$ $\pi \circ$ $\#9$ $91/2^n$ 15^n $113/4^n$ 19^n $\#10$ $103/4^n$ 17^n $131/4^n$ 22^n	PAYWEIGHTS ARE BASED ON ACTUAL LENGTHS.
10:25	#5 2½" 6" 5½" 3¾" #6 4½" 8" 7" 4½"	AND 180 HOURS (GRADE 60 KSI) D = 6d FOR #3 THRU #8 D = 8d FOR #9, #10 AND #11	#10 1074 11 1074 12 #11 12" 19" 14 $\frac{3}{4}$ " 2'-0" #14 18 $\frac{1}{4}$ " 2'-3" 21 $\frac{3}{4}$ " 2'-7"	
3/30/93	NOTE: UNLESS OTHERWISE NOTED DIAMETER "D" IS THE SAME FOR ALL BENDS AND HOOKS ON A BAR.	D = 10d FOR #14 AND #18	#18 24" 3'-0" 2'-4½" 3'-5"	
2				

NTS BETWEEN INE AND THE FOLLOWING LNE. . DUT DIMENSIONS AND ARE LISTED FOR I INCH) NTERLINE OF BAR TO

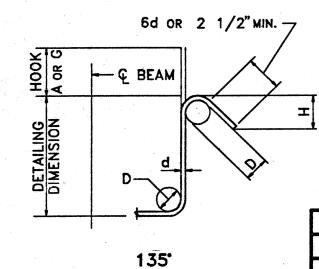


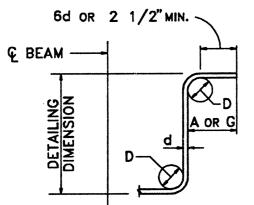
N 180 DEG. TO BE STD. HOOKS. WITH THE PROCEDURES

			СОМІ	PLETE B	ILL OF	REIN	FORCIN	G STEE	L										СОМ	PLE	IE BILL	OF REIN	NFORCI	ING S	TEEL	
MARK NO.		(E) 0.	8_1				DIMENSION	١S				н	F		MARK	(E)	NO.	Ξī				:	DIME	NSIONS	5	
	LOCATION	Z	EACH	В	с	D	E	F'	H H		к	LENGTH	WEIGHT	DEO 'D				EAC	В		С	D		Ε	F	
SIZE		EPOXY SHAPE	VARIE NO.	. IN. FT.	. IN. F	T. IN.	FT. IN	. FT. II	N. FT. 1	N. FT.	IN. F		IN. WEIGHT		SIZE	ΕΡΟΧΥ	SHAI	VARIE NO.	FT.	IN.	FT. IN.	FT. I	N. FT.	IN.	FT. II	N. F1
	SUPERSI	RUCTU	RE	·	 	NORTH AF	PPROACH	SLAB WES	T BRIDGE		L.					SUPERSTR	UCTU	RE		1		NORTH	APPRO	ACH S	LAB EAS	r br
7 AP		E 20	V 1 19	10"									10" 5850	7	2 7 AP1	BOTTOM LONGIT. E	20	V 1		10"						
7 A P	2 BOTTOM LONGIT.	E 20	59	8" 0"							•	59' 10'	8" 0" 286		4 7 AP2	BOTTOM LONGIT. E	20		59' 10'	8" 0"						
7 AP.			V 1 53									53'	0" 897		3 7 AP3	E	20	V 1	53'	0"						—
4 AP	4 T. & B. TRANS.	E 20	56 21	9" 6"		<u></u>						56' 21'	9" 6" 804	5	6 4 AP4	T. & B. TRANS. E	20		56' 21'	9" 6"						
4 AP		E 20	V 2 39				·					39'	8" 790		4 4 AP5		20	V 1	39'	8"						
4 A P	5 T. & B. TRANS.	E 20	4'	3" 6"								4 2'	3" 6" 10		5 4 AP6	T. & B. TRANS. E	20		4 2'	3" 6"						
7 A P	in the second	E 20	V 1 20									20'	11" 2956	3	6 7 AP7	TOP LONGIT. E	20	V 1	20' 59'	11" 5"						
7 A P	B TOP LONGIT.	E 20	V 1 53									59' 53'	5" 0" 447		4 7 AP8	TOP LONGIT. E	20	V 1	59	0"						
			56									56'	4"				20		56'	4"				2		
7 AP		E 20 E 20	20 43	0"								20' 43'	0 [°] 41 0° 88		1 7 AP9 1 7 AP10	TOP LONGIT. E TOP LONGIT. E	20		43'	0"			·			
7 AP1		E 20	10	0"								9'	3" 20		1 7 AP11	TOP LONGIT. E	20		10'	0"						
7 AP1		E 20 E 20	9'	<u> </u>								9' 5'	1" 19 6" 11		1 7 AP12 1 7 AP13	BOTT. LONGIT. E BOTT. LONGIT. E	20 20		9' 5'	6"						
7 AP1		E 20	53									53'	0" 108		1 7 AP14	BOTT. LONGIT. E	20		53'	0"	o' o"	41	0"			
7 AP1 4 AP1	<u></u>	E 14 E 20	12	2'	0" 4	4' O"			2' 1	0" 2'	10"	6' 12'	0" 49 0" 64		4 7 AP15 3 4 AP16	BOTT. TRANS. E BARRIER CURB E	14 20		12'	0"	2 0	4'	0"			2
4 AP1		E 20	22									22'	0" 118	8	3 4 AP17	BARRIER CURB E	20		22'	0"	- 17		. 17			1
4 AP1 4 AP1	and the second se	E 28 E 19	3'	21/2" 21/2"	5" 3 8"	3' 21/2"	8	³³				7' 3'	6" 230 11" 120		6 4 AP18 6 4 AP19	BARRIER CURB E BARRIER CURB E	20 20			21⁄2" 21⁄2"	<u> </u>		/2	8"		
4 AP1 4 AP2		E 20	5'	0 ⁿ	0							5'	0" 27		3 4 AP20		20		5'	0"			_			
												<u></u>														
	SUPERSI			I	L	SOUTH A	I PPROACH	SLAB WES	T BRIDGE	I	I				L	SUPERSTR	UCTU	RE	1	I		SOUTH	APPRO	ACH S	LAB EAS	T BR
7 AF	-	E 20	V 1 19	10"			Τ	1			-	19'	10" 5850	7	2 7 AP1	BOTTOM LONGIT. E	20	V 1	19'	10"	·····			-		
2 1 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2		5 00	59	8" 0"								59' 10'	8" 0" 286		4 7 AP2	BOTTOM LONGIT. E	20		59' 10'	8" 0"						
7 A P		E 20 E 20	V 1 53	0"			+					53'	0 [°] 897		4 7 APZ 3 7 AP3	E	20	V 1	53'	0"	÷					
			56	9" 9"								56' 21'	9" 6" 804		6 4 AP4	T. & B. TRANS. E	20		56' 21'	9" 6"						
4 AP		E 20 E 20	V 1 39	6 8"								<u>39'</u>	8 [°] 790		4 4 AP4		20	V 2		8"						<u> </u>
			4'	3"				_	,			4'	3" 6" 10				20		4' 2'	3" 6"						
4 A P 7 A P		E 20 E 20	2' V 1 20	6" 11"								2 20'	6" 10 11" 2956		6 4 AP6 6 7 AP7			V 1		11"						
			59	5"								59'	5" 0" 447		1 7 AP8	TOP LONGIT. E	20	V 1	59' 53'	5" 0"		*				
7 A P	B TOP LONGIT.	E 20	V 1 53									53' 56'	0" 447 4"		t / AP8		20	V I	56'	4 "		e				
7 A P		E 20	20	0"								20'	0" 41 0" 88		1 7 AP9		20		20' 43'	0" 0"						
7 AP1 7 AP1		E 20 E 20	43									43' 9'	0" 88 3" 20		1 7 AP10 1 7 AP11		20 20		43 10'	0"						
7 AP1	2 BOTT. LONGIT.	E 20	9'	1"				_				e,	1" 19 6" 11		1 7 AP12		20		9' 5'	1" 6"						
7 AP1 7 AP1		E 20 E 20	5'	6" 0"								5' 53'	6 [°] 11 0 [°] 108		1 7 AP13 1 7 AP14	BOTT. LONCT. E BOTT. LONCIT. E	20 20		5 53'	ь 0"						
7 AP1	5 BOTT. TRANS.	E 14		2'	0" 4	4' 0"			2' 1	0" 2'	10"	6'	0" 49		4 7 AP15	BOTT. TRANS. E	14		10'		2' 0"	4'	0"			2
4 AP1 4 AP1		E 20 E 20	12				-					12' 22'	0" 64 0" 118		3 4 AP16 3 4 AP17	BARRIER CURB E	20 20		12' 22	0" 0"	•					
4 AP1	B BARRIER CURB	E 28	3'	21⁄2"	5" 3	3' 21⁄2"	8	33				7'	6" 230	4	6 4 AP18	BARRIER CURB E	20		3'	21⁄2"		1.9	/2"	8"		
4 AP1	i na senera de la companya de la com	E 19 E 20	<u> </u>	<u>2½"</u> 0"	8"							5' 5'	11" 120 0" 27		6 4 AP19 3 4 AP20		19 20	 	5'	2½" 0"	8"			,		
· · · · · · · ·																										
														-		<u> </u>										
+	3		╆╍╊╼╍╄╼╍╸				1									·	1		1				1			

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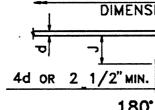
---- marga

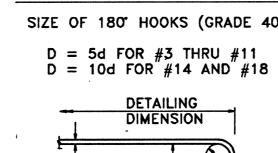




90.

	STIRF	RUP HOOK	DIMENSION	S									
	G	RADES 40-5	0-60 KSI	,									
DAD	D	90° HOOK	135*	HOOK									
	BAR D HOOK HOOK APPROX.												
JIZE	SIZE (IN.) HOOK HOOK APPROX. A OR G A OR G H												
<u></u> #3	11⁄2"	4"	4"	21⁄2"									
#4	2"	41⁄2"	41⁄2"	3"									
#5	21⁄2"	6"	51⁄2"	334"									
# 6	41⁄2"	8"	7"	41⁄2"									
NOTE: THE S	UNLESS	OTHERWISE N	NOTED DIAME	TER "D" IS ON A BAR.									





90. SIZE OF 90° HOOKS (ALL GRADES) AND 180° HOOKS (GRADE 60 KSI) D = 6d FOR #3 THRU #8 D = 8d FOR #9, #10 AND #11 D = 10d FOR #14 AND #18

DIMENSION A OR G

180

SIZE OF 180" HOOKS (GRADE 40 KSI)

IMENSION

	END	HOOK DIM	ENSIONS	
		180° H	IOOKS	90° HOOKS
BAR	D (IN.)	ALL	GRADES	ALL GRADES
5126	()	A OR G	J	A OR G
#3	21⁄4"	5"	3"	6"
#4	3"	6"	4"	8"
# 5⁻	33⁄4"	7"	5"	10"
# 6	41⁄2"	8"	6"	12"
# 7	51⁄4"	10"	7"	14"
# 8	6"	11"	8"	16"
# 9	91⁄2"	15"	113⁄4"	19"
# 10	103⁄4"	17"	131⁄4"	22"
# 11	12"	19"	143⁄4"	2'-0"
#14	181⁄4"	2'-3"	21 3⁄4 "	2'-7"
# 18	24"	3'-0"	2'-41⁄2"	3'-5"

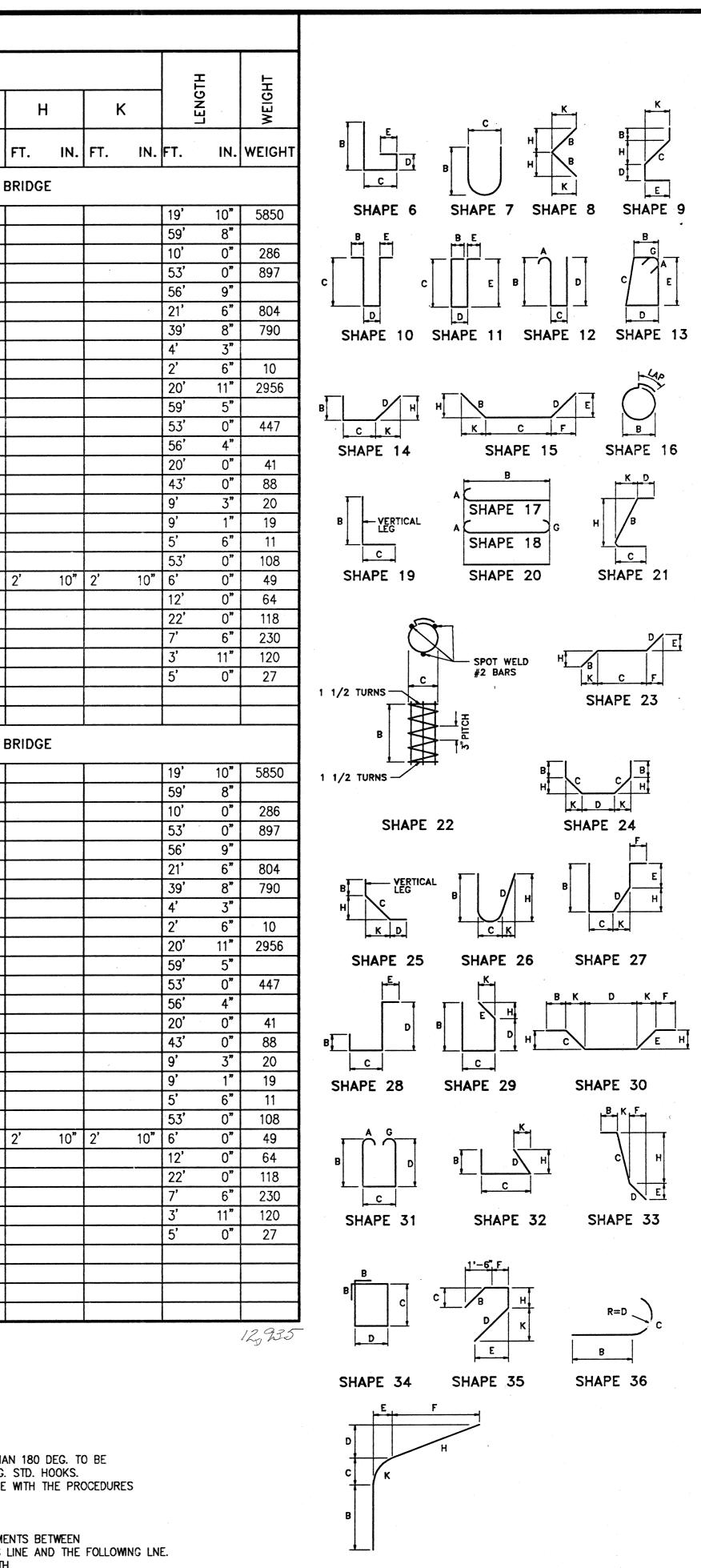
NOTES:

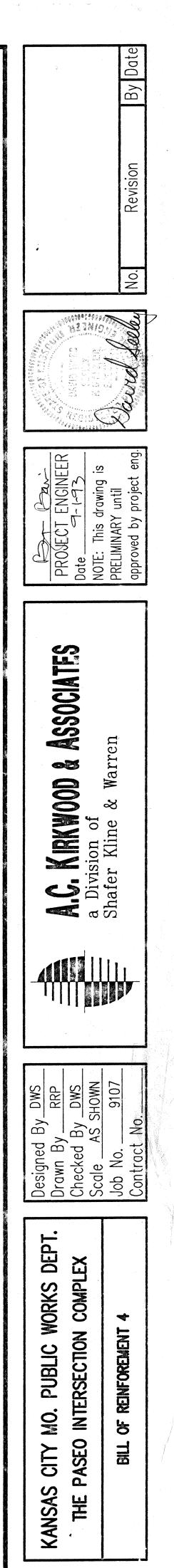
ALL STANDARD HOOKS AND BENDS OTHER THAN 180 DEG. TO BE BENT WITH SAME PROCEDURE AS FOR 90 DEG. STD. HOOKS. HOOKS AND BENDS SHALL BE IN ACCORDANCE WITH THE PROCEDURES AS SHOWN ON THIS SHEET. E - EPOXY COATED REINFORCEMENT.

s – stirrup.

V - BAR DIMENSIONS VARY IN EQUAL INCREMENTS BETWEEN DIMENSIONS SHOWN ON THIS LINE AND THE FOLLOWING LNE. NO. EA. - NUMBER OF BARS OF EACH LENGTH. NOMINAL LENGTHS - ARE BASED ON OUT TO OUT DIMENSIONS SHOWN IN BENDING DIAGRAMS AND ARE LISTED FOR

FABRICATORS USE. (NEAREST INCH) ACTUAL LENGTHS - ARE MEASURED ALONG CENTERLINE OF BAR TO THE NEAREST INCH. PAYWEIGHTS ARE BASED ON ACTUAL LENGTHS.





B H D E SHAPE 9

HAD -

SHAPE 16

H B

SHAPE 21

K C F SHAPE 23

SHAPE 24

СК

SHAPE 27

SHAPE 30

D K F

DE

SHAPE 33

R=D

SHAPE 36

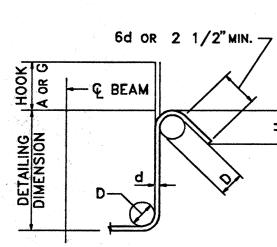
NOTE: This drawing is not to scale. Follow dimensions.

BENDING DIAGRAMS

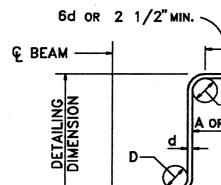
SHAPE 37

					LETE	BIL	L C	DF R	EINF	FORC	ING	STE	ΕL						 			·····			CO	MPLE	ETE BI	LL C	OF REINF	ORCING	STEEL	-	
MARK NO.	(F)	No.	() H						[DIMENS	IONS							H	노		MARK NO.		NO.	ACH (V)			.			DIMENSIO	NS		
		1		1	В	c	;		>	E		F		H		K		LENGTH	WEIGHT	REQ'D.	ш ¥	LOCATION	OXY APE	RIES . EAC	E	3	С	•	D	E		F	
NO. SIZE MARI	C d d	SHAPE	VAR	FT.	IN.	FT.	IN.	FT.	IN.	FT.	IN. F	T.	IN. FT	. IN	N. FT.	IN.	FT.	IN.	WEIGHT	vo.	SIZE MARI		SHA	VAR NO.	FT.	IN.	FT.	IN.	FT. IN.	FT. IN	I. FT.	IN.	FT
S	UBSTRU	CTUR	RE					NORT	ГН АР	PROAC	H WAL	LLS										SUBSTR	UCTUR	RE					SOUTH AF	PROACH	WALLS	,	
98 5 AW2 FTG. W	ALLS A&P	20		2'	۸"												2'	۸"	239	08	5 4.14/2	FTG. WALLS A&B	20		2'	۸"							
4 5 AW3 FTG. W	ALLS A&B	20		3'													3'	+ 5"	14			FTG. WALLS A&B	20		3'	+ 5"							
2 5 AW4 FTG. W/ 2 5 AW5 FTG. W/	a de la companya de l	20 20		20' 21'	<u>6"</u> 2"												20' 21'	6" 2"	43	2		FTG. WALLS A&B FTG. WALLS A&B	20		20' 21'	6" 2"							
2 5 AW6 FTG. W	and the second	20		22'	0"												22'	0"	46			FTG. WALLS A&B	20		22'	 0"							
2 5 AW7 FTG. W		20		16'	7"												16'	7"	ుర	2		FTG. WALLS A&B	20		16'	7"							
68 5 AW8 FTG. W/ 28 5 AW9 FTG. W/		20		17'	<u> </u>					`							5' 17'	6"	390 512	2		FTG. WALLS A&B FTG. WALLS A&B	20	╂╌╂──	5' 17'	<u>6</u> "			*****				
24 5 AW10 FTG. W	وسنبي ومعمد والمستعد	· 20		4'	9"				4 19								4'	9" 7"				FTG. WALLS A&B	20		4'	9"			o) //				
4 5 AW11 FTG. W/ 4 5 AW12 FTG. W/		36 36		1		8' 7'	<u>11"</u> 7"	6 5'	1" 2"								10' 9'	3"	42			FTG. WALLS A&B FTG. WALLS A&B	36	++-	1'	<u>8"</u> 8"	8' 7'	11" 7"	$\frac{6}{5'}$ 1"				
4 5 AW13 FTG. W	ALLS A&B	36		1'	8"	6'	3"	4'	3"			•					7'	11"	34	4	5 AW13	FTG. WALLS A&B	36		1'	8"	6'	3"		<i>.</i>			
4 5 AW14 FTG. W/ 4 5 AW15 FTG. W/	and a second	36 36	┨	1'		4' 3'	9" 5"		4" 5"								6' 5'	5" 1"	26 22			FTG. WALLS A&B FTG. WALLS A&B	36		1'	<u> </u>	4' 3'	9" 5"					
2 5 AW16 FTG. W	ALLS C&D	20		29'	1"		<u> </u>						-		-		29'	1"	61			FTG: WALLS C&D	20		29'	1"			2 5				
2 5 AW17 FTG. W		20		36'	<u>2"</u>									-			36'	2"	75			FTG. WALLS C&D	20		36'	2"							
2 5 AW18 FTG. W/ 2 5 AW19 FTG. W/		20 20		36' 37'	<u> </u>				· .								36' 37'	9"	77 79			FTG. WALLS C&D FTG. WALLS C&D	20	+	36' 37'	<u>11"</u> 9"							
na se de la companya	S A&D	19		4'	2"		10"										5'	0"	522	100	5 AW20	WALLS A&D	19		4'	2"		10"					
in a second a second de la construction de la construction de la construction de la construction de la constru	S A&D S A&D	20 20		16'	4" 0"	1											16' 17'	4" 0"	85 89	5		++	20	$\left\{ \cdot \right\}$	16' 17'	<u> </u>							
5 5 AW23 WA	LL D	20		32'	5"												32'	5"	169	5	5 AW23	WALL D	20		32'	5"							
56 5 AW24 WALL	S B&C	19	V 2	4'	<u>6"</u> 1"		8" 8'										5'	<u>2"</u> 9'	319	56	5 AW24	WALLS B&C	19	V 2	4'	<u>6"</u> 1"		8" 8'					
56 4 AW25 WALL	S B&C	19	V 2	4'	6"		8"										5'		204	56	4 AW25	WALLS B&C	19	V 2	4'	6 [°]		8"					
		20		5'	1" 4"		8'										5'	9'	270		4 41400				5'	1"		8'					
	B&C B&C	20 37	++-	14'	4		61⁄2"	1'	65⁄8"		91⁄4" 2'	· 6	7⁄8" 3'	0)" 1'	93⁄4"	14' 5'	4 9"	230 38	24 10			20	<u> </u>	14'	4	1'	61/2"	1' 65%"	91⁄4	" 2'	67⁄8"	3'
10 4 AW31 WALI	_, B&C	37			1⁄4"	1'	21⁄2"		61⁄4"		75⁄8" 2'		3⁄8" 2	111/2	2" 1'	5"	5'	33/4"	36	10	4 AW31	WALL B&C	37			111/4"	1'	21⁄2"	1' 6¼"		" 2'	6¾"	
	LL D	20 19		31'	<u> </u>		0"										31'	3" 6"	163 365	5 54		+	20 19	$\left \right $	31'	<u>3"</u> 6"		0"					
48 4 AW37 WA		20		26'	8"		, ,		· · · · ·								26'	° 8"	1335	48			20		26'	8"	L						
		20 20		33'	<u>10'</u> 8"												33'	10" 8"	917 1912	24 166		+	20	┨ ┨	33'	<u>10"</u> 8"				2			
		20		5'	3"				,								5'	3"	32	9	4 AW40	+	20	╂╌╂──	5'	3"							
		20		1'	7" o"		c "	-									1'	7"	19	18			20		1'	7 "		0"					
the second s		14 20		2'	8" 6"	2	6"										2'	2" 6"	38 120	18 46	4 AW42 5 AW43		14 20	$\left \right $	2'	8" 6"		6"					
30 5 AW44 WA	LE	20		4'	6"												4'	6"	141	30	5 AW44	WALL E	20		4'	6"					*		
7 5 AW45 WA		20		5' 3'	0" 7"												3'	0" 7"	85	17	5 AW45	WALL F	20		5' 4'	0" 7"							
8 4 AW46 WA	LF	20	V 2	5'	01⁄2"												5'	01⁄2"	59	18	4 AW46	WALL F	20	V	5'	01⁄2"							
1 5 AW47 WA		20		4'	7" 0"												4' 5'	7" 0"	5		5 AW47	WALL F	20	<u> </u>	4' 5'	7" 0"							
		20	V 2	4'	7 "												4'	7"	134	48	4 AW49		20	V 2	4'	7"							
10 4 AW50 WAI		19		3'	<u> </u>		8"										3'	8" 7'	767	210	A AWEO	, , ,	10		3'	8"		0"					
	L F	20	V 2	3'	7'		0										3'	7"	363 673		4 AW50 4 AW51	WALL F	19 20	V 2	3'	11' 7'		8"					
				4'	5"												4'	5"							4'	5"							
		20 20		40' 23'	0" 0"	:											40' 19'	0" 6"	160 46	6	4 AW54 4 AW55	WALL F	20 20		40' 23'	0" 0'				,			
6 4 AW56 WAI	LF	20		40'	0"			****									40'	0"	428	16	4 AW56	WALL F	20		40'	0"							
	LF	20 20		23' 37'	<u> </u>												23' 37'	0" 2"	123 99	8	4 AW57 4 AW59	WALL F	20 20		23' 37'	0" 2"				:			
2 4 AW60 WA		20		30'	<u> </u>						8"						30'	<u> </u>	40	2	4 AW59 4 AW60	WALL F	20		30'	<u> </u>							
02 5 AW61 WAI	LF	20		2'	4"			-									30'	0"	315	202	4 AW61	WALL F	20		2'	4"							
				<u> </u>		I											[[I	II	L								
						OR 2 1	/2" min	" \							DET	AILING	HO A O	DOK DR G											æ				
				မြေ	BEAM -			<u> </u>	1						-	($\overline{}$												*				

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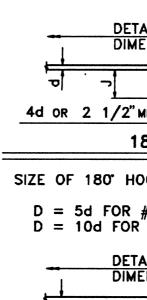
135



	STIRF	RUP HOOK	DIMENSION	S		
	G	RADES 40-5	0-60 KSI			
BAR	D	90° HOOK	135*	ноок		
SIZE	(IN.)	HOOK	HOOK	APPROX.		
		A OR G	A OR G	Н		
#3	11⁄2"	4"	4"	21⁄2"		
#4	2"	41⁄2"	41⁄2"	3"		
# 5	21⁄2"	6"	51⁄2"	3¾"		
# 6	41⁄2"	8"	7"	41⁄2"		
NOTE: UNLESS OTHERWISE NOTED DIAMETER "D" IS						

90.

THE SAME FOR ALL BENDS AND HOOKS ON A BAR.



····· KSI)

MIN.		
80	-	
DOKS	(GRADE	40

80			
OKS	(GRAD	E	40
#3 TI #14	HRU #	11 #1	8

	IRU AND	#11 #18	

		RU				
#1	4	AND) ŧ	¥18	3	

#3 THRU #11 #14 AND #18	

	IRU # AND		

#3 THRU #11 #14 AND #18	

3 THRU #11 #14 AND #18	

•	•
3 THRU #11 #14 AND #18	

43 THRU #11 #14 AND #18	
ILING	

	#11 D #1	8	

5 THRU #11 #14 AND #18	

3 THRU #11 #14 AND #18	
LING NSION	

	ING SION	<i>n</i> · –
A OR G	D_0.	12d

A OR G	D_0	12d
90	•	

SIZE OF 90° HOOKS (ALL GRADES) AND 180° HOOKS (GRADE 60 KSI) D = 6d FOR #3 THRU #8 D = 8d FOR #9, #10 AND #11 D = 10d FOR #14 AND #18

END HOOK DIMENSIONS								
	_	180° H	IOOKS	90° HOOKS				
BAR SIZE	D (IN.)	ALL	GRADES	ALL GRADES				
OILL	(",	A OR G	J	A OR G				
# 3	21⁄4"	5"	3"	6"				
#4	3"	6"	4"	8"				
# 5	33⁄4"	7"	5"	10"				
# 6	41⁄2"	8"	6"	12"				
#7	51⁄4"	10"	7"	14"				
#8	6"	11"	11" 8"					
# 9	91⁄2"	15" 11¾"		19"				
<i>#</i> 10	10¾"	17"	131⁄4"	22"				
#11	12"	19"	14 3⁄ 4"	2'-0"				
<i>#</i> 14	181⁄4"	2'-3"	21 3⁄4 "	2'-7"				
# 18	24"	3'-0"	2'-41⁄2"	3'-5"				

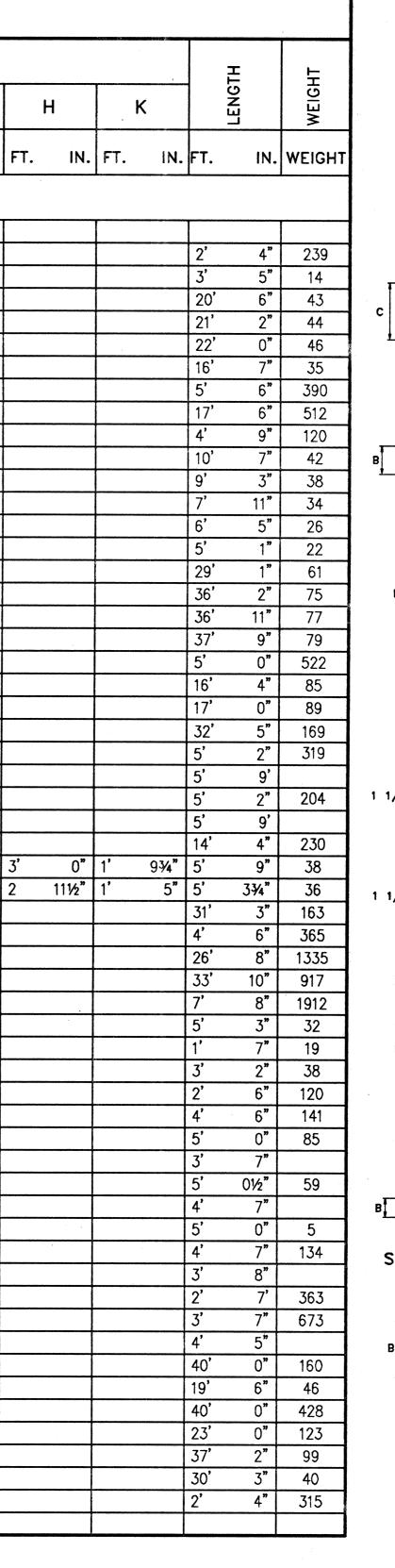
NOTES: ALL STANDARD HOOKS AND BENDS OTHER THAN 180 DEG. TO BE BENT WITH SAME PROCEDURE AS FOR 90 DEG. STD. HOOKS. HOOKS AND BENDS SHALL BE IN ACCORDANCE WITH THE PROCEDURES AS SHOWN ON THIS SHEET. E - EPOXY COATED REINFORCEMENT.

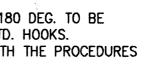
s – stirrup.

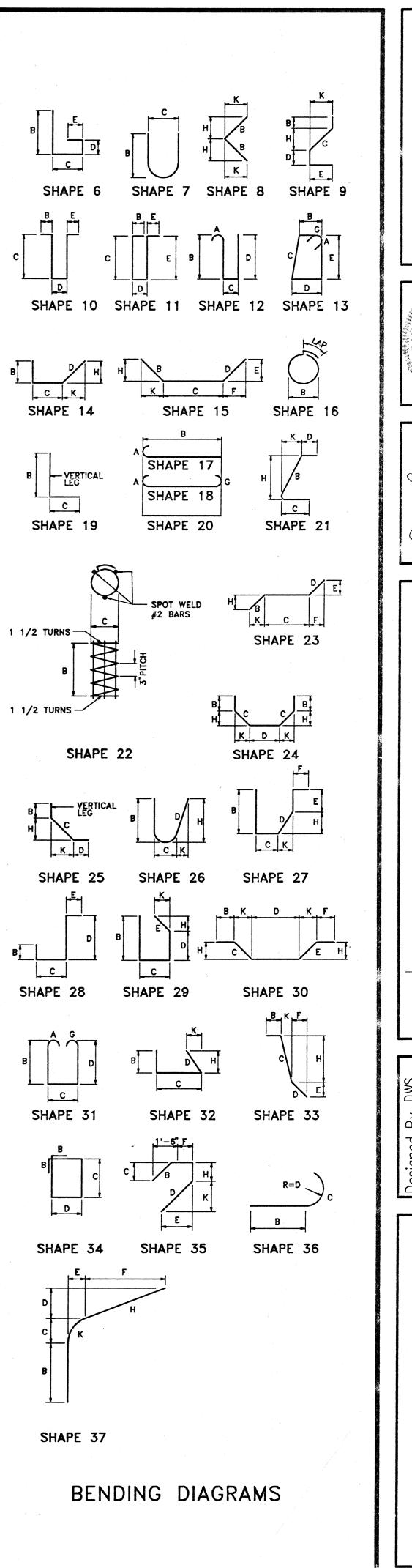
V - BAR DIMENSIONS VARY IN EQUAL INCREMENTS BETWEEN DIMENSIONS SHOWN ON THIS LINE AND THE FOLLOWING LNE. NO. EA. - NUMBER OF BARS OF EACH LENGTH. NOMINAL LENGTHS - ARE BASED ON OUT TO OUT DIMENSIONS

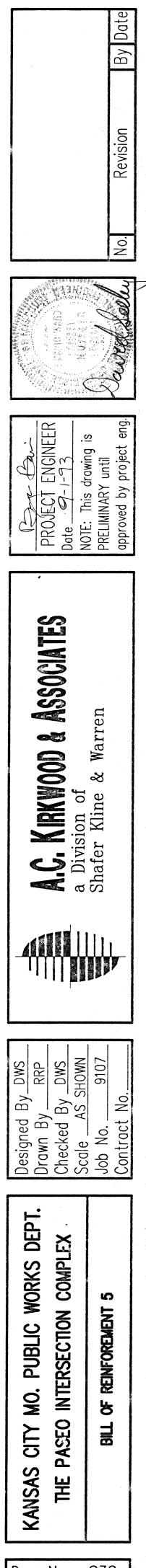
SHOWN IN BENDING DIAGRAMS AND ARE LISTED FOR FABRICATORS USE. (NEAREST INCH) ACTUAL LENGTHS - ARE MEASURED ALONG CENTERLINE OF BAR TO

THE NEAREST INCH. PAYWEIGHTS ARE BASED ON ACTUAL LENGTHS.

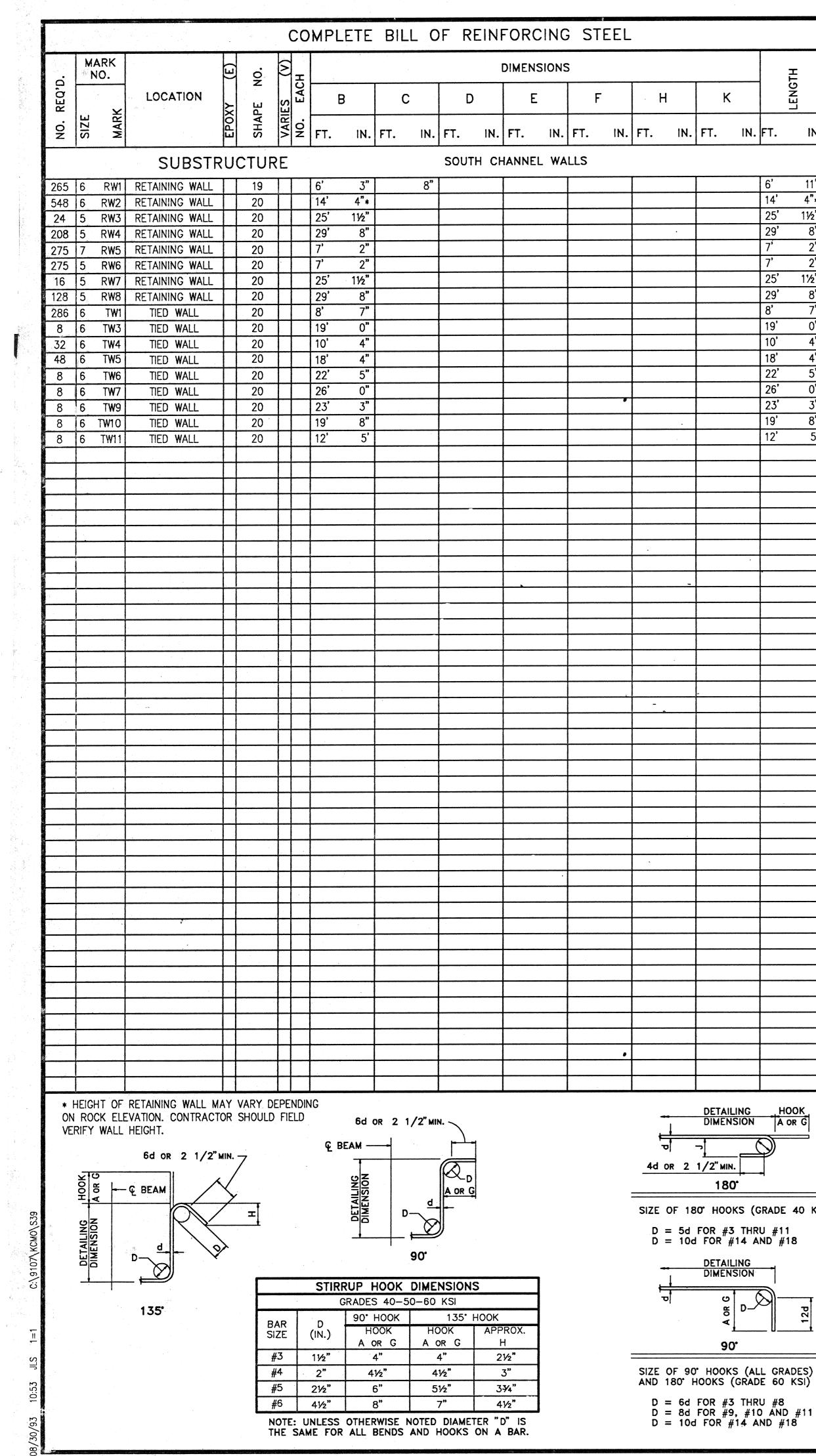








NOTE: This drawing is not to scale. Follow dimensions.



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		H H	F			ARK NO.		(E)	NO.	$(\mathbf{\hat{x}})$	۲							DIMEN	SIONS	5		
K		LENGTH	WEIGHT	REQ'D.		×	LOCATION	XΥ		IES	EACH		В	с			D	E		F		
IN.	FT.	IN.	WEIGHT	NO.	SIZE	MARK		ΕΡΟΧΥ	SHAPE	VARIES	NO.	FT.	IN.	FT.	IN.	FT.	IN.	FT.	IN.	FT.	IN.	FT.
					-		SUBSTR	S U (CTUR	E						NOR	тн сн	ANNEI	L WA	LLS		
	6'	11"	2753	265	6	RW1	RETAINING WALL		19			6'	3"		8"							
	14'	4"*	11797		6	RW2	RETAINING WALL	+	20			14'	4"*									
	25' 29'	1½" 8"	629 6463	J	5 5	RW3 RW4	RETAINING WALL RETAINING WALL		20 20			25' 29'	1½" 8"									
	7'	2"	4028		7	RW5	RETAINING WALL	+	20	+		7'	2"									
	7'	2"	2056		5	RW6	RETAINING WALL		20			7'	2"									
	25'	11/2"	420	16	5	RW7	RETAINING WALL		20			25'	11/2"									
	29'	8"	3960	128	5	RW8	RETAINING WALL		20			29'	8"							4		
	8'	7"	3687		6	TW1	TED WALL		20			8'	7"									
	19'	0"	228	8	6	TW2	TIED WALL		20	$\left \right $		23' 19'	1" 0"									
	10' 18'	4" 4"	497 1322	16 32	6 6	TW3 TW4	TIED WALL	+	20 20			19										
	22'	<u>4</u> 5"	269	-32 +8	6	TW5	TIED WALL		20			18'	<u> </u>									
	26'	0"	312	8	6	TW6	TIED WALL	+	20			22'	5"									
	23'	3"		8	6	TW7			20			26'	0"				•					
	19'	8"		8	6	TW8	TIED WALL		20			13'	3"									
	12'	5'	149																			
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180

SIZE OF 180° HOOKS (GRADE 40 KSI)

DETAILING

D = 6d FOR #3 THRU #8 D = 8d FOR #9, #10 AND #11 D = 10d FOR' #14' AND #18'

	END	HOOK DIM	ENSIONS	
		180° H	IOOKS	90° HOOKS
BAR SIZE	D (IN.)	ALL	GRADES	ALL GRADES
JILL	()	A OR G	J	A OR G
#3	21⁄4"	5"	3"	6"
#4	3"	6"	4"	8"
# 5	33⁄4"	7"	5"	10"
# 6	41⁄2"	8"	6"	12"
# 7	51⁄4"	10"	7"	14"
# 8	6"	11"	8"	16"
# 9	91⁄2"	15"	- 113⁄4"	19"
# 10	10¾"	17"	131⁄4"	22"
# 11	12"	19"	143⁄4"	2'-0"
# 14	181⁄4"	2'-3"	21¾"	2'-7"
# 18	24"	3'-0"	2'-41⁄2"	3'-5"

NOTES:

*

ALL STANDARD HOOKS AND BENDS OTHER THAN 180 DEG. TO BE BENT WITH SAME PROCEDURE AS FOR 90 DEG. STD. HOOKS. HOOKS AND BENDS SHALL BE IN ACCORDANCE WITH THE PROCEDURES AS SHOWN ON THIS SHEET. E - EPOXY COATED REINFORCEMENT.

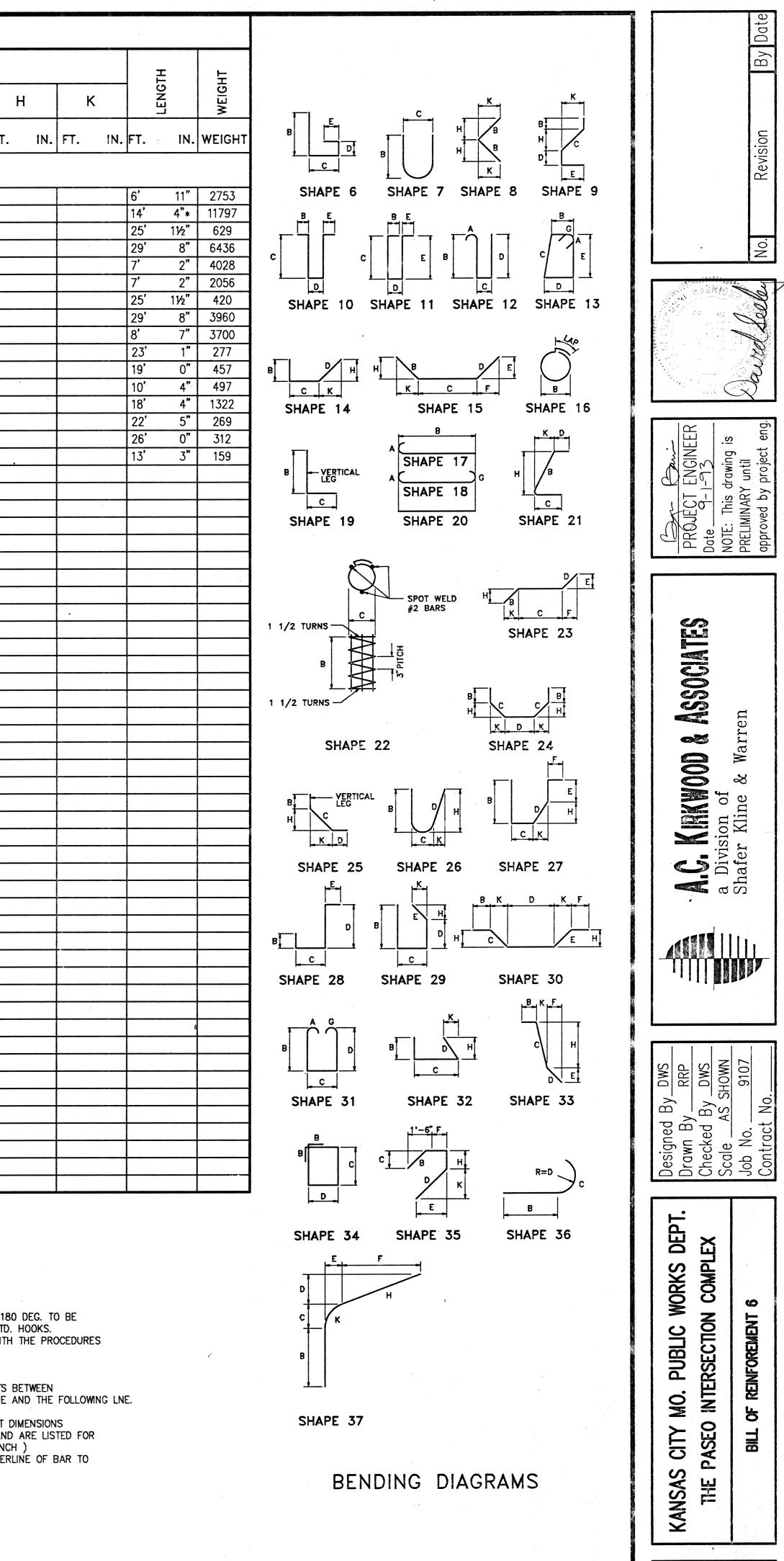
S – STIRRUP.

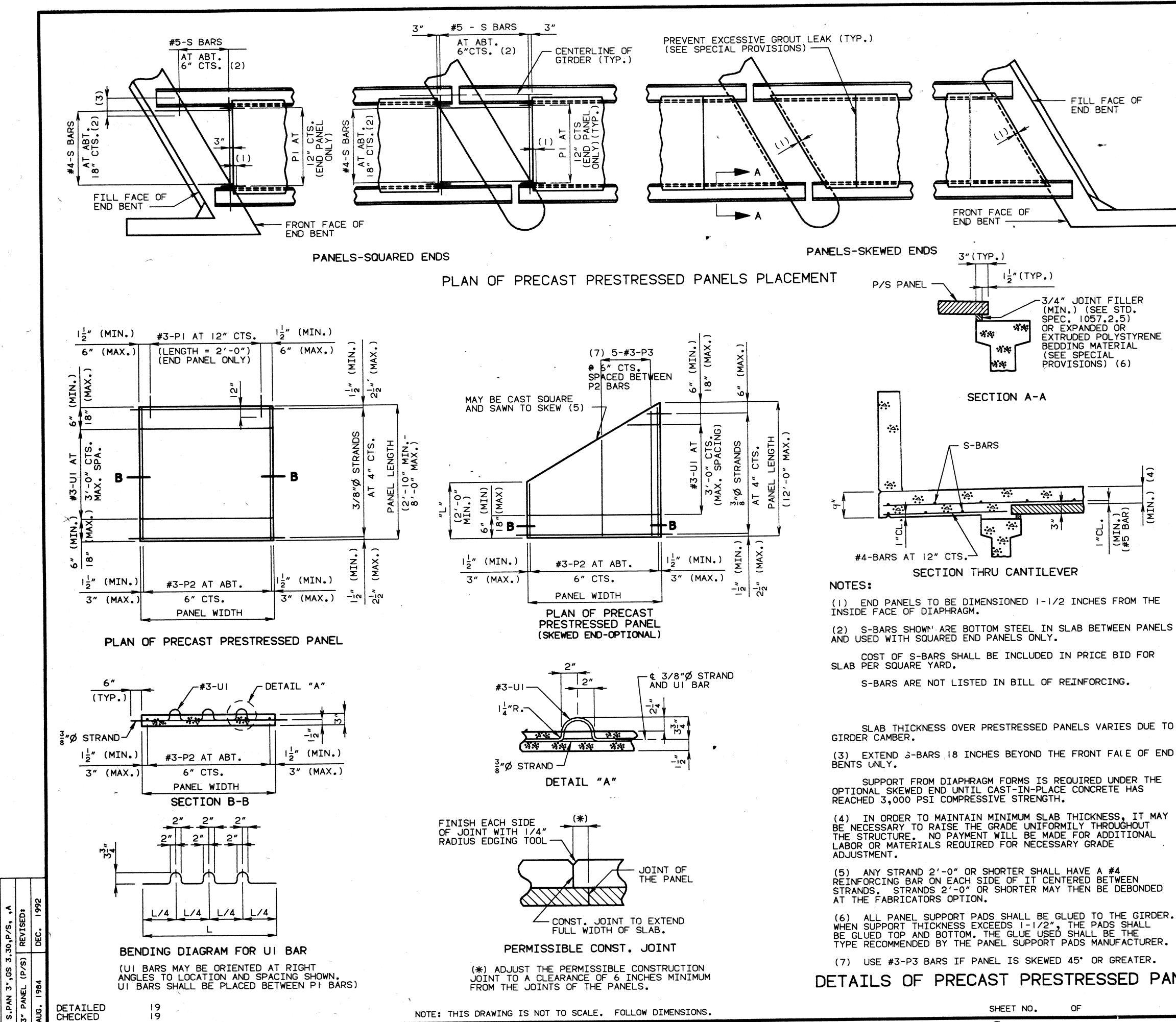
V - BAR DIMENSIONS VARY IN EQUAL INCREMENTS BETWEEN DIMENSIONS SHOWN ON THIS LINE AND THE FOLLOWING LNE. NO. EA. - NUMBER OF BARS OF EACH LENGTH.

NOMINAL LENGTHS - ARE BASED ON OUT TO OUT DIMENSIONS SHOWN IN BENDING DIAGRAMS AND ARE LISTED FOR FABRICATORS USE. (NEAREST INCH) ACTUAL LENGTHS - ARE MEASURED ALONG CENTERLINE OF BAR TO

THE NEAREST INCH. PAYWEIGHTS ARE BASED ON ACTUAL LENGTHS.

0





STATE	PROJ. NO.	SHEET NO.
MO.		

NOTE:

USE SLAB HAUNCHING DIAGRAM ON SHEET NO. FOR DETERMINING THICKNESS OF JOINT FILLER OR POLYSTYRENE BEDDING MATERIAL WITHIN THE LIMITS NOTED BELOW.

GENERAL NOTES:

PRESTRESSED PANELS:

CONCRETE FOR PRESTRESSED PANELS SHALL BE CLASS AI WITH F'C = 5,000 PSI, F'CI = 3,500 PSI.

THE TOP SURFACE OF ALL PANELS SHALL RECEIVE A SCORED FINISH WITH A DEPTH OF SCORING OF 1/8 INCH PERPENDICULAR TO THE PRESTRESSING STRANDS IN THE PANELS (SEE SPECIAL PROVISIONS).

PRESTRESSING TENDONS SHALL BE HIGH-TENSILE STRENGTH UNCOATED SEVEN WIRE (7), LOW-RELAXATION STRANDS FOR PRESTRESSED CONCRETE CONFORMING TO AASHTO M203, EXCEPT THAT NOMINAL DIAMETER OF STRAND = 3/8 INCH AND NOMINAL AREA = (.085 SQ. IN. AND MINIMUM ULTIMATE STRENGTH = 21,250 LBS. (250 KSI). LARGER STRANDS MAY BE USED WITH THE SAME SPACING AND INITIAL TENSION.

INITIAL PRESTRESSING FORCE = 14.9 KIPS/STRAND.

THE METHOD AND SEQUENCE OF RELEASING THE STRANDS SHALL BE SHOWN ON THE SHOP DRAWINGS.

SUITABLE ANCHORAGE DEVICES FOR LIFTING PANELS MAY BE CAST IN PANELS, PROVIDED THEY ARE SHOWN ON THE SHOP DRAWINGS AND APPROVED BY THE ENGINEER. PANEL LENGTHS SHALL BE DETERMINED BY THE CONTRACTOR AND SHOWN ON THE SHOP DRAWINGS.

WHEN SQUARE END PANELS ARE USED AT SKEWED BENTS, IT IS REQUIRED THAT THE SKEWED PORTION BE CAST FULL DEPTH. NO SEPARATE PAYMENT WILL BE MADE FOR THE ADDITIONAL CONCRETE AND REINFORCING REQUIRED.

MINIMUM JOINT FILLER OR POLYSTYRENE BEDDING MATERIAL THICKNESS SHALL BE 3/4 INCH. THICKER JOINT FILLER OR POLYSTRENE BEDDING MATERIAL MAY BE USED ON ONE OR BOTH SIDES OF THE GIRDER TO REDUCE CAST-IN-PLACE CONCRETE THICKNESS, WITHIN TOLERANCES. NO MORE THAN 2 INCHES TOTAL THICKNESS OF JOINT FILLER OR POLYSTYRENE BEDDING MATERIAL SHALL BE USED.

THE SAME THICKNESS OF JOINT FILLER MATERIAL SHALL BE USED UNDER ANY ONE EDGE OF ANY PANEL AND THE MAXIMUM CHANGE IN THICKNESS BETWEEN ADJACENT PANELS SHALL BE 1/4 INCH. THE POLYSTRENE BEDDING MATERIAL MAY BE CUT TO MATCH HAUNCH HEIGHT ABOVE TOP OF FLANGE.

AT THE CONTRACTORS OPTION, THE VARIATION IN SLAB THICKNESS OVER PRESTRESSED PANELS MAY BE ELIMINATED OR REDUCED BY INCREASING AND VARYING THE GIRDER TOP FLANGE THICKNESS. DIMENSIONS SHALL BE SHOWN ON THE SHOP DRAWINGS.

REINFORCING STEEL:

ALL DIMENSIONS ARE OUT TO OUT.

MINIMUM CLEARANCE TO REINFORCING STEEL SHALL BE 1-1/2 INCH, UNLESS OTHERWISE SHOWN.

HOOKS AND BENDS SHALL BE IN ACCORDANCE WITH THE C.R.S.I. MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES, STIRRUP AND TIE DIMENSIONS.

ACTUAL LENGTHS ARE MEASURED ALONG CENTERLINE OF BAR TO THE NEAREST INCH.

THE PRESTRESSED PANEL QUANTITIES ARE NOT INCLUDED IN THE TABLE OF ESTIMATED QUANTITIES FOR ALTERNATE SLABS.

IF UI BARS INTERFERE WITH PLACEMENT OF SLAB STEEL, UI LOOPS MAY BE BENT OVER, AS NECESSARY, TO CLEAR SLAB STEEL.

WELDED WIRE FABRIC OR WELDED DEFORMED BAR MATS PROVIDING A MINIMUM AREA OF REINFORCING PERPENDICULAR TO STRANDS OF 0.22 SQ. IN./FT., WITH SPACING PARALLEL TO STRANDS SUFFICIENT TO INSURE PROPER HANDLING, MAY BE USED IN LIEU OF THE #3-P2 BARS SHOWN. WIRE OR BAR DIAMETER SHALL NOT BE LARGER THAN 0.375 INCHES. THE ABOVE ALTERNATIVE REINFORCEMENT CRITERIA MAY BE USED IN LIEU OF THE #3-P3 BARS, WHEN REQUIRED, AND PLACED OVER A WIDTH OF NOT LESS THAN 2FT.

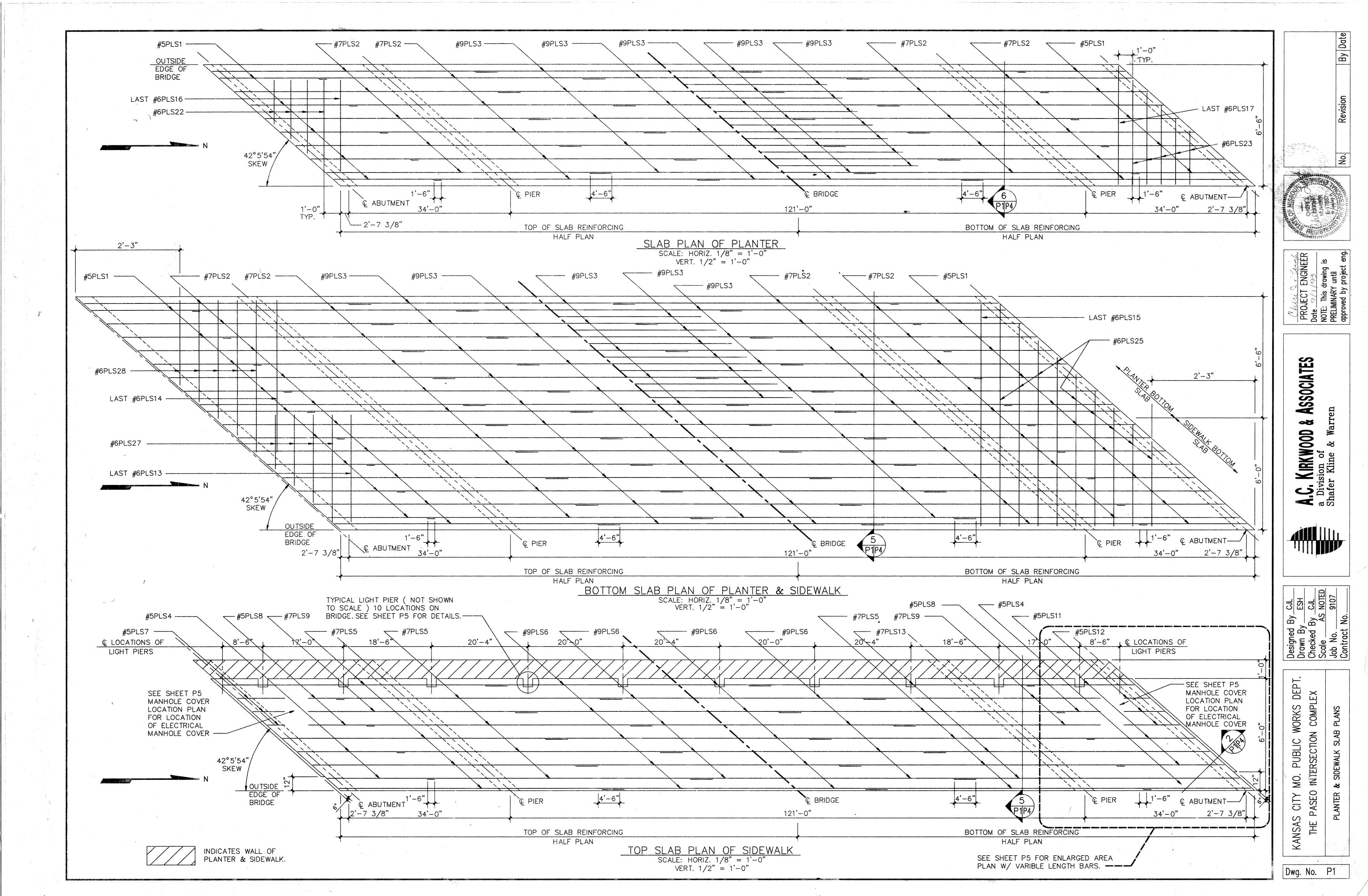
THE REINFORCING STEEL SHALL BE TIED SECURELY TO THE 3/8"\$ STRANDS WITH THE FOLLOWING MAXIMUM SPACING IN EACH DIRECTION: #3-P2 BARS AT 16 INCHES. WELDED WIRE FABRIC OR WELDED DEFORMED BAR MATS AT 24 INCHES.

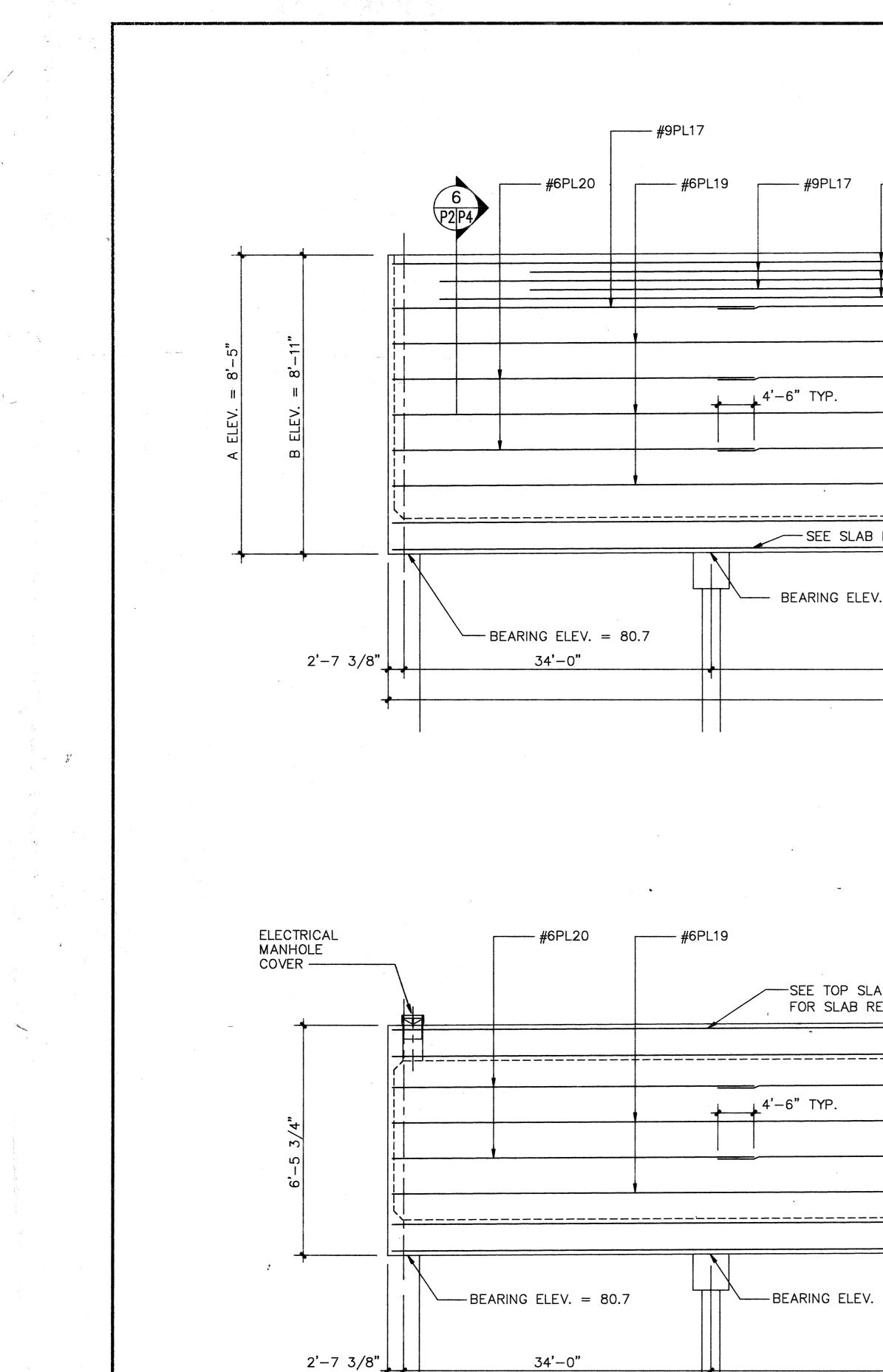
TIE THE #3-UI BARS TO THE #3-P2 BARS, TO THE WELDED WIRE FABRIC OR THE WELDED DEFORMED BAR MATS AT ABOUT 36 INCH CENTERS. ALL REINFORCEMENT OTHER THAN PRESTRESSING STRANDS SHALL BE EPOXY COATED.

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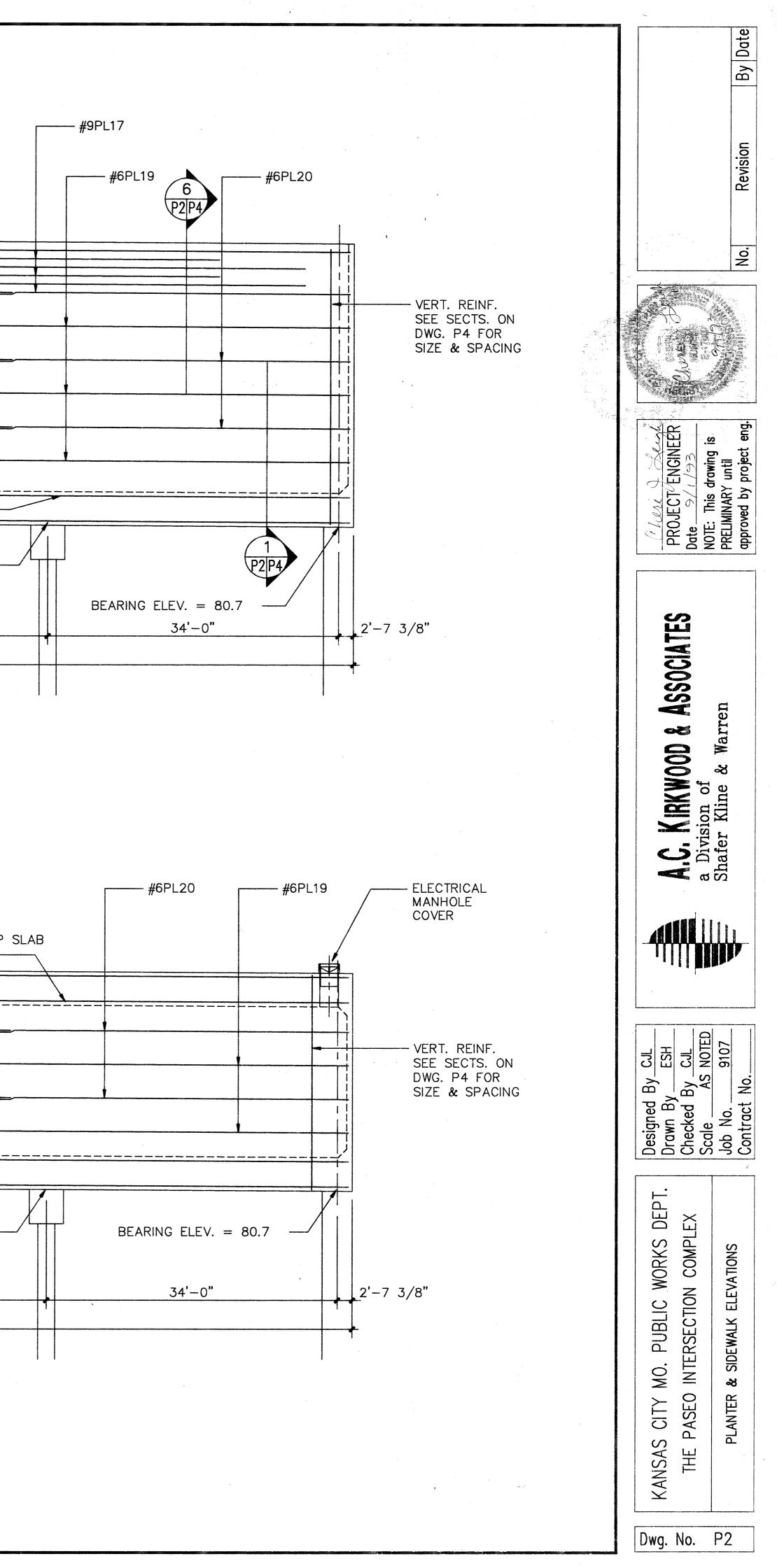
PRES	STRESSED DECK	PANE	ELS
	ALTERNATE A		S 4

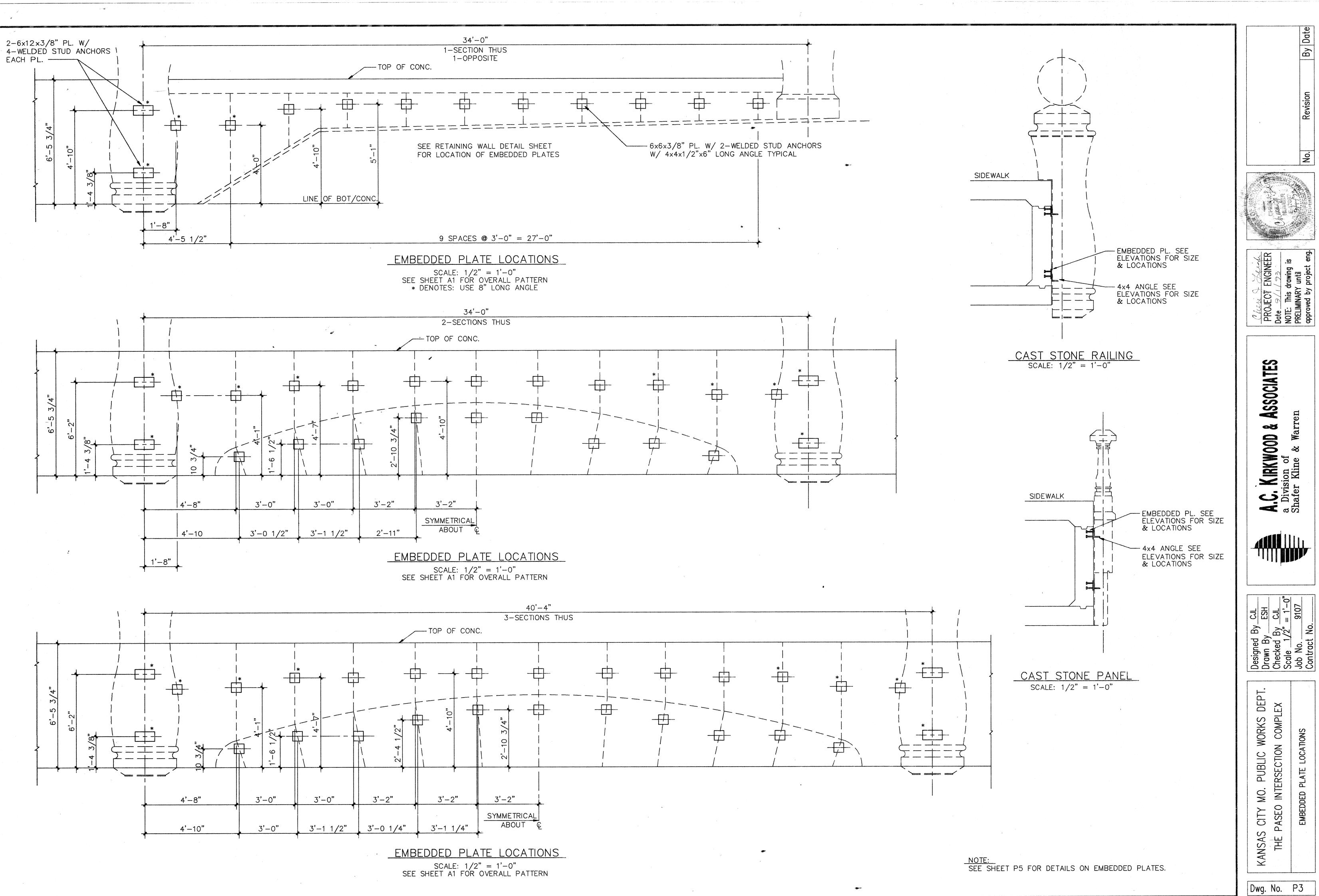
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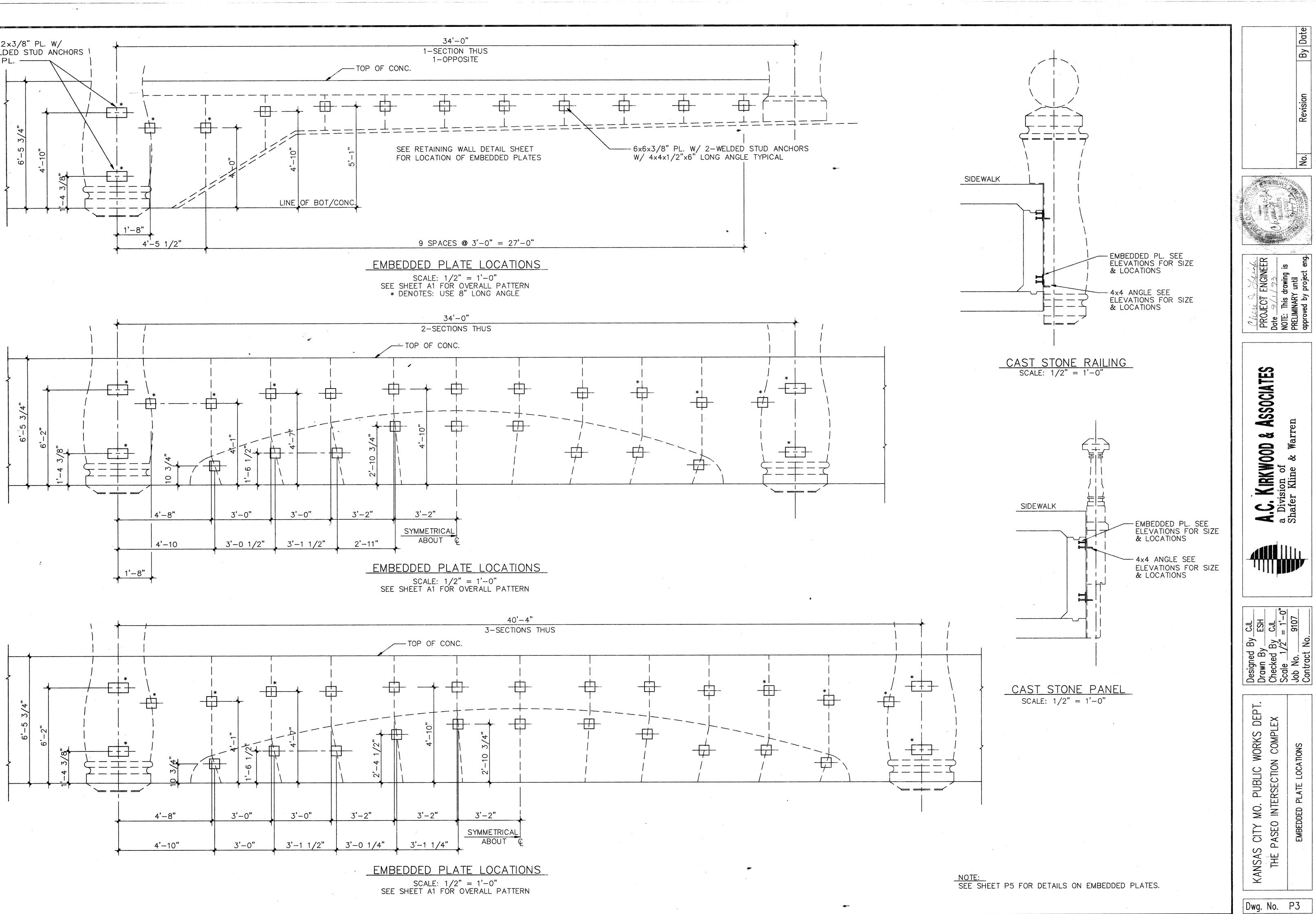




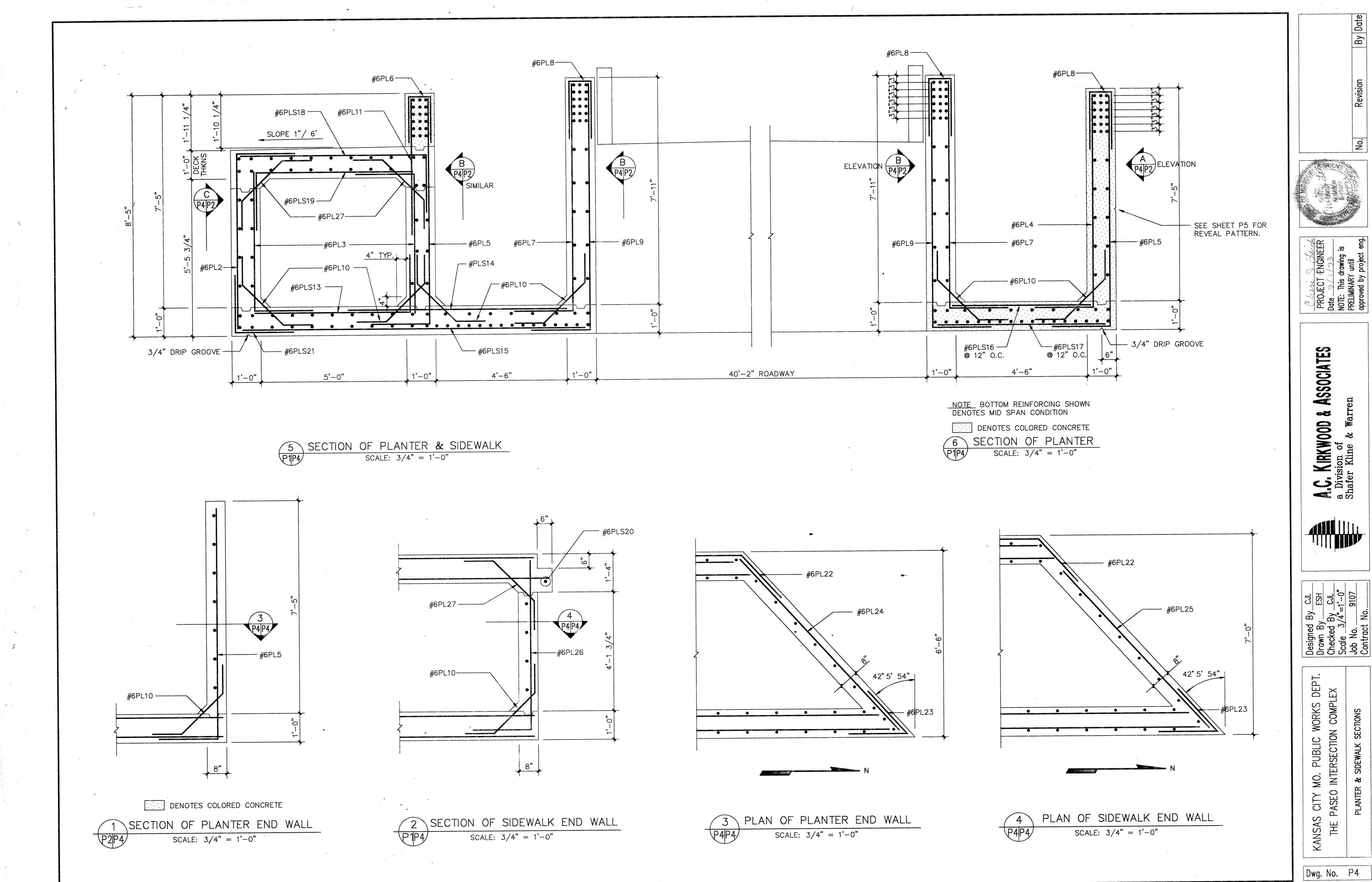
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	#9PL16	#9PL16	#9PL16	#9PL18	#9PL16
#9PL16	#6PL19	#6PL19	#6PL19	#6PL21	·
	γ				
	-			*	
PLAN OF PLANTER FC	R BOTTOM REINFORCIN	 NG	SEE SLAB PLAN O	F PLANTER FOR TOP R	EINFORCING
					and the second se
<i>.</i> = 80.95	- 	Ĺ ∕Ę	ELEV. = 81.2	BEARING ELE	V. = 80.95
	7				, •
60'-6"		194'-2 3/4"		60'-6"	
					ł
	A&B P4P2				
	#6PL19	#6PL19	#6PL19	#6PL21	
AB PLAN OF SIDEWALK EINFORCING				SEE BOTTOM OF FOR SLAB REINF	
	<u>t</u>		<u>t</u>		an a
SEE SLAB PLAN	N OF PLANTER & SIDE	WALK FOR REINFORC			
= 80.95		EL	EV. = 81.2	BEARING ELE	V. = 80.95
60'-6"				60'-6"	•
		194'-2 3/4"			
	C P4P2	ELEVATION SIDEWALK SCALE: HORIZ. 1/8" VERT. 1/2"=1'-			

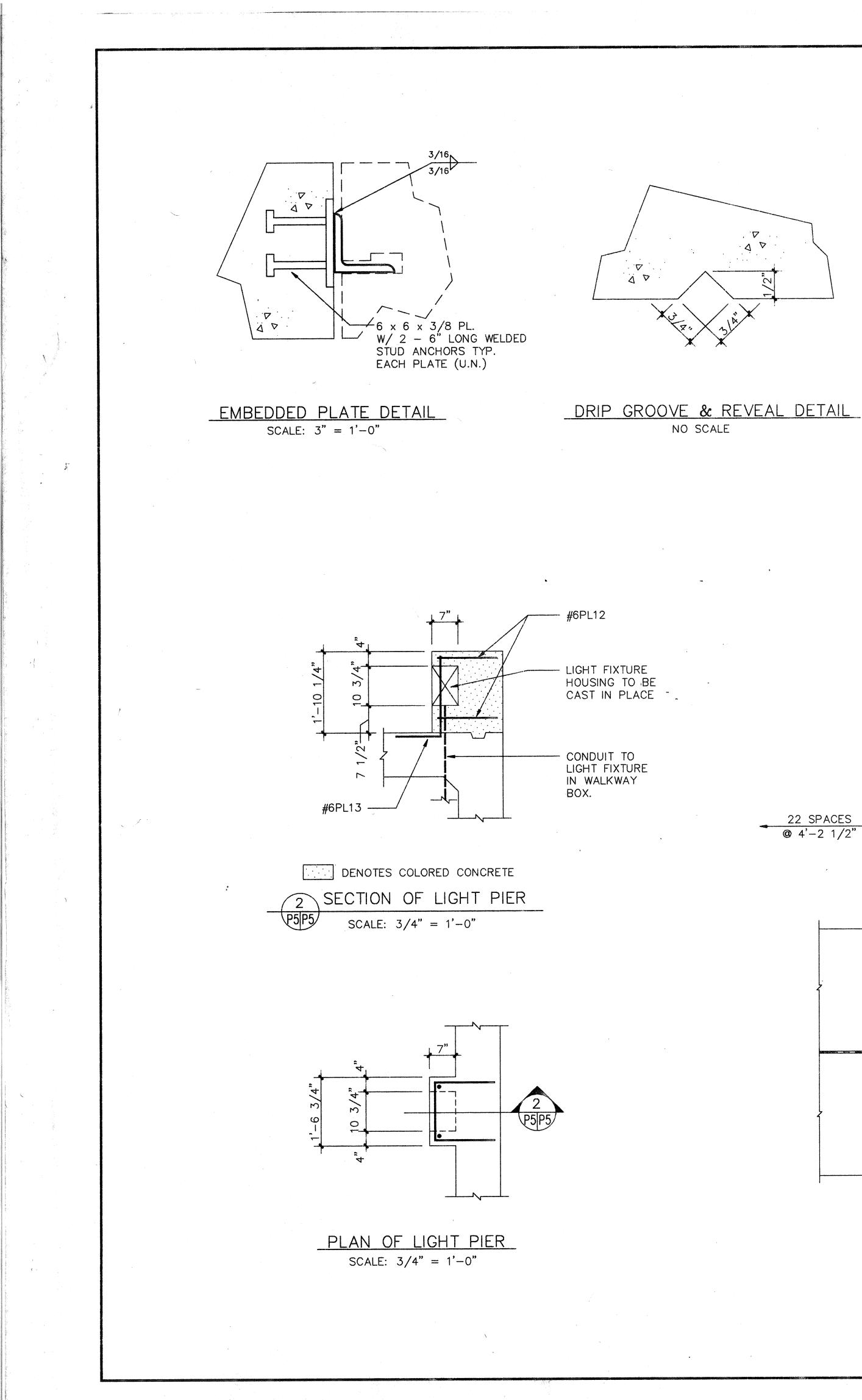


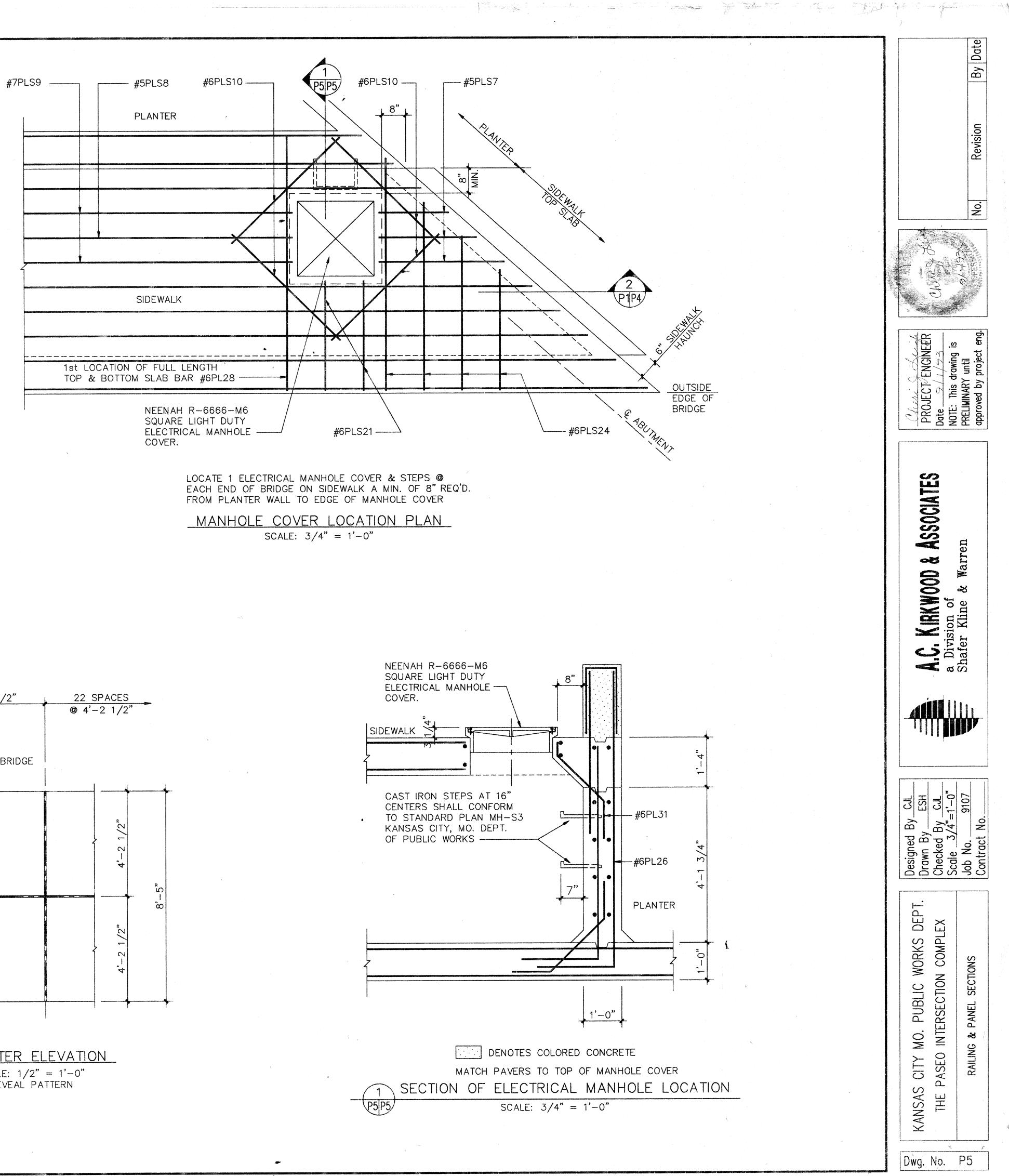


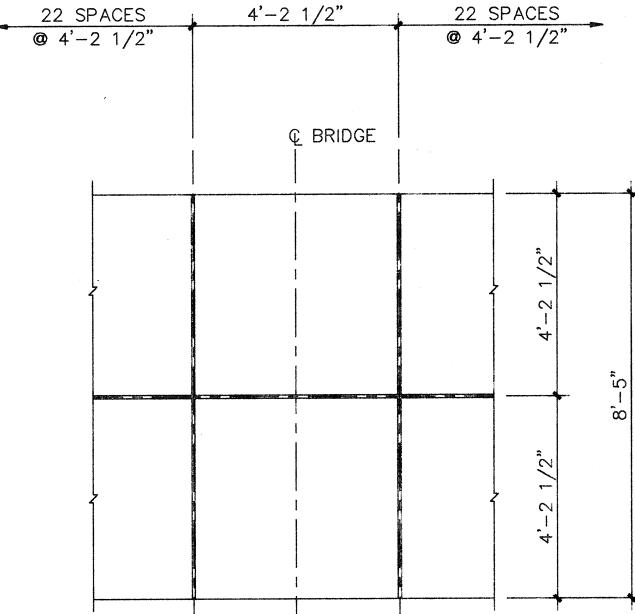


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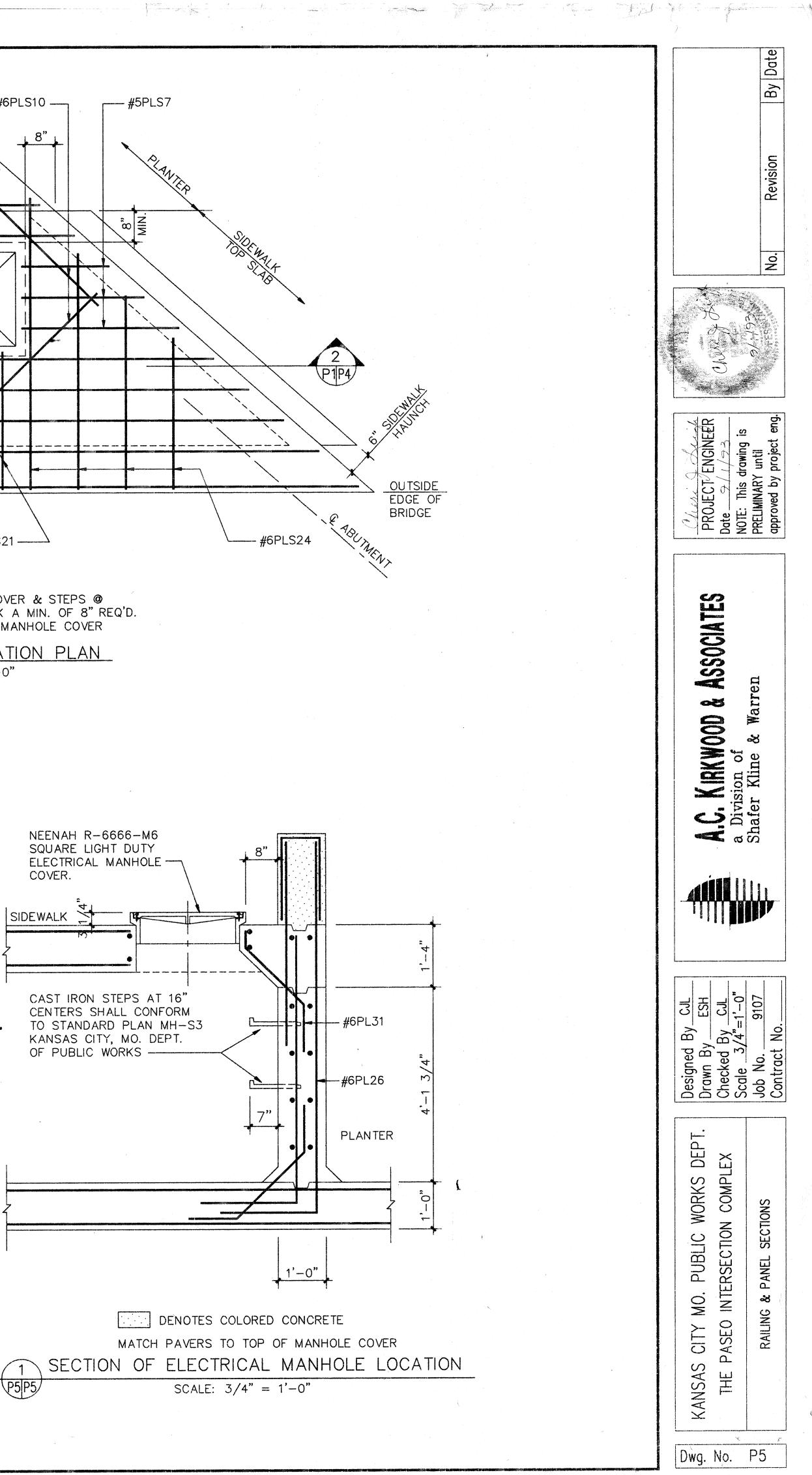


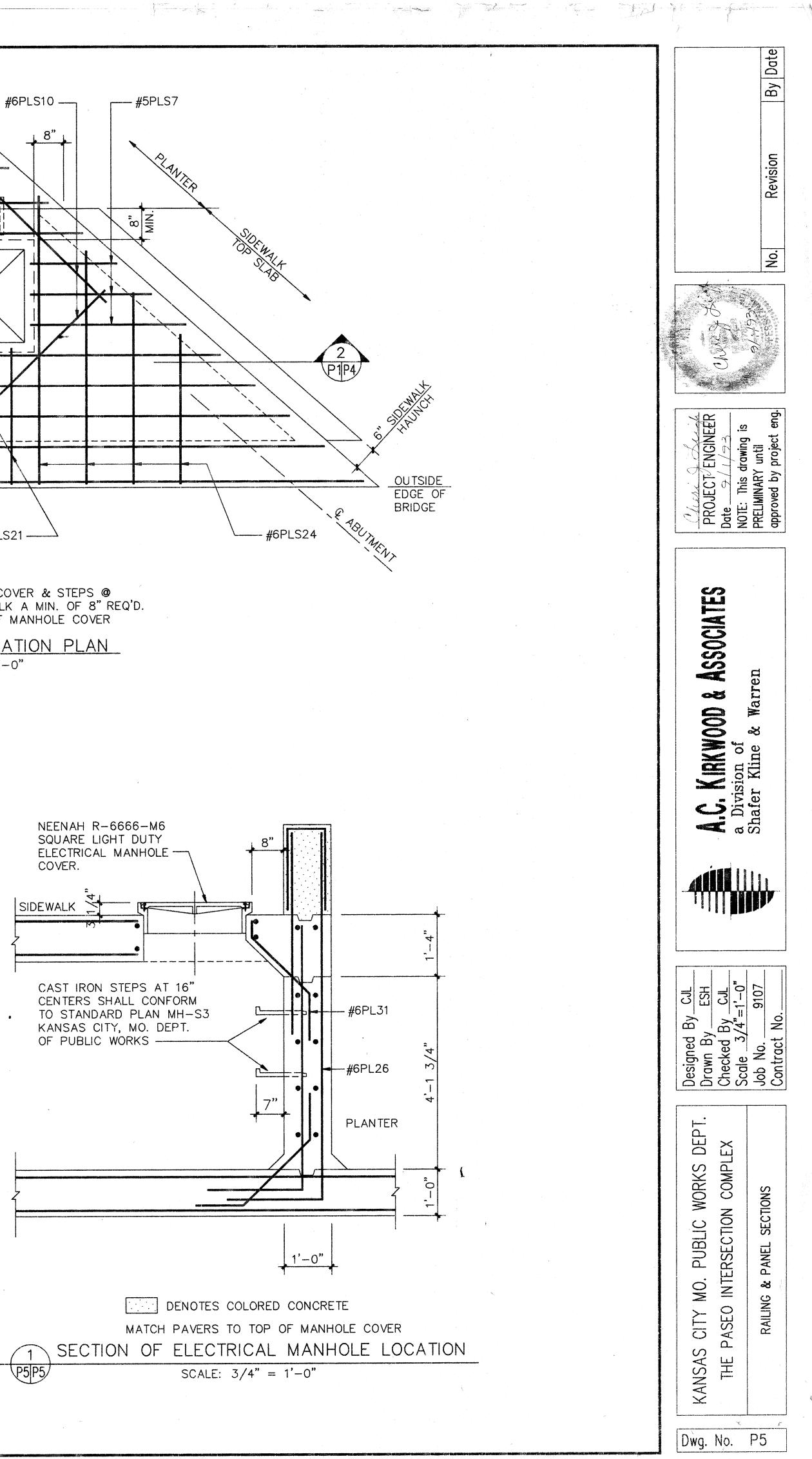






PLANTER ELEVATION SCALE: 1/2" = 1'-0" REVEAL PATTERN





					ETE	BIL	L O	F RE				TEEL													TE BIL	L OF	REINFOR		STEEL				
MARK O NO.	LOCATION	NO.	ACH C		~				DIM	IENSIO	NS		· · · ·				AL WEIGHT		REQ'D.	MARK NO.	LOCATION	NO.	ACH			<u> </u>	DIMEN	NSIONS		·····		NOM	INAL WEIG
		ΡE	ES F	iE	3	C	;	D		E		F	H		K	LENG1						PE XY		В	(; 	D	E	F	H	K	LEN	
NO. SIZE MAR		EPOX	VAR	FT.	IN.	FT.	IN.	FT.	IN. FT	r. IN	N. FT.	IN.	FT. I	N. FT	. IN	FT.	IN. WEIGHT		NO.	MARK		EPOX' SHAPI	VARIE NO.	FT.	IN. FT.	IN. F	Γ. IN. FT.	, IN. F	T. IN.	FT. II	N. FT.	IN. FT.	IN. WEIG
			1 1	71	5"	PLAN	NTER	ON EA	ST BR	RIDGE	1		I			0'	5" 2740		194 ;					[7]	PLA	NTER 0	N WEST BRID	GE		r			5" 074
194 #6PL4 209 #6PL5	WALL	19		8'	2"	2'	0"									10'	5" 2740 2" 3187		· · · ·	#6PL5	WALL WALL	19		8'	2" 2'	0"						10'	5" 274 2" 318
194 #6PL7	WALL	19		7'	11"	2'	0"					********				9'	11" 2886	L	194 j	#6PL7	WALL	19		7'	11" 2'	0"						9'	11" 288
388 #6PL8	WALL	10			0"	1	0"		9"	()"					4'	9" 2765			#6PL8	WALL	10		0'	0" 2'	0"	9"	0"				4'	9" 276
195 #6PL9 398 #6PL10	WALL WALL	· 25		1'	8" 0"	2	0"	1'	0"				1'	5" 1'	5	10	8" 3118 0" 2388		195 _i 398 #	#6PL9	WALL WALL	19		8	8" 2' 0" 2'	0"1'	0"			1' 5	5" 1'	10 5" 4'	8" 311 0" 238
54 #9PL16	WALL	20		60'	0"			•								60'	0" 11016			#9PL16	WALL	20		60'	0"				89147475493498 \$746756 00048715498 \$400487			60'	0" 1101
36 #9PL17	WALL	20		44'	0"											44'	0" 5385			49PL17	WALL	20		44'	0"							44'	0" 538
6 #9PL18 52 #6PL19	WALL WALL	20		27' 60'	<u>6"</u> 0"			***							an ben de Dispurchen d'une - an de statemen	27' 60'	6" 561 0" 4680			49PL18 46PL19	WALL WALL	20		27' 60'	<u>6"</u>							<u> </u>	6" 50 0" 468
16 #6PL20	WALL	20		44'	0"					andy Bengember and With the Barry of Development						44'	0" 1054			6PL20	WALL	20		44'	0"				ł	Joan		44'	0 400
12 #6PL21	WALL	20		27'	6"												6" 495		12 #	#6PL21	WALL	20		27'	6"							27'	6" 49
14 #6PL22	WALL	8		2'	0"			. Na managang pagapatan ng managang s					1' 1		$\frac{15/8}{07/8}$		0" 84 0" 84			6PL22	WALL			2'	0" 0"						1" 11 5/	management of the second se	0" 8 0" 8
14 #6PL23 14 #6PL24	WALL WALL	20		2' 9'	0" 6"								8 3/		0 3/8	4 9'	0" 84 6" 200	F		6PL23 6PL24	WALL WALL	8		2' 9'	<u> </u>					8 5/8	3" 1'10 3	/8 4	0" 8 6" 20
20 # 5PLS1	SLAB	20		24'	6"							******				24'	6 [°] 511			#5PLS1	SLAB	20		24'	6"							24'	6" 5
40 #7PLS2	SLAB	20		60'	0"											60'	0" 4906			7PLS2	SLAB	20		60'	0"							60'	0" 490
27 #9PLS3 177 #6PLS16	SLAB SLAB	20		60' 5'	0"										z	60' 5'	0" 5508 0" 1328	-		9PLS3 6PLS16	SLAB SLAB	20		60' 5'	0"		1					60'	0" 550 0" 132
177 #6PLS17	SLAB	20		6'	3"											6'	3" 1660		L	6PLS17	SLAB	20		6'	3"							6'	3" 166
6 #6PLS20	SLAB	20		4'	9"											4'	9" 23			6PLS20	SLAB			4'	9"							4'	9" 2
8 JEDI 001	SLAB	20		2' 5'	6" ~"											2'	6" 6" 24		<u>R</u> 11	6PLS21	SLAB	20		2' 5'	6"				Rate An and the boots and the second s	-		<u>2'</u> 5'	6" 6" 2
8 #6PLS21	JLAD	20	- v -	5 6'	3"											5 6'	6" 24 3"	-			JLAD		· · · · · · · · · · · · · · · · · · ·	6'	3"	····						5 6'	6" 2 3"
	·····						k SIDE	EWALK	ON EF	AST BI	RIDGE		r			 								PL		k SIDEW	ALK ON WES)E		·····		
193 #6PL1 195 #6PL2	WALL WALL	19		2'	0" 2"		0″ ∩"	1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 -								4' 8'	0" 1158 2" 2389	-		#6PL1 #6PL2	WALL WALL	19		6'	0" 2' 2" 2'	0"				an daar dar waxaya yaar ah		<u>4'</u> 8'	0" 115 2" 238
386 #6PL3	WALL	19		4'	6"	2'	0"	19-19-19-19-19-19-19-19-19-19-19-19-19-1									6 [°] 3764	-		#6PL3	WALL			4'	6" 2'	0"					· · · · · · · · · · · · · · · · · · ·	6'	<u> </u>
208 #6PL5	WALL	19		8'	2"	2'	0"										2" 3172		208	#6PL5	WALL	19)	8'	2" 2'	0"					******	10'	2" 317
194 #6PL6	WALL	10			0" 11"		<u>6"</u>		9"	•					***	3'	9" 1091 11" 2886	-		#6PL6	WALL				0" 1' 11" 2'	6"	9"		~- <u></u>			3'	9" 109 11" 288
194 #6PL7 194 #6PL8	WALL WALL	19			0"	Lan	0"		9"	() "					1	11" 2886 9" 1382	-		#6PL7 #6PL8	WALL WALL	19			0" 2'	0"	9"	0"				4'	<u>11" 288</u> 9" 138
195 #6PL9	WALL	19		8'	8"	2'	0"										8" 3118			#6PL9	WALL	19			8" 2'	0"		_				10'	8" 311
762 #6PL10	WALL	25		1'	0"		0"	1'	0"				1'	5" 1'	5"	4'	0" 4572		762 #		WALL	25		1	0" 2'	0" 1'	0"			1' 5	5" 1'	5" 4'	0" 457
1303 #6PL11 20 #6PL12	WALL	20		4'	6" 10"	1	0"										6" 1303 10" 85		1303 / 20 /	#6PL11 #6PL12	WALL WALL	20		_1	6" 10" 1'	0"						2'	6" 130 10" 8
20 #6PL13	WALL	10			0"	1	4"	1'	4"	() "	de ale - no - n				4'	0" 120			6PL13	WALL	10			0" 1'	4" 1'	4"	0"				4'	0" 12
54 #9PL16	WALL	20		60'	0"									-		60'	0" 11016			49PL16	WALL	20		60'	0"						~	60'	0" 1101
36 #9PL17	WALL	20		40'	<u> </u>											40' 23'	0" 4896 6" 480			49PL17	WALL	20		40' 23'	0" 6"							40' 23'	0" 489 6" 48
6 #9PL18 72 #6PL19	WALL WALL	20		23' 60'	0"											60'	6" 480 0" 6480			49PL18 46PL19	WALL WALL	20		60'	0"							60'	6" 48 0" 648
24 #6PL20	WALL	20		40'	0"											40'	0" 1440			6PL20	WALL.	20)	40'	0"							40'	0" 144
16 #6PL21	WALL	20		25'	2"								A ³ A	<u> </u>	4 5 /08	25'	2" 605			#6PL21	WALL	20			2"				11	<u>^;</u>	· · · · · · · · · · · · · · · · · · ·	25'	2" 60
24 #6PL22 24 #6PL23	WALL WALL	8		2' 2'	0"								8.5/		1 5/8" 10 3/8		0" 144 0" 144			6PL22 6PL23	WALL WALL	8		2'	0" 0"					1 11	1" 11 5/ 3" 1'10 3,	/8 4 /8"4'	0" 14 0" 14
24 #6PL23 14 #6PL24	WALL	20		8'	6"					******					0 0/0	8'	6" 179			6PL24	WALL	20			6"							8'	0" 14 6" 17
8 #6PL25	WALL	20		9'	2"		- 11									9"	2" 110			6PL25	WALL	20)	9'	2"	- 13						9"	2" 11
18 #6PL26 380 #6PL27	WALL WALL	E 25		6'	2" 0"	2'	0" 0"	1'	0"				1'	5"1'	5"	8 4'	2" 221 0" 2280	-		6PL26	WALL WALL	E 25			2" 2' 0" 2'	0" 1'	<u> </u>			1' 4	5" 1'	8' 5" 4'	2" 22 0" 228
8 #6PL31	WALL	E 23			<u> </u>		8"		<u> </u>	8 1/2	2" 8	3 1/2"	1	3"		3'	0" 36			#6PL31	WALL	E 23			4" 1'	8" 1'	0" 8	1/2"	8 1/2"			3" 3'	0" 30
38 #5PLS1	SLAB	E 20		24'	6"												6" 971		38 🛔	#5PLS1	SLAB	E 20		24'	8"							2.4'	8" 97
76 #7PLS2 45 #9PLS3	SLAB	20		60' 60'	<u> </u>											60' 60'	0" 9321 0" 9180			7PLS2	SLAB SLAB	20		60' 60'	0" 0"							60' 60'	0" 9348 0" 9180
6 #5PLS4	SLAB SLAB	E 20		23'	8"	den and a second se											8 ["] 148			9PLS3 5PLS4	SLAB	E 20		23'	8"							23'	<u> </u>
24 #7PLS5 18 #9PLS6	SLAB	E 20		60'	0"											60'	0" 2943		24 #	7PLS5	SLAB	E 20)	60'	0"							60'	0" 294
	SLAB	E 20		60'	0"	1											0" 3876			9PLS6	SLAB	E 20			0"				-			60'	0" 387 10" 29
6	SLAB	E 20		3'	<u>10"</u> 4"										9 6 8 8 8 8 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9	3'	10" 25 4"		0 #	5PLS7	SLAB	E 20) V	3'	10" 4"				•			3'	<u>10" 29</u> 4"
6 #5PLS8	SLAB	E 20		16' 18'	6" 1"	1										16' 18'	6" 108 1"	-	6 #	5PLS8	SLAB	E 20) V	16' 18'	6" 1"							16' 18'	6" 108 1"
6	SLAB	E 20	V	54' 56'	6" 1"			***								54' 56'	6" 678 1"	-	6 #	7PLS9	SLAB	E 20		54' 56'	6" 1"							54' 56'	6" 678 1"
4	SLAB SLAB	E 20 E 20		23'	0" 8"	•										23' 1'	0" 96 8" 12	-		5PLS11 5PLS12	SLAB SLAB	E 20) /	23' 1'	0" 8"							23'	0" 90 8" 1:
190 #6PLS13	SLAB	E 19		3' 5'	<u>3</u> " <u>3</u> "		0"									3' 7'	3" 3" 2069		181 #6		SLAB	E 20		<u> </u>	3" 3" 2'	0"						3' 7'	3" 3" 1968
188 #6PLS14	SLAB	E 20		7'	7"							1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 -				7'	7" 2141		184 #6	SPLS14	SLAB	E 20)	7'	7"	-						7'	7" 2093
184 #6PLS15	SLAB	E 20		12'	3"		~"										3" 3386	- - 	169 #6		SLAB	E 20		12	3"	C.13	~~~~~					12'	3" 3105
181 #6PLS18 181 #6PLS19	SLAB SLAB	E 20 E 20		6' 5'	9" 3"		8" 0"	******				· · · · · · · · · · · · · · · · · · ·			****		5" 2557 3" 1968		181 #6	SPLS18 SPLS19	SLAB SLAB	E 20			9" 2' 3" 2'	8" 0"						9' 7'	5" 255 3" 1968
2 #6PLS20	SLAB	E 20		5'	<u> </u>		~										3 ["] 16	E Contractor de la contra		SPLS20	SLAB	E 20		5'	3"							5'	3" 10
8 #6PLS21	SLAB	E 20	<u> </u>	2'	11"											2'	11" 36			SPLS21	SLAB	E 20		2'	11"							2'	11" 30
8 #6PLS22	SLAB	20		<u>3'</u> 1'	<u>8"</u> 2"					1						<u> 3'</u> 1'	8" 48 2"	F	8 #6	SPLS22	SLAB	20		3 [′] 1 [′]	8 ["] 2"							<u> </u>	8" 48 2"
10 #6PLS23	- SLAB	20	V	5'	<u> </u>											5'	<u>2</u> 11" 55 7"		10 #6	SPLS23	SLAB	20		5'	<u>2</u> 11" 7"							5'	<u> </u>
8 #6PLS24	SLAB	E 20	V	2 6'2 3'2												6'2 1/	/2" 54	F	8 #6	SPLS24	SLAB	E 20		6'21/						9) - 10) - 1		6'2	/ 1/2" 54
18 #6PLS25	SLAB	E 20	V 2	3' 2	1/2" 0'										· · · · · · · · · · · · · · · · · · ·	3' 2 1/ 11'	0" 178		18 #6	PLS25	SLAB	E 19		3'2 1/ 11'	2 0'			•	<u>.</u>			3'2 11'	0" 189
6 #6PLS26	SLAB	E 19		4'	<u>2</u> 6'	2'	0"			*							2" 6" 36		6 #6	SPLS26	SLAB	E 19		3 4'	0' 6' 2'	0"						3' 6'	0" 6" 36
8 #6PLS27	SLAB S	E 20		<u>3</u> 5'	0' 4"		0" 0"	****					<u> </u>		an an an air Bhaile an an ann ann an Angai	5 ⁻ 7'	0" 4" 48		8 #6	PLS27	SLAB	E 20) _V	<u>5</u> '	0' 2' 4" 2'	0″ 0"				,		5' 7'	0" 4" 48
				3'	4"	1	0"				2	1				5'	4"	ŀ			*****				4" 2'	0"						5'	4"
10 #6PLS28	SLAB	E 20		7' '	0" 6"							- 				7' 3'	0" 75 6"		10 #6	PLS28	SLAB	E 20) V	7' 3'	0" 6"					1997 - 1998 - 1998 - 1999 - 1999 - 1999 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 -		7'	0" 75 6"
1				10	0	L	<u> </u>						L			15	<u>v</u>			~					<u>v</u>		e					<u>J</u>	V

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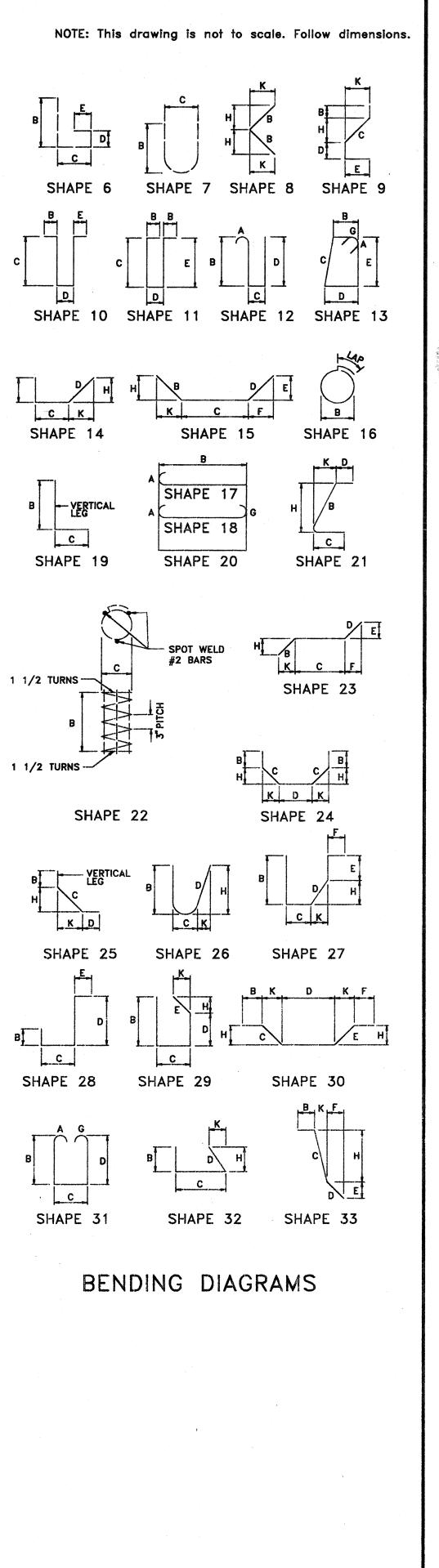
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